

COMMITTENTE:



PROGETTAZIONE:



DIREZIONE TECNICA
U.O. OPERE CIVILI E GESTIONE DELLE VARIANTI

PROGETTO DEFINITIVO

Nuova linea Ferrandina - Matera La Martella per il collegamento di Matera con la rete ferroviaria nazionale

MIGLIORAMENTO SISMICO E OPERE DI COMPLETAMENTO DEI VIADOTTI ESISTENTI DELLA LINEA FERRANDINA MATERA

GEOTECNICA

Relazione di calcolo fondazioni dirette su micropali P21 – P22

SCALA:

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COMMESSA LOTTO FASE ENTE TIPO DOC. OPERA/DISCIPLINA PROGR. REV.

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Rev.	Descrizione	Redatto	Data	Verificato	Data	Approvato	Data	Autorizzato Data
A	EMISSIONE ESECUTIVA	S.Gasperoni	Luglio 2019	M.E.D'Effremo	Luglio 2019	F. Gernone	Luglio 2019	A. Vittozzi Luglio 2019

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Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

Relazione di calcolo fondazioni dirette su micropali
P21 – P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	4 di 91

1. PREMESSA

Nella presente relazione si riporta la verifica delle fondazioni su micropali delle pile P21 e P22 del viadotto Gravina (VI08) per miglioramentosismico nell'ambito della realizzazione della nuova Linea Ferrandina – Matera "La Martella" per il collegamento della città di Matera alla rete ferroviaria nazionale, in particolare con Salerno, per l'accesso al sistema AV/AC, e con Taranto, attraverso la linea Battipaglia-Potenza-Metaponto-Taranto.

In particolare verranno affrontati i seguenti aspetti:

- Descrizione dello stato di fatto delle fondazioni esistenti del viadotto;
- Inquadramento geotecnico: stratigrafia, parametri geotecnici e falda;
- analisi e verifica delle fondazioni esistenti con le nuove azioni sismiche di progetto.

Tutte le analisi svolte nel seguito sono eseguite in conformità alla normativa italiana vigente sulle opere civili (DM 17/01/2018).

2. NORMATIVA E DOCUMENTI DI RIFERIMENTO

2.1 Normativa di riferimento

- [N.1]. Norme Tecniche per le Costruzioni - D.M. 17-01-18 (NTC-2018);
- [N.2]. Circolare N.7 del 21-01-2019, C.S.LL.PP. - Istruzioni per l'Applicazione Norme Tecniche Costruzioni di cui al Decreto Ministeriale 17 gennaio 2018.
- [N.3]. RFI DTC SI PS MA IFS 001 C - Manuale di Progettazione delle Opere Civili - Parte II – Sezione 2 – Ponti e Strutture
- [N.4]. Progetto DOC-Reluis 2005-2008 – linea 3: Valutazione e riduzione del rischio sismico di ponti esistenti – “linee guida e manuale applicativo per la valutazione della sicurezza sismica e il consolidamento dei ponti esistenti in c.a.” marzo 2009

2.2 Documenti di riferimento

- [DC1]. IA5F00D78RHGE0005001 - Nuova Linea Ferrandina – Matera La Martella per il collegamento di Matera con la rete ferroviaria nazionale. Progetto Definitivo. Relazione geotecnica;
- [DC2]. IA5F00D78F6GE0005001 - Nuova Linea Ferrandina – Matera La Martella per il collegamento di Matera con la rete ferroviaria nazionale. Progetto Definitivo. Profilo Geotecnico.
- [DC3]. E.0.2. - Progetto Esecutivo. Costruzione di un tratto di linea tra Matera e Ferrandina in funzione di collegamento diretto della città di Matera alla rete F.S.. Stazione di Ferrandina – Stazione di Matera la Martella. Relazione calcoli statici 2 a parte. Novembre 1985.
- [DC4]. IA5F 01 D 09 RH VI0000 001 A - Nuova Linea Ferrandina – Matera La Martella per il collegamento di Matera con la rete ferroviaria nazionale. Progetto Definitivo. Criteri di calcolo delle fondazioni.
- [DC5]. IA5F 01 D 09 RH VI0800 002 A - Nuova Linea Ferrandina – Matera La Martella per il collegamento di Matera con la rete ferroviaria nazionale. Progetto Definitivo. Relazione stabilità di versante.
- [DC6]. F.8.1.1. e F.8.1.2 - Linea Ferrandina – Matera Centrale. Pianta e profilo longitudinale Tav.1 e 2. Novembre 1985.
- [DC7]. Linea Ferrandina – Matera Centrale. Carpenterie fondazioni e piante. Novembre 1985.
- [DC8]. F.8.8.4.P. - Progetto Esecutivo. Costruzione di un tratto di linea tra Matera e Ferrandina in funzione di collegamento diretto della città di Matera alla rete F.S.. Stazione di Ferrandina – Stazione di Matera la Martella. – Viadotto Gravina - Consolidamento del masso roccioso in corrispondenza della pila 21. Novembre 1985.
- [DC9]. F.8.8.4.P. - Progetto Esecutivo. Costruzione di un tratto di linea tra Matera e Ferrandina in funzione di collegamento diretto della città di Matera alla rete F.S.. Stazione di Ferrandina – Stazione di Matera la Martella. – Viadotto Gravina - Consolidamento del masso roccioso in corrispondenza della pila 22. Novembre 1985.

- [DC10]. Rv.1. Lunghezza pali in opera.
- [DC11]. IA5F01D09ROVI0000001A Relazione tecnico-illustrativa: Materiali, Fattore di confidenza e criteri di calcolo
- [DC12]. IA5F01D09P9VI0800003A VI08 - Pianta scavi e sezione longitudinale tav.1/6
- [DC13]. IA5F01D09P9VI0800004A VI08 - Pianta scavi e sezione longitudinale tav.2/6
- [DC14]. IA5F01D09P9VI0800005A VI08 - Pianta scavi e sezione longitudinale tav.3/6
- [DC15]. IA5F01D09P9VI0800006A VI08 - Pianta scavi e sezione longitudinale tav.4/6
- [DC16]. IA5F01D09P9VI0800007A VI08 - Pianta scavi e sezione longitudinale tav.5/6
- [DC17]. IA5F01D09P9VI0800008A VI08 - Pianta scavi e sezione longitudinale tav.6/6
- [DC18]. IA5F01D09P9VI0800009A VI08 - Pianta fondazione e sezione longitudinale 1
- [DC19]. IA5F01D09P9VI0800010A VI08 - Pianta fondazione e sezione longitudinale 2
- [DC20]. IA6F01D09P9VI0800011A VI08 - Pianta fondazione e sezione longitudinale 3
- [DC21]. IA6F01D09P9VI0800012A VI08 - Pianta fondazione e sezione longitudinale 4
- [DC22]. IA6F01D09P9VI0800013A VI08 - Pianta fondazione e sezione longitudinale 5
- [DC23]. IA5F01D09P9VI0800014A VI08 - Pianta fondazione e sezione longitudinale 6
- [DC24]. IA5F01D09BZVI0800001A Carpenteria interventi fondazioni pile P1-P3 e P26-P27
- [DC25]. IA5F01D09BZVI0800002A Carpenteria interventi fondazioni pile 4-15 e 25
- [DC26]. IA5F01D09BZVI0800003A Carpenteria interventi fondazioni pile 16-20
- [DC27]. IA5F01D09BZVI0800004A Carpenteria interventi fondazioni pile 21-22
- [DC28]. IA5F01D09BZVI0800005A Carpenteria interventi fondazioni pile 23-24

2.3 Programmi di calcolo utilizzati

- **APAL** (G. Guiducci, 2006). Studio Sintesi, Rimini (RN), Italia.
Programma per l'analisi di pali caricati assialmente: curve carico-cedimento trasferimento sforzo assiale.
- **MAP Matrix Analysis of Piles** (G. Guiducci, 1999). Rimini (RN), Italia.
Programma di calcolo per analisi delle sollecitazioni e deformazioni di tipo lineare e non lineare di palificate di fondazione collegate da plinto rigido.

3. MATERIALI

Nel presente progetto per le fondazioni esistenti si utilizzeranno i seguenti materiali.

Calcestruzzo plinto di fondazione, pali $R_{ck} > 25 \text{ MPa}$ C25/30

Calcestruzzo sottoplinto (magrone) $R_{ck} > 15 \text{ MPa}$ C16/20

Acciaio in barre per c.a. FeB 38 K B450C
 $f_{yk} = 380 \text{ MPa}$
 $E = 210000 \text{ MPa}$

Acciaio per armatura micropali Fe 360 S235
 $f_{yk} = 235 \text{ MPa}$
 $E = 210000 \text{ MPa}$

Per i materiali delle opere esistenti si definisce livello di conoscenza $LC = 2$ e quindi nelle verifiche si assume un fattore di confidenza $FC = 1.2$.

4. DESCRIZIONE DELLE OPERE IN FONDAZIONE

4.1 Stato di fatto

Il viadotto è costituito da 27 pile e due spalle.

Le fondazioni delle pile sono costituite da plinti, di dimensioni variabili, fondati su pali (D1200 mm) o su micropali.

Le spalle sono costituite da fondazione su pali D1200 mm.

Inoltre per le pile di scavalco fiume è stato inserito un intervento di consolidamento lungo la scarpata con tiranti attivi (come da schema di figure seguenti).

Di seguito si riportano le tabelle riassuntive con le caratteristiche delle fondazioni esistenti del viadotto in esame:

Nelle figure seguenti sono mostrate le geometrie delle fondazioni.

spalla										
	H tot[m]	dim plinto[m]	h plinto[m]	tipo fondaz.	d pali[mm]	num pali	Lunghezza pali[m]	Armatura longitudinale	spirale gabbia superiore	spirale gabbia inferiore
A	8.00	11.50x9.20x2.00	2.00	pali	1200	9	13.7	28 ϕ 30	ϕ 8 passo 10	ϕ 8 passo 20
B	7.50	11.50x9.20x2.00	2.00	pali	1200	9	24.9÷25.0	28 ϕ 30	ϕ 8 passo 10	ϕ 8 passo 20

pila										
	H tot[m]	dim plinto[m]	h plinto[m]	tipo fondaz.	d pali[mm]	num pali / micropali	Lunghezza pali / micropali[m]	Armatura longitudinale	spirale gabbia superiore	spirale gabbia inferiore
1	6.00	7.40x10.20x2.50	2.50	pali	1200	6	11.3	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
2	6.35	7.40x10.20x2.50	2.50	pali	1200	6	10.1÷10.3	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
3	6.80	7.40x10.20x2.50	2.50	pali	1200	6	9.3÷9.5	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
4	8.20	9.00x10.20x2.50	2.50	pali	1200	8	7.7÷9.5	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
5	9.00	9.00x10.20x2.50	2.50	pali	1200	8	6.6÷7.4	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
6	9.25	9.00x10.20x2.50	2.50	pali	1200	8	5.6÷6.8	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
7	9.00	9.00x10.20x2.50	2.50	pali	1200	8	6.2÷6.9	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
8	8.40	9.00x10.20x2.50	2.50	pali	1200	8	7.4÷8.1	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
9	7.95	9.00x10.20x2.50	2.50	pali	1200	8	7.9÷10.2	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
10	7.70	9.00x10.20x2.50	2.50	pali	1200	8	8.0÷8.7	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20
11	8.20	9.00x10.20x2.50	2.50	pali	1200	8	8.1÷8.9	18 ϕ 24	ϕ 8 passo 10	ϕ 8 passo 20

12	8.95	9.00x10.20x2.50	2.50	pali	1200	8	8.4÷8.7	18 φ24	φ8 passo 10	φ8 passo 20
13	10.45	9.00x10.20x2.50	2.50	pali	1200	8	8.3÷9.1	18 φ24	φ8 passo 10	φ8 passo 20
14	11.70	9.00x10.20x2.50	2.50	pali	1200	8	7.8÷12.2	18 φ24	φ8 passo 10	φ8 passo 20
15	13.30	9.00x10.20x2.50	2.50	pali	1200	8	6.9÷12.0	18 φ24	φ8 passo 10	φ8 passo 20
16	15.80	12.50x9.50x2.50	2.50	micropali	230	58	15	-	-	-
17	18.15	12.50x9.50x2.50	2.50	micropali	230	74	15	-	-	-
18	21.50	12.50x9.50x2.50	2.50	micropali	230	74	15	-	-	-
19	25.00	12.50x9.50x2.50	2.50	micropali	230	74	15	-	-	-
20	23.10	12.50x9.50x2.50	2.50	micropali	230	74	15	-	-	-
21	24.30	19.7x14.7x2.50	2.50	micropali	230	250	28	-	-	-
22	23.60	19.7x14.7x2.50	2.50	micropali	230	250	28	-	-	-
23	20.20	9.60x10.20x2.50	2.50	pali	1200	9	10.5÷10.9	18 φ24	φ8 passo 10	φ8 passo 20
24	16.50	9.60x10.20x2.50	2.50	pali	1200	9	14.3÷15.7	18 φ24	φ8 passo 10	φ8 passo 20
25	12.60	9.00x10.20x2.50	2.50	pali	1200	8	16.2÷17.4	18 φ24	φ8 passo 10	φ8 passo 20
26	9.30	7.40x10.20x2.50	2.50	pali	1200	6	19.9÷20.1	18 φ24	φ8 passo 10	φ8 passo 20
27	6.95	7.40x10.20x2.50	2.50	pali	1200	6	21.3÷22.1	18 φ24	φ8 passo 10	φ8 passo 20

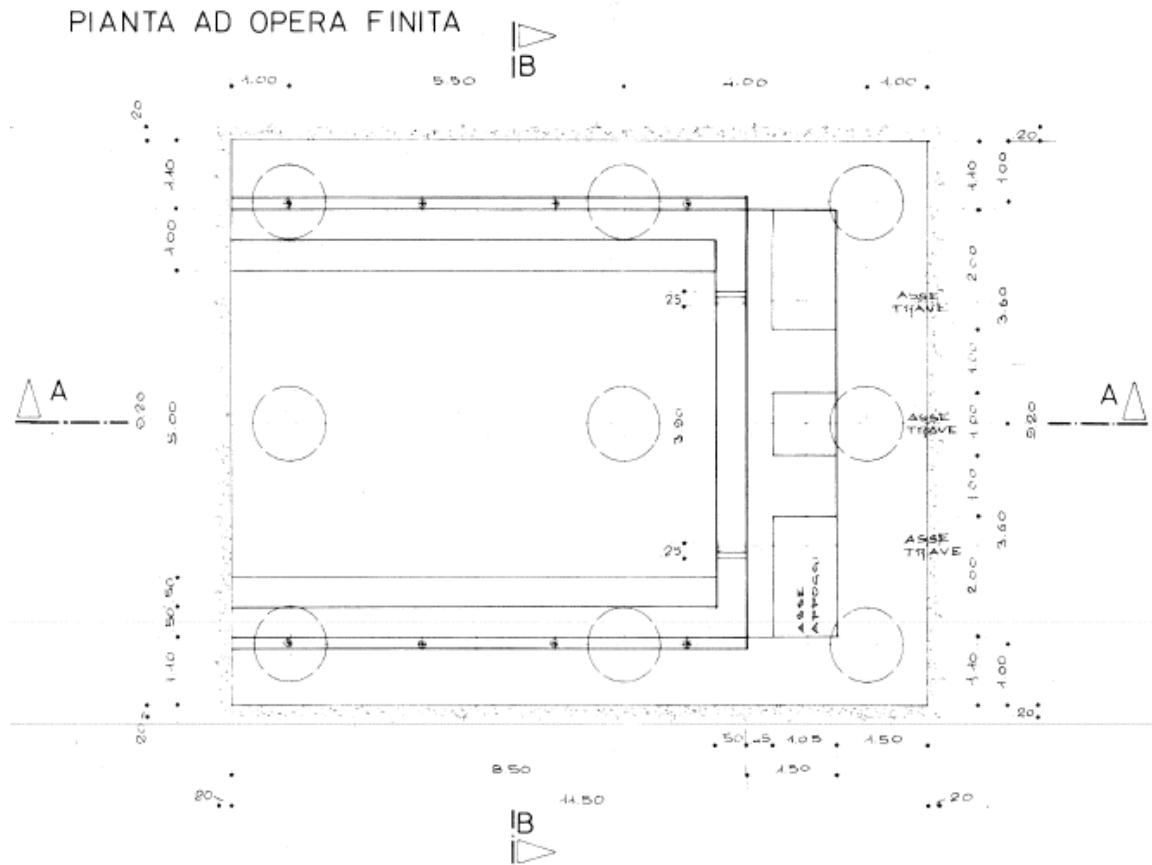


Figura 1 - Pianta Spalle

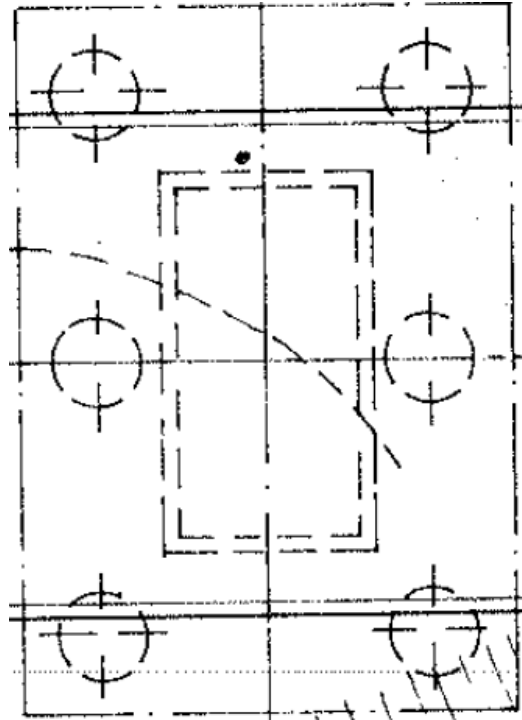


Figura 2 – Pianta Pile a 6 pali (P1÷P3, P26 e P27)

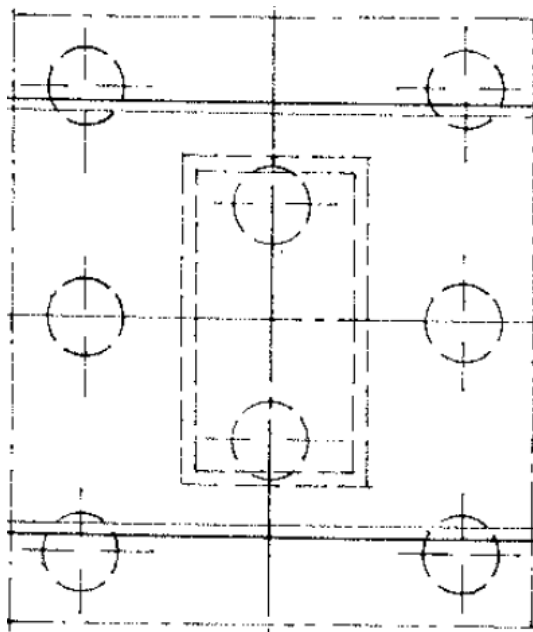


Figura 3 – Pianta Pile a 8 pali (P4÷P15 e P25)

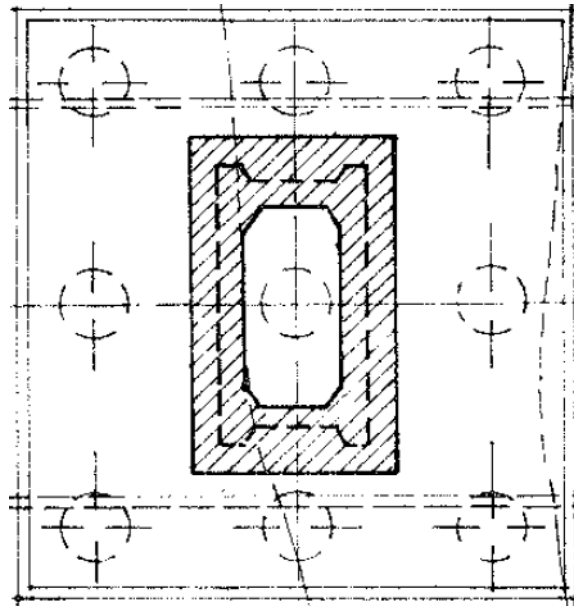


Figura 4 - Pianta Pile a 9 pali (P23 e P24)

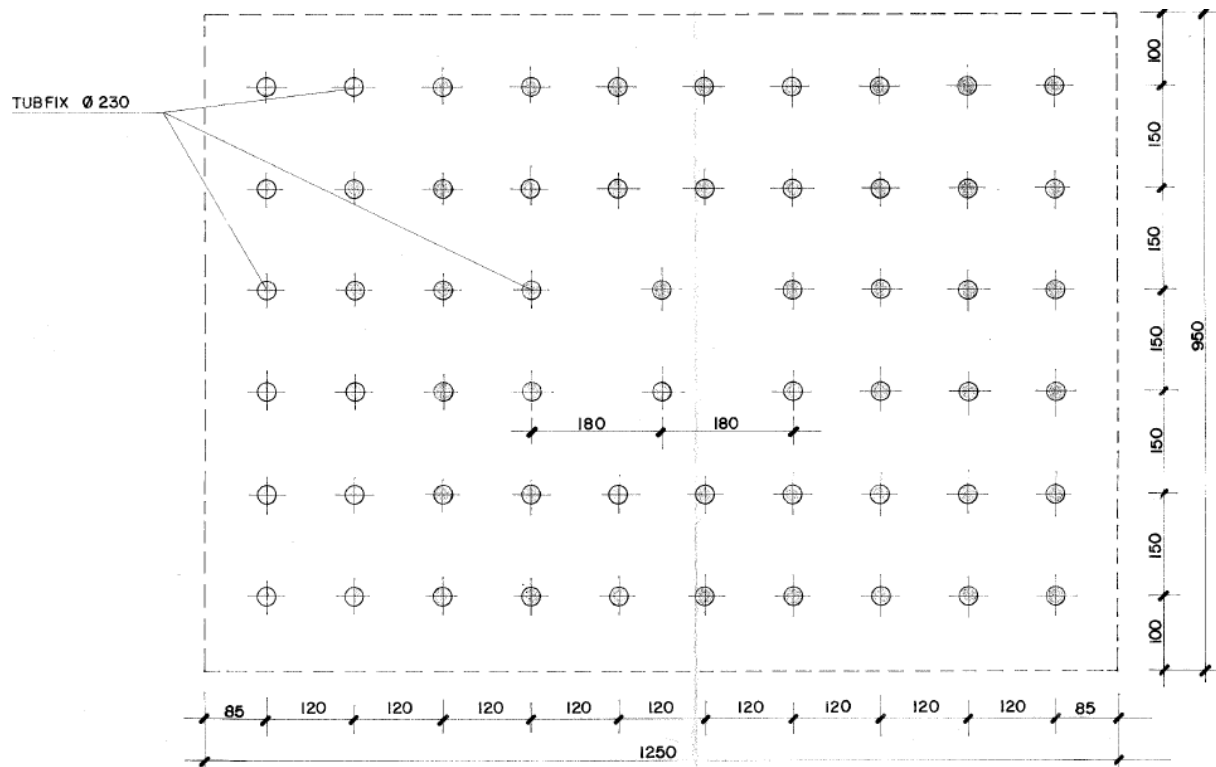


Figura 5 - Pianta pila P16 su fondazione diretta con n°58 micropali

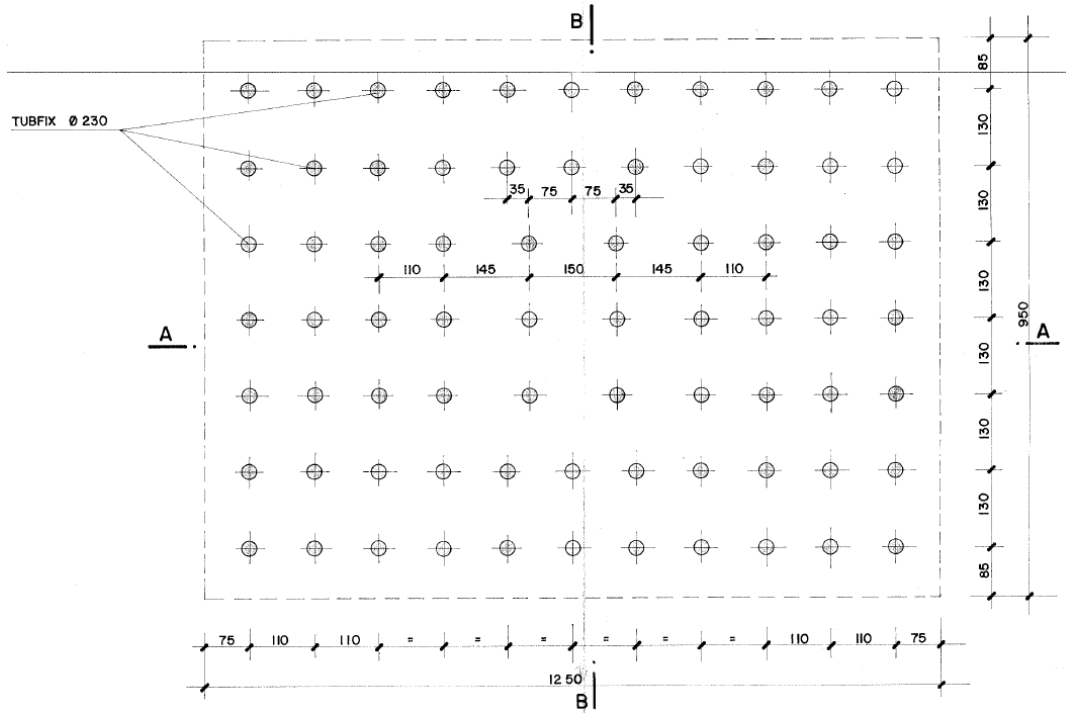


Figura 6 – Pianta pile P17÷P20 su fondazione diretta con n°74 micropali

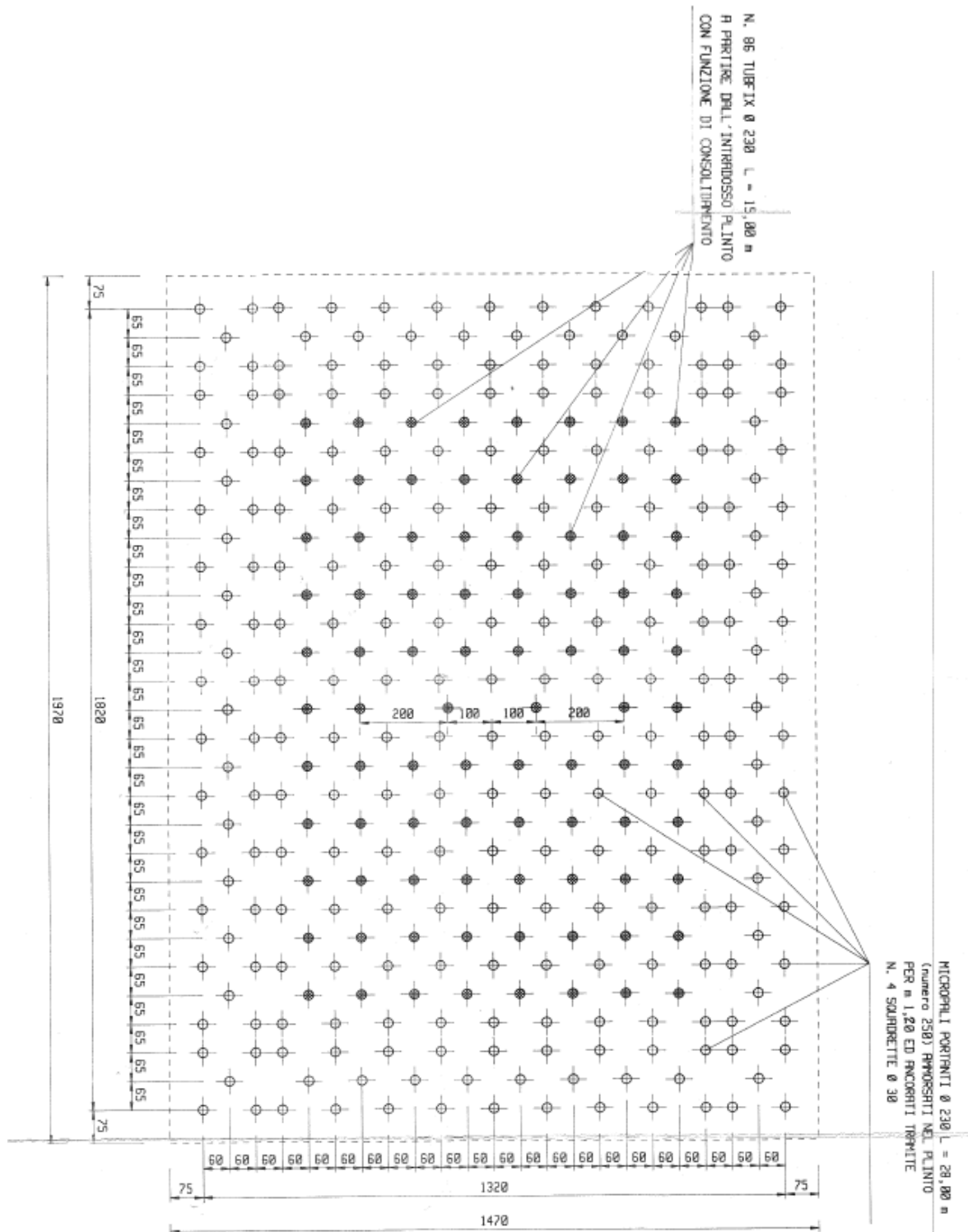


Figura 7 – Pianta pile P21÷P22 su fondazione diretta con 250 micropali

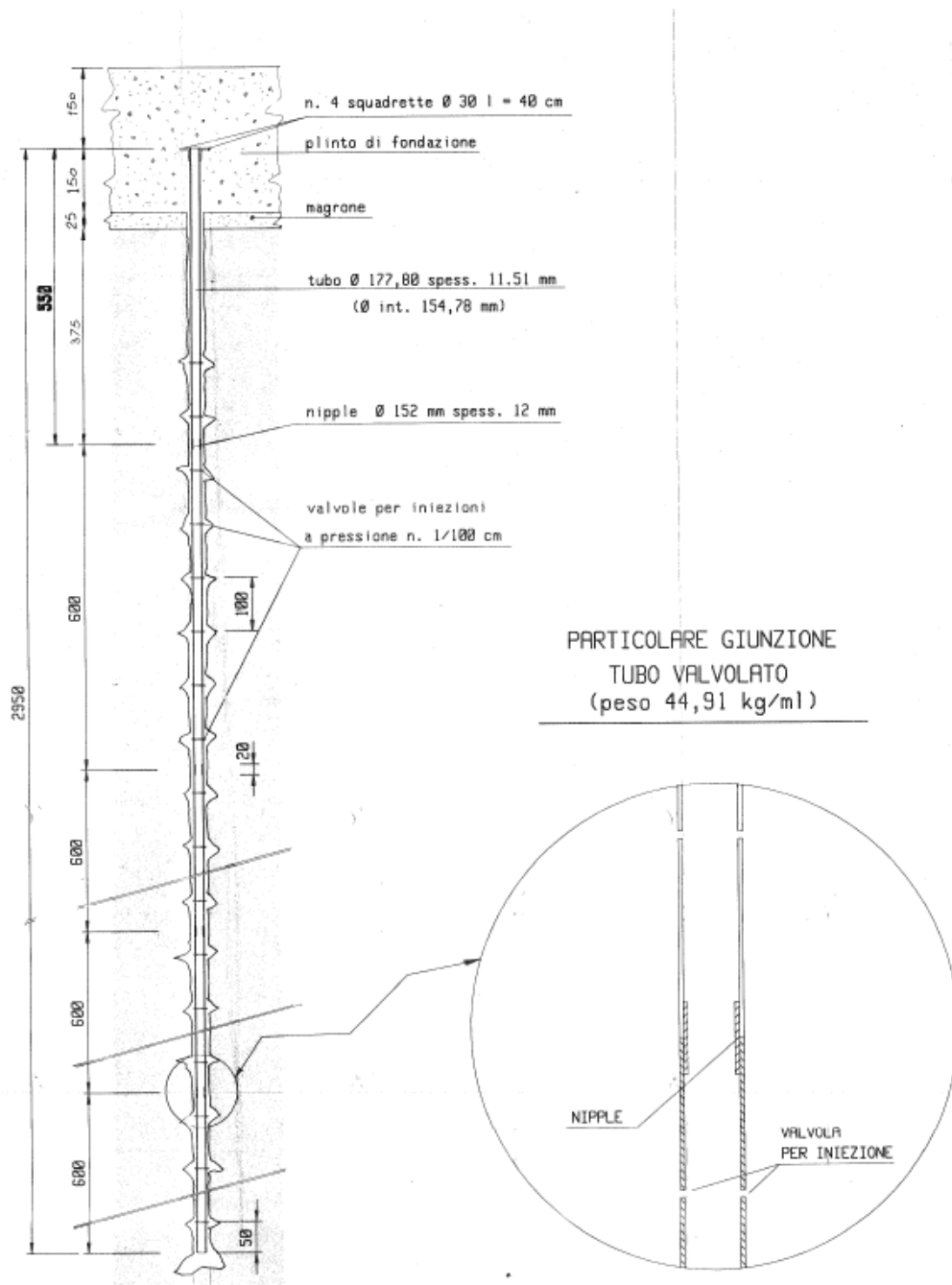


Figura 8 – Micropali pila P21-P22

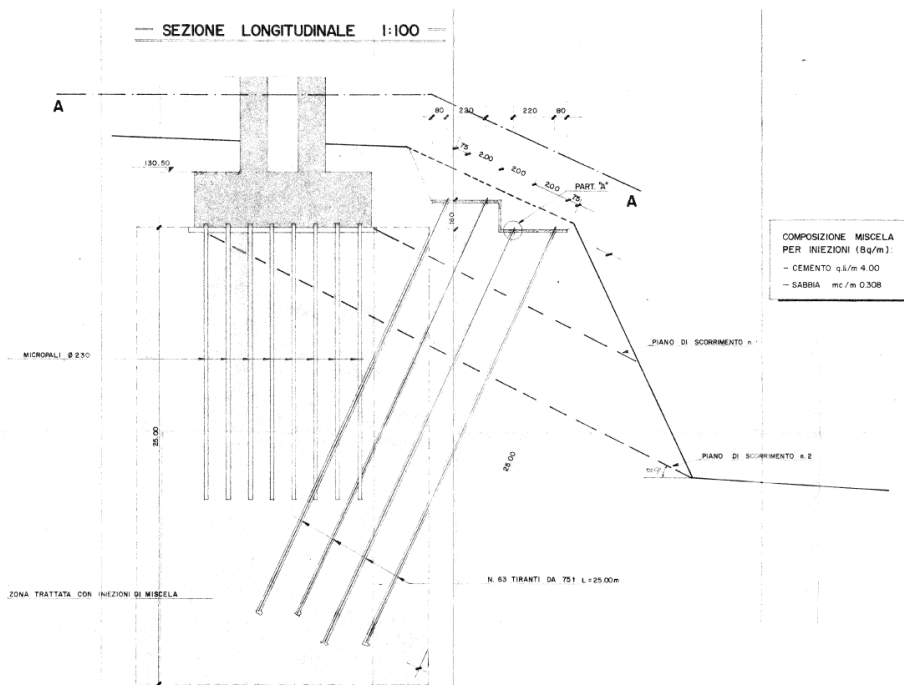


Figura 9 – Intervento di consolidamento scarpata Pila P21

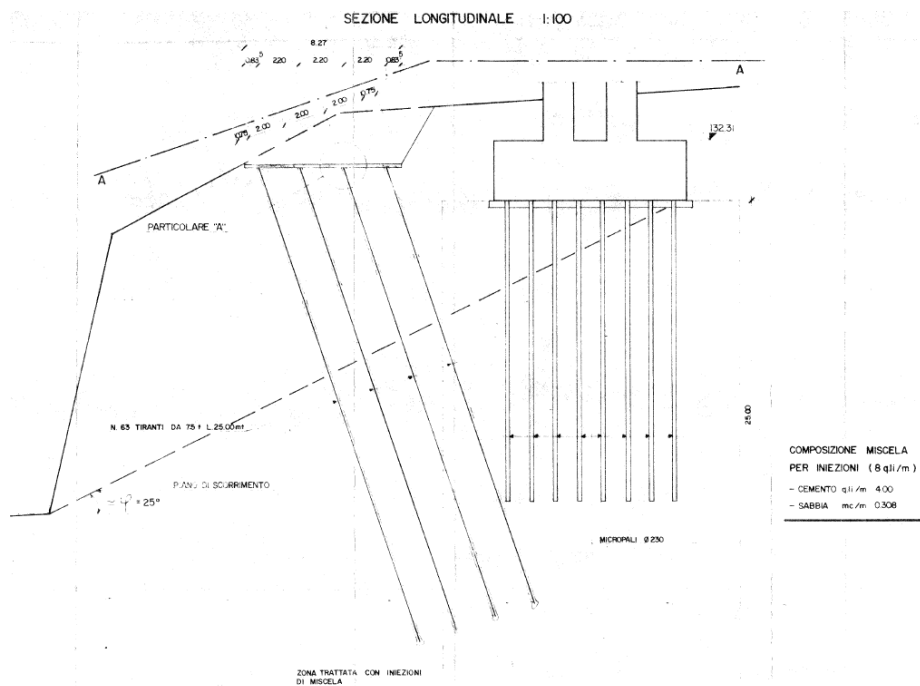


Figura 10 – Intervento di consolidamento scarpata Pila P22

Nella relazione di calcolo opere [DC3] per le fondazioni del viadotto si parla di "interventi di consolidamento" delle fondazioni; in particolare si afferma: "per le spalle e le pile con fondazione diretta sulle calcareniti sub-affioranti, sono adottati in rapporto ai problemi geotecnici interventi particolari" (vedasi interventi indicati nella seguente tabella). Come si evince dal documento [DC3], le fondazioni erano state dimensionate come dirette e gli interventi con micropali erano stati inseriti come consolidamento, definendo il numero di elementi in relazione alle pressioni massime in fondazione e definendo una portata massima dei micropali di 80 t (lunghezza micropali 15 m). L'intervento con iniezioni era finalizzato al riempimento di eventuali cavità/fratture presenti nell'ammasso roccioso interagente con l'opera.

Dalle tavole di progetto si osserva che comunque i micropali sono stati opportunamente ancorati entro il plinto di fondazione, quindi i micropali possono essere considerati come elemento di fondazione e non solo consolidamento.

5. CARATTERIZZAZIONE GEOTECNICA

Nel presente capitolo si riporta la caratterizzazione geotecnica per il viadotto in esame, valutata sulla base dell'interpretazione delle indagini geotecniche svolte in prossimità dell'opera. In generale lungo la linea Ferrandina – Matera sono disponibili le indagini del progetto del 1984, indagini integrative del 1987 nell'ammasso roccioso in corrispondenza delle pile di scavalco e le nuove indagini integrative eseguite per il presente progetto del 2018/2019 (ancora in fase di completamento).

5.1 Indagini pregresse

Lungo il viadotto sono disponibili alcuni sondaggi della vecchia campagna indagine del 1984: M15, M16, M17, M18 e M19, di cui M17 e M18 in corrispondenza delle pile di scavalco P21 e P22 del fiume.

Inoltre, in corrispondenza delle pile P21 e P22 sono stati effettuati sondaggi aggiuntivi per individuare la eventuale presenza di cavità carsiche. Di seguito si riporta una figura con ubicazione dei sondaggi.

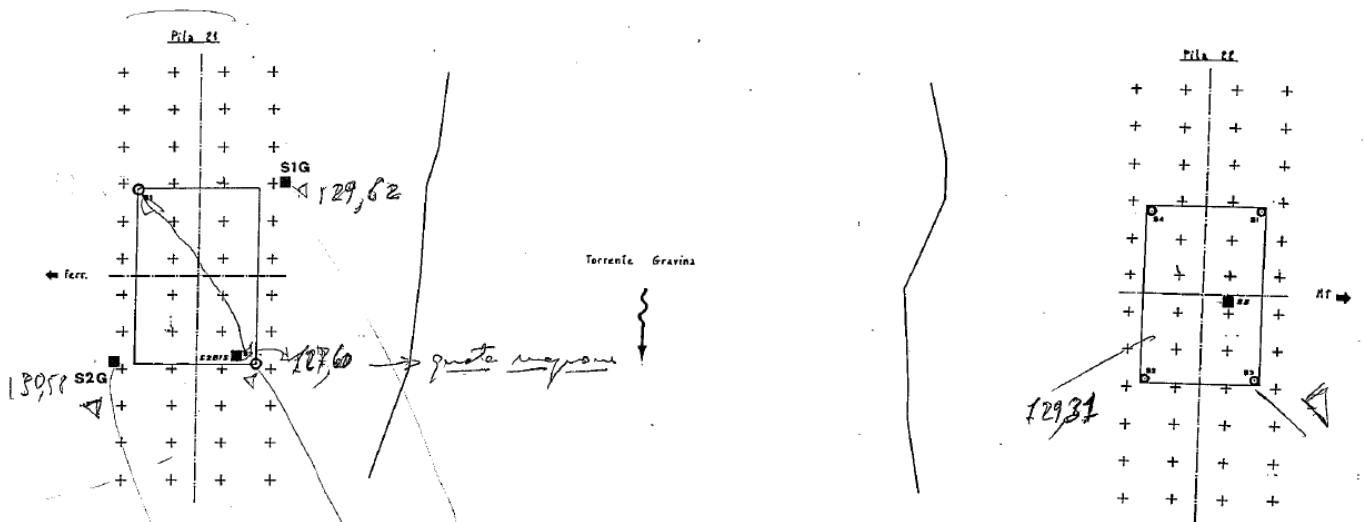


Figura 11: Sondaggi aggiuntivi per rilievo cavità carsiche

5.2 Indagini geotecniche eseguite nel 2018/2019

L'opera in esame è ubicata tra le progressive km 15+139.40 e km 16+030.31.

Sono disponibili le indagini integrative del 2018/2019 elencate nella seguente tabella. In particolare si osserva che:

- i sondaggi S10_DH_50 e S11_DH_50 sono stati eseguiti in corrispondenza del viadotto;
- il sondaggio S6_10 è ubicato poco prima della spalla A del viadotto.

Sondaggi / Indagini sismiche	Profondità [m]	Quota boccaforo [m] s.l.m.	n. campioni indisturbati	n. campioni rimaneggiati	N. prove SPT	n. prove Lefranc /Lugeon	n. prove pressiometriche	n. prove dilatometriche	Piezometro TA; CC / Prova DH/CH
S10_DH_50	30.0	140.949	1	4	3	2	1	-	DH [30]
S11_DH_50	50.0	134.580	-	7	1	2	-	2	DH [50]
S6_10	10.0	147.821	1	1	4	-	-	-	-
Masw10	-	-	-	-	-	-	-	-	-
Masw11	-	-	-	-	-	-	-	-	-
DH [m]: prova Down Hole [profondità prova]									

Non sono disponibili misure piezometriche in corrispondenza del viadotto in esame.

Durante la perforazione il sondaggio S10_DH_50 ha rilevato falda a 27 m da p.c., il sondaggio S11_DH_50 ha rilevato falda profonda, non interferente con le opere. Nel sondaggio S6_10 la falda è assente.

Il piezometro S9_PZ_30, installato nelle vicinanze dell'opera, indica falda a 3 m dal p.c..

Il livello di falda è stato assunto come indicato nel profilo geologico/geotecnico dell'opera.

5.3 Stratigrafia

I sondaggi della campagna integrativa del 2018/2019 evidenziano una copertura di depositi alluvionali terrazzati (unità 1e) di circa 1 m, poi le argille limose da molto consistenti a dure dell'unità 2 (ASP-Argille subappennine) fino a 8 m circa (S10_DH_50), poi le calcareniti di Gravina (unità 3) fino a 11.0 m per il sondaggio S10_DH_50 e fino a 18.0 m per il sondaggio S11_DH_50 con alterazione da media ad intensa, da intatta a fratturata (RQD tra 30 e 100%). La presenza di calcareniti è stata confermata anche dai sondaggi della campagna di indagine del 1984 (M17 e M18) che le intercettano fino alla profondità di 14÷16 m circa. A seguire si intercetta calcilutite (unità 4 – Cc11-8c Calcere di Altamura) a struttura compatta con alterazione da media ad elevata fino alla massima profondità investigata (50 m) (RQD tra 20 e 70%).

Di seguito si riporta uno stralcio del profilo geotecnico ricostruito sulla base delle nuove indagini dell'area.

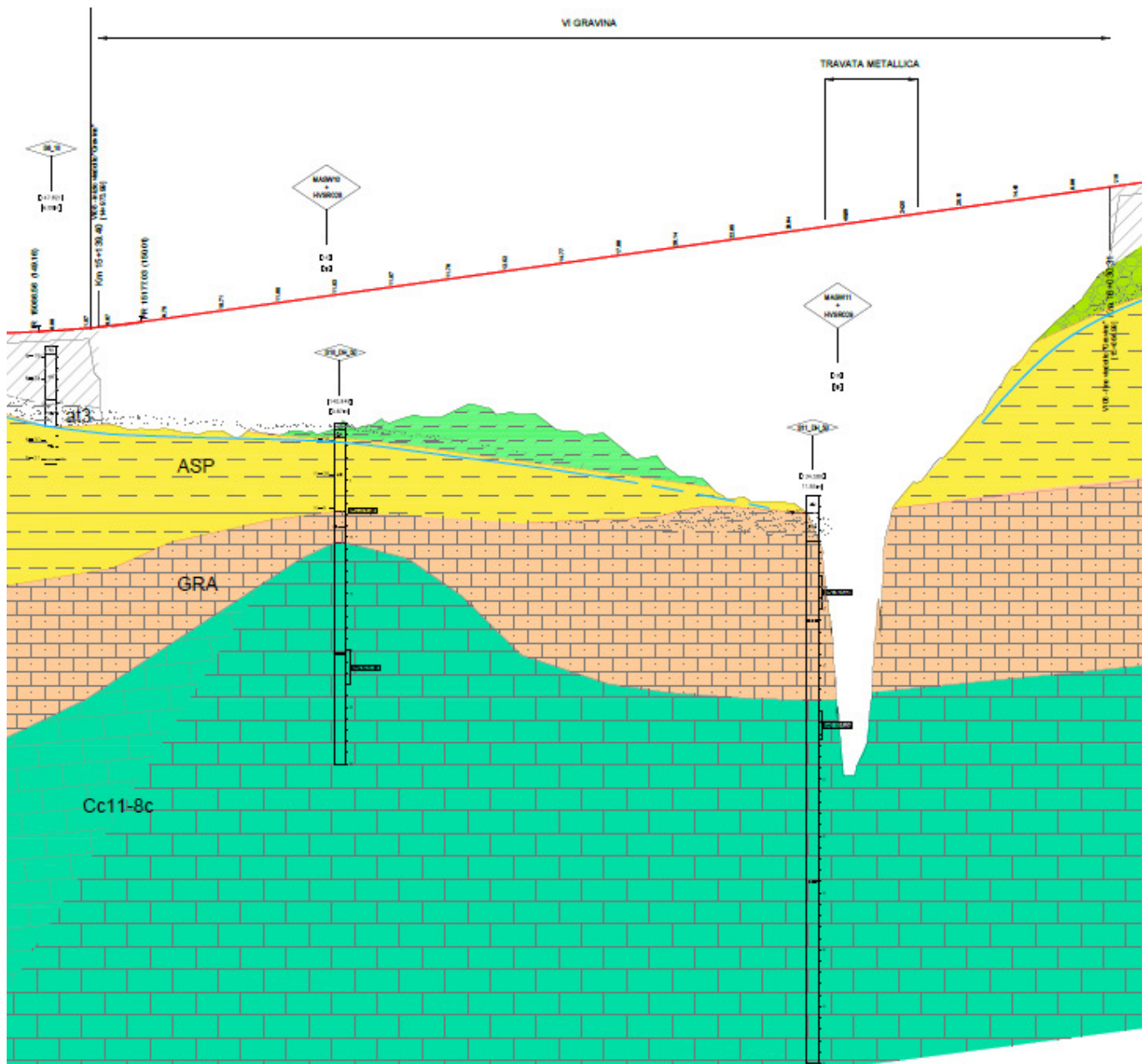


Figura 12 – Stralcio profilo geotecnico

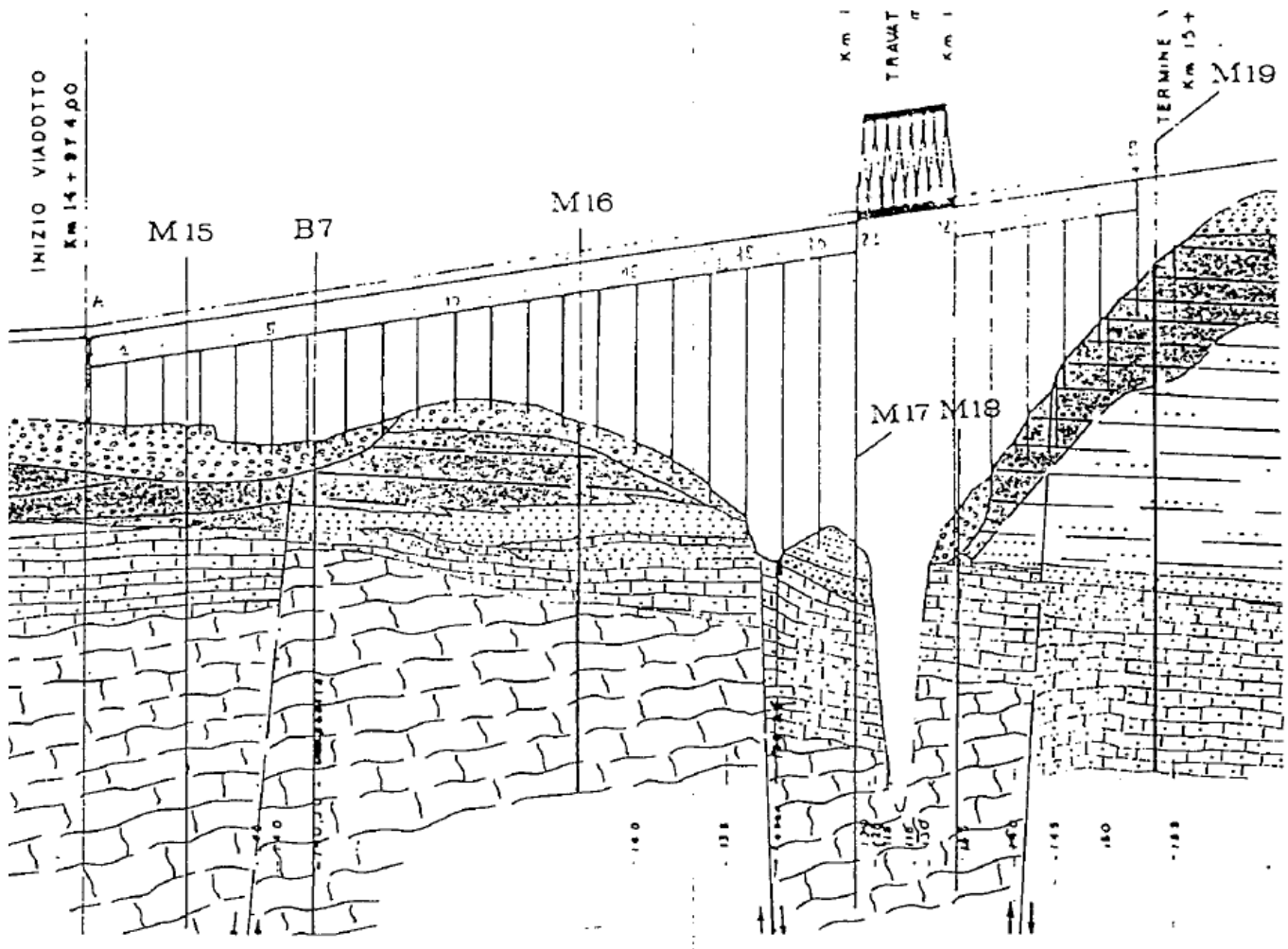


Figura 13 – Profilo geologico 1984

5.4 Sintesi dei parametri geotecnici di progetto

Nel seguito si riassumono i parametri geotecnici di progetto per le unità intercettate interagenti con le fondazioni.

Unità 1e – Depositi terrazzati del Fiume Bradano

$\gamma = 19.0 \text{ kN/m}^3$	peso di volume naturale
$c' = 0 \text{ kPa}$	coesione drenata
$\varphi' = 21 \div 23^\circ$	angolo di resistenza al taglio
$V_s = 130 \div 400 \text{ m/s}$	velocità delle onde di taglio
$E_o = 200 \div 500 \text{ MPa}$	modulo di deformazione elastico iniziale
$k = 3.5 \cdot 10^{-5} \div 4.5 \cdot 10^{-4} \text{ m/s}$	coefficiente di permeabilità

Unità 2 – Argille subappennine

$\gamma = 19.0 \div 21.0 \text{ kN/m}^3$	peso di volume naturale
$c' = 27 \div 32 \text{ kPa}$	coesione drenata
$\varphi' = 22 \div 24^\circ$	angolo di resistenza al taglio
$c_u = 100 \div 300 \text{ kPa}$	resistenza al taglio in condizioni non drenate
$V_s = 130 \div 210 \text{ m/s}$	velocità delle onde di taglio
$E_o = 120 \div 200 \text{ MPa}$	modulo di deformazione elastico iniziale
$k = 9.0 \cdot 10^{-8} \div 1.0 \cdot 10^{-7} \text{ m/s}$	coefficiente di permeabilità

Unità 3 – Calcareniti di Gravina

$\gamma = 19.0 \div 21.0 \text{ kN/m}^3$	peso di volume naturale
GSI = 37	Geological Strength Index
$\sigma_{ci} = 3 \text{ MPa}$	resistenza a compressione della roccia intatta
$m_i = 9$	costante per la roccia intatta
$c' = 20 \text{ kPa}$	coesione drenata
$\varphi' = 32^\circ$	angolo di resistenza al taglio
$V_s = 600 \div 1300 \text{ m/s}$	velocità delle onde di taglio
$E_o = 4000 \div 12300 \text{ MPa}$	modulo di deformazione elastico iniziale
$k = 8.0 \cdot 10^{-7} \div 1.0 \cdot 10^{-7} \text{ m/s}$	coefficiente di permeabilità

Unità 4 – Calcarea di Altamura

$\gamma = 24.0 \div 26.0 \text{ kN/m}^3$	peso di volume naturale
GSI = 45	Geological Strength Index
$\sigma_{ci} = 42 \text{ MPa}$	resistenza a compressione della roccia intatta
$m_i = 9$	costante per la roccia intatta
$c' = 130 \text{ kPa}$	coesione drenata
$\varphi' = 40^\circ$	angolo di resistenza al taglio
$V_s = 1400 \div 1800 \text{ m/s}$	velocità delle onde di taglio
$E_o = 15600 \div 21500 \text{ MPa}$	modulo di deformazione elastico iniziale
$k = 2.0 \cdot 10^{-7} \div 1.0 \cdot 10^{-5} \text{ m/s}$	coefficiente di permeabilità

I moduli di deformabilità "operativi" (E') da adottare per le opere di sostegno e per le fondazioni, saranno assunti pari a $1/3 \div 1/5$ di quello iniziale (E0).

I parametri di resistenza di progetto per le unità rocciose (calcareniti di Gravina e calcari di Bari) sono stati individuati con il programma "Roclab" a partire dalla caratterizzazione geomeccanica di base indicata nella relazione geotecnica generale valutata sulla base delle nuove indagini eseguite.

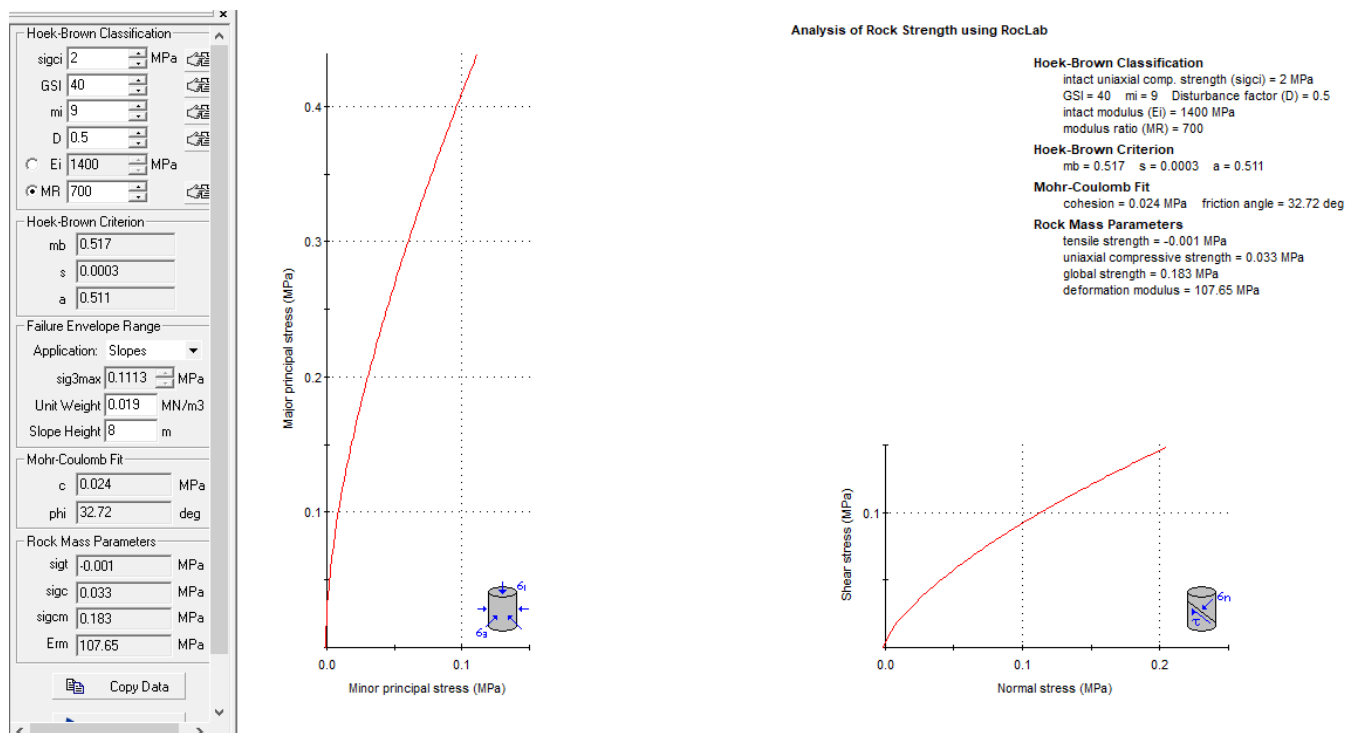
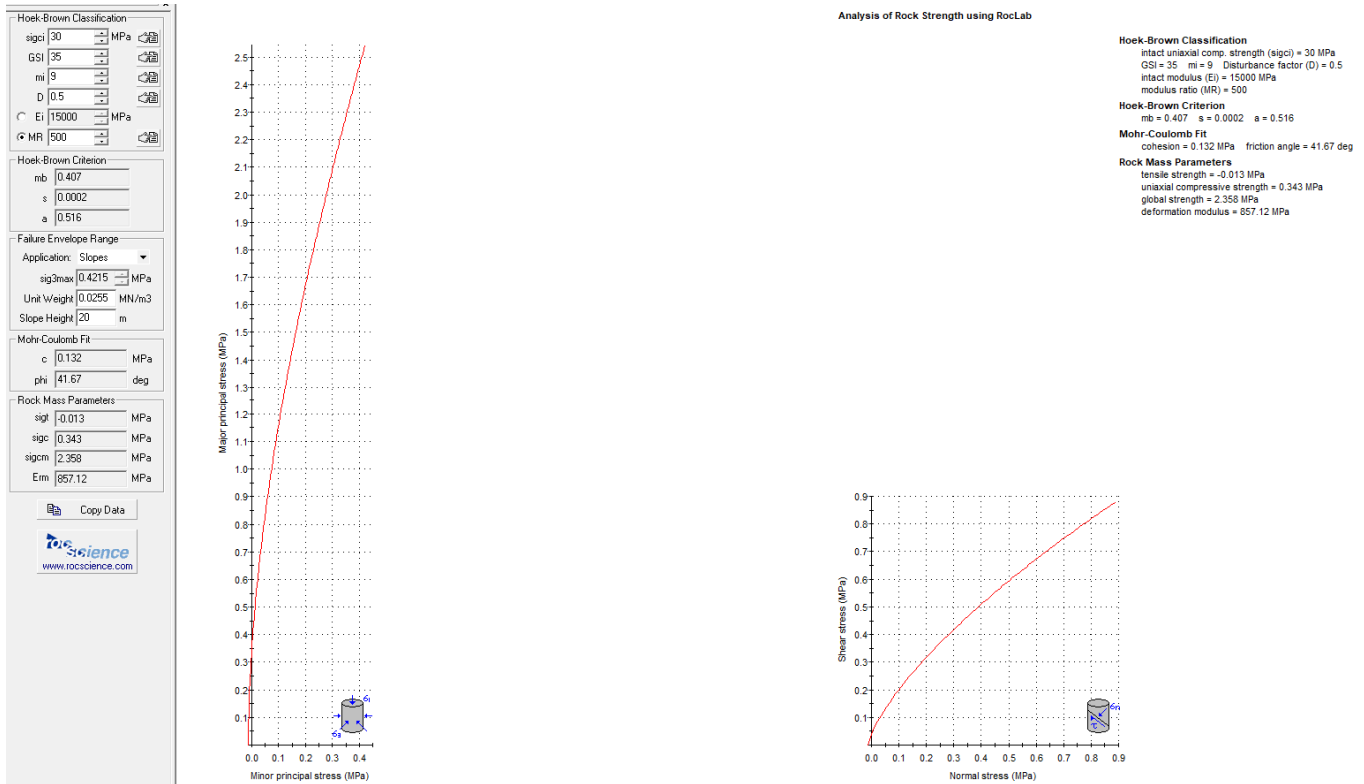


Figura 14 - Calcareniti di Gravina



5.5 Categoria di sottosuolo

Dalle indagini sismiche Down-Hole (S10_DH_50 e S11_DH_50) e dalle indagini sismiche Masw (Masw10 e Masw11), si definisce una categoria di sottosuolo sismica di tipo E con i seguenti valori di $V_{s,eq}$ (vedasi tabelle riportate di seguito):

- S10_DH_50: $V_{s,eq} = 208$ m/s
- S11_DH_50: $V_{s,eq} = 131$ m/s
- Masw10: $V_{s,eq} = 249$ m/s
- Masw11: $V_{s,eq} = 228$ m/s

Tabella 1 - S10_DH_50

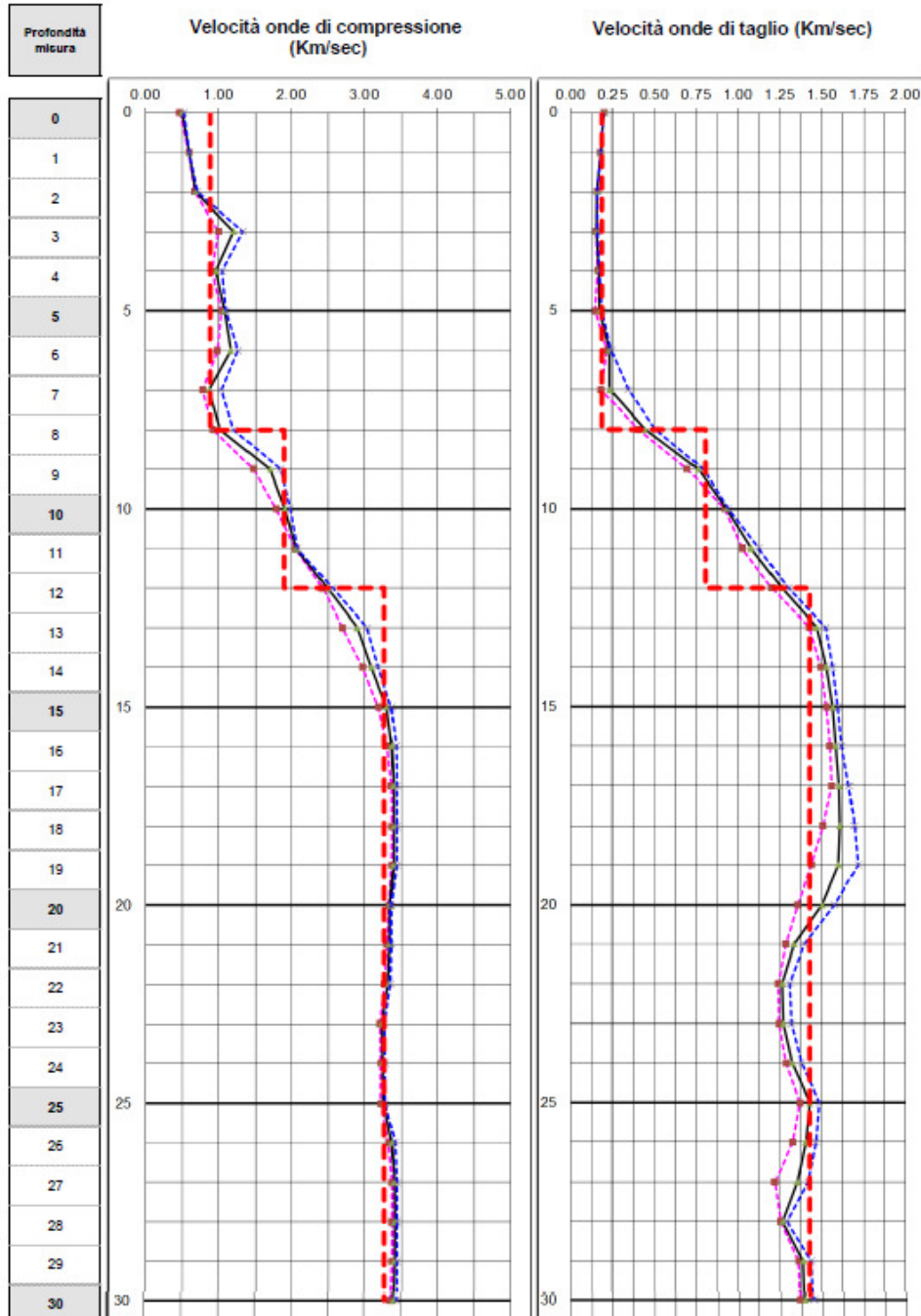


Tabella 2 - S11_DH_50

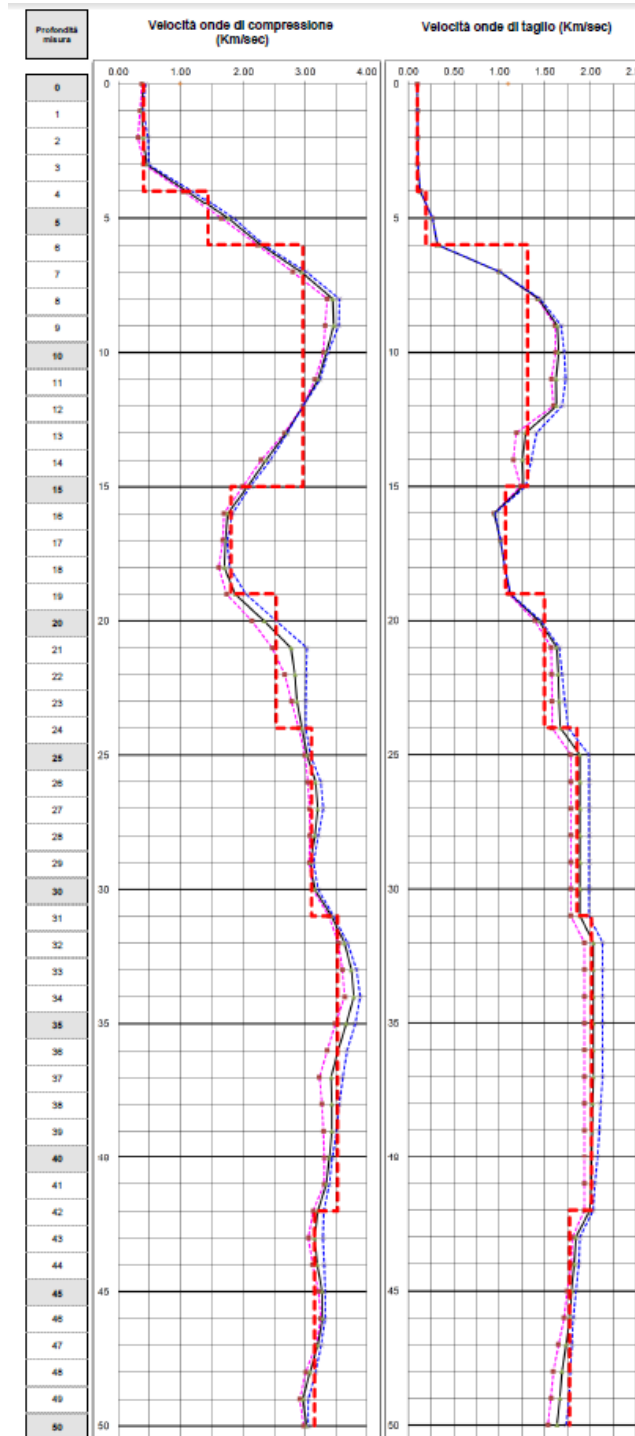
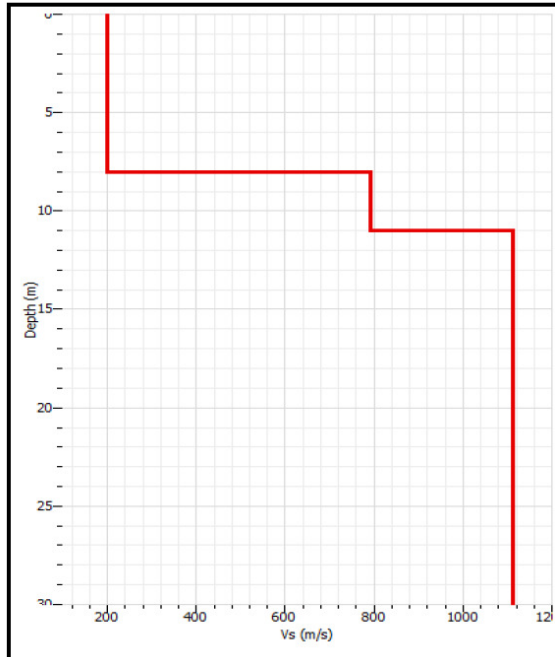


Tabella 3 - Masw10

PROFILO VELOCITA' ONDE DI TAGLIO



CALCOLO VS30

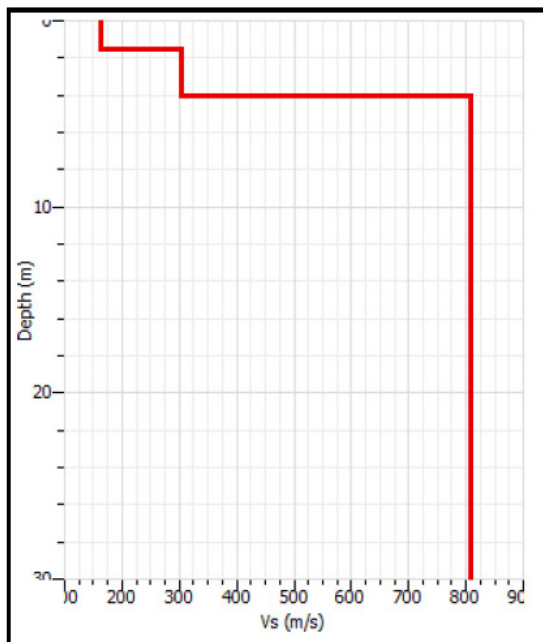
SPESSORE	PROFONDITA'	Vs	SPESSORE/Vs
8	0	198	0.04040404
3	8	790	0.003797468
19	11	1110	
	30		0.044201509

$V_{Seq11} = 249$

Seismic classification of soils
(It. D.M. 17/01/2018) **E**

Tabella 4 - Masw11

PROFILO VELOCITA' ONDE DI TAGLIO



CALCOLO VS30

SPESSORE	PROFONDITA'	Vs	SPESSORE/Vs
1.5	0	160	0.009375
2.6	1.5	301	0.008637874
25.9	4.1	810	
	30		0.018012874

$V_{Seq30} = 228$

Seismic classification of soils
(It. D.M. 17/01/2018) **E**

6. VALUTAZIONE CAPACITÀ PORTANTE MICROPALI

La verifica di capacità portante per i micropali è stata svolta con la metodologia di Bustamante e Doix, considerando l'Approccio 2 (A1+M1+R3) di normativa e quindi con i seguenti coefficienti parziali sulle resistenze di base e laterale:

- N. 5 verticale di indagine, da cui $\xi_3 = 1.50$,
- F_{SL} = fattore di sicurezza per la portata laterale a compressione ($=\xi_3 \cdot \gamma_s = 1.725$).
- $F_{SL,t}$ = fattore di sicurezza per la portata laterale a trazione ($=\xi_3 \cdot \gamma_s = 1.875$).
- Portata di base nulla.

Quindi per i micropali di fondazione si ha:

$La = 24.0$ m lunghezza attiva portante ($28.0-4.0 = 24.0$ m) delle pile P21-P22

$Dp = 230$ mm diametro di perforazione

Dalle tavole di progetto del 1985 risultano micropali con valvole ogni metro per iniezione a pressione. Comunque ipotizzando una corretta esecuzione dei micropali, secondo le indicazioni di progetto, si ipotizza un valore di resistenza di adesione micropalo-terreno: $\alpha\tau = 200$ kPa (Bustamante e Doix), da cui si stima:

P21-P22:

$$Rk = \pi \cdot Dp \cdot \alpha\tau \cdot La = 3466 \text{ kN} \quad \text{resistenza caratteristica}$$

$$Rd = Rk / F_{SL} = 2009 \text{ kN} \quad \text{resistenza di progetto a compressione}$$

$$Rdt = Rk / F_{SL,t} = 1848 \text{ kN} \quad \text{resistenza di progetto a trazione}$$

7. ANALISI PALIFICATE DI FONDAZIONE

7.1 Premessa

Nel presente capitolo si riporta l'analisi delle palificate di fondazione, le cui metodologie di calcolo sono dettagliatamente esposte nell'apposito documento [DC4]. Nel seguito si riportano quindi i principali elementi utilizzati per il calcolo, quali geometria palificata, sistema di riferimento globale e dei carichi, carichi, ed infine i risultati.

7.2 Metodologia di calcolo

Per l'analisi della fondazione su pali singoli è stato utilizzato il codice di calcolo MAP (G. Guiducci), le cui metodologie sono esposte nel documento [DC4].

Nelle seguenti figure sono riportati i sistemi di riferimento globale e locale con le convenzioni sui segni delle variabili adottate.

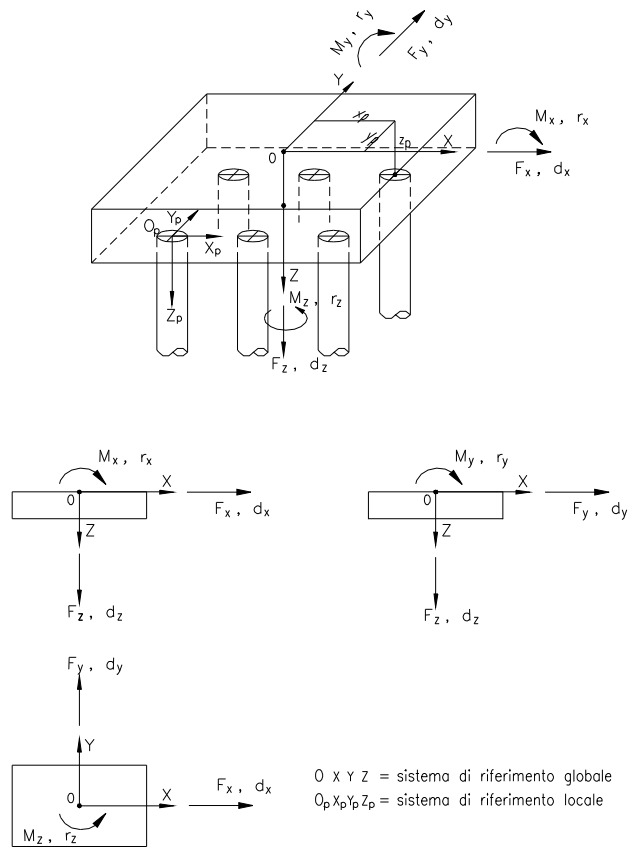


Figura 16– Definizione dei sistemi di riferimento globali adottati nelle analisi delle palificate

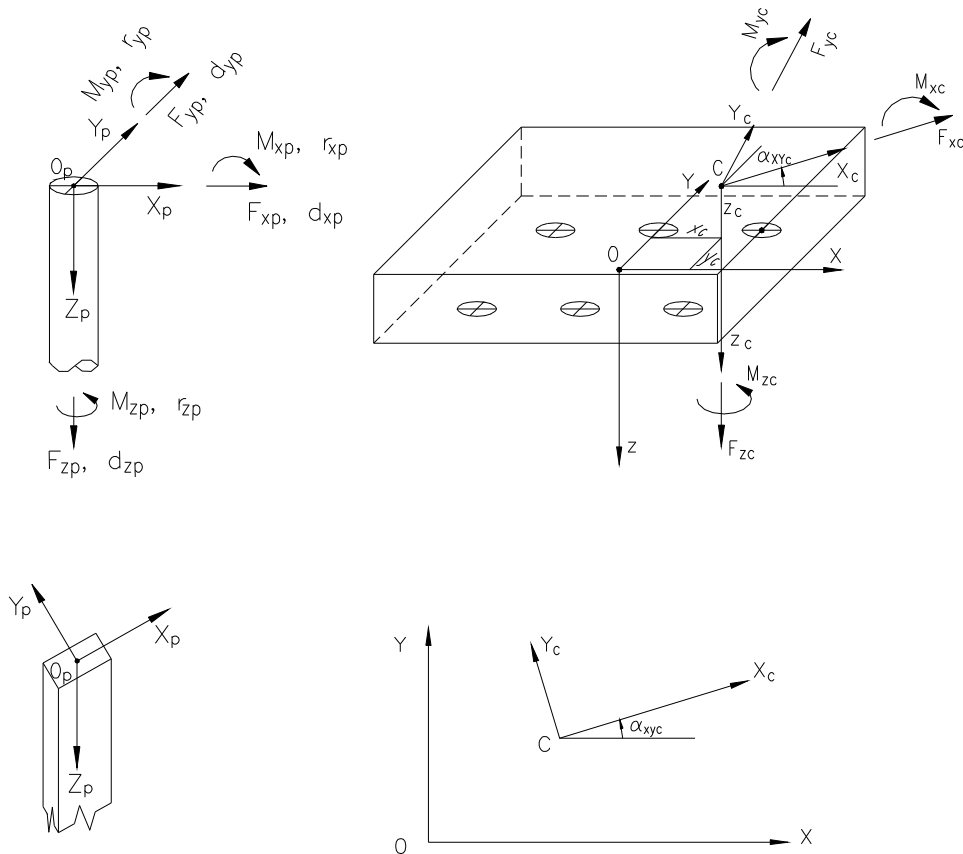


Figura 17– Definizione dei sistemi di riferimento locali e delle convenzioni sui centri di carico adottati nelle analisi delle palificate

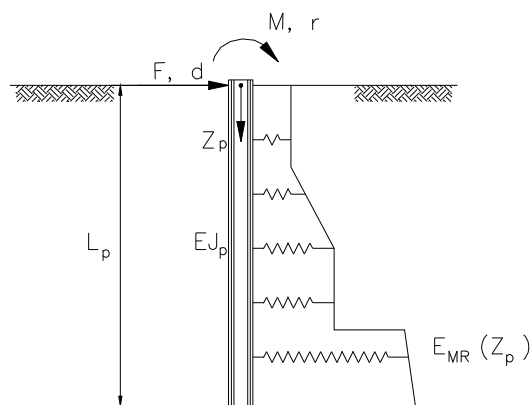


Figura 18– Pali soggetti a carichi trasversali: moduli di reazione del terreno

7.3 Caratterizzazione della palificata

La struttura della palificata è stata modellata ipotizzando un plinto infinitamente rigido. Per i pali di fondazione, la condizione di vincolo in testa è quella di incastro perfetto con il plinto. Ciascun micropalo di fondazione è stato modellato con una rigidità flessionale ottenuta considerando la somma dei due contributi: tubo in acciaio e malta di perforazione, da cui si ottiene:

- microapli esistenti e micropali integrativi di progetto ($D_p = 230$ mm tubo $d_e = 177.8$ mm, $s_p = 11$ mm):

$$EJ = (EJ)_{\text{tubo}} + (EJ)_{\text{malta}} = 4228 + 2747 = 6975 \text{ kN m}^2.$$

Comportamento del palo soggetto a carichi verticali

La curva carico-cedimento del singolo palo viene caratterizzata attraverso la seguente relazione lineare:

$$dz = [Fz / Ak]$$

dove:

dz = spostamento verticale a testa palo/micropalo;

Fz = carico assiale a testa palo/micropalo.

Per le palificate in esame è stata valutata la curva carico-cedimento del singolo palo/micropalo con il programma APAL che utilizza il metodo delle curve di trasferimento riferite al fusto ed alla base dei pali sviluppate da Reese e O'Neill (curve t-s e q-s). Nel caso in esame, si ottiene la curva carico-cedimento mostrata nella figura seguente. In Appendice A sono riportati i tabulati di calcolo completi. Quindi per le palificate in esame si assume la seguente rigidità assiale per i pali/micropali di fondazione:

$$Ak = 100000 \text{ kN/m per micropalo } D_p = 230 \text{ mm}$$

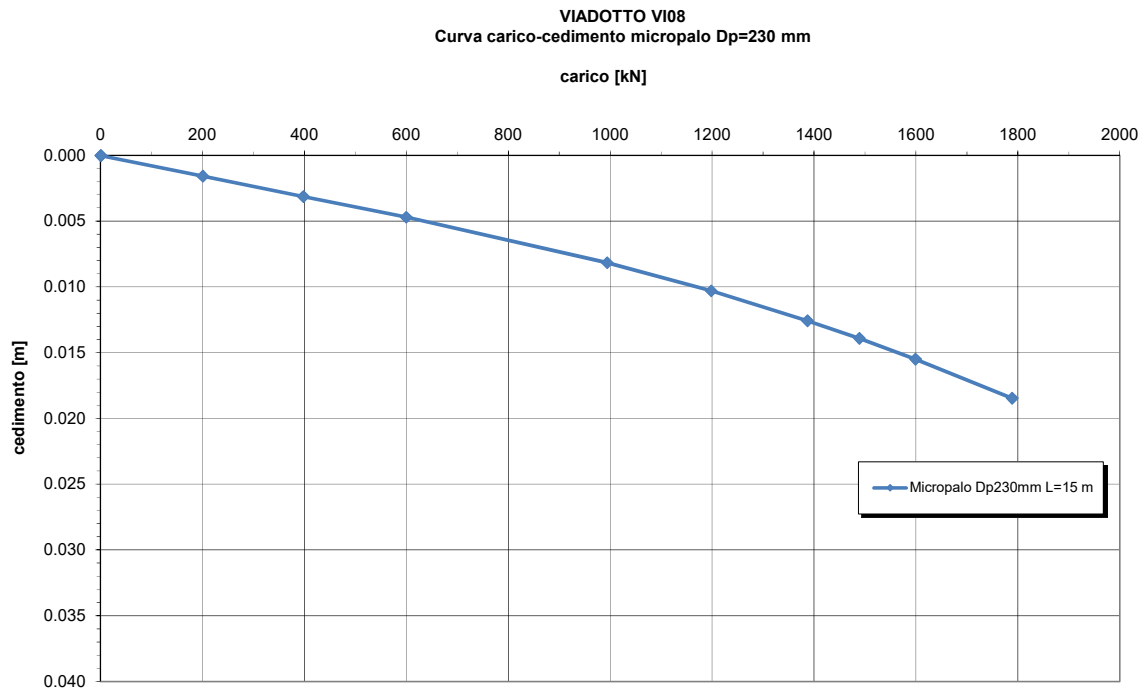


Figura 19– curva carico – cedimento - micropalo

Comportamento del palo soggetto ai carichi orizzontali

Lo studio dell'interazione tra palo soggetto ai carichi orizzontali ed il terreno viene effettuato ricorrendo alla teoria di Matlock e Reese che si basa sul noto modello di suolo alla Winkler (elastico-lineare), caratterizzato da un modulo di reazione orizzontale del terreno (E_{MR}) definito come il rapporto fra la reazione del terreno per unità di lunghezza del palo (p) ed il corrispondente spostamento orizzontale (y): $E_{MR} = p / y$. Definito il coefficiente di sottofondo alla Winkler (K_W), per un palo di diametro D , si ha questa relazione con il modulo di reazione orizzontale palo-terreno:

$$E_{MR} = K_W \cdot D$$

Nel caso in esame le fondazioni profonde interagiscono con il substrato calcarenitico per il quale si assume un valore massimo del coefficiente di Winkler di 100000 kN/m³.

Quindi si è assunto il seguente andamento per il modulo di reazione micropalo-terreno con la profondità a partire da testa micropalo (diametro di perforazione $D_p=230$ mm):

Prof. m	E kN/m ²
0.00	23000.0
40.00	23000.0

7.4 Carichi

Di seguito si riportano i carichi agenti a quota intradosso plinto, baricentro fondazione, forniti dal progettista strutturale per le palificate in esame.

Per le pile i carichi sono agenti a quota intradosso plinto, a baricentro fondazione, ma non includono il peso del plinto di fondazione, che è stato quindi valutato a parte con la seguente tabella.

Pila	L	B	h1	A	V1	Vtot	PP
	[m]	[m]	[m]	[m2]	[m3]	[m3]	[kN]
P21-P22	19.7	14.7	2.5	289.59	723.975	724.0	18099

7.5 Risultati P21-P22

Nella seguente tabella si sintetizzano le massime sollecitazioni a testa palo/micropalo per le condizioni di carico analizzate. Inoltre si riportano anche le deformazioni massime del plinto. In **Appendice B** si riportano i tabulati di calcolo completi.

Tabella 6 – pile P21÷P22 su micropali – Analisi SLV

LINEA FS FERRANDINA MATERA

VI08 Pile P21 e P22 - Analisi SLV

250 pali D = 230 mm

Sollecitazioni massime in sommita' ai pali

	Fz kN	M kN*m	T kN	palo	c.d.c.
S.1	653.2	20.9	47.5	1	5
S.2	-323.6	20.9	47.5	1	6
S.3	-62.5	33.0	68.9	16	7
S.4	-62.5	33.0	68.9	16	7
T.1	631.3	30.8	64.2	1	7
T.2	-301.8	30.8	64.2	1	8

S.1: cond. di carico con Sforzo Normale Massimo

VI08 - P22 - SLV_x

S.2: cond. di carico con Sforzo Normale Minimo

VI08 - P22 - SLV_x

S.3: cond. di carico con Momento Massimo

VI08 - P22 - SLV_y

S.4: cond. di carico con Taglio Massimo

VI08 - P22 - SLV_y

T.1: cond. di carico con Tensione Massima (sez. interamente reagente)

VI08 - P22 - SLV_y

T.2: cond. di carico con Tensione Minima (sez. interamente reagente)

VI08 - P22 - SLV_y

Deformazioni massime del plinto

	dz mm	dx mm	rx mRad	dy mm	ry mRad	c.d.c.
D.1	1.748	2.148	.558	.746	.115	1
D.2	1.587	-2.236	-.573	-.757	-.114	6
D.3	1.587	-2.236	-.573	-.757	-.114	6
D.4	1.713	.671	.171	2.522	.381	7
D.5	1.745	.645	.168	2.485	.382	3

D.1: cond. di carico con dz massimo

VI08 - P21 - SLV_x

D.2: cond. di carico con dx massimo

VI08 - P22 - SLV_x

D.3: cond. di carico con rx massimo

VI08 - P22 - SLV_x

D.4: cond. di carico con dy massimo

VI08 - P22 - SLV_y

D.5: cond. di carico con ry massimo

VI08 - P21 - SLV_y

8. VERIFICHE GEOTECNICHE

8.1 Verifica capacità portante fondazioni su pali

Le verifiche di capacità portante dei pali sono condotte in accordo alla normativa vigente (NTC 2018), in particolare si è fatto riferimento alla combinazione (A1+M1+R3). Nelle seguenti tabelle si sintetizzano le massime sollecitazioni in testa ai pali (N_{max}) per le analisi eseguite, le lunghezze dei pali e le relative portate di progetto a compressione (Q_d).

Le curve di capacità portante sono riportate al capitolo 6.

Le verifiche di portanza dei micropali sono riportate nella seguente tabella, da cui si osserva che le verifiche di portanza sono soddisfatte in quanto la resistenza di progetto è sempre maggiore della massima sollecitazione assiale di progetto sia a compressione che a trazione.

Tabella 7 – Verifica capacità portante micropali esistenti $D_p=230$ mm

Pila	N_{max} [kN] compressione	R_d [kN] compressione	$N_{max,t}$ [kN] trazione	$R_{d,t}$ [kN] trazione	$L_{micropalo}$ [m]
P21, P22	653	2009	324	1848	28.0

8.2 Verifica carico limite orizzontale

Per la verifica del carico limite orizzontale si fa generalmente riferimento alla teoria di Broms per il caso di pali con rotazione in testa impedita. Le metodologie di calcolo sono riportate nel documento [DC4].

Il valore della azione resistente di progetto (H_d) è valutato dal valore caratteristico della resistenza (H_k) con il fattore di correlazione $\xi_3 = 1.5$ (cinque verticali di indagine), il coefficiente parziale $\gamma_T = 1.3$ ed un fattore di effetto gruppo orizzontale assunto cautelativamente pari a 0.80.

Per la resistenza del terreno si considerano i valori medi di resistenza laterale lungo il palo, valutati in tensioni totali in quanto le condizioni di carico massime sono sismiche. Inoltre le fondazioni sono in roccia e quindi la resistenza verrà dedotta dalla resistenza a compressione monoassiale minima indicata per la roccia ($\sigma_c=3\text{MPa}$)

Nella seguente tabella si riassumono i risultati delle verifiche. La verifica a carico limite orizzontale è sempre soddisfatta, in quanto la massima azione di taglio di progetto, valutata con effetto gruppo orizzontale, risulta sempre inferiore all'azione di taglio resistente ($H_{max,EG} = H_{max}/0.8 < H_d = H_k / (\xi_3 \cdot \gamma_T)$).

OPERA	D [mm]	L [m]	My [kNm]	φ [°]	cu [kPa]	Hmax [kN]	Hk [kN]	Hd [kN]
P21-P22	230	28.0	67.4 (de=177.8mm sp=11mm)	-	1500 ($\sigma_c/2$)	337.55	173.1	69 (associato a trazione massima N=-63 kN)

My = momento di plasticizzazione.
 Hmax = azione massima orizzontale di progetto.
 $Hd = Hk / (\gamma_R \cdot \xi_3)$ = resistenza orizzontale di progetto.
 D = diametro palo.
 L = lunghezza palo.

9. VERIFICHE STRUTTURALI

9.1 Verifica dell'armatura tubolare in acciaio dei micropali

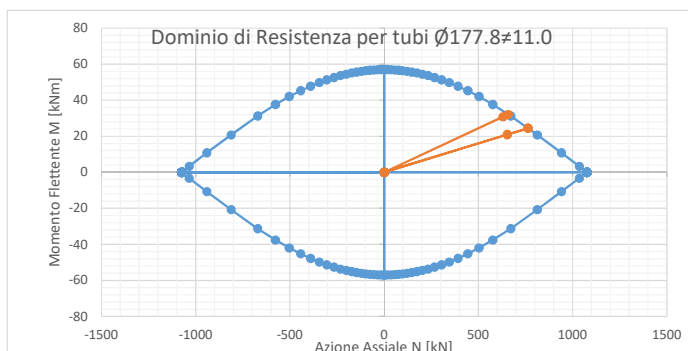
La verifica è svolta in accordo alle normative vigenti NTC 2018 con riferimento alle massime sollecitazioni individuate dall'analisi eseguita.

Tutti i micropali esistenti sono armati con tubo de= 177.8 mm sp=11 mm acciaio ex. Fe360 (S235).

Per i materiali delle opere esistenti si definisce livello di conoscenza LC = 2 e quindi nelle verifiche si assume un fattore di confidenza FC = 1.2.

Come si evince dalla seguente tabella la verifica di resistenza dell'armatura tubolare è soddisfatta per i micropali esistenti della fondazione delle pile P21-P22.

Sezione	AZIONI DI PROGETTO (F _{Ed})			RESISTENZE DI PROGETTO (R _d)						VERIFICA (>1)	
	N _{Ed} kN	M _{Ed} kNm	V _{Ed} kN	N _{pl,Rd} kN	M _{c,Rd} kNm	V _{c,Rd} kN	M _{y,V,Rd} kNm	M _{N,y,Rd} kNm	M _{y,V,Rd} kNm	(R _d /F _{Ed}) _{min}	
-										--	
P22 - cdc SLV-x	653	21	48	1075	57	395	57	24	24	1.17	OK
P22 - cdc SLV-y	631	31	64	1075	57	395	57	32	32	1.04	OK



10. APPENDICE A: ANALISI CURVA CARICO-CEDIMENTO. TABULATI DI CALCOLO APAL

10.1 Micropalo esistente DP230mm

GRAVINA D230mm

Caratteristiche palo

Lunghezza = 15.00 m
 Diametro = .23 m
 Modulo elastico = 20000000.00 kPa
 Peso di volume immerso = 5.00 kN/m³

Carico in sommità al palo = .00 kN

Caratteristiche alla base del palo

Pressione limite specifica = 5000.000 kPa
 Parametro deformazione limite = .200
 cedimento per press. lim. = .200 x D = 46. mm
 Parametro forma curva = .090

Caratteristiche di adesione laterale

prof. m	ades. limite kPa	s. lim. mm
.00	100.0	5.0
3.00	100.0	5.0
3.10	220.0	5.0
20.00	220.0	5.0

Definizione cedimenti del terreno

prof. m	cedimento mm
.00	.01
.50	.01
1.00	.01
1.50	.01
2.00	.01
2.50	.01
3.00	.01
3.50	.01
4.00	.01
4.50	.01
5.00	.01
5.50	.01
6.00	.01
6.50	.01
7.00	.01
7.50	.01
8.00	.01
8.50	.01
9.00	.01
9.50	.01
10.00	.01
10.50	.01
11.00	.01

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	.7	.01
.50	.7	.01
1.00	.7	.01
1.50	.7	.01
2.00	.8	.01
2.50	.8	.01
3.00	.8	.01
3.50	.7	.01
4.00	.7	.01
4.50	.6	.01
5.00	.6	.01
5.50	.5	.01
6.00	.5	.01
6.50	.4	.01
7.00	.4	.01
7.50	.4	.01
8.00	.4	.01
8.50	.3	.01
9.00	.3	.01
9.50	.3	.01
10.00	.3	.01
10.50	.3	.01
11.00	.3	.01

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	40 di 91

11.50	.3	.01
12.00	.3	.01
12.50	.3	.01
13.00	.3	.01
13.50	.3	.01
14.00	.3	.01
14.50	.3	.01
15.00	.3	.01

Carico in sommità al palo = 1.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	.7	.01
.50	.7	.01
1.00	.7	.01
1.50	.7	.01
2.00	.8	.01
2.50	.8	.01
3.00	.8	.01
3.50	.7	.01
4.00	.7	.01
4.50	.6	.01
5.00	.6	.01
5.50	.5	.01
6.00	.5	.01
6.50	.4	.01
7.00	.4	.01
7.50	.4	.01
8.00	.4	.01
8.50	.3	.01
9.00	.3	.01
9.50	.3	.01
10.00	.3	.01
10.50	.3	.01
11.00	.3	.01
11.50	.3	.01
12.00	.3	.01
12.50	.3	.01
13.00	.3	.01
13.50	.3	.01
14.00	.3	.01
14.50	.3	.01
15.00	.3	.01

Carico in sommità al palo = 200.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	200.9	1.58
.50	190.5	1.46
1.00	180.8	1.35
1.50	171.9	1.25
2.00	163.7	1.15
2.50	156.3	1.05
3.00	149.4	.96
3.50	135.7	.87
4.00	123.2	.79
4.50	111.8	.72
5.00	101.5	.66
5.50	92.0	.60

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	41 di 91

6.00	83.4	.55
6.50	75.6	.50
7.00	68.4	.46
7.50	61.9	.42
8.00	56.0	.38
8.50	50.5	.35
9.00	45.5	.32
9.50	40.9	.29
10.00	36.7	.27
10.50	32.9	.25
11.00	29.3	.23
11.50	26.0	.21
12.00	22.9	.20
12.50	20.0	.19
13.00	17.3	.18
13.50	14.8	.17
14.00	12.4	.16
14.50	10.1	.15
15.00	7.9	.15

Carico in sommità al palo = 400.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	398.1	3.13
.50	377.3	2.89
1.00	358.1	2.67
1.50	340.4	2.46
2.00	324.2	2.26
2.50	309.3	2.07
3.00	295.8	1.89
3.50	268.6	1.72
4.00	243.8	1.56
4.50	221.3	1.42
5.00	200.7	1.30
5.50	182.1	1.18
6.00	165.0	1.08
6.50	149.5	.98
7.00	135.4	.90
7.50	122.4	.82
8.00	110.6	.75
8.50	99.8	.69
9.00	89.9	.63
9.50	80.8	.58
10.00	72.5	.53
10.50	64.8	.49
11.00	57.7	.45
11.50	51.1	.42
12.00	45.0	.39
12.50	39.3	.37
13.00	33.9	.34
13.50	28.8	.33
14.00	24.0	.31
14.50	19.4	.30
15.00	14.9	.29

Carico in sommità al palo = 600.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	600.0	4.71

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	42 di 91

.50	568.7	4.36
1.00	539.7	4.02
1.50	513.0	3.71
2.00	488.5	3.40
2.50	466.1	3.12
3.00	445.7	2.84
3.50	404.7	2.59
4.00	367.4	2.35
4.50	333.4	2.14
5.00	302.5	1.95
5.50	274.3	1.78
6.00	248.6	1.62
6.50	225.2	1.48
7.00	203.9	1.35
7.50	184.4	1.23
8.00	166.5	1.13
8.50	150.2	1.03
9.00	135.3	.95
9.50	121.6	.87
10.00	109.0	.80
10.50	97.4	.74
11.00	86.6	.68
11.50	76.7	.63
12.00	67.4	.59
12.50	58.8	.55
13.00	50.6	.52
13.50	42.9	.49
14.00	35.6	.47
14.50	28.6	.45
15.00	21.8	.43

Carico in sommità al palo = 1000.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	994.4	8.16
.50	958.4	7.57
1.00	922.3	7.01
1.50	886.3	6.46
2.00	850.3	5.94
2.50	814.3	5.44
3.00	778.5	4.96
3.50	706.8	4.52
4.00	641.6	4.11
4.50	582.2	3.74
5.00	528.1	3.41
5.50	478.9	3.10
6.00	434.0	2.83
6.50	393.1	2.58
7.00	355.7	2.36
7.50	321.6	2.15
8.00	290.4	1.97
8.50	261.9	1.80
9.00	235.8	1.65
9.50	211.7	1.52
10.00	189.6	1.40
10.50	169.3	1.29
11.00	150.4	1.19
11.50	132.9	1.11
12.00	116.6	1.03
12.50	101.3	.97
13.00	87.0	.91
13.50	73.4	.86
14.00	60.4	.82
14.50	48.0	.79
15.00	35.9	.76

Carico in sommità al palo = 1200.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	1198.2	10.30
.50	1162.2	9.59
1.00	1126.2	8.90
1.50	1090.1	8.23
2.00	1054.1	7.59
2.50	1018.1	6.96
3.00	982.1	6.36
3.50	902.7	5.79
4.00	823.3	5.27
4.50	747.1	4.80
5.00	677.7	4.37
5.50	614.4	3.98
6.00	556.8	3.63
6.50	504.2	3.31
7.00	456.3	3.02
7.50	412.5	2.76
8.00	372.4	2.53
8.50	335.7	2.31
9.00	302.1	2.12
9.50	271.2	1.95
10.00	242.8	1.79
10.50	216.6	1.66
11.00	192.3	1.53
11.50	169.8	1.42
12.00	148.7	1.33
12.50	129.1	1.24
13.00	110.5	1.17
13.50	93.0	1.11
14.00	76.2	1.06
14.50	60.1	1.02
15.00	44.5	.99

Carico in sommità al palo = 1400.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	1387.2	12.57
.50	1351.2	11.74
1.00	1315.2	10.94
1.50	1279.2	10.16
2.00	1243.1	9.40
2.50	1207.1	8.66
3.00	1171.1	7.95
3.50	1091.7	7.27
4.00	1012.3	6.64
4.50	932.9	6.05
5.00	853.6	5.51
5.50	774.2	5.02
6.00	701.5	4.58
6.50	635.2	4.18
7.00	574.7	3.81
7.50	519.4	3.48
8.00	468.9	3.19
8.50	422.6	2.92
9.00	380.1	2.68
9.50	341.1	2.46
10.00	305.2	2.27
10.50	272.1	2.09
11.00	241.4	1.94
11.50	212.9	1.80
12.00	186.3	1.68
12.50	161.3	1.58
13.00	137.8	1.49
13.50	115.5	1.41
14.00	94.2	1.35
14.50	73.7	1.30

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	44 di 91

15.00 53.8 1.26

Carico in sommità al palo = 1500.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	1489.0	13.92
.50	1453.0	13.04
1.00	1416.9	12.17
1.50	1380.9	11.33
2.00	1344.9	10.51
2.50	1308.9	9.71
3.00	1272.8	8.94
3.50	1193.5	8.20
4.00	1114.1	7.50
4.50	1034.7	6.86
5.00	955.3	6.26
5.50	875.9	5.71
6.00	796.6	5.20
6.50	721.2	4.75
7.00	652.5	4.33
7.50	589.6	3.96
8.00	532.2	3.62
8.50	479.6	3.32
9.00	431.3	3.04
9.50	387.0	2.80
10.00	346.1	2.58
10.50	308.4	2.38
11.00	273.5	2.20
11.50	241.0	2.05
12.00	210.7	1.91
12.50	182.3	1.80
13.00	155.4	1.69
13.50	130.0	1.61
14.00	105.7	1.54
14.50	82.2	1.48
15.00	59.5	1.44

Carico in sommità al palo = 1600.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	1599.3	15.49
.50	1563.3	14.54
1.00	1527.2	13.61
1.50	1491.2	12.70
2.00	1455.2	11.81
2.50	1419.2	10.95
3.00	1383.1	10.10
3.50	1303.8	9.30
4.00	1224.4	8.54
4.50	1145.0	7.82
5.00	1065.6	7.16
5.50	986.2	6.54
6.00	906.9	5.97
6.50	827.5	5.45
7.00	748.5	4.97
7.50	676.4	4.55
8.00	610.4	4.16
8.50	549.9	3.81
9.00	494.4	3.50

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	45 di 91

9.50	443.5	3.21
10.00	396.5	2.96
10.50	353.1	2.74
11.00	312.9	2.53
11.50	275.6	2.36
12.00	240.7	2.20
12.50	207.9	2.07
13.00	177.0	1.95
13.50	147.6	1.85
14.00	119.5	1.77
14.50	92.4	1.71
15.00	66.1	1.66

Carico in sommità al palo = 1800.00 kN

STAMPA DEI RISULTATI

prof. m	sforzo normale kN	cedimento mm
.00	1788.9	18.47
.50	1752.9	17.40
1.00	1716.9	16.36
1.50	1680.9	15.34
2.00	1644.9	14.34
2.50	1608.8	13.36
3.00	1572.8	12.40
3.50	1493.4	11.48
4.00	1414.0	10.60
4.50	1334.7	9.77
5.00	1255.3	9.00
5.50	1175.9	8.26
6.00	1096.5	7.58
6.50	1017.2	6.94
7.00	937.8	6.36
7.50	858.4	5.82
8.00	779.0	5.32
8.50	701.6	4.88
9.00	630.5	4.48
9.50	565.2	4.12
10.00	504.9	3.80
10.50	449.3	3.51
11.00	397.6	3.25
11.50	349.6	3.03
12.00	304.7	2.83
12.50	262.5	2.66
13.00	222.6	2.52
13.50	184.7	2.39
14.00	148.3	2.29
14.50	113.2	2.21
15.00	79.0	2.16

STAMPA tabella carichi-cedimenti a testa palo

carico kN	cedimento mm
.7	.01
.7	.01
200.9	1.58
398.1	3.13
600.0	4.71
994.4	8.16
1198.2	10.30
1387.2	12.57
1489.0	13.92
1599.3	15.49
1788.9	18.47

STAMPA tabella carichi a testa palo-sforzi normali massimi

carico a testa palo kN	sforzo normale massimo kN	profondita' sf. massimo m
.7	.8	3.00
.7	.8	3.00
200.9	200.9	.00
398.1	398.1	.00
600.0	600.0	.00
994.4	994.4	.00
1198.2	1198.2	.00
1387.2	1387.2	.00
1489.0	1489.0	.00
1599.3	1599.3	.00
1788.9	1788.9	.00

11. APPENDICE B: ANALISI DELLE PALIFICATE DI FONDAZIONE. TABULATI DI CALCOLO MAP

11.1 Pile P20, P21 - Analisi SLV

M A P - Matrix Analysis of Piles
 Programma per l'analisi di palificate collegate da un plinto rigido
 (C) G.Guiducci, S.G.I. - luglio 1994

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LINEA FS FERRANDINA MATERA
 VI08 Pile P21 e P22 - Analisi SLV

Geometria Palificata

palo	vin	X m	Y m	Z m	axz deg	ayz deg	axy deg	Box m	Boy m
1	0	6.600	9.100	.000	.00	.00	.00	.23	.00
2	0	6.600	7.800	.000	.00	.00	.00	.23	.00
3	0	6.600	7.150	.000	.00	.00	.00	.23	.00
4	0	6.600	5.850	.000	.00	.00	.00	.23	.00
5	0	6.600	4.550	.000	.00	.00	.00	.23	.00
6	0	6.600	3.250	.000	.00	.00	.00	.23	.00
7	0	6.600	1.950	.000	.00	.00	.00	.23	.00
8	0	6.600	.650	.000	.00	.00	.00	.23	.00
9	0	6.600	-.650	.000	.00	.00	.00	.23	.00
10	0	6.600	-1.950	.000	.00	.00	.00	.23	.00
11	0	6.600	-3.250	.000	.00	.00	.00	.23	.00
12	0	6.600	-4.550	.000	.00	.00	.00	.23	.00
13	0	6.600	-5.850	.000	.00	.00	.00	.23	.00
14	0	6.600	-7.150	.000	.00	.00	.00	.23	.00
15	0	6.600	-7.800	.000	.00	.00	.00	.23	.00
16	0	6.600	-9.100	.000	.00	.00	.00	.23	.00
17	0	6.000	8.450	.000	.00	.00	.00	.23	.00
18	0	6.000	6.500	.000	.00	.00	.00	.23	.00
19	0	6.000	5.200	.000	.00	.00	.00	.23	.00
20	0	6.000	3.900	.000	.00	.00	.00	.23	.00
21	0	6.000	2.600	.000	.00	.00	.00	.23	.00
22	0	6.000	1.300	.000	.00	.00	.00	.23	.00

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	47 di 91

23	0	6.000	.000	.000	.00	.00	.00	.23	.00
24	0	6.000	-1.300	.000	.00	.00	.00	.23	.00
25	0	6.000	-2.600	.000	.00	.00	.00	.23	.00
26	0	6.000	-3.900	.000	.00	.00	.00	.23	.00
27	0	6.000	-5.200	.000	.00	.00	.00	.23	.00
28	0	6.000	-6.500	.000	.00	.00	.00	.23	.00
29	0	6.000	-8.450	.000	.00	.00	.00	.23	.00
30	0	5.400	9.100	.000	.00	.00	.00	.23	.00
31	0	5.400	7.800	.000	.00	.00	.00	.23	.00
32	0	5.400	7.150	.000	.00	.00	.00	.23	.00
33	0	5.400	5.850	.000	.00	.00	.00	.23	.00
34	0	5.400	4.550	.000	.00	.00	.00	.23	.00
35	0	5.400	3.250	.000	.00	.00	.00	.23	.00
36	0	5.400	1.950	.000	.00	.00	.00	.23	.00
37	0	5.400	.650	.000	.00	.00	.00	.23	.00
38	0	5.400	-.650	.000	.00	.00	.00	.23	.00
39	0	5.400	-1.950	.000	.00	.00	.00	.23	.00
40	0	5.400	-3.250	.000	.00	.00	.00	.23	.00
41	0	5.400	-4.550	.000	.00	.00	.00	.23	.00
42	0	5.400	-5.850	.000	.00	.00	.00	.23	.00
43	0	5.400	-7.150	.000	.00	.00	.00	.23	.00
44	0	5.400	-7.800	.000	.00	.00	.00	.23	.00
45	0	5.400	-9.100	.000	.00	.00	.00	.23	.00
46	0	4.800	9.100	.000	.00	.00	.00	.23	.00
47	0	4.800	7.800	.000	.00	.00	.00	.23	.00
48	0	4.800	7.150	.000	.00	.00	.00	.23	.00
49	0	4.800	5.850	.000	.00	.00	.00	.23	.00
50	0	4.800	4.550	.000	.00	.00	.00	.23	.00
51	0	4.800	3.250	.000	.00	.00	.00	.23	.00
52	0	4.800	1.950	.000	.00	.00	.00	.23	.00
53	0	4.800	.650	.000	.00	.00	.00	.23	.00
54	0	4.800	-.650	.000	.00	.00	.00	.23	.00
55	0	4.800	-1.950	.000	.00	.00	.00	.23	.00
56	0	4.800	-3.250	.000	.00	.00	.00	.23	.00
57	0	4.800	-4.550	.000	.00	.00	.00	.23	.00
58	0	4.800	-5.850	.000	.00	.00	.00	.23	.00
59	0	4.800	-7.150	.000	.00	.00	.00	.23	.00
60	0	4.800	-7.800	.000	.00	.00	.00	.23	.00
61	0	4.800	-9.100	.000	.00	.00	.00	.23	.00
62	0	4.200	8.450	.000	.00	.00	.00	.23	.00
63	0	4.200	-8.450	.000	.00	.00	.00	.23	.00
64	0	3.600	9.100	.000	.00	.00	.00	.23	.00
65	0	3.600	7.800	.000	.00	.00	.00	.23	.00
66	0	3.600	7.150	.000	.00	.00	.00	.23	.00
67	0	3.600	5.850	.000	.00	.00	.00	.23	.00
68	0	3.600	4.550	.000	.00	.00	.00	.23	.00
69	0	3.600	3.250	.000	.00	.00	.00	.23	.00
70	0	3.600	1.950	.000	.00	.00	.00	.23	.00
71	0	3.600	.650	.000	.00	.00	.00	.23	.00
72	0	3.600	-.650	.000	.00	.00	.00	.23	.00
73	0	3.600	-1.950	.000	.00	.00	.00	.23	.00
74	0	3.600	-3.250	.000	.00	.00	.00	.23	.00
75	0	3.600	-4.550	.000	.00	.00	.00	.23	.00
76	0	3.600	-5.850	.000	.00	.00	.00	.23	.00
77	0	3.600	-7.150	.000	.00	.00	.00	.23	.00
78	0	3.600	-7.800	.000	.00	.00	.00	.23	.00
79	0	3.600	-9.100	.000	.00	.00	.00	.23	.00
80	0	3.000	8.450	.000	.00	.00	.00	.23	.00
81	0	3.000	-8.450	.000	.00	.00	.00	.23	.00
82	0	2.400	9.100	.000	.00	.00	.00	.23	.00
83	0	2.400	7.800	.000	.00	.00	.00	.23	.00
84	0	2.400	7.150	.000	.00	.00	.00	.23	.00
85	0	2.400	5.850	.000	.00	.00	.00	.23	.00
86	0	2.400	4.550	.000	.00	.00	.00	.23	.00
87	0	2.400	3.250	.000	.00	.00	.00	.23	.00
88	0	2.400	1.950	.000	.00	.00	.00	.23	.00
89	0	2.400	.650	.000	.00	.00	.00	.23	.00
90	0	2.400	-.650	.000	.00	.00	.00	.23	.00
91	0	2.400	-1.950	.000	.00	.00	.00	.23	.00
92	0	2.400	-3.250	.000	.00	.00	.00	.23	.00
93	0	2.400	-4.550	.000	.00	.00	.00	.23	.00
94	0	2.400	-5.850	.000	.00	.00	.00	.23	.00
95	0	2.400	-7.150	.000	.00	.00	.00	.23	.00
96	0	2.400	-7.800	.000	.00	.00	.00	.23	.00
97	0	2.400	-9.100	.000	.00	.00	.00	.23	.00
98	0	1.800	8.450	.000	.00	.00	.00	.23	.00

Relazione di calcolo fondazioni dirette su micropali
 P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	48 di 91

99	0	1.800	-8.450	.000	.00	.00	.00	.23	.00
100	0	1.200	9.100	.000	.00	.00	.00	.23	.00
101	0	1.200	7.800	.000	.00	.00	.00	.23	.00
102	0	1.200	7.150	.000	.00	.00	.00	.23	.00
103	0	1.200	5.850	.000	.00	.00	.00	.23	.00
104	0	1.200	4.550	.000	.00	.00	.00	.23	.00
105	0	1.200	3.250	.000	.00	.00	.00	.23	.00
106	0	1.200	1.950	.000	.00	.00	.00	.23	.00
107	0	1.200	.650	.000	.00	.00	.00	.23	.00
108	0	1.200	-.650	.000	.00	.00	.00	.23	.00
109	0	1.200	-1.950	.000	.00	.00	.00	.23	.00
110	0	1.200	-3.250	.000	.00	.00	.00	.23	.00
111	0	1.200	-4.550	.000	.00	.00	.00	.23	.00
112	0	1.200	-5.850	.000	.00	.00	.00	.23	.00
113	0	1.200	-7.150	.000	.00	.00	.00	.23	.00
114	0	1.200	-7.800	.000	.00	.00	.00	.23	.00
115	0	1.200	-9.100	.000	.00	.00	.00	.23	.00
116	0	.600	8.450	.000	.00	.00	.00	.23	.00
117	0	.600	-8.450	.000	.00	.00	.00	.23	.00
118	0	.000	9.100	.000	.00	.00	.00	.23	.00
119	0	.000	7.800	.000	.00	.00	.00	.23	.00
120	0	.000	7.150	.000	.00	.00	.00	.23	.00
121	0	.000	5.850	.000	.00	.00	.00	.23	.00
122	0	.000	4.550	.000	.00	.00	.00	.23	.00
123	0	.000	3.250	.000	.00	.00	.00	.23	.00
124	0	.000	1.950	.000	.00	.00	.00	.23	.00
125	0	.000	.650	.000	.00	.00	.00	.23	.00
126	0	.000	-.650	.000	.00	.00	.00	.23	.00
127	0	.000	-1.950	.000	.00	.00	.00	.23	.00
128	0	.000	-3.250	.000	.00	.00	.00	.23	.00
129	0	.000	-4.550	.000	.00	.00	.00	.23	.00
130	0	.000	-5.850	.000	.00	.00	.00	.23	.00
131	0	.000	-7.150	.000	.00	.00	.00	.23	.00
132	0	.000	-7.800	.000	.00	.00	.00	.23	.00
133	0	.000	-9.100	.000	.00	.00	.00	.23	.00
134	0	-.600	8.450	.000	.00	.00	.00	.23	.00
135	0	-.600	-8.450	.000	.00	.00	.00	.23	.00
136	0	-1.200	9.100	.000	.00	.00	.00	.23	.00
137	0	-1.200	7.800	.000	.00	.00	.00	.23	.00
138	0	-1.200	7.150	.000	.00	.00	.00	.23	.00
139	0	-1.200	5.850	.000	.00	.00	.00	.23	.00
140	0	-1.200	4.550	.000	.00	.00	.00	.23	.00
141	0	-1.200	3.250	.000	.00	.00	.00	.23	.00
142	0	-1.200	1.950	.000	.00	.00	.00	.23	.00
143	0	-1.200	.650	.000	.00	.00	.00	.23	.00
144	0	-1.200	-.650	.000	.00	.00	.00	.23	.00
145	0	-1.200	-1.950	.000	.00	.00	.00	.23	.00
146	0	-1.200	-3.250	.000	.00	.00	.00	.23	.00
147	0	-1.200	-4.550	.000	.00	.00	.00	.23	.00
148	0	-1.200	-5.850	.000	.00	.00	.00	.23	.00
149	0	-1.200	-7.150	.000	.00	.00	.00	.23	.00
150	0	-1.200	-7.800	.000	.00	.00	.00	.23	.00
151	0	-1.200	-9.100	.000	.00	.00	.00	.23	.00
152	0	-1.800	8.450	.000	.00	.00	.00	.23	.00
153	0	-1.800	-8.450	.000	.00	.00	.00	.23	.00
154	0	-2.400	9.100	.000	.00	.00	.00	.23	.00
155	0	-2.400	7.800	.000	.00	.00	.00	.23	.00
156	0	-2.400	7.150	.000	.00	.00	.00	.23	.00
157	0	-2.400	5.850	.000	.00	.00	.00	.23	.00
158	0	-2.400	4.550	.000	.00	.00	.00	.23	.00
159	0	-2.400	3.250	.000	.00	.00	.00	.23	.00
160	0	-2.400	1.950	.000	.00	.00	.00	.23	.00
161	0	-2.400	.650	.000	.00	.00	.00	.23	.00
162	0	-2.400	-.650	.000	.00	.00	.00	.23	.00
163	0	-2.400	-1.950	.000	.00	.00	.00	.23	.00
164	0	-2.400	-3.250	.000	.00	.00	.00	.23	.00
165	0	-2.400	-4.550	.000	.00	.00	.00	.23	.00
166	0	-2.400	-5.850	.000	.00	.00	.00	.23	.00
167	0	-2.400	-7.150	.000	.00	.00	.00	.23	.00
168	0	-2.400	-7.800	.000	.00	.00	.00	.23	.00
169	0	-2.400	-9.100	.000	.00	.00	.00	.23	.00
170	0	-3.000	8.450	.000	.00	.00	.00	.23	.00
171	0	-3.000	-8.450	.000	.00	.00	.00	.23	.00
172	0	-3.600	9.100	.000	.00	.00	.00	.23	.00
173	0	-3.600	7.800	.000	.00	.00	.00	.23	.00
174	0	-3.600	7.150	.000	.00	.00	.00	.23	.00

Relazione di calcolo fondazioni dirette su micropali
P21 – P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	49 di 91

175	0	-3.600	5.850	.000	.00	.00	.00	.23	.00
176	0	-3.600	4.550	.000	.00	.00	.00	.23	.00
177	0	-3.600	3.250	.000	.00	.00	.00	.23	.00
178	0	-3.600	1.950	.000	.00	.00	.00	.23	.00
179	0	-3.600	.650	.000	.00	.00	.00	.23	.00
180	0	-3.600	-.650	.000	.00	.00	.00	.23	.00
181	0	-3.600	-1.950	.000	.00	.00	.00	.23	.00
182	0	-3.600	-3.250	.000	.00	.00	.00	.23	.00
183	0	-3.600	-4.550	.000	.00	.00	.00	.23	.00
184	0	-3.600	-5.850	.000	.00	.00	.00	.23	.00
185	0	-3.600	-7.150	.000	.00	.00	.00	.23	.00
186	0	-3.600	-7.800	.000	.00	.00	.00	.23	.00
187	0	-3.600	-9.100	.000	.00	.00	.00	.23	.00
188	0	-4.200	8.450	.000	.00	.00	.00	.23	.00
189	0	-4.200	-8.450	.000	.00	.00	.00	.23	.00
190	0	-4.800	9.100	.000	.00	.00	.00	.23	.00
191	0	-4.800	7.800	.000	.00	.00	.00	.23	.00
192	0	-4.800	7.150	.000	.00	.00	.00	.23	.00
193	0	-4.800	5.850	.000	.00	.00	.00	.23	.00
194	0	-4.800	4.550	.000	.00	.00	.00	.23	.00
195	0	-4.800	3.250	.000	.00	.00	.00	.23	.00
196	0	-4.800	1.950	.000	.00	.00	.00	.23	.00
197	0	-4.800	.650	.000	.00	.00	.00	.23	.00
198	0	-4.800	-.650	.000	.00	.00	.00	.23	.00
199	0	-4.800	-1.950	.000	.00	.00	.00	.23	.00
200	0	-4.800	-3.250	.000	.00	.00	.00	.23	.00
201	0	-4.800	-4.550	.000	.00	.00	.00	.23	.00
202	0	-4.800	-5.850	.000	.00	.00	.00	.23	.00
203	0	-4.800	-7.150	.000	.00	.00	.00	.23	.00
204	0	-4.800	-7.800	.000	.00	.00	.00	.23	.00
205	0	-4.800	-9.100	.000	.00	.00	.00	.23	.00
206	0	-5.400	9.100	.000	.00	.00	.00	.23	.00
207	0	-5.400	7.800	.000	.00	.00	.00	.23	.00
208	0	-5.400	7.150	.000	.00	.00	.00	.23	.00
209	0	-5.400	5.850	.000	.00	.00	.00	.23	.00
210	0	-5.400	4.550	.000	.00	.00	.00	.23	.00
211	0	-5.400	3.250	.000	.00	.00	.00	.23	.00
212	0	-5.400	1.950	.000	.00	.00	.00	.23	.00
213	0	-5.400	.650	.000	.00	.00	.00	.23	.00
214	0	-5.400	-.650	.000	.00	.00	.00	.23	.00
215	0	-5.400	-1.950	.000	.00	.00	.00	.23	.00
216	0	-5.400	-3.250	.000	.00	.00	.00	.23	.00
217	0	-5.400	-4.550	.000	.00	.00	.00	.23	.00
218	0	-5.400	-5.850	.000	.00	.00	.00	.23	.00
219	0	-5.400	-7.150	.000	.00	.00	.00	.23	.00
220	0	-5.400	-7.800	.000	.00	.00	.00	.23	.00
221	0	-5.400	-9.100	.000	.00	.00	.00	.23	.00
222	0	-6.000	8.450	.000	.00	.00	.00	.23	.00
223	0	-6.000	6.500	.000	.00	.00	.00	.23	.00
224	0	-6.000	5.200	.000	.00	.00	.00	.23	.00
225	0	-6.000	3.900	.000	.00	.00	.00	.23	.00
226	0	-6.000	2.600	.000	.00	.00	.00	.23	.00
227	0	-6.000	1.300	.000	.00	.00	.00	.23	.00
228	0	-6.000	.000	.000	.00	.00	.00	.23	.00
229	0	-6.000	-1.300	.000	.00	.00	.00	.23	.00
230	0	-6.000	-2.600	.000	.00	.00	.00	.23	.00
231	0	-6.000	-3.900	.000	.00	.00	.00	.23	.00
232	0	-6.000	-5.200	.000	.00	.00	.00	.23	.00
233	0	-6.000	-6.500	.000	.00	.00	.00	.23	.00
234	0	-6.000	-8.450	.000	.00	.00	.00	.23	.00
235	0	-6.600	9.100	.000	.00	.00	.00	.23	.00
236	0	-6.600	7.800	.000	.00	.00	.00	.23	.00
237	0	-6.600	7.150	.000	.00	.00	.00	.23	.00
238	0	-6.600	5.850	.000	.00	.00	.00	.23	.00
239	0	-6.600	4.550	.000	.00	.00	.00	.23	.00
240	0	-6.600	3.250	.000	.00	.00	.00	.23	.00
241	0	-6.600	1.950	.000	.00	.00	.00	.23	.00
242	0	-6.600	.650	.000	.00	.00	.00	.23	.00
243	0	-6.600	-.650	.000	.00	.00	.00	.23	.00
244	0	-6.600	-1.950	.000	.00	.00	.00	.23	.00
245	0	-6.600	-3.250	.000	.00	.00	.00	.23	.00
246	0	-6.600	-4.550	.000	.00	.00	.00	.23	.00
247	0	-6.600	-5.850	.000	.00	.00	.00	.23	.00
248	0	-6.600	-7.150	.000	.00	.00	.00	.23	.00
249	0	-6.600	-7.800	.000	.00	.00	.00	.23	.00
250	0	-6.600	-9.100	.000	.00	.00	.00	.23	.00

vin = 0 - incastro; 1 - cerniera; 2 - appoggio
X, Y, Z = Coordinate testa pali
axz = Inclinazione palo nel piano Xp Z rispetto alla verticale
(positiva se verso Xp positivo)
ayz = Inclinazione palo nel piano Yp Z rispetto alla verticale
(positiva se verso Yp positivo)
axy = Rotazione assi Xp Yp (positiva se antioraria)
Box = Lato dell'elemento parallelo all'asse Xp
Boy = Lato dell'elemento parallelo all'asse Yp
se Boy = 0 D = Box: diametro
altrimenti D = $\sqrt{\text{Box} * \text{Boy} * 1.273}$: diametro equivalente

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Caratterizzazione dei pali soggetti a carichi assiali e torsionali
(uguali per tutti i pali)

palo	AK kN/m	TK kN*m/rad
1	100000.	.0

AK = Rigidezza assiale palo-terreno
TK = Rigidezza torsionale palo-terreno

Baricentro palificata: Xg = .000 m Yg = .000 m
Rotazione direzioni princip. di inerzia: .00 deg

Caratterizzazione del terreno per pali soggetti a carichi trasversali

Terreno tipo 1

Prof. m	E kN/m2
.00	23000.0
40.00	23000.0

Caratterizzazione dei pali soggetti a carichi trasversali
(uguali per tutti i pali)

palo	Lp m	EJx kN*m2	Itx	Ridx	EJy kN*m2	Ity	Ridy
1	15.00	6975.	1	1.000	6975.	1	1.000

Lp = Lunghezza palo (compreso eventuale tratto fuori terra)
EJ = Rigidezza flessionale del palo
It = Tipo di terreno
Rid = Moltiplicatore del modulo di reazione orizzontale

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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LINEA FS FERRANDINA MATERA
VI08 Pile P21 e P22 - Analisi SLV

CONDIZIONE DI CARICO 1
VI08 - P21 - SLV_x

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	25596.4	11209.9	261289.6	4139.8	97519.9	1189.6
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
43695.4	11209.9	261289.6	4139.8	97519.9	1189.6

Punto di applic. carico verticale: Xv = 5.980 m Yv = 2.232 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.748	2.148	.558	.746	.115	.004

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	647.7	44.0	-19.1	17.1	-8.1	.0	20.8
2	632.8	44.1	-19.2	17.1	-8.1	.0	20.9
3	625.3	44.2	-19.2	17.1	-8.1	.0	20.9
4	610.4	44.3	-19.3	17.1	-8.1	.0	20.9
5	595.5	44.4	-19.3	17.1	-8.1	.0	21.0
6	580.6	44.6	-19.4	17.1	-8.1	.0	21.0
7	565.7	44.7	-19.5	17.1	-8.1	.0	21.1
8	550.8	44.8	-19.5	17.1	-8.1	.0	21.2
9	535.9	44.9	-19.6	17.1	-8.1	.0	21.2
10	521.0	45.0	-19.6	17.1	-8.1	.0	21.3
11	506.1	45.1	-19.7	17.1	-8.1	.0	21.3
12	491.2	45.2	-19.8	17.1	-8.1	.0	21.4
13	476.3	45.4	-19.8	17.1	-8.1	.0	21.4
14	461.4	45.5	-19.9	17.1	-8.1	.0	21.5

Relazione di calcolo fondazioni dirette su micropali
 P21 – P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	53 di 91

15	453.9	45.5	-19.9	17.1	-8.1	.0	21.5
16	439.0	45.6	-20.0	17.1	-8.1	.0	21.6
17	606.7	44.1	-19.2	17.1	-8.1	.0	20.8
18	584.4	44.3	-19.3	17.1	-8.1	.0	20.9
19	569.5	44.4	-19.3	17.1	-8.1	.0	21.0
20	554.6	44.5	-19.4	17.1	-8.1	.0	21.0
21	539.7	44.6	-19.4	17.1	-8.1	.0	21.1
22	524.8	44.7	-19.5	17.1	-8.1	.0	21.1
23	509.9	44.8	-19.6	17.1	-8.1	.0	21.2
24	494.9	45.0	-19.6	17.1	-8.1	.0	21.2
25	480.0	45.1	-19.7	17.1	-8.1	.0	21.3
26	465.1	45.2	-19.7	17.1	-8.1	.0	21.3
27	450.2	45.3	-19.8	17.1	-8.1	.0	21.4
28	435.3	45.4	-19.9	17.1	-8.1	.0	21.5
29	413.0	45.6	-19.9	17.1	-8.1	.0	21.5
30	580.7	44.0	-19.1	17.0	-8.1	.0	20.8
31	565.8	44.1	-19.2	17.0	-8.1	.0	20.8
32	558.3	44.2	-19.2	17.0	-8.1	.0	20.9
33	543.4	44.3	-19.3	17.0	-8.1	.0	20.9
34	528.5	44.4	-19.3	17.0	-8.1	.0	21.0
35	513.6	44.6	-19.4	17.0	-8.1	.0	21.0
36	498.7	44.7	-19.5	17.0	-8.1	.0	21.1
37	483.8	44.8	-19.5	17.0	-8.1	.0	21.1
38	468.9	44.9	-19.6	17.0	-8.1	.0	21.2
39	454.0	45.0	-19.6	17.0	-8.1	.0	21.2
40	439.1	45.1	-19.7	17.0	-8.1	.0	21.3
41	424.2	45.2	-19.8	17.0	-8.1	.0	21.4
42	409.3	45.4	-19.8	17.0	-8.1	.0	21.4
43	394.4	45.5	-19.9	17.0	-8.1	.0	21.5
44	386.9	45.5	-19.9	17.0	-8.1	.0	21.5
45	372.0	45.6	-20.0	17.0	-8.1	.0	21.6
46	547.2	44.0	-19.1	17.0	-8.1	.0	20.8
47	532.3	44.1	-19.2	17.0	-8.1	.0	20.8
48	524.8	44.2	-19.2	17.0	-8.1	.0	20.9
49	509.9	44.3	-19.3	17.0	-8.1	.0	20.9
50	495.0	44.4	-19.3	17.0	-8.1	.0	21.0
51	480.1	44.6	-19.4	17.0	-8.1	.0	21.0
52	465.2	44.7	-19.5	17.0	-8.1	.0	21.1
53	450.3	44.8	-19.5	17.0	-8.1	.0	21.1
54	435.4	44.9	-19.6	17.0	-8.1	.0	21.2
55	420.5	45.0	-19.6	17.0	-8.1	.0	21.2
56	405.6	45.1	-19.7	17.0	-8.1	.0	21.3
57	390.7	45.2	-19.8	17.0	-8.1	.0	21.3
58	375.8	45.4	-19.8	17.0	-8.1	.0	21.4
59	360.9	45.5	-19.9	17.0	-8.1	.0	21.5
60	353.4	45.5	-19.9	17.0	-8.1	.0	21.5
61	338.5	45.6	-20.0	17.0	-8.1	.0	21.5
62	506.2	44.1	-19.2	16.9	-8.0	.0	20.8
63	312.4	45.6	-19.9	16.9	-8.0	.0	21.5
64	480.2	44.0	-19.1	16.9	-8.0	.0	20.7
65	465.3	44.1	-19.2	16.9	-8.0	.0	20.8
66	457.8	44.2	-19.2	16.9	-8.0	.0	20.8
67	442.9	44.3	-19.3	16.9	-8.0	.0	20.9
68	428.0	44.4	-19.3	16.9	-8.0	.0	20.9
69	413.1	44.6	-19.4	16.9	-8.0	.0	21.0
70	398.2	44.7	-19.5	16.9	-8.0	.0	21.1
71	383.3	44.8	-19.5	16.9	-8.0	.0	21.1
72	368.4	44.9	-19.6	16.9	-8.0	.0	21.2
73	353.5	45.0	-19.6	16.9	-8.0	.0	21.2
74	338.6	45.1	-19.7	16.9	-8.0	.0	21.3
75	323.7	45.2	-19.8	16.9	-8.0	.0	21.3
76	308.8	45.4	-19.8	16.9	-8.0	.0	21.4
77	293.8	45.5	-19.9	16.9	-8.0	.0	21.4
78	286.4	45.5	-19.9	16.9	-8.0	.0	21.5
79	271.5	45.6	-20.0	16.9	-8.0	.0	21.5
80	439.2	44.1	-19.2	16.8	-8.0	.0	20.8
81	245.4	45.6	-19.9	16.8	-8.0	.0	21.5
82	413.1	44.0	-19.1	16.8	-8.0	.0	20.7
83	398.2	44.1	-19.2	16.8	-8.0	.0	20.8
84	390.8	44.2	-19.2	16.8	-8.0	.0	20.8
85	375.9	44.3	-19.3	16.8	-8.0	.0	20.9
86	361.0	44.4	-19.3	16.8	-8.0	.0	20.9
87	346.1	44.6	-19.4	16.8	-8.0	.0	21.0
88	331.2	44.7	-19.5	16.8	-8.0	.0	21.0
89	316.3	44.8	-19.5	16.8	-8.0	.0	21.1
90	301.4	44.9	-19.6	16.8	-8.0	.0	21.1

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	54 di 91

91	286.5	45.0	-19.6	16.8	-8.0	.0	21.2
92	271.5	45.1	-19.7	16.8	-8.0	.0	21.3
93	256.6	45.2	-19.8	16.8	-8.0	.0	21.3
94	241.7	45.4	-19.8	16.8	-8.0	.0	21.4
95	226.8	45.5	-19.9	16.8	-8.0	.0	21.4
96	219.4	45.5	-19.9	16.8	-8.0	.0	21.4
97	204.5	45.6	-20.0	16.8	-8.0	.0	21.5
98	372.2	44.1	-19.2	16.7	-7.9	.0	20.7
99	178.4	45.6	-19.9	16.7	-7.9	.0	21.5
100	346.1	44.0	-19.1	16.7	-7.9	.0	20.7
101	331.2	44.1	-19.2	16.7	-7.9	.0	20.8
102	323.8	44.2	-19.2	16.7	-7.9	.0	20.8
103	308.9	44.3	-19.3	16.7	-7.9	.0	20.8
104	294.0	44.4	-19.3	16.7	-7.9	.0	20.9
105	279.1	44.6	-19.4	16.7	-7.9	.0	21.0
106	264.2	44.7	-19.5	16.7	-7.9	.0	21.0
107	249.2	44.8	-19.5	16.7	-7.9	.0	21.1
108	234.3	44.9	-19.6	16.7	-7.9	.0	21.1
109	219.4	45.0	-19.6	16.7	-7.9	.0	21.2
110	204.5	45.1	-19.7	16.7	-7.9	.0	21.2
111	189.6	45.2	-19.8	16.7	-7.9	.0	21.3
112	174.7	45.4	-19.8	16.7	-7.9	.0	21.3
113	159.8	45.5	-19.9	16.7	-7.9	.0	21.4
114	152.4	45.5	-19.9	16.7	-7.9	.0	21.4
115	137.5	45.6	-20.0	16.7	-7.9	.0	21.5
116	305.2	44.1	-19.2	16.6	-7.9	.0	20.7
117	111.4	45.6	-19.9	16.6	-7.9	.0	21.4
118	279.1	44.0	-19.1	16.6	-7.8	.0	20.7
119	264.2	44.1	-19.2	16.6	-7.8	.0	20.7
120	256.8	44.2	-19.2	16.6	-7.8	.0	20.8
121	241.9	44.3	-19.3	16.6	-7.8	.0	20.8
122	226.9	44.4	-19.3	16.6	-7.8	.0	20.9
123	212.0	44.6	-19.4	16.6	-7.8	.0	20.9
124	197.1	44.7	-19.5	16.6	-7.8	.0	21.0
125	182.2	44.8	-19.5	16.6	-7.8	.0	21.0
126	167.3	44.9	-19.6	16.6	-7.8	.0	21.1
127	152.4	45.0	-19.6	16.6	-7.8	.0	21.2
128	137.5	45.1	-19.7	16.6	-7.8	.0	21.2
129	122.6	45.2	-19.8	16.6	-7.8	.0	21.3
130	107.7	45.4	-19.8	16.6	-7.8	.0	21.3
131	92.8	45.5	-19.9	16.6	-7.8	.0	21.4
132	85.4	45.5	-19.9	16.6	-7.8	.0	21.4
133	70.4	45.6	-20.0	16.6	-7.8	.0	21.5
134	238.2	44.1	-19.2	16.5	-7.8	.0	20.7
135	44.4	45.6	-19.9	16.5	-7.8	.0	21.4
136	212.1	44.0	-19.1	16.5	-7.8	.0	20.7
137	197.2	44.1	-19.2	16.5	-7.8	.0	20.7
138	189.7	44.2	-19.2	16.5	-7.8	.0	20.7
139	174.8	44.3	-19.3	16.5	-7.8	.0	20.8
140	159.9	44.4	-19.3	16.5	-7.8	.0	20.9
141	145.0	44.6	-19.4	16.5	-7.8	.0	20.9
142	130.1	44.7	-19.5	16.5	-7.8	.0	21.0
143	115.2	44.8	-19.5	16.5	-7.8	.0	21.0
144	100.3	44.9	-19.6	16.5	-7.8	.0	21.1
145	85.4	45.0	-19.6	16.5	-7.8	.0	21.1
146	70.5	45.1	-19.7	16.5	-7.8	.0	21.2
147	55.6	45.2	-19.8	16.5	-7.8	.0	21.2
148	40.7	45.4	-19.8	16.5	-7.8	.0	21.3
149	25.8	45.5	-19.9	16.5	-7.8	.0	21.4
150	18.3	45.5	-19.9	16.5	-7.8	.0	21.4
151	3.4	45.6	-20.0	16.5	-7.8	.0	21.4
152	171.1	44.1	-19.2	16.4	-7.8	.0	20.7
153	-22.6	45.6	-19.9	16.4	-7.8	.0	21.4
154	145.1	44.0	-19.1	16.3	-7.7	.0	20.6
155	130.2	44.1	-19.2	16.3	-7.7	.0	20.7
156	122.7	44.2	-19.2	16.3	-7.7	.0	20.7
157	107.8	44.3	-19.3	16.3	-7.7	.0	20.8
158	92.9	44.4	-19.3	16.3	-7.7	.0	20.8
159	78.0	44.6	-19.4	16.3	-7.7	.0	20.9
160	63.1	44.7	-19.5	16.3	-7.7	.0	20.9
161	48.2	44.8	-19.5	16.3	-7.7	.0	21.0
162	33.3	44.9	-19.6	16.3	-7.7	.0	21.1
163	18.4	45.0	-19.6	16.3	-7.7	.0	21.1
164	3.5	45.1	-19.7	16.3	-7.7	.0	21.2
165	-11.4	45.2	-19.8	16.3	-7.7	.0	21.2
166	-26.3	45.4	-19.8	16.3	-7.7	.0	21.3

Relazione di calcolo fondazioni dirette su micropali
 P21 – P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	55 di 91

167	-41.2	45.5	-19.9	16.3	-7.7	.0	21.3
168	-48.7	45.5	-19.9	16.3	-7.7	.0	21.4
169	-63.6	45.6	-20.0	16.3	-7.7	.0	21.4
170	104.1	44.1	-19.2	16.3	-7.7	.0	20.7
171	-89.6	45.6	-19.9	16.3	-7.7	.0	21.4
172	78.1	44.0	-19.1	16.2	-7.7	.0	20.6
173	63.2	44.1	-19.2	16.2	-7.7	.0	20.7
174	55.7	44.2	-19.2	16.2	-7.7	.0	20.7
175	40.8	44.3	-19.3	16.2	-7.7	.0	20.8
176	25.9	44.4	-19.3	16.2	-7.7	.0	20.8
177	11.0	44.6	-19.4	16.2	-7.7	.0	20.9
178	-3.9	44.7	-19.5	16.2	-7.7	.0	20.9
179	-18.8	44.8	-19.5	16.2	-7.7	.0	21.0
180	-33.7	44.9	-19.6	16.2	-7.7	.0	21.0
181	-48.6	45.0	-19.6	16.2	-7.7	.0	21.1
182	-63.5	45.1	-19.7	16.2	-7.7	.0	21.2
183	-78.4	45.2	-19.8	16.2	-7.7	.0	21.2
184	-93.3	45.4	-19.8	16.2	-7.7	.0	21.3
185	-108.2	45.5	-19.9	16.2	-7.7	.0	21.3
186	-115.7	45.5	-19.9	16.2	-7.7	.0	21.3
187	-130.6	45.6	-20.0	16.2	-7.7	.0	21.4
188	37.1	44.1	-19.2	16.2	-7.6	.0	20.6
189	-156.7	45.6	-19.9	16.2	-7.6	.0	21.4
190	11.1	44.0	-19.1	16.1	-7.6	.0	20.6
191	-3.8	44.1	-19.2	16.1	-7.6	.0	20.7
192	-11.3	44.2	-19.2	16.1	-7.6	.0	20.7
193	-26.2	44.3	-19.3	16.1	-7.6	.0	20.7
194	-41.1	44.4	-19.3	16.1	-7.6	.0	20.8
195	-56.0	44.6	-19.4	16.1	-7.6	.0	20.9
196	-70.9	44.7	-19.5	16.1	-7.6	.0	20.9
197	-85.8	44.8	-19.5	16.1	-7.6	.0	21.0
198	-100.7	44.9	-19.6	16.1	-7.6	.0	21.0
199	-115.6	45.0	-19.6	16.1	-7.6	.0	21.1
200	-130.5	45.1	-19.7	16.1	-7.6	.0	21.1
201	-145.4	45.2	-19.8	16.1	-7.6	.0	21.2
202	-160.3	45.4	-19.8	16.1	-7.6	.0	21.2
203	-175.3	45.5	-19.9	16.1	-7.6	.0	21.3
204	-182.7	45.5	-19.9	16.1	-7.6	.0	21.3
205	-197.6	45.6	-20.0	16.1	-7.6	.0	21.4
206	-22.4	44.0	-19.1	16.1	-7.6	.0	20.6
207	-37.4	44.1	-19.2	16.1	-7.6	.0	20.6
208	-44.8	44.2	-19.2	16.1	-7.6	.0	20.7
209	-59.7	44.3	-19.3	16.1	-7.6	.0	20.7
210	-74.6	44.4	-19.3	16.1	-7.6	.0	20.8
211	-89.5	44.6	-19.4	16.1	-7.6	.0	20.8
212	-104.4	44.7	-19.5	16.1	-7.6	.0	20.9
213	-119.3	44.8	-19.5	16.1	-7.6	.0	21.0
214	-134.2	44.9	-19.6	16.1	-7.6	.0	21.0
215	-149.1	45.0	-19.6	16.1	-7.6	.0	21.1
216	-164.0	45.1	-19.7	16.1	-7.6	.0	21.1
217	-178.9	45.2	-19.8	16.1	-7.6	.0	21.2
218	-193.9	45.4	-19.8	16.1	-7.6	.0	21.2
219	-208.8	45.5	-19.9	16.1	-7.6	.0	21.3
220	-216.2	45.5	-19.9	16.1	-7.6	.0	21.3
221	-231.1	45.6	-20.0	16.1	-7.6	.0	21.4
222	-63.4	44.1	-19.2	16.0	-7.6	.0	20.6
223	-85.8	44.3	-19.3	16.0	-7.6	.0	20.7
224	-100.7	44.4	-19.3	16.0	-7.6	.0	20.7
225	-115.6	44.5	-19.4	16.0	-7.6	.0	20.8
226	-130.5	44.6	-19.4	16.0	-7.6	.0	20.9
227	-145.4	44.7	-19.5	16.0	-7.6	.0	20.9
228	-160.3	44.8	-19.6	16.0	-7.6	.0	21.0
229	-175.2	45.0	-19.6	16.0	-7.6	.0	21.0
230	-190.1	45.1	-19.7	16.0	-7.6	.0	21.1
231	-205.0	45.2	-19.7	16.0	-7.6	.0	21.1
232	-219.9	45.3	-19.8	16.0	-7.6	.0	21.2
233	-234.8	45.4	-19.9	16.0	-7.6	.0	21.3
234	-257.2	45.6	-19.9	16.0	-7.6	.0	21.3
235	-89.5	44.0	-19.1	16.0	-7.5	.0	20.6
236	-104.4	44.1	-19.2	16.0	-7.5	.0	20.6
237	-111.8	44.2	-19.2	16.0	-7.5	.0	20.7
238	-126.7	44.3	-19.3	16.0	-7.5	.0	20.7
239	-141.6	44.4	-19.3	16.0	-7.5	.0	20.8
240	-156.5	44.6	-19.4	16.0	-7.5	.0	20.8
241	-171.4	44.7	-19.5	16.0	-7.5	.0	20.9
242	-186.3	44.8	-19.5	16.0	-7.5	.0	20.9



Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	56 di 91

243	-201.2	44.9	-19.6	16.0	-7.5	.0	21.0
244	-216.2	45.0	-19.6	16.0	-7.5	.0	21.0
245	-231.1	45.1	-19.7	16.0	-7.5	.0	21.1
246	-246.0	45.2	-19.8	16.0	-7.5	.0	21.2
247	-260.9	45.4	-19.8	16.0	-7.5	.0	21.2
248	-275.8	45.5	-19.9	16.0	-7.5	.0	21.3
249	-283.2	45.5	-19.9	16.0	-7.5	.0	21.3
250	-298.1	45.6	-20.0	16.0	-7.5	.0	21.4

 $M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$

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LINEA FS FERRANDINA MATERA
VI08 Pile P21 e P22 - Analisi SLV

CONDIZIONE DI CARICO 2
VI08 - P21 - SLV_x

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	22201.9	-11209.9	-260797.6	-4139.8	-97519.9	-1189.6
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
40300.9	-11209.9	-260797.6	-4139.8	-97519.9	-1189.6

Punto di applic. carico verticale: Xv = -6.471 m Yv = -2.420 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.612	-2.148	-.557	-.746	-.115	-.004

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	-311.0	-44.0	19.1	-17.1	8.1	.0	20.8
2	-296.1	-44.1	19.2	-17.1	8.1	.0	20.9
3	-288.7	-44.2	19.2	-17.1	8.1	.0	20.9
4	-273.8	-44.3	19.3	-17.1	8.1	.0	20.9
5	-258.9	-44.4	19.4	-17.1	8.1	.0	21.0
6	-244.0	-44.6	19.4	-17.1	8.1	.0	21.1
7	-229.1	-44.7	19.5	-17.1	8.1	.0	21.1
8	-214.1	-44.8	19.5	-17.1	8.1	.0	21.2
9	-199.2	-44.9	19.6	-17.1	8.1	.0	21.2
10	-184.3	-45.0	19.7	-17.1	8.1	.0	21.3
11	-169.4	-45.1	19.7	-17.1	8.1	.0	21.3
12	-154.5	-45.2	19.8	-17.1	8.1	.0	21.4
13	-139.6	-45.4	19.8	-17.1	8.1	.0	21.4
14	-124.7	-45.5	19.9	-17.1	8.1	.0	21.5

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	58 di 91

15	-117.3	-45.5	19.9	-17.1	8.1	.0	21.5
16	-102.4	-45.6	20.0	-17.1	8.1	.0	21.6
17	-270.1	-44.1	19.2	-17.1	8.1	.0	20.8
18	-247.8	-44.3	19.3	-17.1	8.1	.0	20.9
19	-232.9	-44.4	19.3	-17.1	8.1	.0	21.0
20	-218.0	-44.5	19.4	-17.1	8.1	.0	21.0
21	-203.1	-44.6	19.4	-17.1	8.1	.0	21.1
22	-188.2	-44.7	19.5	-17.1	8.1	.0	21.1
23	-173.2	-44.8	19.6	-17.1	8.1	.0	21.2
24	-158.3	-45.0	19.6	-17.1	8.1	.0	21.2
25	-143.4	-45.1	19.7	-17.1	8.1	.0	21.3
26	-128.5	-45.2	19.7	-17.1	8.1	.0	21.3
27	-113.6	-45.3	19.8	-17.1	8.1	.0	21.4
28	-98.7	-45.4	19.9	-17.1	8.1	.0	21.5
29	-76.4	-45.6	20.0	-17.1	8.1	.0	21.5
30	-244.1	-44.0	19.1	-17.0	8.1	.0	20.8
31	-229.2	-44.1	19.2	-17.0	8.1	.0	20.8
32	-221.8	-44.2	19.2	-17.0	8.1	.0	20.9
33	-206.9	-44.3	19.3	-17.0	8.1	.0	20.9
34	-192.0	-44.4	19.4	-17.0	8.1	.0	21.0
35	-177.1	-44.6	19.4	-17.0	8.1	.0	21.0
36	-162.2	-44.7	19.5	-17.0	8.1	.0	21.1
37	-147.3	-44.8	19.5	-17.0	8.1	.0	21.1
38	-132.4	-44.9	19.6	-17.0	8.1	.0	21.2
39	-117.4	-45.0	19.7	-17.0	8.1	.0	21.3
40	-102.5	-45.1	19.7	-17.0	8.1	.0	21.3
41	-87.6	-45.2	19.8	-17.0	8.1	.0	21.4
42	-72.7	-45.4	19.8	-17.0	8.1	.0	21.4
43	-57.8	-45.5	19.9	-17.0	8.1	.0	21.5
44	-50.4	-45.5	19.9	-17.0	8.1	.0	21.5
45	-35.5	-45.6	20.0	-17.0	8.1	.0	21.6
46	-210.7	-44.0	19.1	-17.0	8.1	.0	20.8
47	-195.8	-44.1	19.2	-17.0	8.1	.0	20.8
48	-188.3	-44.2	19.2	-17.0	8.1	.0	20.9
49	-173.4	-44.3	19.3	-17.0	8.1	.0	20.9
50	-158.5	-44.4	19.4	-17.0	8.1	.0	21.0
51	-143.6	-44.6	19.4	-17.0	8.1	.0	21.0
52	-128.7	-44.7	19.5	-17.0	8.1	.0	21.1
53	-113.8	-44.8	19.5	-17.0	8.1	.0	21.1
54	-98.9	-44.9	19.6	-17.0	8.1	.0	21.2
55	-84.0	-45.0	19.7	-17.0	8.1	.0	21.2
56	-69.1	-45.1	19.7	-17.0	8.1	.0	21.3
57	-54.2	-45.2	19.8	-17.0	8.1	.0	21.4
58	-39.3	-45.4	19.8	-17.0	8.1	.0	21.4
59	-24.4	-45.5	19.9	-17.0	8.1	.0	21.5
60	-16.9	-45.5	19.9	-17.0	8.1	.0	21.5
61	-2.0	-45.6	20.0	-17.0	8.1	.0	21.5
62	-169.8	-44.1	19.2	-16.9	8.0	.0	20.8
63	24.0	-45.6	20.0	-16.9	8.0	.0	21.5
64	-143.8	-44.0	19.1	-16.9	8.0	.0	20.8
65	-128.9	-44.1	19.2	-16.9	8.0	.0	20.8
66	-121.4	-44.2	19.2	-16.9	8.0	.0	20.8
67	-106.5	-44.3	19.3	-16.9	8.0	.0	20.9
68	-91.6	-44.4	19.4	-16.9	8.0	.0	20.9
69	-76.7	-44.6	19.4	-16.9	8.0	.0	21.0
70	-61.8	-44.7	19.5	-16.9	8.0	.0	21.1
71	-46.9	-44.8	19.5	-16.9	8.0	.0	21.1
72	-32.0	-44.9	19.6	-16.9	8.0	.0	21.2
73	-17.1	-45.0	19.7	-16.9	8.0	.0	21.2
74	-2.2	-45.1	19.7	-16.9	8.0	.0	21.3
75	12.7	-45.2	19.8	-16.9	8.0	.0	21.3
76	27.6	-45.4	19.8	-16.9	8.0	.0	21.4
77	42.5	-45.5	19.9	-16.9	8.0	.0	21.4
78	50.0	-45.5	19.9	-16.9	8.0	.0	21.5
79	64.9	-45.6	20.0	-16.9	8.0	.0	21.5
80	-102.9	-44.1	19.2	-16.8	8.0	.0	20.8
81	90.9	-45.6	20.0	-16.8	8.0	.0	21.5
82	-76.9	-44.0	19.1	-16.8	8.0	.0	20.7
83	-62.0	-44.1	19.2	-16.8	8.0	.0	20.8
84	-54.6	-44.2	19.2	-16.8	8.0	.0	20.8
85	-39.7	-44.3	19.3	-16.8	8.0	.0	20.9
86	-24.7	-44.4	19.4	-16.8	8.0	.0	20.9
87	-9.8	-44.6	19.4	-16.8	8.0	.0	21.0
88	5.1	-44.7	19.5	-16.8	8.0	.0	21.0
89	20.0	-44.8	19.5	-16.8	8.0	.0	21.1
90	34.9	-44.9	19.6	-16.8	8.0	.0	21.1

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91	49.8	-45.0	19.7	-16.8	8.0	.0	21.2
92	64.7	-45.1	19.7	-16.8	8.0	.0	21.3
93	79.6	-45.2	19.8	-16.8	8.0	.0	21.3
94	94.5	-45.4	19.8	-16.8	8.0	.0	21.4
95	109.4	-45.5	19.9	-16.8	8.0	.0	21.4
96	116.9	-45.5	19.9	-16.8	8.0	.0	21.5
97	131.8	-45.6	20.0	-16.8	8.0	.0	21.5
98	-36.0	-44.1	19.2	-16.7	7.9	.0	20.7
99	157.8	-45.6	20.0	-16.7	7.9	.0	21.5
100	-10.0	-44.0	19.1	-16.7	7.9	.0	20.7
101	4.9	-44.1	19.2	-16.7	7.9	.0	20.8
102	12.3	-44.2	19.2	-16.7	7.9	.0	20.8
103	27.2	-44.3	19.3	-16.7	7.9	.0	20.8
104	42.1	-44.4	19.4	-16.7	7.9	.0	20.9
105	57.1	-44.6	19.4	-16.7	7.9	.0	21.0
106	72.0	-44.7	19.5	-16.7	7.9	.0	21.0
107	86.9	-44.8	19.5	-16.7	7.9	.0	21.1
108	101.8	-44.9	19.6	-16.7	7.9	.0	21.1
109	116.7	-45.0	19.7	-16.7	7.9	.0	21.2
110	131.6	-45.1	19.7	-16.7	7.9	.0	21.2
111	146.5	-45.2	19.8	-16.7	7.9	.0	21.3
112	161.4	-45.4	19.8	-16.7	7.9	.0	21.3
113	176.3	-45.5	19.9	-16.7	7.9	.0	21.4
114	183.7	-45.5	19.9	-16.7	7.9	.0	21.4
115	198.6	-45.6	20.0	-16.7	7.9	.0	21.5
116	30.9	-44.1	19.2	-16.6	7.9	.0	20.7
117	224.6	-45.6	20.0	-16.6	7.9	.0	21.5
118	56.9	-44.0	19.1	-16.6	7.8	.0	20.7
119	71.8	-44.1	19.2	-16.6	7.8	.0	20.7
120	79.2	-44.2	19.2	-16.6	7.8	.0	20.8
121	94.1	-44.3	19.3	-16.6	7.8	.0	20.8
122	109.0	-44.4	19.4	-16.6	7.8	.0	20.9
123	123.9	-44.6	19.4	-16.6	7.8	.0	20.9
124	138.8	-44.7	19.5	-16.6	7.8	.0	21.0
125	153.8	-44.8	19.5	-16.6	7.8	.0	21.1
126	168.7	-44.9	19.6	-16.6	7.8	.0	21.1
127	183.6	-45.0	19.7	-16.6	7.8	.0	21.2
128	198.5	-45.1	19.7	-16.6	7.8	.0	21.2
129	213.4	-45.2	19.8	-16.6	7.8	.0	21.3
130	228.3	-45.4	19.8	-16.6	7.8	.0	21.3
131	243.2	-45.5	19.9	-16.6	7.8	.0	21.4
132	250.6	-45.5	19.9	-16.6	7.8	.0	21.4
133	265.5	-45.6	20.0	-16.6	7.8	.0	21.5
134	97.8	-44.1	19.2	-16.5	7.8	.0	20.7
135	291.5	-45.6	20.0	-16.5	7.8	.0	21.4
136	123.8	-44.0	19.1	-16.5	7.8	.0	20.7
137	138.7	-44.1	19.2	-16.5	7.8	.0	20.7
138	146.1	-44.2	19.2	-16.5	7.8	.0	20.8
139	161.0	-44.3	19.3	-16.5	7.8	.0	20.8
140	175.9	-44.4	19.4	-16.5	7.8	.0	20.9
141	190.8	-44.6	19.4	-16.5	7.8	.0	20.9
142	205.7	-44.7	19.5	-16.5	7.8	.0	21.0
143	220.6	-44.8	19.5	-16.5	7.8	.0	21.0
144	235.5	-44.9	19.6	-16.5	7.8	.0	21.1
145	250.5	-45.0	19.7	-16.5	7.8	.0	21.1
146	265.4	-45.1	19.7	-16.5	7.8	.0	21.2
147	280.3	-45.2	19.8	-16.5	7.8	.0	21.3
148	295.2	-45.4	19.8	-16.5	7.8	.0	21.3
149	310.1	-45.5	19.9	-16.5	7.8	.0	21.4
150	317.5	-45.5	19.9	-16.5	7.8	.0	21.4
151	332.4	-45.6	20.0	-16.5	7.8	.0	21.4
152	164.7	-44.1	19.2	-16.4	7.8	.0	20.7
153	358.4	-45.6	20.0	-16.4	7.8	.0	21.4
154	190.6	-44.0	19.1	-16.3	7.7	.0	20.6
155	205.6	-44.1	19.2	-16.3	7.7	.0	20.7
156	213.0	-44.2	19.2	-16.3	7.7	.0	20.7
157	227.9	-44.3	19.3	-16.3	7.7	.0	20.8
158	242.8	-44.4	19.4	-16.3	7.7	.0	20.8
159	257.7	-44.6	19.4	-16.3	7.7	.0	20.9
160	272.6	-44.7	19.5	-16.3	7.7	.0	21.0
161	287.5	-44.8	19.5	-16.3	7.7	.0	21.0
162	302.4	-44.9	19.6	-16.3	7.7	.0	21.1
163	317.3	-45.0	19.7	-16.3	7.7	.0	21.1
164	332.2	-45.1	19.7	-16.3	7.7	.0	21.2
165	347.2	-45.2	19.8	-16.3	7.7	.0	21.2
166	362.1	-45.4	19.8	-16.3	7.7	.0	21.3

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167	377.0	-45.5	19.9	-16.3	7.7	.0	21.3
168	384.4	-45.5	19.9	-16.3	7.7	.0	21.4
169	399.3	-45.6	20.0	-16.3	7.7	.0	21.4
170	231.5	-44.1	19.2	-16.3	7.7	.0	20.7
171	425.3	-45.6	20.0	-16.3	7.7	.0	21.4
172	257.5	-44.0	19.1	-16.2	7.7	.0	20.6
173	272.4	-44.1	19.2	-16.2	7.7	.0	20.7
174	279.9	-44.2	19.2	-16.2	7.7	.0	20.7
175	294.8	-44.3	19.3	-16.2	7.7	.0	20.8
176	309.7	-44.4	19.4	-16.2	7.7	.0	20.8
177	324.6	-44.6	19.4	-16.2	7.7	.0	20.9
178	339.5	-44.7	19.5	-16.2	7.7	.0	20.9
179	354.4	-44.8	19.5	-16.2	7.7	.0	21.0
180	369.3	-44.9	19.6	-16.2	7.7	.0	21.0
181	384.2	-45.0	19.7	-16.2	7.7	.0	21.1
182	399.1	-45.1	19.7	-16.2	7.7	.0	21.2
183	414.0	-45.2	19.8	-16.2	7.7	.0	21.2
184	428.9	-45.4	19.8	-16.2	7.7	.0	21.3
185	443.9	-45.5	19.9	-16.2	7.7	.0	21.3
186	451.3	-45.5	19.9	-16.2	7.7	.0	21.4
187	466.2	-45.6	20.0	-16.2	7.7	.0	21.4
188	298.4	-44.1	19.2	-16.2	7.6	.0	20.6
189	492.2	-45.6	20.0	-16.2	7.6	.0	21.4
190	324.4	-44.0	19.1	-16.1	7.6	.0	20.6
191	339.3	-44.1	19.2	-16.1	7.6	.0	20.7
192	346.8	-44.2	19.2	-16.1	7.6	.0	20.7
193	361.7	-44.3	19.3	-16.1	7.6	.0	20.7
194	376.6	-44.4	19.4	-16.1	7.6	.0	20.8
195	391.5	-44.6	19.4	-16.1	7.6	.0	20.9
196	406.4	-44.7	19.5	-16.1	7.6	.0	20.9
197	421.3	-44.8	19.5	-16.1	7.6	.0	21.0
198	436.2	-44.9	19.6	-16.1	7.6	.0	21.0
199	451.1	-45.0	19.7	-16.1	7.6	.0	21.1
200	466.0	-45.1	19.7	-16.1	7.6	.0	21.1
201	480.9	-45.2	19.8	-16.1	7.6	.0	21.2
202	495.8	-45.4	19.8	-16.1	7.6	.0	21.2
203	510.7	-45.5	19.9	-16.1	7.6	.0	21.3
204	518.2	-45.5	19.9	-16.1	7.6	.0	21.3
205	533.1	-45.6	20.0	-16.1	7.6	.0	21.4
206	357.9	-44.0	19.1	-16.1	7.6	.0	20.6
207	372.8	-44.1	19.2	-16.1	7.6	.0	20.7
208	380.2	-44.2	19.2	-16.1	7.6	.0	20.7
209	395.1	-44.3	19.3	-16.1	7.6	.0	20.7
210	410.0	-44.4	19.4	-16.1	7.6	.0	20.8
211	424.9	-44.6	19.4	-16.1	7.6	.0	20.8
212	439.9	-44.7	19.5	-16.1	7.6	.0	20.9
213	454.8	-44.8	19.5	-16.1	7.6	.0	21.0
214	469.7	-44.9	19.6	-16.1	7.6	.0	21.0
215	484.6	-45.0	19.7	-16.1	7.6	.0	21.1
216	499.5	-45.1	19.7	-16.1	7.6	.0	21.1
217	514.4	-45.2	19.8	-16.1	7.6	.0	21.2
218	529.3	-45.4	19.8	-16.1	7.6	.0	21.2
219	544.2	-45.5	19.9	-16.1	7.6	.0	21.3
220	551.6	-45.5	19.9	-16.1	7.6	.0	21.3
221	566.5	-45.6	20.0	-16.1	7.6	.0	21.4
222	398.8	-44.1	19.2	-16.0	7.6	.0	20.6
223	421.1	-44.3	19.3	-16.0	7.6	.0	20.7
224	436.0	-44.4	19.3	-16.0	7.6	.0	20.8
225	450.9	-44.5	19.4	-16.0	7.6	.0	20.8
226	465.8	-44.6	19.4	-16.0	7.6	.0	20.9
227	480.8	-44.7	19.5	-16.0	7.6	.0	20.9
228	495.7	-44.8	19.6	-16.0	7.6	.0	21.0
229	510.6	-45.0	19.6	-16.0	7.6	.0	21.0
230	525.5	-45.1	19.7	-16.0	7.6	.0	21.1
231	540.4	-45.2	19.7	-16.0	7.6	.0	21.1
232	555.3	-45.3	19.8	-16.0	7.6	.0	21.2
233	570.2	-45.4	19.9	-16.0	7.6	.0	21.3
234	592.5	-45.6	20.0	-16.0	7.6	.0	21.3
235	424.8	-44.0	19.1	-16.0	7.5	.0	20.6
236	439.7	-44.1	19.2	-16.0	7.5	.0	20.6
237	447.1	-44.2	19.2	-16.0	7.5	.0	20.7
238	462.0	-44.3	19.3	-16.0	7.5	.0	20.7
239	476.9	-44.4	19.4	-16.0	7.5	.0	20.8
240	491.8	-44.6	19.4	-16.0	7.5	.0	20.8
241	506.7	-44.7	19.5	-16.0	7.5	.0	20.9
242	521.6	-44.8	19.5	-16.0	7.5	.0	20.9



Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

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243	536.6	-44.9	19.6	-16.0	7.5	.0	21.0
244	551.5	-45.0	19.7	-16.0	7.5	.0	21.1
245	566.4	-45.1	19.7	-16.0	7.5	.0	21.1
246	581.3	-45.2	19.8	-16.0	7.5	.0	21.2
247	596.2	-45.4	19.8	-16.0	7.5	.0	21.2
248	611.1	-45.5	19.9	-16.0	7.5	.0	21.3
249	618.5	-45.5	19.9	-16.0	7.5	.0	21.3
250	633.4	-45.6	20.0	-16.0	7.5	.0	21.4

 $M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$

Relazione di calcolo fondazioni dirette su micropali
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LINEA FS FERRANDINA MATERA
VI08 Pile P21 e P22 - Analisi SLV

CONDIZIONE DI CARICO 3
VI08 - P21 - SLV_y

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	25521.3	3365.0	78572.0	13799.3	325064.4	3965.2
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
43620.3	3365.0	78572.0	13799.3	325064.4	3965.2

Punto di applic. carico verticale: Xv = 1.801 m Yv = 7.452 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.745	.645	.168	2.485	.382	.012

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	633.1	10.8	-4.5	57.1	-27.2	.0	27.5
2	583.4	11.2	-4.7	57.1	-27.2	.0	27.6
3	558.6	11.4	-4.8	57.1	-27.2	.0	27.6
4	508.9	11.7	-5.0	57.1	-27.2	.0	27.6
5	459.2	12.1	-5.2	57.1	-27.2	.0	27.6
6	409.5	12.5	-5.4	57.1	-27.2	.0	27.7
7	359.8	12.9	-5.6	57.1	-27.2	.0	27.7
8	310.2	13.3	-5.8	57.1	-27.2	.0	27.8
9	260.5	13.7	-6.0	57.1	-27.2	.0	27.8
10	210.8	14.0	-6.2	57.1	-27.2	.0	27.8
11	161.1	14.4	-6.4	57.1	-27.2	.0	27.9
12	111.4	14.8	-6.6	57.1	-27.2	.0	27.9
13	61.7	15.2	-6.8	57.1	-27.2	.0	28.0
14	12.1	15.6	-7.0	57.1	-27.2	.0	28.0

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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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15	-12.8	15.8	-7.1	57.1	-27.2	.0	28.1
16	-62.5	16.1	-7.3	57.1	-27.2	.0	28.1
17	598.2	11.0	-4.6	57.0	-27.1	.0	27.4
18	523.7	11.5	-4.9	57.0	-27.1	.0	27.5
19	474.0	11.9	-5.1	57.0	-27.1	.0	27.5
20	424.3	12.3	-5.3	57.0	-27.1	.0	27.6
21	374.6	12.7	-5.5	57.0	-27.1	.0	27.6
22	324.9	13.1	-5.7	57.0	-27.1	.0	27.6
23	275.2	13.5	-5.9	57.0	-27.1	.0	27.7
24	225.6	13.8	-6.1	57.0	-27.1	.0	27.7
25	175.9	14.2	-6.3	57.0	-27.1	.0	27.8
26	126.2	14.6	-6.5	57.0	-27.1	.0	27.8
27	76.5	15.0	-6.7	57.0	-27.1	.0	27.9
28	26.8	15.4	-6.9	57.0	-27.1	.0	27.9
29	-47.7	16.0	-7.2	57.0	-27.1	.0	28.0
30	612.9	10.8	-4.5	56.8	-27.0	.0	27.3
31	563.3	11.2	-4.7	56.8	-27.0	.0	27.4
32	538.4	11.4	-4.8	56.8	-27.0	.0	27.4
33	488.7	11.7	-5.0	56.8	-27.0	.0	27.4
34	439.1	12.1	-5.2	56.8	-27.0	.0	27.5
35	389.4	12.5	-5.4	56.8	-27.0	.0	27.5
36	339.7	12.9	-5.6	56.8	-27.0	.0	27.5
37	290.0	13.3	-5.8	56.8	-27.0	.0	27.6
38	240.3	13.7	-6.0	56.8	-27.0	.0	27.6
39	190.6	14.0	-6.2	56.8	-27.0	.0	27.7
40	141.0	14.4	-6.4	56.8	-27.0	.0	27.7
41	91.3	14.8	-6.6	56.8	-27.0	.0	27.8
42	41.6	15.2	-6.8	56.8	-27.0	.0	27.8
43	-8.1	15.6	-7.0	56.8	-27.0	.0	27.9
44	-32.9	15.8	-7.1	56.8	-27.0	.0	27.9
45	-82.6	16.1	-7.3	56.8	-27.0	.0	27.9
46	602.9	10.8	-4.5	56.6	-26.9	.0	27.2
47	553.2	11.2	-4.7	56.6	-26.9	.0	27.3
48	528.3	11.4	-4.8	56.6	-26.9	.0	27.3
49	478.7	11.7	-5.0	56.6	-26.9	.0	27.3
50	429.0	12.1	-5.2	56.6	-26.9	.0	27.4
51	379.3	12.5	-5.4	56.6	-26.9	.0	27.4
52	329.6	12.9	-5.6	56.6	-26.9	.0	27.4
53	279.9	13.3	-5.8	56.6	-26.9	.0	27.5
54	230.2	13.7	-6.0	56.6	-26.9	.0	27.5
55	180.6	14.0	-6.2	56.6	-26.9	.0	27.6
56	130.9	14.4	-6.4	56.6	-26.9	.0	27.6
57	81.2	14.8	-6.6	56.6	-26.9	.0	27.7
58	31.5	15.2	-6.8	56.6	-26.9	.0	27.7
59	-18.2	15.6	-7.0	56.6	-26.9	.0	27.8
60	-43.0	15.8	-7.1	56.6	-26.9	.0	27.8
61	-92.7	16.1	-7.3	56.6	-26.9	.0	27.8
62	568.0	11.0	-4.6	56.4	-26.8	.0	27.2
63	-77.9	16.0	-7.2	56.4	-26.8	.0	27.7
64	582.7	10.8	-4.5	56.3	-26.7	.0	27.1
65	533.0	11.2	-4.7	56.3	-26.7	.0	27.1
66	508.2	11.4	-4.8	56.3	-26.7	.0	27.1
67	458.5	11.7	-5.0	56.3	-26.7	.0	27.2
68	408.8	12.1	-5.2	56.3	-26.7	.0	27.2
69	359.1	12.5	-5.4	56.3	-26.7	.0	27.2
70	309.5	12.9	-5.6	56.3	-26.7	.0	27.3
71	259.8	13.3	-5.8	56.3	-26.7	.0	27.3
72	210.1	13.7	-6.0	56.3	-26.7	.0	27.4
73	160.4	14.0	-6.2	56.3	-26.7	.0	27.4
74	110.7	14.4	-6.4	56.3	-26.7	.0	27.4
75	61.0	14.8	-6.6	56.3	-26.7	.0	27.5
76	11.4	15.2	-6.8	56.3	-26.7	.0	27.5
77	-38.3	15.6	-7.0	56.3	-26.7	.0	27.6
78	-63.2	15.8	-7.1	56.3	-26.7	.0	27.6
79	-112.9	16.1	-7.3	56.3	-26.7	.0	27.7
80	547.8	11.0	-4.6	56.1	-26.6	.0	27.0
81	-98.1	16.0	-7.2	56.1	-26.6	.0	27.5
82	562.6	10.8	-4.5	55.9	-26.5	.0	26.9
83	512.9	11.2	-4.7	55.9	-26.5	.0	26.9
84	488.0	11.4	-4.8	55.9	-26.5	.0	26.9
85	438.4	11.7	-5.0	55.9	-26.5	.0	27.0
86	388.7	12.1	-5.2	55.9	-26.5	.0	27.0
87	339.0	12.5	-5.4	55.9	-26.5	.0	27.0
88	289.3	12.9	-5.6	55.9	-26.5	.0	27.1
89	239.6	13.3	-5.8	55.9	-26.5	.0	27.1
90	189.9	13.7	-6.0	55.9	-26.5	.0	27.2

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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91	140.3	14.0	-6.2	55.9	-26.5	.0	27.2
92	90.6	14.4	-6.4	55.9	-26.5	.0	27.3
93	40.9	14.8	-6.6	55.9	-26.5	.0	27.3
94	-8.8	15.2	-6.8	55.9	-26.5	.0	27.4
95	-58.5	15.6	-7.0	55.9	-26.5	.0	27.4
96	-83.3	15.8	-7.1	55.9	-26.5	.0	27.4
97	-133.0	16.1	-7.3	55.9	-26.5	.0	27.5
98	527.7	11.0	-4.6	55.7	-26.4	.0	26.8
99	-118.2	16.0	-7.2	55.7	-26.4	.0	27.4
100	542.4	10.8	-4.5	55.6	-26.3	.0	26.7
101	492.7	11.2	-4.7	55.6	-26.3	.0	26.7
102	467.9	11.4	-4.8	55.6	-26.3	.0	26.8
103	418.2	11.7	-5.0	55.6	-26.3	.0	26.8
104	368.5	12.1	-5.2	55.6	-26.3	.0	26.8
105	318.8	12.5	-5.4	55.6	-26.3	.0	26.9
106	269.2	12.9	-5.6	55.6	-26.3	.0	26.9
107	219.5	13.3	-5.8	55.6	-26.3	.0	26.9
108	169.8	13.7	-6.0	55.6	-26.3	.0	27.0
109	120.1	14.0	-6.2	55.6	-26.3	.0	27.0
110	70.4	14.4	-6.4	55.6	-26.3	.0	27.1
111	20.7	14.8	-6.6	55.6	-26.3	.0	27.1
112	-28.9	15.2	-6.8	55.6	-26.3	.0	27.2
113	-78.6	15.6	-7.0	55.6	-26.3	.0	27.2
114	-103.5	15.8	-7.1	55.6	-26.3	.0	27.3
115	-153.2	16.1	-7.3	55.6	-26.3	.0	27.3
116	507.5	11.0	-4.6	55.4	-26.2	.0	26.6
117	-138.4	16.0	-7.2	55.4	-26.2	.0	27.2
118	522.3	10.8	-4.5	55.2	-26.1	.0	26.5
119	472.6	11.2	-4.7	55.2	-26.1	.0	26.6
120	447.7	11.4	-4.8	55.2	-26.1	.0	26.6
121	398.1	11.7	-5.0	55.2	-26.1	.0	26.6
122	348.4	12.1	-5.2	55.2	-26.1	.0	26.6
123	298.7	12.5	-5.4	55.2	-26.1	.0	26.7
124	249.0	12.9	-5.6	55.2	-26.1	.0	26.7
125	199.3	13.3	-5.8	55.2	-26.1	.0	26.8
126	149.6	13.7	-6.0	55.2	-26.1	.0	26.8
127	100.0	14.0	-6.2	55.2	-26.1	.0	26.9
128	50.3	14.4	-6.4	55.2	-26.1	.0	26.9
129	.6	14.8	-6.6	55.2	-26.1	.0	27.0
130	-49.1	15.2	-6.8	55.2	-26.1	.0	27.0
131	-98.8	15.6	-7.0	55.2	-26.1	.0	27.1
132	-123.6	15.8	-7.1	55.2	-26.1	.0	27.1
133	-173.3	16.1	-7.3	55.2	-26.1	.0	27.1
134	487.3	11.0	-4.6	55.0	-26.0	.0	26.4
135	-158.5	16.0	-7.2	55.0	-26.0	.0	27.0
136	502.1	10.8	-4.5	54.8	-26.0	.0	26.3
137	452.4	11.2	-4.7	54.8	-26.0	.0	26.4
138	427.6	11.4	-4.8	54.8	-26.0	.0	26.4
139	377.9	11.7	-5.0	54.8	-26.0	.0	26.4
140	328.2	12.1	-5.2	54.8	-26.0	.0	26.5
141	278.5	12.5	-5.4	54.8	-26.0	.0	26.5
142	228.9	12.9	-5.6	54.8	-26.0	.0	26.5
143	179.2	13.3	-5.8	54.8	-26.0	.0	26.6
144	129.5	13.7	-6.0	54.8	-26.0	.0	26.6
145	79.8	14.0	-6.2	54.8	-26.0	.0	26.7
146	30.1	14.4	-6.4	54.8	-26.0	.0	26.7
147	-19.6	14.8	-6.6	54.8	-26.0	.0	26.8
148	-69.2	15.2	-6.8	54.8	-26.0	.0	26.8
149	-118.9	15.6	-7.0	54.8	-26.0	.0	26.9
150	-143.8	15.8	-7.1	54.8	-26.0	.0	26.9
151	-193.5	16.1	-7.3	54.8	-26.0	.0	27.0
152	467.2	11.0	-4.6	54.7	-25.9	.0	26.3
153	-178.7	16.0	-7.2	54.7	-25.9	.0	26.8
154	482.0	10.8	-4.5	54.5	-25.8	.0	26.2
155	432.3	11.2	-4.7	54.5	-25.8	.0	26.2
156	407.4	11.4	-4.8	54.5	-25.8	.0	26.2
157	357.8	11.7	-5.0	54.5	-25.8	.0	26.2
158	308.1	12.1	-5.2	54.5	-25.8	.0	26.3
159	258.4	12.5	-5.4	54.5	-25.8	.0	26.3
160	208.7	12.9	-5.6	54.5	-25.8	.0	26.4
161	159.0	13.3	-5.8	54.5	-25.8	.0	26.4
162	109.3	13.7	-6.0	54.5	-25.8	.0	26.5
163	59.7	14.0	-6.2	54.5	-25.8	.0	26.5
164	10.0	14.4	-6.4	54.5	-25.8	.0	26.5
165	-39.7	14.8	-6.6	54.5	-25.8	.0	26.6
166	-89.4	15.2	-6.8	54.5	-25.8	.0	26.6

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	65 di 91

167	-139.1	15.6	-7.0	54.5	-25.8	.0	26.7
168	-163.9	15.8	-7.1	54.5	-25.8	.0	26.7
169	-213.6	16.1	-7.3	54.5	-25.8	.0	26.8
170	447.0	11.0	-4.6	54.3	-25.7	.0	26.1
171	-198.8	16.0	-7.2	54.3	-25.7	.0	26.7
172	461.8	10.8	-4.5	54.1	-25.6	.0	26.0
173	412.1	11.2	-4.7	54.1	-25.6	.0	26.0
174	387.3	11.4	-4.8	54.1	-25.6	.0	26.0
175	337.6	11.7	-5.0	54.1	-25.6	.0	26.1
176	287.9	12.1	-5.2	54.1	-25.6	.0	26.1
177	238.2	12.5	-5.4	54.1	-25.6	.0	26.1
178	188.6	12.9	-5.6	54.1	-25.6	.0	26.2
179	138.9	13.3	-5.8	54.1	-25.6	.0	26.2
180	89.2	13.7	-6.0	54.1	-25.6	.0	26.3
181	39.5	14.0	-6.2	54.1	-25.6	.0	26.3
182	-10.2	14.4	-6.4	54.1	-25.6	.0	26.4
183	-59.9	14.8	-6.6	54.1	-25.6	.0	26.4
184	-109.5	15.2	-6.8	54.1	-25.6	.0	26.5
185	-159.2	15.6	-7.0	54.1	-25.6	.0	26.5
186	-184.1	15.8	-7.1	54.1	-25.6	.0	26.5
187	-233.8	16.1	-7.3	54.1	-25.6	.0	26.6
188	426.9	11.0	-4.6	54.0	-25.5	.0	25.9
189	-219.0	16.0	-7.2	54.0	-25.5	.0	26.5
190	441.7	10.8	-4.5	53.8	-25.4	.0	25.8
191	392.0	11.2	-4.7	53.8	-25.4	.0	25.8
192	367.1	11.4	-4.8	53.8	-25.4	.0	25.8
193	317.5	11.7	-5.0	53.8	-25.4	.0	25.9
194	267.8	12.1	-5.2	53.8	-25.4	.0	25.9
195	218.1	12.5	-5.4	53.8	-25.4	.0	26.0
196	168.4	12.9	-5.6	53.8	-25.4	.0	26.0
197	118.7	13.3	-5.8	53.8	-25.4	.0	26.0
198	69.0	13.7	-6.0	53.8	-25.4	.0	26.1
199	19.4	14.0	-6.2	53.8	-25.4	.0	26.1
200	-30.3	14.4	-6.4	53.8	-25.4	.0	26.2
201	-80.0	14.8	-6.6	53.8	-25.4	.0	26.2
202	-129.7	15.2	-6.8	53.8	-25.4	.0	26.3
203	-179.4	15.6	-7.0	53.8	-25.4	.0	26.3
204	-204.2	15.8	-7.1	53.8	-25.4	.0	26.4
205	-253.9	16.1	-7.3	53.8	-25.4	.0	26.4
206	431.6	10.8	-4.5	53.6	-25.3	.0	25.7
207	381.9	11.2	-4.7	53.6	-25.3	.0	25.7
208	357.1	11.4	-4.8	53.6	-25.3	.0	25.8
209	307.4	11.7	-5.0	53.6	-25.3	.0	25.8
210	257.7	12.1	-5.2	53.6	-25.3	.0	25.8
211	208.0	12.5	-5.4	53.6	-25.3	.0	25.9
212	158.3	12.9	-5.6	53.6	-25.3	.0	25.9
213	108.6	13.3	-5.8	53.6	-25.3	.0	26.0
214	59.0	13.7	-6.0	53.6	-25.3	.0	26.0
215	9.3	14.0	-6.2	53.6	-25.3	.0	26.1
216	-40.4	14.4	-6.4	53.6	-25.3	.0	26.1
217	-90.1	14.8	-6.6	53.6	-25.3	.0	26.1
218	-139.8	15.2	-6.8	53.6	-25.3	.0	26.2
219	-189.5	15.6	-7.0	53.6	-25.3	.0	26.3
220	-214.3	15.8	-7.1	53.6	-25.3	.0	26.3
221	-264.0	16.1	-7.3	53.6	-25.3	.0	26.3
222	396.7	11.0	-4.6	53.4	-25.2	.0	25.6
223	322.1	11.5	-4.9	53.4	-25.2	.0	25.7
224	272.5	11.9	-5.1	53.4	-25.2	.0	25.7
225	222.8	12.3	-5.3	53.4	-25.2	.0	25.8
226	173.1	12.7	-5.5	53.4	-25.2	.0	25.8
227	123.4	13.1	-5.7	53.4	-25.2	.0	25.8
228	73.7	13.5	-5.9	53.4	-25.2	.0	25.9
229	24.0	13.8	-6.1	53.4	-25.2	.0	25.9
230	-25.6	14.2	-6.3	53.4	-25.2	.0	26.0
231	-75.3	14.6	-6.5	53.4	-25.2	.0	26.0
232	-125.0	15.0	-6.7	53.4	-25.2	.0	26.1
233	-174.7	15.4	-6.9	53.4	-25.2	.0	26.1
234	-249.2	16.0	-7.2	53.4	-25.2	.0	26.2
235	411.4	10.8	-4.5	53.3	-25.1	.0	25.5
236	361.8	11.2	-4.7	53.3	-25.1	.0	25.6
237	336.9	11.4	-4.8	53.3	-25.1	.0	25.6
238	287.2	11.7	-5.0	53.3	-25.1	.0	25.6
239	237.5	12.1	-5.2	53.3	-25.1	.0	25.7
240	187.9	12.5	-5.4	53.3	-25.1	.0	25.7
241	138.2	12.9	-5.6	53.3	-25.1	.0	25.7
242	88.5	13.3	-5.8	53.3	-25.1	.0	25.8



Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	66 di 91

243	38.8	13.7	-6.0	53.3	-25.1	.0	25.8
244	-10.9	14.0	-6.2	53.3	-25.1	.0	25.9
245	-60.6	14.4	-6.4	53.3	-25.1	.0	25.9
246	-110.2	14.8	-6.6	53.3	-25.1	.0	26.0
247	-159.9	15.2	-6.8	53.3	-25.1	.0	26.0
248	-209.6	15.6	-7.0	53.3	-25.1	.0	26.1
249	-234.5	15.8	-7.1	53.3	-25.1	.0	26.1
250	-284.1	16.1	-7.3	53.3	-25.1	.0	26.2

 $M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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LINEA FS FERRANDINA MATERA
VI08 Pile P21 e P22 - Analisi SLV

CONDIZIONE DI CARICO 4
VI08 - P21 - SLV_y

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	22277.0	-3365.0	-78080.0	-13799.3	-325064.4	-3965.2
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
40376.0	-3365.0	-78080.0	-13799.3	-325064.4	-3965.2

Punto di applic. carico verticale: Xv = -1.934 m Yv = -8.051 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.615	-.644	-.167	-2.485	-.382	-.012

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	-296.4	-10.8	4.5	-57.1	27.2	.0	27.5
2	-246.7	-11.2	4.7	-57.1	27.2	.0	27.6
3	-221.9	-11.4	4.8	-57.1	27.2	.0	27.6
4	-172.2	-11.7	5.0	-57.1	27.2	.0	27.6
5	-122.5	-12.1	5.2	-57.1	27.2	.0	27.6
6	-72.9	-12.5	5.4	-57.1	27.2	.0	27.7
7	-23.2	-12.9	5.6	-57.1	27.2	.0	27.7
8	26.5	-13.3	5.8	-57.1	27.2	.0	27.8
9	76.2	-13.7	6.0	-57.1	27.2	.0	27.8
10	125.9	-14.0	6.2	-57.1	27.2	.0	27.8
11	175.6	-14.4	6.4	-57.1	27.2	.0	27.9
12	225.2	-14.8	6.6	-57.1	27.2	.0	27.9
13	274.9	-15.2	6.8	-57.1	27.2	.0	28.0
14	324.6	-15.6	7.0	-57.1	27.2	.0	28.0

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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15	349.5	-15.8	7.1	-57.1	27.2	.0	28.1
16	399.1	-16.1	7.3	-57.1	27.2	.0	28.1
17	-261.6	-11.0	4.6	-57.0	27.1	.0	27.4
18	-187.1	-11.5	4.9	-57.0	27.1	.0	27.5
19	-137.4	-11.9	5.1	-57.0	27.1	.0	27.5
20	-87.7	-12.3	5.3	-57.0	27.1	.0	27.6
21	-38.0	-12.7	5.5	-57.0	27.1	.0	27.6
22	11.7	-13.1	5.7	-57.0	27.1	.0	27.6
23	61.4	-13.5	5.9	-57.0	27.1	.0	27.7
24	111.1	-13.8	6.1	-57.0	27.1	.0	27.7
25	160.7	-14.2	6.3	-57.0	27.1	.0	27.8
26	210.4	-14.6	6.5	-57.0	27.1	.0	27.8
27	260.1	-15.0	6.7	-57.0	27.1	.0	27.9
28	309.8	-15.4	6.9	-57.0	27.1	.0	27.9
29	384.3	-16.0	7.2	-57.0	27.1	.0	28.0
30	-276.4	-10.8	4.5	-56.8	27.0	.0	27.3
31	-226.7	-11.2	4.7	-56.8	27.0	.0	27.4
32	-201.9	-11.4	4.8	-56.8	27.0	.0	27.4
33	-152.2	-11.7	5.0	-56.8	27.0	.0	27.4
34	-102.5	-12.1	5.2	-56.8	27.0	.0	27.5
35	-52.8	-12.5	5.4	-56.8	27.0	.0	27.5
36	-3.1	-12.9	5.6	-56.8	27.0	.0	27.5
37	46.5	-13.3	5.8	-56.8	27.0	.0	27.6
38	96.2	-13.7	6.0	-56.8	27.0	.0	27.6
39	145.9	-14.0	6.2	-56.8	27.0	.0	27.7
40	195.6	-14.4	6.4	-56.8	27.0	.0	27.7
41	245.3	-14.8	6.6	-56.8	27.0	.0	27.8
42	295.0	-15.2	6.8	-56.8	27.0	.0	27.8
43	344.6	-15.6	7.0	-56.8	27.0	.0	27.9
44	369.5	-15.8	7.1	-56.8	27.0	.0	27.9
45	419.2	-16.1	7.3	-56.8	27.0	.0	27.9
46	-266.4	-10.8	4.5	-56.6	26.9	.0	27.2
47	-216.7	-11.2	4.7	-56.6	26.9	.0	27.3
48	-191.9	-11.4	4.8	-56.6	26.9	.0	27.3
49	-142.2	-11.7	5.0	-56.6	26.9	.0	27.3
50	-92.5	-12.1	5.2	-56.6	26.9	.0	27.4
51	-42.8	-12.5	5.4	-56.6	26.9	.0	27.4
52	6.9	-12.9	5.6	-56.6	26.9	.0	27.4
53	56.6	-13.3	5.8	-56.6	26.9	.0	27.5
54	106.2	-13.7	6.0	-56.6	26.9	.0	27.5
55	155.9	-14.0	6.2	-56.6	26.9	.0	27.6
56	205.6	-14.4	6.4	-56.6	26.9	.0	27.6
57	255.3	-14.8	6.6	-56.6	26.9	.0	27.7
58	305.0	-15.2	6.8	-56.6	26.9	.0	27.7
59	354.7	-15.6	7.0	-56.6	26.9	.0	27.8
60	379.5	-15.8	7.1	-56.6	26.9	.0	27.8
61	429.2	-16.1	7.3	-56.6	26.9	.0	27.8
62	-231.5	-11.0	4.6	-56.4	26.8	.0	27.2
63	414.4	-16.0	7.2	-56.4	26.8	.0	27.7
64	-246.4	-10.8	4.5	-56.3	26.7	.0	27.1
65	-196.7	-11.2	4.7	-56.3	26.7	.0	27.1
66	-171.8	-11.4	4.8	-56.3	26.7	.0	27.1
67	-122.2	-11.7	5.0	-56.3	26.7	.0	27.2
68	-72.5	-12.1	5.2	-56.3	26.7	.0	27.2
69	-22.8	-12.5	5.4	-56.3	26.7	.0	27.2
70	26.9	-12.9	5.6	-56.3	26.7	.0	27.3
71	76.6	-13.3	5.8	-56.3	26.7	.0	27.3
72	126.3	-13.7	6.0	-56.3	26.7	.0	27.4
73	175.9	-14.0	6.2	-56.3	26.7	.0	27.4
74	225.6	-14.4	6.4	-56.3	26.7	.0	27.4
75	275.3	-14.8	6.6	-56.3	26.7	.0	27.5
76	325.0	-15.2	6.8	-56.3	26.7	.0	27.5
77	374.7	-15.6	7.0	-56.3	26.7	.0	27.6
78	399.5	-15.8	7.1	-56.3	26.7	.0	27.6
79	449.2	-16.1	7.3	-56.3	26.7	.0	27.7
80	-211.5	-11.0	4.6	-56.1	26.6	.0	27.0
81	434.4	-16.0	7.2	-56.1	26.6	.0	27.5
82	-226.3	-10.8	4.5	-55.9	26.5	.0	26.9
83	-176.7	-11.2	4.7	-55.9	26.5	.0	26.9
84	-151.8	-11.4	4.8	-55.9	26.5	.0	26.9
85	-102.1	-11.7	5.0	-55.9	26.5	.0	27.0
86	-52.4	-12.1	5.2	-55.9	26.5	.0	27.0
87	-2.8	-12.5	5.4	-55.9	26.5	.0	27.0
88	46.9	-12.9	5.6	-55.9	26.5	.0	27.1
89	96.6	-13.3	5.8	-55.9	26.5	.0	27.1
90	146.3	-13.7	6.0	-55.9	26.5	.0	27.2

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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91	196.0	-14.0	6.2	-55.9	26.5	.0	27.2
92	245.7	-14.4	6.4	-55.9	26.5	.0	27.3
93	295.3	-14.8	6.6	-55.9	26.5	.0	27.3
94	345.0	-15.2	6.8	-55.9	26.5	.0	27.4
95	394.7	-15.6	7.0	-55.9	26.5	.0	27.4
96	419.6	-15.8	7.1	-55.9	26.5	.0	27.4
97	469.2	-16.1	7.3	-55.9	26.5	.0	27.5
98	-191.5	-11.0	4.6	-55.7	26.4	.0	26.8
99	454.4	-16.0	7.2	-55.7	26.4	.0	27.4
100	-206.3	-10.8	4.5	-55.6	26.3	.0	26.7
101	-156.6	-11.2	4.7	-55.6	26.3	.0	26.7
102	-131.8	-11.4	4.8	-55.6	26.3	.0	26.8
103	-82.1	-11.7	5.0	-55.6	26.3	.0	26.8
104	-32.4	-12.1	5.2	-55.6	26.3	.0	26.8
105	17.3	-12.5	5.4	-55.6	26.3	.0	26.9
106	67.0	-12.9	5.6	-55.6	26.3	.0	26.9
107	116.6	-13.3	5.8	-55.6	26.3	.0	26.9
108	166.3	-13.7	6.0	-55.6	26.3	.0	27.0
109	216.0	-14.0	6.2	-55.6	26.3	.0	27.0
110	265.7	-14.4	6.4	-55.6	26.3	.0	27.1
111	315.4	-14.8	6.6	-55.6	26.3	.0	27.1
112	365.1	-15.2	6.8	-55.6	26.3	.0	27.2
113	414.7	-15.6	7.0	-55.6	26.3	.0	27.2
114	439.6	-15.8	7.1	-55.6	26.3	.0	27.3
115	489.3	-16.1	7.3	-55.6	26.3	.0	27.3
116	-171.5	-11.0	4.6	-55.4	26.2	.0	26.6
117	474.4	-16.0	7.2	-55.4	26.2	.0	27.2
118	-186.3	-10.8	4.5	-55.2	26.1	.0	26.5
119	-136.6	-11.2	4.7	-55.2	26.1	.0	26.6
120	-111.8	-11.4	4.8	-55.2	26.1	.0	26.6
121	-62.1	-11.7	5.0	-55.2	26.1	.0	26.6
122	-12.4	-12.1	5.2	-55.2	26.1	.0	26.6
123	37.3	-12.5	5.4	-55.2	26.1	.0	26.7
124	87.0	-12.9	5.6	-55.2	26.1	.0	26.7
125	136.7	-13.3	5.8	-55.2	26.1	.0	26.8
126	186.3	-13.7	6.0	-55.2	26.1	.0	26.8
127	236.0	-14.0	6.2	-55.2	26.1	.0	26.9
128	285.7	-14.4	6.4	-55.2	26.1	.0	26.9
129	335.4	-14.8	6.6	-55.2	26.1	.0	27.0
130	385.1	-15.2	6.8	-55.2	26.1	.0	27.0
131	434.8	-15.6	7.0	-55.2	26.1	.0	27.1
132	459.6	-15.8	7.1	-55.2	26.1	.0	27.1
133	509.3	-16.1	7.3	-55.2	26.1	.0	27.1
134	-151.4	-11.0	4.6	-55.0	26.0	.0	26.4
135	494.5	-16.0	7.2	-55.0	26.0	.0	27.0
136	-166.3	-10.8	4.5	-54.8	26.0	.0	26.3
137	-116.6	-11.2	4.7	-54.8	26.0	.0	26.4
138	-91.7	-11.4	4.8	-54.8	26.0	.0	26.4
139	-42.0	-11.7	5.0	-54.8	26.0	.0	26.4
140	7.6	-12.1	5.2	-54.8	26.0	.0	26.5
141	57.3	-12.5	5.4	-54.8	26.0	.0	26.5
142	107.0	-12.9	5.6	-54.8	26.0	.0	26.5
143	156.7	-13.3	5.8	-54.8	26.0	.0	26.6
144	206.4	-13.7	6.0	-54.8	26.0	.0	26.6
145	256.1	-14.0	6.2	-54.8	26.0	.0	26.7
146	305.7	-14.4	6.4	-54.8	26.0	.0	26.7
147	355.4	-14.8	6.6	-54.8	26.0	.0	26.8
148	405.1	-15.2	6.8	-54.8	26.0	.0	26.8
149	454.8	-15.6	7.0	-54.8	26.0	.0	26.9
150	479.6	-15.8	7.1	-54.8	26.0	.0	26.9
151	529.3	-16.1	7.3	-54.8	26.0	.0	27.0
152	-131.4	-11.0	4.6	-54.7	25.9	.0	26.3
153	514.5	-16.0	7.2	-54.7	25.9	.0	26.8
154	-146.2	-10.8	4.5	-54.5	25.8	.0	26.2
155	-96.5	-11.2	4.7	-54.5	25.8	.0	26.2
156	-71.7	-11.4	4.8	-54.5	25.8	.0	26.2
157	-22.0	-11.7	5.0	-54.5	25.8	.0	26.2
158	27.7	-12.1	5.2	-54.5	25.8	.0	26.3
159	77.3	-12.5	5.4	-54.5	25.8	.0	26.3
160	127.0	-12.9	5.6	-54.5	25.8	.0	26.4
161	176.7	-13.3	5.8	-54.5	25.8	.0	26.4
162	226.4	-13.7	6.0	-54.5	25.8	.0	26.5
163	276.1	-14.0	6.2	-54.5	25.8	.0	26.5
164	325.8	-14.4	6.4	-54.5	25.8	.0	26.5
165	375.5	-14.8	6.6	-54.5	25.8	.0	26.6
166	425.1	-15.2	6.8	-54.5	25.8	.0	26.6

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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167	474.8	-15.6	7.0	-54.5	25.8	.0	26.7
168	499.7	-15.8	7.1	-54.5	25.8	.0	26.7
169	549.3	-16.1	7.3	-54.5	25.8	.0	26.8
170	-111.4	-11.0	4.6	-54.3	25.7	.0	26.1
171	534.5	-16.0	7.2	-54.3	25.7	.0	26.7
172	-126.2	-10.8	4.5	-54.1	25.6	.0	26.0
173	-76.5	-11.2	4.7	-54.1	25.6	.0	26.0
174	-51.7	-11.4	4.8	-54.1	25.6	.0	26.0
175	-2.0	-11.7	5.0	-54.1	25.6	.0	26.1
176	47.7	-12.1	5.2	-54.1	25.6	.0	26.1
177	97.4	-12.5	5.4	-54.1	25.6	.0	26.1
178	147.1	-12.9	5.6	-54.1	25.6	.0	26.2
179	196.7	-13.3	5.8	-54.1	25.6	.0	26.2
180	246.4	-13.7	6.0	-54.1	25.6	.0	26.3
181	296.1	-14.0	6.2	-54.1	25.6	.0	26.3
182	345.8	-14.4	6.4	-54.1	25.6	.0	26.4
183	395.5	-14.8	6.6	-54.1	25.6	.0	26.4
184	445.2	-15.2	6.8	-54.1	25.6	.0	26.5
185	494.8	-15.6	7.0	-54.1	25.6	.0	26.5
186	519.7	-15.8	7.1	-54.1	25.6	.0	26.5
187	569.4	-16.1	7.3	-54.1	25.6	.0	26.6
188	-91.3	-11.0	4.6	-54.0	25.5	.0	25.9
189	554.5	-16.0	7.2	-54.0	25.5	.0	26.5
190	-106.2	-10.8	4.5	-53.8	25.4	.0	25.8
191	-56.5	-11.2	4.7	-53.8	25.4	.0	25.8
192	-31.6	-11.4	4.8	-53.8	25.4	.0	25.8
193	18.0	-11.7	5.0	-53.8	25.4	.0	25.9
194	67.7	-12.1	5.2	-53.8	25.4	.0	25.9
195	117.4	-12.5	5.4	-53.8	25.4	.0	26.0
196	167.1	-12.9	5.6	-53.8	25.4	.0	26.0
197	216.8	-13.3	5.8	-53.8	25.4	.0	26.1
198	266.5	-13.7	6.0	-53.8	25.4	.0	26.1
199	316.1	-14.0	6.2	-53.8	25.4	.0	26.1
200	365.8	-14.4	6.4	-53.8	25.4	.0	26.2
201	415.5	-14.8	6.6	-53.8	25.4	.0	26.2
202	465.2	-15.2	6.8	-53.8	25.4	.0	26.3
203	514.9	-15.6	7.0	-53.8	25.4	.0	26.3
204	539.7	-15.8	7.1	-53.8	25.4	.0	26.4
205	589.4	-16.1	7.3	-53.8	25.4	.0	26.4
206	-96.2	-10.8	4.5	-53.6	25.3	.0	25.7
207	-46.5	-11.2	4.7	-53.6	25.3	.0	25.7
208	-21.6	-11.4	4.8	-53.6	25.3	.0	25.8
209	28.1	-11.7	5.0	-53.6	25.3	.0	25.8
210	77.7	-12.1	5.2	-53.6	25.3	.0	25.8
211	127.4	-12.5	5.4	-53.6	25.3	.0	25.9
212	177.1	-12.9	5.6	-53.6	25.3	.0	25.9
213	226.8	-13.3	5.8	-53.6	25.3	.0	26.0
214	276.5	-13.7	6.0	-53.6	25.3	.0	26.0
215	326.2	-14.0	6.2	-53.6	25.3	.0	26.1
216	375.8	-14.4	6.4	-53.6	25.3	.0	26.1
217	425.5	-14.8	6.6	-53.6	25.3	.0	26.2
218	475.2	-15.2	6.8	-53.6	25.3	.0	26.2
219	524.9	-15.6	7.0	-53.6	25.3	.0	26.3
220	549.7	-15.8	7.1	-53.6	25.3	.0	26.3
221	599.4	-16.1	7.3	-53.6	25.3	.0	26.3
222	-61.3	-11.0	4.6	-53.4	25.2	.0	25.6
223	13.2	-11.5	4.9	-53.4	25.2	.0	25.7
224	62.9	-11.9	5.1	-53.4	25.2	.0	25.7
225	112.6	-12.3	5.3	-53.4	25.2	.0	25.8
226	162.3	-12.7	5.5	-53.4	25.2	.0	25.8
227	212.0	-13.1	5.7	-53.4	25.2	.0	25.8
228	261.6	-13.5	5.9	-53.4	25.2	.0	25.9
229	311.3	-13.8	6.1	-53.4	25.2	.0	25.9
230	361.0	-14.2	6.3	-53.4	25.2	.0	26.0
231	410.7	-14.6	6.5	-53.4	25.2	.0	26.0
232	460.4	-15.0	6.7	-53.4	25.2	.0	26.1
233	510.1	-15.4	6.9	-53.4	25.2	.0	26.1
234	584.6	-16.0	7.2	-53.4	25.2	.0	26.2
235	-76.1	-10.8	4.5	-53.3	25.1	.0	25.5
236	-26.4	-11.2	4.7	-53.3	25.1	.0	25.6
237	-1.6	-11.4	4.8	-53.3	25.1	.0	25.6
238	48.1	-11.7	5.0	-53.3	25.1	.0	25.6
239	97.8	-12.1	5.2	-53.3	25.1	.0	25.7
240	147.4	-12.5	5.4	-53.3	25.1	.0	25.7
241	197.1	-12.9	5.6	-53.3	25.1	.0	25.7
242	246.8	-13.3	5.8	-53.3	25.1	.0	25.8



Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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243	296.5	-13.7	6.0	-53.3	25.1	.0	25.8
244	346.2	-14.0	6.2	-53.3	25.1	.0	25.9
245	395.9	-14.4	6.4	-53.3	25.1	.0	25.9
246	445.5	-14.8	6.6	-53.3	25.1	.0	26.0
247	495.2	-15.2	6.8	-53.3	25.1	.0	26.0
248	544.9	-15.6	7.0	-53.3	25.1	.0	26.1
249	569.8	-15.8	7.1	-53.3	25.1	.0	26.1
250	619.4	-16.1	7.3	-53.3	25.1	.0	26.2

$$M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$$

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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LINEA FS FERRANDINA MATERA
VI08 Pile P21 e P22 - Analisi SLV

CONDIZIONE DI CARICO 5
VI08 - P22 - SLV_x

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	24776.0	11691.4	267577.3	4206.3	97230.1	4930.2
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
42875.0	11691.4	267577.3	4206.3	97230.1	4930.2

Punto di applic. carico verticale: Xv = 6.241 m Yv = 2.268 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.715	2.235	.572	.757	.114	.015

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	653.2	43.4	-18.7	19.2	-9.2	.0	20.9
2	638.3	43.9	-19.0	19.2	-9.2	.0	21.1
3	630.9	44.1	-19.1	19.2	-9.2	.0	21.2
4	616.0	44.6	-19.4	19.2	-9.2	.0	21.4
5	601.1	45.1	-19.6	19.2	-9.2	.0	21.7
6	586.3	45.6	-19.8	19.2	-9.2	.0	21.9
7	571.4	46.1	-20.1	19.2	-9.2	.0	22.1
8	556.5	46.5	-20.3	19.2	-9.2	.0	22.3
9	541.7	47.0	-20.6	19.2	-9.2	.0	22.6
10	526.8	47.5	-20.8	19.2	-9.2	.0	22.8
11	511.9	48.0	-21.1	19.2	-9.2	.0	23.0
12	497.1	48.4	-21.3	19.2	-9.2	.0	23.3
13	482.2	48.9	-21.6	19.2	-9.2	.0	23.5
14	467.3	49.4	-21.8	19.2	-9.2	.0	23.7

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	73 di 91

15	459.9	49.6	-22.0	19.2	-9.2	.0	23.8
16	445.0	50.1	-22.2	19.2	-9.2	.0	24.1
17	611.4	43.7	-18.9	19.0	-9.1	.0	20.9
18	589.1	44.4	-19.2	19.0	-9.1	.0	21.3
19	574.2	44.9	-19.5	19.0	-9.1	.0	21.5
20	559.4	45.3	-19.7	19.0	-9.1	.0	21.7
21	544.5	45.8	-20.0	19.0	-9.1	.0	22.0
22	529.6	46.3	-20.2	19.0	-9.1	.0	22.2
23	514.8	46.8	-20.5	19.0	-9.1	.0	22.4
24	499.9	47.2	-20.7	19.0	-9.1	.0	22.6
25	485.0	47.7	-21.0	19.0	-9.1	.0	22.9
26	470.2	48.2	-21.2	19.0	-9.1	.0	23.1
27	455.3	48.7	-21.5	19.0	-9.1	.0	23.3
28	440.4	49.1	-21.7	19.0	-9.1	.0	23.5
29	418.1	49.9	-22.1	19.0	-9.1	.0	23.9
30	584.5	43.4	-18.7	18.8	-9.0	.0	20.8
31	569.6	43.9	-19.0	18.8	-9.0	.0	21.0
32	562.2	44.1	-19.1	18.8	-9.0	.0	21.1
33	547.3	44.6	-19.4	18.8	-9.0	.0	21.3
34	532.5	45.1	-19.6	18.8	-9.0	.0	21.6
35	517.6	45.6	-19.8	18.8	-9.0	.0	21.8
36	502.7	46.1	-20.1	18.8	-9.0	.0	22.0
37	487.9	46.5	-20.3	18.8	-9.0	.0	22.3
38	473.0	47.0	-20.6	18.8	-9.0	.0	22.5
39	458.1	47.5	-20.8	18.8	-9.0	.0	22.7
40	443.3	48.0	-21.1	18.8	-9.0	.0	22.9
41	428.4	48.4	-21.3	18.8	-9.0	.0	23.2
42	413.5	48.9	-21.6	18.8	-9.0	.0	23.4
43	398.7	49.4	-21.8	18.8	-9.0	.0	23.6
44	391.2	49.6	-22.0	18.8	-9.0	.0	23.7
45	376.4	50.1	-22.2	18.8	-9.0	.0	24.0
46	550.2	43.4	-18.7	18.6	-8.9	.0	20.7
47	535.3	43.9	-19.0	18.6	-8.9	.0	21.0
48	527.9	44.1	-19.1	18.6	-8.9	.0	21.1
49	513.0	44.6	-19.4	18.6	-8.9	.0	21.3
50	498.2	45.1	-19.6	18.6	-8.9	.0	21.5
51	483.3	45.6	-19.8	18.6	-8.9	.0	21.8
52	468.4	46.1	-20.1	18.6	-8.9	.0	22.0
53	453.6	46.5	-20.3	18.6	-8.9	.0	22.2
54	438.7	47.0	-20.6	18.6	-8.9	.0	22.4
55	423.8	47.5	-20.8	18.6	-8.9	.0	22.7
56	408.9	48.0	-21.1	18.6	-8.9	.0	22.9
57	394.1	48.4	-21.3	18.6	-8.9	.0	23.1
58	379.2	48.9	-21.6	18.6	-8.9	.0	23.3
59	364.3	49.4	-21.8	18.6	-8.9	.0	23.6
60	356.9	49.6	-22.0	18.6	-8.9	.0	23.7
61	342.0	50.1	-22.2	18.6	-8.9	.0	23.9
62	508.4	43.7	-18.9	18.4	-8.8	.0	20.8
63	315.2	49.9	-22.1	18.4	-8.8	.0	23.8
64	481.5	43.4	-18.7	18.1	-8.7	.0	20.6
65	466.7	43.9	-19.0	18.1	-8.7	.0	20.9
66	459.2	44.1	-19.1	18.1	-8.7	.0	21.0
67	444.4	44.6	-19.4	18.1	-8.7	.0	21.2
68	429.5	45.1	-19.6	18.1	-8.7	.0	21.4
69	414.6	45.6	-19.8	18.1	-8.7	.0	21.7
70	399.8	46.1	-20.1	18.1	-8.7	.0	21.9
71	384.9	46.5	-20.3	18.1	-8.7	.0	22.1
72	370.0	47.0	-20.6	18.1	-8.7	.0	22.3
73	355.2	47.5	-20.8	18.1	-8.7	.0	22.6
74	340.3	48.0	-21.1	18.1	-8.7	.0	22.8
75	325.4	48.4	-21.3	18.1	-8.7	.0	23.0
76	310.6	48.9	-21.6	18.1	-8.7	.0	23.3
77	295.7	49.4	-21.8	18.1	-8.7	.0	23.5
78	288.3	49.6	-22.0	18.1	-8.7	.0	23.6
79	273.4	50.1	-22.2	18.1	-8.7	.0	23.8
80	439.8	43.7	-18.9	17.9	-8.6	.0	20.7
81	246.5	49.9	-22.1	17.9	-8.6	.0	23.7
82	412.9	43.4	-18.7	17.7	-8.4	.0	20.5
83	398.0	43.9	-19.0	17.7	-8.4	.0	20.8
84	390.6	44.1	-19.1	17.7	-8.4	.0	20.9
85	375.7	44.6	-19.4	17.7	-8.4	.0	21.1
86	360.8	45.1	-19.6	17.7	-8.4	.0	21.3
87	346.0	45.6	-19.8	17.7	-8.4	.0	21.6
88	331.1	46.1	-20.1	17.7	-8.4	.0	21.8
89	316.2	46.5	-20.3	17.7	-8.4	.0	22.0
90	301.4	47.0	-20.6	17.7	-8.4	.0	22.3

Relazione di calcolo fondazioni dirette su micropali
P21 – P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	74 di 91

91	286.5	47.5	-20.8	17.7	-8.4	.0	22.5
92	271.6	48.0	-21.1	17.7	-8.4	.0	22.7
93	256.8	48.4	-21.3	17.7	-8.4	.0	22.9
94	241.9	48.9	-21.6	17.7	-8.4	.0	23.2
95	227.0	49.4	-21.8	17.7	-8.4	.0	23.4
96	219.6	49.6	-22.0	17.7	-8.4	.0	23.5
97	204.7	50.1	-22.2	17.7	-8.4	.0	23.8
98	371.1	43.7	-18.9	17.5	-8.3	.0	20.6
99	177.8	49.9	-22.1	17.5	-8.3	.0	23.6
100	344.2	43.4	-18.7	17.3	-8.2	.0	20.5
101	329.4	43.9	-19.0	17.3	-8.2	.0	20.7
102	321.9	44.1	-19.1	17.3	-8.2	.0	20.8
103	307.1	44.6	-19.4	17.3	-8.2	.0	21.0
104	292.2	45.1	-19.6	17.3	-8.2	.0	21.3
105	277.3	45.6	-19.8	17.3	-8.2	.0	21.5
106	262.5	46.1	-20.1	17.3	-8.2	.0	21.7
107	247.6	46.5	-20.3	17.3	-8.2	.0	21.9
108	232.7	47.0	-20.6	17.3	-8.2	.0	22.2
109	217.9	47.5	-20.8	17.3	-8.2	.0	22.4
110	203.0	48.0	-21.1	17.3	-8.2	.0	22.6
111	188.1	48.4	-21.3	17.3	-8.2	.0	22.9
112	173.3	48.9	-21.6	17.3	-8.2	.0	23.1
113	158.4	49.4	-21.8	17.3	-8.2	.0	23.3
114	151.0	49.6	-22.0	17.3	-8.2	.0	23.4
115	136.1	50.1	-22.2	17.3	-8.2	.0	23.7
116	302.5	43.7	-18.9	17.0	-8.1	.0	20.5
117	109.2	49.9	-22.1	17.0	-8.1	.0	23.5
118	275.6	43.4	-18.7	16.8	-8.0	.0	20.4
119	260.7	43.9	-19.0	16.8	-8.0	.0	20.6
120	253.3	44.1	-19.1	16.8	-8.0	.0	20.7
121	238.4	44.6	-19.4	16.8	-8.0	.0	20.9
122	223.5	45.1	-19.6	16.8	-8.0	.0	21.2
123	208.7	45.6	-19.8	16.8	-8.0	.0	21.4
124	193.8	46.1	-20.1	16.8	-8.0	.0	21.6
125	178.9	46.5	-20.3	16.8	-8.0	.0	21.9
126	164.1	47.0	-20.6	16.8	-8.0	.0	22.1
127	149.2	47.5	-20.8	16.8	-8.0	.0	22.3
128	134.3	48.0	-21.1	16.8	-8.0	.0	22.5
129	119.5	48.4	-21.3	16.8	-8.0	.0	22.8
130	104.6	48.9	-21.6	16.8	-8.0	.0	23.0
131	89.7	49.4	-21.8	16.8	-8.0	.0	23.2
132	82.3	49.6	-22.0	16.8	-8.0	.0	23.4
133	67.4	50.1	-22.2	16.8	-8.0	.0	23.6
134	233.8	43.7	-18.9	16.6	-7.9	.0	20.4
135	40.5	49.9	-22.1	16.6	-7.9	.0	23.4
136	206.9	43.4	-18.7	16.4	-7.8	.0	20.3
137	192.0	43.9	-19.0	16.4	-7.8	.0	20.5
138	184.6	44.1	-19.1	16.4	-7.8	.0	20.6
139	169.7	44.6	-19.4	16.4	-7.8	.0	20.8
140	154.9	45.1	-19.6	16.4	-7.8	.0	21.1
141	140.0	45.6	-19.8	16.4	-7.8	.0	21.3
142	125.1	46.1	-20.1	16.4	-7.8	.0	21.5
143	110.3	46.5	-20.3	16.4	-7.8	.0	21.8
144	95.4	47.0	-20.6	16.4	-7.8	.0	22.0
145	80.5	47.5	-20.8	16.4	-7.8	.0	22.2
146	65.7	48.0	-21.1	16.4	-7.8	.0	22.5
147	50.8	48.4	-21.3	16.4	-7.8	.0	22.7
148	35.9	48.9	-21.6	16.4	-7.8	.0	22.9
149	21.1	49.4	-21.8	16.4	-7.8	.0	23.2
150	13.6	49.6	-22.0	16.4	-7.8	.0	23.3
151	-1.2	50.1	-22.2	16.4	-7.8	.0	23.5
152	165.2	43.7	-18.9	16.2	-7.6	.0	20.3
153	-28.1	49.9	-22.1	16.2	-7.6	.0	23.4
154	138.3	43.4	-18.7	15.9	-7.5	.0	20.2
155	123.4	43.9	-19.0	15.9	-7.5	.0	20.4
156	116.0	44.1	-19.1	15.9	-7.5	.0	20.5
157	101.1	44.6	-19.4	15.9	-7.5	.0	20.8
158	86.2	45.1	-19.6	15.9	-7.5	.0	21.0
159	71.4	45.6	-19.8	15.9	-7.5	.0	21.2
160	56.5	46.1	-20.1	15.9	-7.5	.0	21.5
161	41.6	46.5	-20.3	15.9	-7.5	.0	21.7
162	26.8	47.0	-20.6	15.9	-7.5	.0	21.9
163	11.9	47.5	-20.8	15.9	-7.5	.0	22.2
164	-3.0	48.0	-21.1	15.9	-7.5	.0	22.4
165	-17.8	48.4	-21.3	15.9	-7.5	.0	22.6
166	-32.7	48.9	-21.6	15.9	-7.5	.0	22.9

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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167	-47.6	49.4	-21.8	15.9	-7.5	.0	23.1
168	-55.0	49.6	-22.0	15.9	-7.5	.0	23.2
169	-69.9	50.1	-22.2	15.9	-7.5	.0	23.4
170	96.5	43.7	-18.9	15.7	-7.4	.0	20.3
171	-96.8	49.9	-22.1	15.7	-7.4	.0	23.3
172	69.6	43.4	-18.7	15.5	-7.3	.0	20.1
173	54.7	43.9	-19.0	15.5	-7.3	.0	20.3
174	47.3	44.1	-19.1	15.5	-7.3	.0	20.4
175	32.4	44.6	-19.4	15.5	-7.3	.0	20.7
176	17.6	45.1	-19.6	15.5	-7.3	.0	20.9
177	2.7	45.6	-19.8	15.5	-7.3	.0	21.1
178	-12.2	46.1	-20.1	15.5	-7.3	.0	21.4
179	-27.0	46.5	-20.3	15.5	-7.3	.0	21.6
180	-41.9	47.0	-20.6	15.5	-7.3	.0	21.8
181	-56.8	47.5	-20.8	15.5	-7.3	.0	22.1
182	-71.6	48.0	-21.1	15.5	-7.3	.0	22.3
183	-86.5	48.4	-21.3	15.5	-7.3	.0	22.5
184	-101.4	48.9	-21.6	15.5	-7.3	.0	22.8
185	-116.2	49.4	-21.8	15.5	-7.3	.0	23.0
186	-123.7	49.6	-22.0	15.5	-7.3	.0	23.1
187	-138.5	50.1	-22.2	15.5	-7.3	.0	23.4
188	27.8	43.7	-18.9	15.3	-7.2	.0	20.2
189	-165.4	49.9	-22.1	15.3	-7.2	.0	23.2
190	1.0	43.4	-18.7	15.1	-7.1	.0	20.0
191	-13.9	43.9	-19.0	15.1	-7.1	.0	20.3
192	-21.3	44.1	-19.1	15.1	-7.1	.0	20.4
193	-36.2	44.6	-19.4	15.1	-7.1	.0	20.6
194	-51.1	45.1	-19.6	15.1	-7.1	.0	20.8
195	-65.9	45.6	-19.8	15.1	-7.1	.0	21.1
196	-80.8	46.1	-20.1	15.1	-7.1	.0	21.3
197	-95.7	46.5	-20.3	15.1	-7.1	.0	21.5
198	-110.6	47.0	-20.6	15.1	-7.1	.0	21.8
199	-125.4	47.5	-20.8	15.1	-7.1	.0	22.0
200	-140.3	48.0	-21.1	15.1	-7.1	.0	22.2
201	-155.2	48.4	-21.3	15.1	-7.1	.0	22.5
202	-170.0	48.9	-21.6	15.1	-7.1	.0	22.7
203	-184.9	49.4	-21.8	15.1	-7.1	.0	22.9
204	-192.3	49.6	-22.0	15.1	-7.1	.0	23.1
205	-207.2	50.1	-22.2	15.1	-7.1	.0	23.3
206	-33.4	43.4	-18.7	14.8	-7.0	.0	20.0
207	-48.2	43.9	-19.0	14.8	-7.0	.0	20.2
208	-55.7	44.1	-19.1	14.8	-7.0	.0	20.3
209	-70.5	44.6	-19.4	14.8	-7.0	.0	20.6
210	-85.4	45.1	-19.6	14.8	-7.0	.0	20.8
211	-100.3	45.6	-19.8	14.8	-7.0	.0	21.0
212	-115.1	46.1	-20.1	14.8	-7.0	.0	21.3
213	-130.0	46.5	-20.3	14.8	-7.0	.0	21.5
214	-144.9	47.0	-20.6	14.8	-7.0	.0	21.7
215	-159.7	47.5	-20.8	14.8	-7.0	.0	22.0
216	-174.6	48.0	-21.1	14.8	-7.0	.0	22.2
217	-189.5	48.4	-21.3	14.8	-7.0	.0	22.4
218	-204.3	48.9	-21.6	14.8	-7.0	.0	22.7
219	-219.2	49.4	-21.8	14.8	-7.0	.0	22.9
220	-226.6	49.6	-22.0	14.8	-7.0	.0	23.0
221	-241.5	50.1	-22.2	14.8	-7.0	.0	23.3
222	-75.1	43.7	-18.9	14.6	-6.8	.0	20.1
223	-97.4	44.4	-19.2	14.6	-6.8	.0	20.4
224	-112.3	44.9	-19.5	14.6	-6.8	.0	20.6
225	-127.2	45.3	-19.7	14.6	-6.8	.0	20.9
226	-142.0	45.8	-20.0	14.6	-6.8	.0	21.1
227	-156.9	46.3	-20.2	14.6	-6.8	.0	21.3
228	-171.8	46.8	-20.5	14.6	-6.8	.0	21.6
229	-186.6	47.2	-20.7	14.6	-6.8	.0	21.8
230	-201.5	47.7	-21.0	14.6	-6.8	.0	22.1
231	-216.4	48.2	-21.2	14.6	-6.8	.0	22.3
232	-231.2	48.7	-21.5	14.6	-6.8	.0	22.5
233	-246.1	49.1	-21.7	14.6	-6.8	.0	22.8
234	-268.4	49.9	-22.1	14.6	-6.8	.0	23.1
235	-102.0	43.4	-18.7	14.4	-6.7	.0	19.9
236	-116.9	43.9	-19.0	14.4	-6.7	.0	20.1
237	-124.3	44.1	-19.1	14.4	-6.7	.0	20.3
238	-139.2	44.6	-19.4	14.4	-6.7	.0	20.5
239	-154.1	45.1	-19.6	14.4	-6.7	.0	20.7
240	-168.9	45.6	-19.8	14.4	-6.7	.0	21.0
241	-183.8	46.1	-20.1	14.4	-6.7	.0	21.2
242	-198.7	46.5	-20.3	14.4	-6.7	.0	21.4



Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

Relazione di calcolo fondazioni dirette su micropali
P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	76 di 91

243	-213.5	47.0	-20.6	14.4	-6.7	.0	21.7
244	-228.4	47.5	-20.8	14.4	-6.7	.0	21.9
245	-243.3	48.0	-21.1	14.4	-6.7	.0	22.1
246	-258.1	48.4	-21.3	14.4	-6.7	.0	22.4
247	-273.0	48.9	-21.6	14.4	-6.7	.0	22.6
248	-287.9	49.4	-21.8	14.4	-6.7	.0	22.8
249	-295.3	49.6	-22.0	14.4	-6.7	.0	23.0
250	-310.2	50.1	-22.2	14.4	-6.7	.0	23.2

 $M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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LINEA FS FERRANDINA MATERA
VI08 Pile P21 e P22 - Analisi SLV

CONDIZIONE DI CARICO 6
VI08 - P22 - SLV_x

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	21576.8	-11691.4	-268069.3	-4206.3	-97230.1	-4930.2
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
39675.8	-11691.4	-268069.3	-4206.3	-97230.1	-4930.2

Punto di applic. carico verticale: Xv = -6.756 m Yv = -2.451 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.587	-2.236	-.573	-.757	-.114	-.015

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	-323.6	-43.4	18.7	-19.2	9.2	.0	20.9
2	-308.8	-43.9	19.0	-19.2	9.2	.0	21.1
3	-301.3	-44.1	19.1	-19.2	9.2	.0	21.2
4	-286.5	-44.6	19.3	-19.2	9.2	.0	21.4
5	-271.6	-45.1	19.6	-19.2	9.2	.0	21.7
6	-256.7	-45.6	19.8	-19.2	9.2	.0	21.9
7	-241.9	-46.1	20.1	-19.2	9.2	.0	22.1
8	-227.0	-46.5	20.3	-19.2	9.2	.0	22.3
9	-212.1	-47.0	20.6	-19.2	9.2	.0	22.6
10	-197.3	-47.5	20.8	-19.2	9.2	.0	22.8
11	-182.4	-48.0	21.1	-19.2	9.2	.0	23.0
12	-167.5	-48.4	21.3	-19.2	9.2	.0	23.2
13	-152.7	-48.9	21.6	-19.2	9.2	.0	23.5
14	-137.8	-49.4	21.8	-19.2	9.2	.0	23.7

Relazione di calcolo fondazioni dirette su micropali
 P21 – P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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15	-130.4	-49.6	22.0	-19.2	9.2	.0	23.8
16	-115.5	-50.1	22.2	-19.2	9.2	.0	24.0
17	-281.8	-43.7	18.8	-19.0	9.1	.0	20.9
18	-259.5	-44.4	19.2	-19.0	9.1	.0	21.3
19	-244.7	-44.9	19.5	-19.0	9.1	.0	21.5
20	-229.8	-45.3	19.7	-19.0	9.1	.0	21.7
21	-214.9	-45.8	20.0	-19.0	9.1	.0	22.0
22	-200.1	-46.3	20.2	-19.0	9.1	.0	22.2
23	-185.2	-46.8	20.5	-19.0	9.1	.0	22.4
24	-170.3	-47.2	20.7	-19.0	9.1	.0	22.6
25	-155.5	-47.7	21.0	-19.0	9.1	.0	22.9
26	-140.6	-48.2	21.2	-19.0	9.1	.0	23.1
27	-125.7	-48.7	21.5	-19.0	9.1	.0	23.3
28	-110.9	-49.1	21.7	-19.0	9.1	.0	23.5
29	-88.5	-49.9	22.1	-19.0	9.1	.0	23.9
30	-254.9	-43.4	18.7	-18.8	9.0	.0	20.8
31	-240.0	-43.9	19.0	-18.8	9.0	.0	21.0
32	-232.6	-44.1	19.1	-18.8	9.0	.0	21.1
33	-217.7	-44.6	19.3	-18.8	9.0	.0	21.3
34	-202.8	-45.1	19.6	-18.8	9.0	.0	21.6
35	-188.0	-45.6	19.8	-18.8	9.0	.0	21.8
36	-173.1	-46.1	20.1	-18.8	9.0	.0	22.0
37	-158.2	-46.5	20.3	-18.8	9.0	.0	22.2
38	-143.4	-47.0	20.6	-18.8	9.0	.0	22.5
39	-128.5	-47.5	20.8	-18.8	9.0	.0	22.7
40	-113.6	-48.0	21.1	-18.8	9.0	.0	22.9
41	-98.8	-48.4	21.3	-18.8	9.0	.0	23.2
42	-83.9	-48.9	21.6	-18.8	9.0	.0	23.4
43	-69.0	-49.4	21.8	-18.8	9.0	.0	23.6
44	-61.6	-49.6	22.0	-18.8	9.0	.0	23.7
45	-46.7	-50.1	22.2	-18.8	9.0	.0	24.0
46	-220.5	-43.4	18.7	-18.6	8.9	.0	20.7
47	-205.6	-43.9	19.0	-18.6	8.9	.0	21.0
48	-198.2	-44.1	19.1	-18.6	8.9	.0	21.1
49	-183.3	-44.6	19.3	-18.6	8.9	.0	21.3
50	-168.4	-45.1	19.6	-18.6	8.9	.0	21.5
51	-153.6	-45.6	19.8	-18.6	8.9	.0	21.7
52	-138.7	-46.1	20.1	-18.6	8.9	.0	22.0
53	-123.8	-46.5	20.3	-18.6	8.9	.0	22.2
54	-109.0	-47.0	20.6	-18.6	8.9	.0	22.4
55	-94.1	-47.5	20.8	-18.6	8.9	.0	22.7
56	-79.2	-48.0	21.1	-18.6	8.9	.0	22.9
57	-64.4	-48.4	21.3	-18.6	8.9	.0	23.1
58	-49.5	-48.9	21.6	-18.6	8.9	.0	23.3
59	-34.6	-49.4	21.8	-18.6	8.9	.0	23.6
60	-27.2	-49.6	22.0	-18.6	8.9	.0	23.7
61	-12.3	-50.1	22.2	-18.6	8.9	.0	23.9
62	-178.7	-43.7	18.8	-18.4	8.8	.0	20.8
63	14.6	-49.9	22.1	-18.4	8.8	.0	23.8
64	-151.7	-43.4	18.7	-18.1	8.7	.0	20.6
65	-136.8	-43.9	19.0	-18.1	8.7	.0	20.9
66	-129.4	-44.1	19.1	-18.1	8.7	.0	21.0
67	-114.5	-44.6	19.3	-18.1	8.7	.0	21.2
68	-99.7	-45.1	19.6	-18.1	8.7	.0	21.4
69	-84.8	-45.6	19.8	-18.1	8.7	.0	21.7
70	-69.9	-46.1	20.1	-18.1	8.7	.0	21.9
71	-55.1	-46.5	20.3	-18.1	8.7	.0	22.1
72	-40.2	-47.0	20.6	-18.1	8.7	.0	22.3
73	-25.3	-47.5	20.8	-18.1	8.7	.0	22.6
74	-10.5	-48.0	21.1	-18.1	8.7	.0	22.8
75	4.4	-48.4	21.3	-18.1	8.7	.0	23.0
76	19.3	-48.9	21.6	-18.1	8.7	.0	23.3
77	34.1	-49.4	21.8	-18.1	8.7	.0	23.5
78	41.6	-49.6	22.0	-18.1	8.7	.0	23.6
79	56.4	-50.1	22.2	-18.1	8.7	.0	23.8
80	-109.9	-43.7	18.8	-17.9	8.6	.0	20.7
81	83.4	-49.9	22.1	-17.9	8.6	.0	23.7
82	-82.9	-43.4	18.7	-17.7	8.4	.0	20.5
83	-68.1	-43.9	19.0	-17.7	8.4	.0	20.8
84	-60.6	-44.1	19.1	-17.7	8.4	.0	20.9
85	-45.8	-44.6	19.3	-17.7	8.4	.0	21.1
86	-30.9	-45.1	19.6	-17.7	8.4	.0	21.3
87	-16.0	-45.6	19.8	-17.7	8.4	.0	21.6
88	-1.2	-46.1	20.1	-17.7	8.4	.0	21.8
89	13.7	-46.5	20.3	-17.7	8.4	.0	22.0
90	28.6	-47.0	20.6	-17.7	8.4	.0	22.2

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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91	43.4	-47.5	20.8	-17.7	8.4	.0	22.5
92	58.3	-48.0	21.1	-17.7	8.4	.0	22.7
93	73.2	-48.4	21.3	-17.7	8.4	.0	22.9
94	88.0	-48.9	21.6	-17.7	8.4	.0	23.2
95	102.9	-49.4	21.8	-17.7	8.4	.0	23.4
96	110.3	-49.6	22.0	-17.7	8.4	.0	23.5
97	125.2	-50.1	22.2	-17.7	8.4	.0	23.7
98	-41.1	-43.7	18.8	-17.5	8.3	.0	20.6
99	152.2	-49.9	22.1	-17.5	8.3	.0	23.6
100	-14.1	-43.4	18.7	-17.3	8.2	.0	20.4
101	.7	-43.9	19.0	-17.3	8.2	.0	20.7
102	8.2	-44.1	19.1	-17.3	8.2	.0	20.8
103	23.0	-44.6	19.3	-17.3	8.2	.0	21.0
104	37.9	-45.1	19.6	-17.3	8.2	.0	21.2
105	52.8	-45.6	19.8	-17.3	8.2	.0	21.5
106	67.6	-46.1	20.1	-17.3	8.2	.0	21.7
107	82.5	-46.5	20.3	-17.3	8.2	.0	21.9
108	97.4	-47.0	20.6	-17.3	8.2	.0	22.2
109	112.2	-47.5	20.8	-17.3	8.2	.0	22.4
110	127.1	-48.0	21.1	-17.3	8.2	.0	22.6
111	142.0	-48.4	21.3	-17.3	8.2	.0	22.9
112	156.8	-48.9	21.6	-17.3	8.2	.0	23.1
113	171.7	-49.4	21.8	-17.3	8.2	.0	23.3
114	179.1	-49.6	22.0	-17.3	8.2	.0	23.4
115	194.0	-50.1	22.2	-17.3	8.2	.0	23.7
116	27.7	-43.7	18.8	-17.0	8.1	.0	20.5
117	220.9	-49.9	22.1	-17.0	8.1	.0	23.5
118	54.6	-43.4	18.7	-16.8	8.0	.0	20.4
119	69.5	-43.9	19.0	-16.8	8.0	.0	20.6
120	76.9	-44.1	19.1	-16.8	8.0	.0	20.7
121	91.8	-44.6	19.3	-16.8	8.0	.0	20.9
122	106.7	-45.1	19.6	-16.8	8.0	.0	21.2
123	121.5	-45.6	19.8	-16.8	8.0	.0	21.4
124	136.4	-46.1	20.1	-16.8	8.0	.0	21.6
125	151.3	-46.5	20.3	-16.8	8.0	.0	21.8
126	166.1	-47.0	20.6	-16.8	8.0	.0	22.1
127	181.0	-47.5	20.8	-16.8	8.0	.0	22.3
128	195.9	-48.0	21.1	-16.8	8.0	.0	22.5
129	210.7	-48.4	21.3	-16.8	8.0	.0	22.8
130	225.6	-48.9	21.6	-16.8	8.0	.0	23.0
131	240.5	-49.4	21.8	-16.8	8.0	.0	23.2
132	247.9	-49.6	22.0	-16.8	8.0	.0	23.4
133	262.8	-50.1	22.2	-16.8	8.0	.0	23.6
134	96.5	-43.7	18.8	-16.6	7.9	.0	20.4
135	289.7	-49.9	22.1	-16.6	7.9	.0	23.4
136	123.4	-43.4	18.7	-16.4	7.8	.0	20.3
137	138.3	-43.9	19.0	-16.4	7.8	.0	20.5
138	145.7	-44.1	19.1	-16.4	7.8	.0	20.6
139	160.6	-44.6	19.3	-16.4	7.8	.0	20.8
140	175.4	-45.1	19.6	-16.4	7.8	.0	21.1
141	190.3	-45.6	19.8	-16.4	7.8	.0	21.3
142	205.2	-46.1	20.1	-16.4	7.8	.0	21.5
143	220.0	-46.5	20.3	-16.4	7.8	.0	21.8
144	234.9	-47.0	20.6	-16.4	7.8	.0	22.0
145	249.8	-47.5	20.8	-16.4	7.8	.0	22.2
146	264.6	-48.0	21.1	-16.4	7.8	.0	22.5
147	279.5	-48.4	21.3	-16.4	7.8	.0	22.7
148	294.4	-48.9	21.6	-16.4	7.8	.0	22.9
149	309.2	-49.4	21.8	-16.4	7.8	.0	23.2
150	316.7	-49.6	22.0	-16.4	7.8	.0	23.3
151	331.5	-50.1	22.2	-16.4	7.8	.0	23.5
152	165.2	-43.7	18.8	-16.2	7.6	.0	20.3
153	358.5	-49.9	22.1	-16.2	7.6	.0	23.4
154	192.2	-43.4	18.7	-15.9	7.5	.0	20.2
155	207.1	-43.9	19.0	-15.9	7.5	.0	20.4
156	214.5	-44.1	19.1	-15.9	7.5	.0	20.5
157	229.4	-44.6	19.3	-15.9	7.5	.0	20.8
158	244.2	-45.1	19.6	-15.9	7.5	.0	21.0
159	259.1	-45.6	19.8	-15.9	7.5	.0	21.2
160	274.0	-46.1	20.1	-15.9	7.5	.0	21.5
161	288.8	-46.5	20.3	-15.9	7.5	.0	21.7
162	303.7	-47.0	20.6	-15.9	7.5	.0	21.9
163	318.6	-47.5	20.8	-15.9	7.5	.0	22.2
164	333.4	-48.0	21.1	-15.9	7.5	.0	22.4
165	348.3	-48.4	21.3	-15.9	7.5	.0	22.6
166	363.2	-48.9	21.6	-15.9	7.5	.0	22.9

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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167	378.0	-49.4	21.8	-15.9	7.5	.0	23.1
168	385.5	-49.6	22.0	-15.9	7.5	.0	23.2
169	400.3	-50.1	22.2	-15.9	7.5	.0	23.4
170	234.0	-43.7	18.8	-15.7	7.4	.0	20.3
171	427.3	-49.9	22.1	-15.7	7.4	.0	23.3
172	261.0	-43.4	18.7	-15.5	7.3	.0	20.1
173	275.8	-43.9	19.0	-15.5	7.3	.0	20.3
174	283.3	-44.1	19.1	-15.5	7.3	.0	20.4
175	298.1	-44.6	19.3	-15.5	7.3	.0	20.7
176	313.0	-45.1	19.6	-15.5	7.3	.0	20.9
177	327.9	-45.6	19.8	-15.5	7.3	.0	21.1
178	342.7	-46.1	20.1	-15.5	7.3	.0	21.4
179	357.6	-46.5	20.3	-15.5	7.3	.0	21.6
180	372.5	-47.0	20.6	-15.5	7.3	.0	21.8
181	387.3	-47.5	20.8	-15.5	7.3	.0	22.1
182	402.2	-48.0	21.1	-15.5	7.3	.0	22.3
183	417.1	-48.4	21.3	-15.5	7.3	.0	22.5
184	431.9	-48.9	21.6	-15.5	7.3	.0	22.8
185	446.8	-49.4	21.8	-15.5	7.3	.0	23.0
186	454.2	-49.6	22.0	-15.5	7.3	.0	23.1
187	469.1	-50.1	22.2	-15.5	7.3	.0	23.4
188	302.8	-43.7	18.8	-15.3	7.2	.0	20.2
189	496.1	-49.9	22.1	-15.3	7.2	.0	23.2
190	329.7	-43.4	18.7	-15.1	7.1	.0	20.0
191	344.6	-43.9	19.0	-15.1	7.1	.0	20.2
192	352.0	-44.1	19.1	-15.1	7.1	.0	20.4
193	366.9	-44.6	19.3	-15.1	7.1	.0	20.6
194	381.8	-45.1	19.6	-15.1	7.1	.0	20.8
195	396.6	-45.6	19.8	-15.1	7.1	.0	21.1
196	411.5	-46.1	20.1	-15.1	7.1	.0	21.3
197	426.4	-46.5	20.3	-15.1	7.1	.0	21.5
198	441.2	-47.0	20.6	-15.1	7.1	.0	21.8
199	456.1	-47.5	20.8	-15.1	7.1	.0	22.0
200	471.0	-48.0	21.1	-15.1	7.1	.0	22.2
201	485.8	-48.4	21.3	-15.1	7.1	.0	22.5
202	500.7	-48.9	21.6	-15.1	7.1	.0	22.7
203	515.6	-49.4	21.8	-15.1	7.1	.0	22.9
204	523.0	-49.6	22.0	-15.1	7.1	.0	23.1
205	537.9	-50.1	22.2	-15.1	7.1	.0	23.3
206	364.1	-43.4	18.7	-14.8	7.0	.0	20.0
207	379.0	-43.9	19.0	-14.8	7.0	.0	20.2
208	386.4	-44.1	19.1	-14.8	7.0	.0	20.3
209	401.3	-44.6	19.3	-14.8	7.0	.0	20.6
210	416.2	-45.1	19.6	-14.8	7.0	.0	20.8
211	431.0	-45.6	19.8	-14.8	7.0	.0	21.0
212	445.9	-46.1	20.1	-14.8	7.0	.0	21.3
213	460.8	-46.5	20.3	-14.8	7.0	.0	21.5
214	475.6	-47.0	20.6	-14.8	7.0	.0	21.7
215	490.5	-47.5	20.8	-14.8	7.0	.0	22.0
216	505.4	-48.0	21.1	-14.8	7.0	.0	22.2
217	520.2	-48.4	21.3	-14.8	7.0	.0	22.4
218	535.1	-48.9	21.6	-14.8	7.0	.0	22.7
219	550.0	-49.4	21.8	-14.8	7.0	.0	22.9
220	557.4	-49.6	22.0	-14.8	7.0	.0	23.0
221	572.3	-50.1	22.2	-14.8	7.0	.0	23.3
222	406.0	-43.7	18.8	-14.6	6.8	.0	20.1
223	428.3	-44.4	19.2	-14.6	6.8	.0	20.4
224	443.1	-44.9	19.5	-14.6	6.8	.0	20.6
225	458.0	-45.3	19.7	-14.6	6.8	.0	20.9
226	472.9	-45.8	20.0	-14.6	6.8	.0	21.1
227	487.7	-46.3	20.2	-14.6	6.8	.0	21.3
228	502.6	-46.8	20.5	-14.6	6.8	.0	21.6
229	517.5	-47.2	20.7	-14.6	6.8	.0	21.8
230	532.3	-47.7	21.0	-14.6	6.8	.0	22.0
231	547.2	-48.2	21.2	-14.6	6.8	.0	22.3
232	562.1	-48.7	21.5	-14.6	6.8	.0	22.5
233	576.9	-49.1	21.7	-14.6	6.8	.0	22.8
234	599.2	-49.9	22.1	-14.6	6.8	.0	23.1
235	432.9	-43.4	18.7	-14.4	6.7	.0	19.9
236	447.8	-43.9	19.0	-14.4	6.7	.0	20.1
237	455.2	-44.1	19.1	-14.4	6.7	.0	20.2
238	470.1	-44.6	19.3	-14.4	6.7	.0	20.5
239	484.9	-45.1	19.6	-14.4	6.7	.0	20.7
240	499.8	-45.6	19.8	-14.4	6.7	.0	20.9
241	514.7	-46.1	20.1	-14.4	6.7	.0	21.2
242	529.5	-46.5	20.3	-14.4	6.7	.0	21.4

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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243	544.4	-47.0	20.6	-14.4	6.7	.0	21.7
244	559.3	-47.5	20.8	-14.4	6.7	.0	21.9
245	574.1	-48.0	21.1	-14.4	6.7	.0	22.1
246	589.0	-48.4	21.3	-14.4	6.7	.0	22.4
247	603.9	-48.9	21.6	-14.4	6.7	.0	22.6
248	618.7	-49.4	21.8	-14.4	6.7	.0	22.8
249	626.2	-49.6	22.0	-14.4	6.7	.0	23.0
250	641.0	-50.1	22.2	-14.4	6.7	.0	23.2

$$M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$$

Relazione di calcolo fondazioni dirette su micropali
 P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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 LINEA FS FERRANDINA MATERA
 VI08 Pile P21 e P22 - Analisi SLV

 CONDIZIONE DI CARICO 7
 VI08 - P22 - SLV_y

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	24737.8	3509.7	80115.6	14020.8	324099.0	16433.8
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
42836.8	3509.7	80115.6	14020.8	324099.0	16433.8

Punto di applic. carico verticale: Xv = 1.870 m Yv = 7.566 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.713	.671	.171	2.522	.381	.051

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	631.3	2.9	-.4	64.2	-30.8	.0	30.8
2	581.7	4.5	-1.2	64.2	-30.8	.0	30.8
3	557.0	5.3	-1.6	64.2	-30.8	.0	30.8
4	507.4	6.9	-2.4	64.2	-30.8	.0	30.9
5	457.9	8.5	-3.3	64.2	-30.8	.0	31.0
6	408.3	10.1	-4.1	64.2	-30.8	.0	31.1
7	358.7	11.7	-4.9	64.2	-30.8	.0	31.2
8	309.2	13.2	-5.7	64.2	-30.8	.0	31.3
9	259.6	14.8	-6.6	64.2	-30.8	.0	31.5
10	210.1	16.4	-7.4	64.2	-30.8	.0	31.7
11	160.5	18.0	-8.2	64.2	-30.8	.0	31.9
12	111.0	19.6	-9.0	64.2	-30.8	.0	32.1
13	61.4	21.2	-9.9	64.2	-30.8	.0	32.3
14	11.9	22.8	-10.7	64.2	-30.8	.0	32.6

Relazione di calcolo fondazioni dirette su micropali
 P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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15	-12.9	23.6	-11.1	64.2	-30.8	.0	32.7
16	-62.5	25.2	-11.9	64.2	-30.8	.0	33.0
17	596.2	3.7	-.8	63.4	-30.4	.0	30.4
18	521.9	6.1	-2.0	63.4	-30.4	.0	30.5
19	472.4	7.7	-2.8	63.4	-30.4	.0	30.6
20	422.8	9.3	-3.7	63.4	-30.4	.0	30.6
21	373.2	10.9	-4.5	63.4	-30.4	.0	30.8
22	323.7	12.4	-5.3	63.4	-30.4	.0	30.9
23	274.1	14.0	-6.1	63.4	-30.4	.0	31.0
24	224.6	15.6	-7.0	63.4	-30.4	.0	31.2
25	175.0	17.2	-7.8	63.4	-30.4	.0	31.4
26	125.5	18.8	-8.6	63.4	-30.4	.0	31.6
27	75.9	20.4	-9.5	63.4	-30.4	.0	31.9
28	26.3	22.0	-10.3	63.4	-30.4	.0	32.1
29	-48.0	24.4	-11.5	63.4	-30.4	.0	32.5
30	610.7	2.9	-.4	62.7	-30.0	.0	30.0
31	561.2	4.5	-1.2	62.7	-30.0	.0	30.1
32	536.4	5.3	-1.6	62.7	-30.0	.0	30.1
33	486.9	6.9	-2.4	62.7	-30.0	.0	30.1
34	437.3	8.5	-3.3	62.7	-30.0	.0	30.2
35	387.7	10.1	-4.1	62.7	-30.0	.0	30.3
36	338.2	11.7	-4.9	62.7	-30.0	.0	30.4
37	288.6	13.2	-5.7	62.7	-30.0	.0	30.6
38	239.1	14.8	-6.6	62.7	-30.0	.0	30.7
39	189.5	16.4	-7.4	62.7	-30.0	.0	30.9
40	140.0	18.0	-8.2	62.7	-30.0	.0	31.1
41	90.4	19.6	-9.0	62.7	-30.0	.0	31.4
42	40.8	21.2	-9.9	62.7	-30.0	.0	31.6
43	-8.7	22.8	-10.7	62.7	-30.0	.0	31.9
44	-33.5	23.6	-11.1	62.7	-30.0	.0	32.0
45	-83.0	25.2	-11.9	62.7	-30.0	.0	32.3
46	600.5	2.9	-.4	62.0	-29.7	.0	29.7
47	550.9	4.5	-1.2	62.0	-29.7	.0	29.7
48	526.1	5.3	-1.6	62.0	-29.7	.0	29.7
49	476.6	6.9	-2.4	62.0	-29.7	.0	29.8
50	427.0	8.5	-3.3	62.0	-29.7	.0	29.8
51	377.5	10.1	-4.1	62.0	-29.7	.0	29.9
52	327.9	11.7	-4.9	62.0	-29.7	.0	30.1
53	278.4	13.2	-5.7	62.0	-29.7	.0	30.2
54	228.8	14.8	-6.6	62.0	-29.7	.0	30.4
55	179.2	16.4	-7.4	62.0	-29.7	.0	30.6
56	129.7	18.0	-8.2	62.0	-29.7	.0	30.8
57	80.1	19.6	-9.0	62.0	-29.7	.0	31.0
58	30.6	21.2	-9.9	62.0	-29.7	.0	31.3
59	-19.0	22.8	-10.7	62.0	-29.7	.0	31.5
60	-43.8	23.6	-11.1	62.0	-29.7	.0	31.7
61	-93.3	25.2	-11.9	62.0	-29.7	.0	32.0
62	565.4	3.7	-.8	61.2	-29.3	.0	29.3
63	-78.8	24.4	-11.5	61.2	-29.3	.0	31.5
64	579.9	2.9	-.4	60.5	-28.9	.0	28.9
65	530.4	4.5	-1.2	60.5	-28.9	.0	28.9
66	505.6	5.3	-1.6	60.5	-28.9	.0	28.9
67	456.0	6.9	-2.4	60.5	-28.9	.0	29.0
68	406.5	8.5	-3.3	60.5	-28.9	.0	29.1
69	356.9	10.1	-4.1	60.5	-28.9	.0	29.2
70	307.4	11.7	-4.9	60.5	-28.9	.0	29.3
71	257.8	13.2	-5.7	60.5	-28.9	.0	29.5
72	208.2	14.8	-6.6	60.5	-28.9	.0	29.6
73	158.7	16.4	-7.4	60.5	-28.9	.0	29.8
74	109.1	18.0	-8.2	60.5	-28.9	.0	30.0
75	59.6	19.6	-9.0	60.5	-28.9	.0	30.3
76	10.0	21.2	-9.9	60.5	-28.9	.0	30.5
77	-39.5	22.8	-10.7	60.5	-28.9	.0	30.8
78	-64.3	23.6	-11.1	60.5	-28.9	.0	31.0
79	-113.9	25.2	-11.9	60.5	-28.9	.0	31.3
80	544.9	3.7	-.8	59.8	-28.5	.0	28.5
81	-99.4	24.4	-11.5	59.8	-28.5	.0	30.8
82	559.4	2.9	-.4	59.0	-28.1	.0	28.1
83	509.8	4.5	-1.2	59.0	-28.1	.0	28.2
84	485.0	5.3	-1.6	59.0	-28.1	.0	28.2
85	435.5	6.9	-2.4	59.0	-28.1	.0	28.2
86	385.9	8.5	-3.3	59.0	-28.1	.0	28.3
87	336.4	10.1	-4.1	59.0	-28.1	.0	28.4
88	286.8	11.7	-4.9	59.0	-28.1	.0	28.6
89	237.2	13.2	-5.7	59.0	-28.1	.0	28.7
90	187.7	14.8	-6.6	59.0	-28.1	.0	28.9

Relazione di calcolo fondazioni dirette su micropali
 P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	84 di 91

91	138.1	16.4	-7.4	59.0	-28.1	.0	29.1
92	88.6	18.0	-8.2	59.0	-28.1	.0	29.3
93	39.0	19.6	-9.0	59.0	-28.1	.0	29.6
94	-10.5	21.2	-9.9	59.0	-28.1	.0	29.8
95	-60.1	22.8	-10.7	59.0	-28.1	.0	30.1
96	-84.9	23.6	-11.1	59.0	-28.1	.0	30.2
97	-134.4	25.2	-11.9	59.0	-28.1	.0	30.6
98	524.3	3.7	-.8	58.3	-27.8	.0	27.8
99	-119.9	24.4	-11.5	58.3	-27.8	.0	30.0
100	538.8	2.9	-.4	57.6	-27.4	.0	27.4
101	489.2	4.5	-1.2	57.6	-27.4	.0	27.4
102	464.5	5.3	-1.6	57.6	-27.4	.0	27.4
103	414.9	6.9	-2.4	57.6	-27.4	.0	27.5
104	365.4	8.5	-3.3	57.6	-27.4	.0	27.6
105	315.8	10.1	-4.1	57.6	-27.4	.0	27.7
106	266.2	11.7	-4.9	57.6	-27.4	.0	27.8
107	216.7	13.2	-5.7	57.6	-27.4	.0	28.0
108	167.1	14.8	-6.6	57.6	-27.4	.0	28.1
109	117.6	16.4	-7.4	57.6	-27.4	.0	28.3
110	68.0	18.0	-8.2	57.6	-27.4	.0	28.6
111	18.5	19.6	-9.0	57.6	-27.4	.0	28.8
112	-31.1	21.2	-9.9	57.6	-27.4	.0	29.1
113	-80.7	22.8	-10.7	57.6	-27.4	.0	29.4
114	-105.4	23.6	-11.1	57.6	-27.4	.0	29.5
115	-155.0	25.2	-11.9	57.6	-27.4	.0	29.9
116	503.7	3.7	-.8	56.8	-27.0	.0	27.0
117	-140.5	24.4	-11.5	56.8	-27.0	.0	29.3
118	518.2	2.9	-.4	56.1	-26.6	.0	26.6
119	468.7	4.5	-1.2	56.1	-26.6	.0	26.6
120	443.9	5.3	-1.6	56.1	-26.6	.0	26.7
121	394.4	6.9	-2.4	56.1	-26.6	.0	26.7
122	344.8	8.5	-3.3	56.1	-26.6	.0	26.8
123	295.2	10.1	-4.1	56.1	-26.6	.0	26.9
124	245.7	11.7	-4.9	56.1	-26.6	.0	27.1
125	196.1	13.2	-5.7	56.1	-26.6	.0	27.2
126	146.6	14.8	-6.6	56.1	-26.6	.0	27.4
127	97.0	16.4	-7.4	56.1	-26.6	.0	27.6
128	47.5	18.0	-8.2	56.1	-26.6	.0	27.8
129	-2.1	19.6	-9.0	56.1	-26.6	.0	28.1
130	-51.7	21.2	-9.9	56.1	-26.6	.0	28.4
131	-101.2	22.8	-10.7	56.1	-26.6	.0	28.7
132	-126.0	23.6	-11.1	56.1	-26.6	.0	28.8
133	-175.5	25.2	-11.9	56.1	-26.6	.0	29.2
134	483.2	3.7	-.8	55.3	-26.2	.0	26.2
135	-161.0	24.4	-11.5	55.3	-26.2	.0	28.6
136	497.7	2.9	-.4	54.6	-25.8	.0	25.8
137	448.1	4.5	-1.2	54.6	-25.8	.0	25.9
138	423.4	5.3	-1.6	54.6	-25.8	.0	25.9
139	373.8	6.9	-2.4	54.6	-25.8	.0	26.0
140	324.2	8.5	-3.3	54.6	-25.8	.0	26.0
141	274.7	10.1	-4.1	54.6	-25.8	.0	26.2
142	225.1	11.7	-4.9	54.6	-25.8	.0	26.3
143	175.6	13.2	-5.7	54.6	-25.8	.0	26.5
144	126.0	14.8	-6.6	54.6	-25.8	.0	26.7
145	76.5	16.4	-7.4	54.6	-25.8	.0	26.9
146	26.9	18.0	-8.2	54.6	-25.8	.0	27.1
147	-22.7	19.6	-9.0	54.6	-25.8	.0	27.4
148	-72.2	21.2	-9.9	54.6	-25.8	.0	27.7
149	-121.8	22.8	-10.7	54.6	-25.8	.0	28.0
150	-146.5	23.6	-11.1	54.6	-25.8	.0	28.1
151	-196.1	25.2	-11.9	54.6	-25.8	.0	28.5
152	462.6	3.7	-.8	53.9	-25.5	.0	25.5
153	-181.6	24.4	-11.5	53.9	-25.5	.0	27.9
154	477.1	2.9	-.4	53.1	-25.1	.0	25.1
155	427.6	4.5	-1.2	53.1	-25.1	.0	25.1
156	402.8	5.3	-1.6	53.1	-25.1	.0	25.1
157	353.2	6.9	-2.4	53.1	-25.1	.0	25.2
158	303.7	8.5	-3.3	53.1	-25.1	.0	25.3
159	254.1	10.1	-4.1	53.1	-25.1	.0	25.4
160	204.6	11.7	-4.9	53.1	-25.1	.0	25.6
161	155.0	13.2	-5.7	53.1	-25.1	.0	25.7
162	105.5	14.8	-6.6	53.1	-25.1	.0	25.9
163	55.9	16.4	-7.4	53.1	-25.1	.0	26.1
164	6.3	18.0	-8.2	53.1	-25.1	.0	26.4
165	-43.2	19.6	-9.0	53.1	-25.1	.0	26.7
166	-92.8	21.2	-9.9	53.1	-25.1	.0	27.0

Relazione di calcolo fondazioni dirette su micropali
 P21 - P22

PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	85 di 91

167	-142.3	22.8	-10.7	53.1	-25.1	.0	27.3
168	-167.1	23.6	-11.1	53.1	-25.1	.0	27.4
169	-216.7	25.2	-11.9	53.1	-25.1	.0	27.8
170	442.1	3.7	-.8	52.4	-24.7	.0	24.7
171	-202.2	24.4	-11.5	52.4	-24.7	.0	27.3
172	456.6	2.9	-.4	51.7	-24.3	.0	24.3
173	407.0	4.5	-1.2	51.7	-24.3	.0	24.3
174	382.2	5.3	-1.6	51.7	-24.3	.0	24.4
175	332.7	6.9	-2.4	51.7	-24.3	.0	24.4
176	283.1	8.5	-3.3	51.7	-24.3	.0	24.5
177	233.6	10.1	-4.1	51.7	-24.3	.0	24.7
178	184.0	11.7	-4.9	51.7	-24.3	.0	24.8
179	134.5	13.2	-5.7	51.7	-24.3	.0	25.0
180	84.9	14.8	-6.6	51.7	-24.3	.0	25.2
181	35.3	16.4	-7.4	51.7	-24.3	.0	25.4
182	-14.2	18.0	-8.2	51.7	-24.3	.0	25.7
183	-63.8	19.6	-9.0	51.7	-24.3	.0	25.9
184	-113.3	21.2	-9.9	51.7	-24.3	.0	26.2
185	-162.9	22.8	-10.7	51.7	-24.3	.0	26.6
186	-187.7	23.6	-11.1	51.7	-24.3	.0	26.7
187	-237.2	25.2	-11.9	51.7	-24.3	.0	27.1
188	421.5	3.7	-.8	50.9	-23.9	.0	23.9
189	-222.7	24.4	-11.5	50.9	-23.9	.0	26.6
190	436.0	2.9	-.4	50.2	-23.6	.0	23.6
191	386.5	4.5	-1.2	50.2	-23.6	.0	23.6
192	361.7	5.3	-1.6	50.2	-23.6	.0	23.6
193	312.1	6.9	-2.4	50.2	-23.6	.0	23.7
194	262.6	8.5	-3.3	50.2	-23.6	.0	23.8
195	213.0	10.1	-4.1	50.2	-23.6	.0	23.9
196	163.5	11.7	-4.9	50.2	-23.6	.0	24.1
197	113.9	13.2	-5.7	50.2	-23.6	.0	24.2
198	64.3	14.8	-6.6	50.2	-23.6	.0	24.5
199	14.8	16.4	-7.4	50.2	-23.6	.0	24.7
200	-34.8	18.0	-8.2	50.2	-23.6	.0	24.9
201	-84.3	19.6	-9.0	50.2	-23.6	.0	25.2
202	-133.9	21.2	-9.9	50.2	-23.6	.0	25.5
203	-183.4	22.8	-10.7	50.2	-23.6	.0	25.9
204	-208.2	23.6	-11.1	50.2	-23.6	.0	26.0
205	-257.8	25.2	-11.9	50.2	-23.6	.0	26.4
206	425.7	2.9	-.4	49.5	-23.2	.0	23.2
207	376.2	4.5	-1.2	49.5	-23.2	.0	23.2
208	351.4	5.3	-1.6	49.5	-23.2	.0	23.2
209	301.8	6.9	-2.4	49.5	-23.2	.0	23.3
210	252.3	8.5	-3.3	49.5	-23.2	.0	23.4
211	202.7	10.1	-4.1	49.5	-23.2	.0	23.5
212	153.2	11.7	-4.9	49.5	-23.2	.0	23.7
213	103.6	13.2	-5.7	49.5	-23.2	.0	23.9
214	54.1	14.8	-6.6	49.5	-23.2	.0	24.1
215	4.5	16.4	-7.4	49.5	-23.2	.0	24.3
216	-45.1	18.0	-8.2	49.5	-23.2	.0	24.6
217	-94.6	19.6	-9.0	49.5	-23.2	.0	24.9
218	-144.2	21.2	-9.9	49.5	-23.2	.0	25.2
219	-193.7	22.8	-10.7	49.5	-23.2	.0	25.5
220	-218.5	23.6	-11.1	49.5	-23.2	.0	25.7
221	-268.1	25.2	-11.9	49.5	-23.2	.0	26.1
222	390.7	3.7	-.8	48.7	-22.8	.0	22.8
223	316.3	6.1	-2.0	48.7	-22.8	.0	22.9
224	266.8	7.7	-2.8	48.7	-22.8	.0	23.0
225	217.2	9.3	-3.7	48.7	-22.8	.0	23.1
226	167.7	10.9	-4.5	48.7	-22.8	.0	23.2
227	118.1	12.4	-5.3	48.7	-22.8	.0	23.4
228	68.6	14.0	-6.1	48.7	-22.8	.0	23.6
229	19.0	15.6	-7.0	48.7	-22.8	.0	23.8
230	-30.6	17.2	-7.8	48.7	-22.8	.0	24.1
231	-80.1	18.8	-8.6	48.7	-22.8	.0	24.4
232	-129.7	20.4	-9.5	48.7	-22.8	.0	24.7
233	-179.2	22.0	-10.3	48.7	-22.8	.0	25.0
234	-253.6	24.4	-11.5	48.7	-22.8	.0	25.5
235	405.2	2.9	-.4	48.0	-22.4	.0	22.4
236	355.6	4.5	-1.2	48.0	-22.4	.0	22.4
237	330.8	5.3	-1.6	48.0	-22.4	.0	22.5
238	281.3	6.9	-2.4	48.0	-22.4	.0	22.5
239	231.7	8.5	-3.3	48.0	-22.4	.0	22.6
240	182.2	10.1	-4.1	48.0	-22.4	.0	22.8
241	132.6	11.7	-4.9	48.0	-22.4	.0	22.9
242	83.1	13.2	-5.7	48.0	-22.4	.0	23.1



Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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243	33.5	14.8	-6.6	48.0	-22.4	.0	23.3
244	-16.1	16.4	-7.4	48.0	-22.4	.0	23.6
245	-65.6	18.0	-8.2	48.0	-22.4	.0	23.9
246	-115.2	19.6	-9.0	48.0	-22.4	.0	24.2
247	-164.7	21.2	-9.9	48.0	-22.4	.0	24.5
248	-214.3	22.8	-10.7	48.0	-22.4	.0	24.8
249	-239.1	23.6	-11.1	48.0	-22.4	.0	25.0
250	-288.6	25.2	-11.9	48.0	-22.4	.0	25.4

 $M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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LINEA FS FERRANDINA MATERA
VI08 Pile P21 e P22 - Analisi SLV

CONDIZIONE DI CARICO 8
VI08 - P22 - SLV_y

Coordinate Centri di Carico (c.c.)

c.c.	Xc m	Yc m	Zc m	Alfc deg
1	.000	.000	.000	.00
2	.000	.000	.000	.00

Componenti di Azioni Esterne riferite ai Centri di Carico

c.c.	Fzc kN	Fxc kN	Mxc kN*m	Fyc kN	Myc kN*m	Mzc kN*m
1	21615.0	-3509.7	-80607.6	-14020.8	-324099.0	-16433.8
2	18099.0	.0	.0	.0	.0	.0

Componenti di Carico Risultanti (riferimento globale)

Fz kN	Fx kN	Mx kN*m	Fy kN	My kN*m	Mz kN*m
39714.0	-3509.7	-80607.6	-14020.8	-324099.0	-16433.8

Punto di applic. carico verticale: Xv = -2.030 m Yv = -8.161 m

Componenti di Spostamento del Plinto (riferimento globale)

dz mm	dx mm	rx mRad	dy mm	ry mRad	rz mRad
1.589	-.671	-.172	-2.522	-.381	-.051

Sollecitazioni in Sommita' ai Singoli Pali (riferimento locale)

palo	Fzp kN	Fxp kN	Mxp kN*m	Fyp kN	Myp kN*m	Mzp kN*m	Mris kN*m
1	-301.8	-2.9	.4	-64.2	30.8	.0	30.8
2	-252.2	-4.5	1.2	-64.2	30.8	.0	30.8
3	-227.4	-5.3	1.6	-64.2	30.8	.0	30.8
4	-177.9	-6.9	2.4	-64.2	30.8	.0	30.9
5	-128.3	-8.5	3.2	-64.2	30.8	.0	31.0
6	-78.8	-10.1	4.1	-64.2	30.8	.0	31.1
7	-29.2	-11.7	4.9	-64.2	30.8	.0	31.2
8	20.3	-13.2	5.7	-64.2	30.8	.0	31.3
9	69.9	-14.8	6.6	-64.2	30.8	.0	31.5
10	119.4	-16.4	7.4	-64.2	30.8	.0	31.7
11	169.0	-18.0	8.2	-64.2	30.8	.0	31.9
12	218.6	-19.6	9.0	-64.2	30.8	.0	32.1
13	268.1	-21.2	9.9	-64.2	30.8	.0	32.3
14	317.7	-22.8	10.7	-64.2	30.8	.0	32.6

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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15	342.5	-23.6	11.1	-64.2	30.8	.0	32.7
16	392.0	-25.2	11.9	-64.2	30.8	.0	33.0
17	-266.7	-3.7	.8	-63.4	30.4	.0	30.4
18	-192.3	-6.1	2.0	-63.4	30.4	.0	30.5
19	-142.8	-7.7	2.8	-63.4	30.4	.0	30.6
20	-93.2	-9.3	3.7	-63.4	30.4	.0	30.6
21	-43.7	-10.9	4.5	-63.4	30.4	.0	30.8
22	5.9	-12.4	5.3	-63.4	30.4	.0	30.9
23	55.5	-14.0	6.1	-63.4	30.4	.0	31.0
24	105.0	-15.6	7.0	-63.4	30.4	.0	31.2
25	154.6	-17.2	7.8	-63.4	30.4	.0	31.4
26	204.1	-18.8	8.6	-63.4	30.4	.0	31.6
27	253.7	-20.4	9.4	-63.4	30.4	.0	31.9
28	303.2	-22.0	10.3	-63.4	30.4	.0	32.1
29	377.6	-24.4	11.5	-63.4	30.4	.0	32.5
30	-281.1	-2.9	.4	-62.7	30.0	.0	30.0
31	-231.5	-4.5	1.2	-62.7	30.0	.0	30.1
32	-206.8	-5.3	1.6	-62.7	30.0	.0	30.1
33	-157.2	-6.9	2.4	-62.7	30.0	.0	30.1
34	-107.7	-8.5	3.2	-62.7	30.0	.0	30.2
35	-58.1	-10.1	4.1	-62.7	30.0	.0	30.3
36	-8.5	-11.7	4.9	-62.7	30.0	.0	30.4
37	41.0	-13.2	5.7	-62.7	30.0	.0	30.6
38	90.6	-14.8	6.6	-62.7	30.0	.0	30.7
39	140.1	-16.4	7.4	-62.7	30.0	.0	30.9
40	189.7	-18.0	8.2	-62.7	30.0	.0	31.1
41	239.2	-19.6	9.0	-62.7	30.0	.0	31.4
42	288.8	-21.2	9.9	-62.7	30.0	.0	31.6
43	338.4	-22.8	10.7	-62.7	30.0	.0	31.9
44	363.1	-23.6	11.1	-62.7	30.0	.0	32.0
45	412.7	-25.2	11.9	-62.7	30.0	.0	32.3
46	-270.8	-2.9	.4	-62.0	29.7	.0	29.7
47	-221.2	-4.5	1.2	-62.0	29.7	.0	29.7
48	-196.4	-5.3	1.6	-62.0	29.7	.0	29.7
49	-146.9	-6.9	2.4	-62.0	29.7	.0	29.8
50	-97.3	-8.5	3.2	-62.0	29.7	.0	29.8
51	-47.8	-10.1	4.1	-62.0	29.7	.0	29.9
52	1.8	-11.7	4.9	-62.0	29.7	.0	30.1
53	51.4	-13.2	5.7	-62.0	29.7	.0	30.2
54	100.9	-14.8	6.6	-62.0	29.7	.0	30.4
55	150.5	-16.4	7.4	-62.0	29.7	.0	30.6
56	200.0	-18.0	8.2	-62.0	29.7	.0	30.8
57	249.6	-19.6	9.0	-62.0	29.7	.0	31.0
58	299.1	-21.2	9.9	-62.0	29.7	.0	31.3
59	348.7	-22.8	10.7	-62.0	29.7	.0	31.5
60	373.5	-23.6	11.1	-62.0	29.7	.0	31.7
61	423.0	-25.2	11.9	-62.0	29.7	.0	32.0
62	-235.6	-3.7	.8	-61.2	29.3	.0	29.3
63	408.6	-24.4	11.5	-61.2	29.3	.0	31.5
64	-250.1	-2.9	.4	-60.5	28.9	.0	28.9
65	-200.5	-4.5	1.2	-60.5	28.9	.0	28.9
66	-175.7	-5.3	1.6	-60.5	28.9	.0	28.9
67	-126.2	-6.9	2.4	-60.5	28.9	.0	29.0
68	-76.6	-8.5	3.2	-60.5	28.9	.0	29.1
69	-27.1	-10.1	4.1	-60.5	28.9	.0	29.2
70	22.5	-11.7	4.9	-60.5	28.9	.0	29.3
71	72.0	-13.2	5.7	-60.5	28.9	.0	29.5
72	121.6	-14.8	6.6	-60.5	28.9	.0	29.6
73	171.1	-16.4	7.4	-60.5	28.9	.0	29.8
74	220.7	-18.0	8.2	-60.5	28.9	.0	30.0
75	270.3	-19.6	9.0	-60.5	28.9	.0	30.3
76	319.8	-21.2	9.9	-60.5	28.9	.0	30.5
77	369.4	-22.8	10.7	-60.5	28.9	.0	30.8
78	394.2	-23.6	11.1	-60.5	28.9	.0	31.0
79	443.7	-25.2	11.9	-60.5	28.9	.0	31.3
80	-215.0	-3.7	.8	-59.8	28.5	.0	28.5
81	429.3	-24.4	11.5	-59.8	28.5	.0	30.8
82	-229.4	-2.9	.4	-59.0	28.1	.0	28.1
83	-179.8	-4.5	1.2	-59.0	28.1	.0	28.2
84	-155.1	-5.3	1.6	-59.0	28.1	.0	28.2
85	-105.5	-6.9	2.4	-59.0	28.1	.0	28.2
86	-56.0	-8.5	3.2	-59.0	28.1	.0	28.3
87	-6.4	-10.1	4.1	-59.0	28.1	.0	28.4
88	43.2	-11.7	4.9	-59.0	28.1	.0	28.6
89	92.7	-13.2	5.7	-59.0	28.1	.0	28.7
90	142.3	-14.8	6.6	-59.0	28.1	.0	28.9

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
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91	191.8	-16.4	7.4	-59.0	28.1	.0	29.1
92	241.4	-18.0	8.2	-59.0	28.1	.0	29.3
93	290.9	-19.6	9.0	-59.0	28.1	.0	29.5
94	340.5	-21.2	9.9	-59.0	28.1	.0	29.8
95	390.1	-22.8	10.7	-59.0	28.1	.0	30.1
96	414.8	-23.6	11.1	-59.0	28.1	.0	30.2
97	464.4	-25.2	11.9	-59.0	28.1	.0	30.6
98	-194.3	-3.7	.8	-58.3	27.8	.0	27.8
99	450.0	-24.4	11.5	-58.3	27.8	.0	30.0
100	-208.7	-2.9	.4	-57.6	27.4	.0	27.4
101	-159.2	-4.5	1.2	-57.6	27.4	.0	27.4
102	-134.4	-5.3	1.6	-57.6	27.4	.0	27.4
103	-84.8	-6.9	2.4	-57.6	27.4	.0	27.5
104	-35.3	-8.5	3.2	-57.6	27.4	.0	27.6
105	14.3	-10.1	4.1	-57.6	27.4	.0	27.7
106	63.8	-11.7	4.9	-57.6	27.4	.0	27.8
107	113.4	-13.2	5.7	-57.6	27.4	.0	28.0
108	163.0	-14.8	6.6	-57.6	27.4	.0	28.1
109	212.5	-16.4	7.4	-57.6	27.4	.0	28.3
110	262.1	-18.0	8.2	-57.6	27.4	.0	28.6
111	311.6	-19.6	9.0	-57.6	27.4	.0	28.8
112	361.2	-21.2	9.9	-57.6	27.4	.0	29.1
113	410.7	-22.8	10.7	-57.6	27.4	.0	29.4
114	435.5	-23.6	11.1	-57.6	27.4	.0	29.5
115	485.1	-25.2	11.9	-57.6	27.4	.0	29.9
116	-173.6	-3.7	.8	-56.8	27.0	.0	27.0
117	470.6	-24.4	11.5	-56.8	27.0	.0	29.3
118	-188.0	-2.9	.4	-56.1	26.6	.0	26.6
119	-138.5	-4.5	1.2	-56.1	26.6	.0	26.6
120	-113.7	-5.3	1.6	-56.1	26.6	.0	26.7
121	-64.1	-6.9	2.4	-56.1	26.6	.0	26.7
122	-14.6	-8.5	3.2	-56.1	26.6	.0	26.8
123	35.0	-10.1	4.1	-56.1	26.6	.0	26.9
124	84.5	-11.7	4.9	-56.1	26.6	.0	27.1
125	134.1	-13.2	5.7	-56.1	26.6	.0	27.2
126	183.6	-14.8	6.6	-56.1	26.6	.0	27.4
127	233.2	-16.4	7.4	-56.1	26.6	.0	27.6
128	282.7	-18.0	8.2	-56.1	26.6	.0	27.8
129	332.3	-19.6	9.0	-56.1	26.6	.0	28.1
130	381.9	-21.2	9.9	-56.1	26.6	.0	28.4
131	431.4	-22.8	10.7	-56.1	26.6	.0	28.7
132	456.2	-23.6	11.1	-56.1	26.6	.0	28.8
133	505.8	-25.2	11.9	-56.1	26.6	.0	29.2
134	-152.9	-3.7	.8	-55.3	26.2	.0	26.2
135	491.3	-24.4	11.5	-55.3	26.2	.0	28.6
136	-167.4	-2.9	.4	-54.6	25.8	.0	25.8
137	-117.8	-4.5	1.2	-54.6	25.8	.0	25.9
138	-93.0	-5.3	1.6	-54.6	25.8	.0	25.9
139	-43.5	-6.9	2.4	-54.6	25.8	.0	26.0
140	6.1	-8.5	3.2	-54.6	25.8	.0	26.0
141	55.6	-10.1	4.1	-54.6	25.8	.0	26.2
142	105.2	-11.7	4.9	-54.6	25.8	.0	26.3
143	154.8	-13.2	5.7	-54.6	25.8	.0	26.5
144	204.3	-14.8	6.6	-54.6	25.8	.0	26.7
145	253.9	-16.4	7.4	-54.6	25.8	.0	26.9
146	303.4	-18.0	8.2	-54.6	25.8	.0	27.1
147	353.0	-19.6	9.0	-54.6	25.8	.0	27.4
148	402.5	-21.2	9.9	-54.6	25.8	.0	27.7
149	452.1	-22.8	10.7	-54.6	25.8	.0	28.0
150	476.9	-23.6	11.1	-54.6	25.8	.0	28.1
151	526.4	-25.2	11.9	-54.6	25.8	.0	28.5
152	-132.2	-3.7	.8	-53.9	25.5	.0	25.5
153	512.0	-24.4	11.5	-53.9	25.5	.0	27.9
154	-146.7	-2.9	.4	-53.1	25.1	.0	25.1
155	-97.1	-4.5	1.2	-53.1	25.1	.0	25.1
156	-72.3	-5.3	1.6	-53.1	25.1	.0	25.1
157	-22.8	-6.9	2.4	-53.1	25.1	.0	25.2
158	26.8	-8.5	3.2	-53.1	25.1	.0	25.3
159	76.3	-10.1	4.1	-53.1	25.1	.0	25.4
160	125.9	-11.7	4.9	-53.1	25.1	.0	25.6
161	175.4	-13.2	5.7	-53.1	25.1	.0	25.7
162	225.0	-14.8	6.6	-53.1	25.1	.0	25.9
163	274.6	-16.4	7.4	-53.1	25.1	.0	26.1
164	324.1	-18.0	8.2	-53.1	25.1	.0	26.4
165	373.7	-19.6	9.0	-53.1	25.1	.0	26.7
166	423.2	-21.2	9.9	-53.1	25.1	.0	26.9

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	90 di 91

167	472.8	-22.8	10.7	-53.1	25.1	.0	27.3
168	497.6	-23.6	11.1	-53.1	25.1	.0	27.4
169	547.1	-25.2	11.9	-53.1	25.1	.0	27.8
170	-111.6	-3.7	.8	-52.4	24.7	.0	24.7
171	532.7	-24.4	11.5	-52.4	24.7	.0	27.3
172	-126.0	-2.9	.4	-51.7	24.3	.0	24.3
173	-76.4	-4.5	1.2	-51.7	24.3	.0	24.3
174	-51.7	-5.3	1.6	-51.7	24.3	.0	24.4
175	-2.1	-6.9	2.4	-51.7	24.3	.0	24.4
176	47.4	-8.5	3.2	-51.7	24.3	.0	24.5
177	97.0	-10.1	4.1	-51.7	24.3	.0	24.7
178	146.6	-11.7	4.9	-51.7	24.3	.0	24.8
179	196.1	-13.2	5.7	-51.7	24.3	.0	25.0
180	245.7	-14.8	6.6	-51.7	24.3	.0	25.2
181	295.2	-16.4	7.4	-51.7	24.3	.0	25.4
182	344.8	-18.0	8.2	-51.7	24.3	.0	25.7
183	394.3	-19.6	9.0	-51.7	24.3	.0	25.9
184	443.9	-21.2	9.9	-51.7	24.3	.0	26.2
185	493.5	-22.8	10.7	-51.7	24.3	.0	26.6
186	518.2	-23.6	11.1	-51.7	24.3	.0	26.7
187	567.8	-25.2	11.9	-51.7	24.3	.0	27.1
188	-90.9	-3.7	.8	-50.9	23.9	.0	23.9
189	553.4	-24.4	11.5	-50.9	23.9	.0	26.6
190	-105.3	-2.9	.4	-50.2	23.6	.0	23.6
191	-55.8	-4.5	1.2	-50.2	23.6	.0	23.6
192	-31.0	-5.3	1.6	-50.2	23.6	.0	23.6
193	18.6	-6.9	2.4	-50.2	23.6	.0	23.7
194	68.1	-8.5	3.2	-50.2	23.6	.0	23.8
195	117.7	-10.1	4.1	-50.2	23.6	.0	23.9
196	167.2	-11.7	4.9	-50.2	23.6	.0	24.1
197	216.8	-13.2	5.7	-50.2	23.6	.0	24.2
198	266.4	-14.8	6.6	-50.2	23.6	.0	24.4
199	315.9	-16.4	7.4	-50.2	23.6	.0	24.7
200	365.5	-18.0	8.2	-50.2	23.6	.0	24.9
201	415.0	-19.6	9.0	-50.2	23.6	.0	25.2
202	464.6	-21.2	9.9	-50.2	23.6	.0	25.5
203	514.1	-22.8	10.7	-50.2	23.6	.0	25.9
204	538.9	-23.6	11.1	-50.2	23.6	.0	26.0
205	588.5	-25.2	11.9	-50.2	23.6	.0	26.4
206	-95.0	-2.9	.4	-49.5	23.2	.0	23.2
207	-45.4	-4.5	1.2	-49.5	23.2	.0	23.2
208	-20.6	-5.3	1.6	-49.5	23.2	.0	23.2
209	28.9	-6.9	2.4	-49.5	23.2	.0	23.3
210	78.5	-8.5	3.2	-49.5	23.2	.0	23.4
211	128.0	-10.1	4.1	-49.5	23.2	.0	23.5
212	177.6	-11.7	4.9	-49.5	23.2	.0	23.7
213	227.1	-13.2	5.7	-49.5	23.2	.0	23.9
214	276.7	-14.8	6.6	-49.5	23.2	.0	24.1
215	326.3	-16.4	7.4	-49.5	23.2	.0	24.3
216	375.8	-18.0	8.2	-49.5	23.2	.0	24.6
217	425.4	-19.6	9.0	-49.5	23.2	.0	24.9
218	474.9	-21.2	9.9	-49.5	23.2	.0	25.2
219	524.5	-22.8	10.7	-49.5	23.2	.0	25.5
220	549.3	-23.6	11.1	-49.5	23.2	.0	25.7
221	598.8	-25.2	11.9	-49.5	23.2	.0	26.1
222	-59.9	-3.7	.8	-48.7	22.8	.0	22.8
223	14.5	-6.1	2.0	-48.7	22.8	.0	22.9
224	64.0	-7.7	2.8	-48.7	22.8	.0	23.0
225	113.6	-9.3	3.7	-48.7	22.8	.0	23.1
226	163.1	-10.9	4.5	-48.7	22.8	.0	23.2
227	212.7	-12.4	5.3	-48.7	22.8	.0	23.4
228	262.3	-14.0	6.1	-48.7	22.8	.0	23.6
229	311.8	-15.6	7.0	-48.7	22.8	.0	23.8
230	361.4	-17.2	7.8	-48.7	22.8	.0	24.1
231	410.9	-18.8	8.6	-48.7	22.8	.0	24.4
232	460.5	-20.4	9.4	-48.7	22.8	.0	24.7
233	510.0	-22.0	10.3	-48.7	22.8	.0	25.0
234	584.4	-24.4	11.5	-48.7	22.8	.0	25.5
235	-74.3	-2.9	.4	-48.0	22.4	.0	22.4
236	-24.7	-4.5	1.2	-48.0	22.4	.0	22.4
237	.0	-5.3	1.6	-48.0	22.4	.0	22.5
238	49.6	-6.9	2.4	-48.0	22.4	.0	22.5
239	99.2	-8.5	3.2	-48.0	22.4	.0	22.6
240	148.7	-10.1	4.1	-48.0	22.4	.0	22.8
241	198.3	-11.7	4.9	-48.0	22.4	.0	22.9
242	247.8	-13.2	5.7	-48.0	22.4	.0	23.1



Nuova linea Ferrandina - Matera La Martella

VIADOTTO "GRAVINA"

Relazione di calcolo fondazioni dirette su micropali
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PROGETTO	LOTTO	CODIFICA	DOCUMENT	REV.	FOGLIO
IA5F	01	D09 CL	VI 0803003	A	91 di 91

243	297.4	-14.8	6.6	-48.0	22.4	.0	23.3
244	346.9	-16.4	7.4	-48.0	22.4	.0	23.6
245	396.5	-18.0	8.2	-48.0	22.4	.0	23.9
246	446.0	-19.6	9.0	-48.0	22.4	.0	24.2
247	495.6	-21.2	9.9	-48.0	22.4	.0	24.5
248	545.2	-22.8	10.7	-48.0	22.4	.0	24.8
249	569.9	-23.6	11.1	-48.0	22.4	.0	25.0
250	619.5	-25.2	11.9	-48.0	22.4	.0	25.4

 $M_{ris} = (M_{xp}^2 + M_{yp}^2)^{0.5}$