

# S.S. N. 9 "VIA EMILIA"

## VARIANTE DI CASALPUSTERLENGO ED ELIMINAZIONE PASSAGGIO A LIVELLO SULLA S.P. EX S.S. N.234

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## 1 GENERALITÀ

### 1.1 PREMESSE

La presente relazione riporta i calcoli statici relativi alla progettazione esecutiva del Cavalcavia Strada Vicinale Delle Coste, intersecante alla Pk. 5+059.88 la Variante di Casalpusterlengo alla S.S. n. 9 "Via Emilia". L'opera in oggetto presenta uno schema statico di trave continua su tre campate aventi luci tra gli assi di appoggio pari a 31+38+31 m.

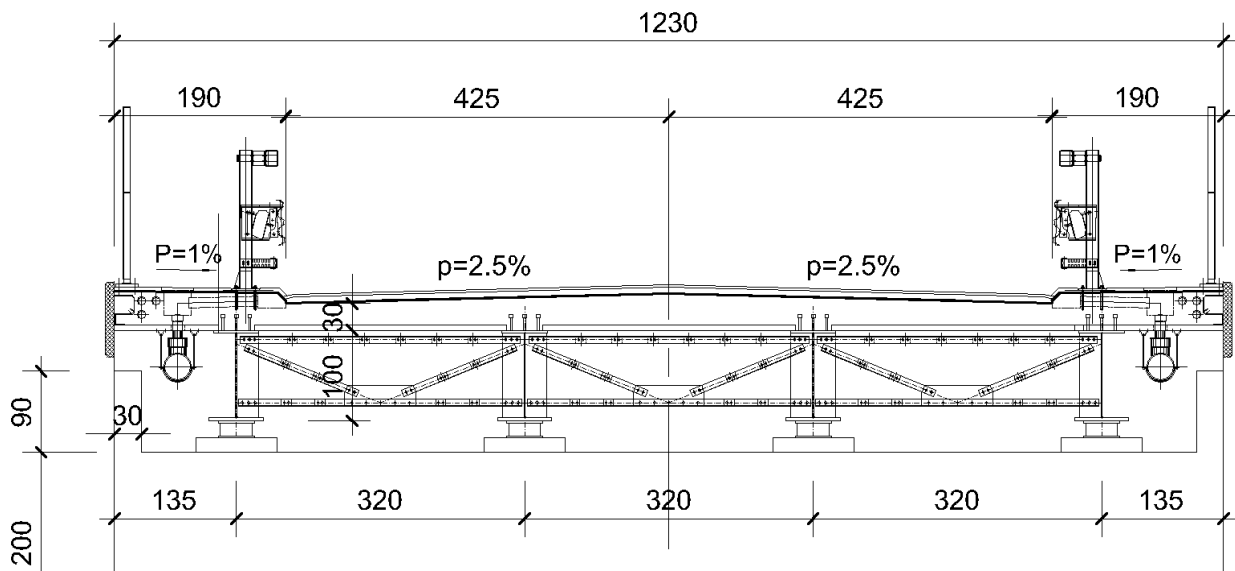
L'impalcato è composto, da quattro travi continue in acciaio a doppio T ad altezza variabile sostenenti la soletta di scorrimento stradale in calcestruzzo armato ordinario.

In riguardo agli aspetti sismici, l'impalcato è isolato alla sommità delle pile e spalle mediante isolatori elastomerici armati.

**L'impalcato, in previsione di un futuro allargamento della strada è stato calcolato per una sezione tipo F2 anziché F urbana; la larghezza totale dell'impalcato rimane 12.30 m.**

#### Dimensioni dell'impalcato:

Larghezza marciapiedi	= 1.90 m	<b>(2.90 m)</b>
Larghezza carreggiata, categoria F2	= 8.50 m	<b>(6,50 m)</b>
Larghezza totale soletta	= 12.30 m	
Numero delle travi	= 4	
Interasse travi	= 3.20 m	
Altezza delle travi (variabile)	= 1.00÷2.00 m	
Spessore soletta	= 30 cm	(lastra 6 cm + getto 24 cm)



**FIGURA 1 - SEZIONE TIPICA IMPALCATO (SU SPALLA)**



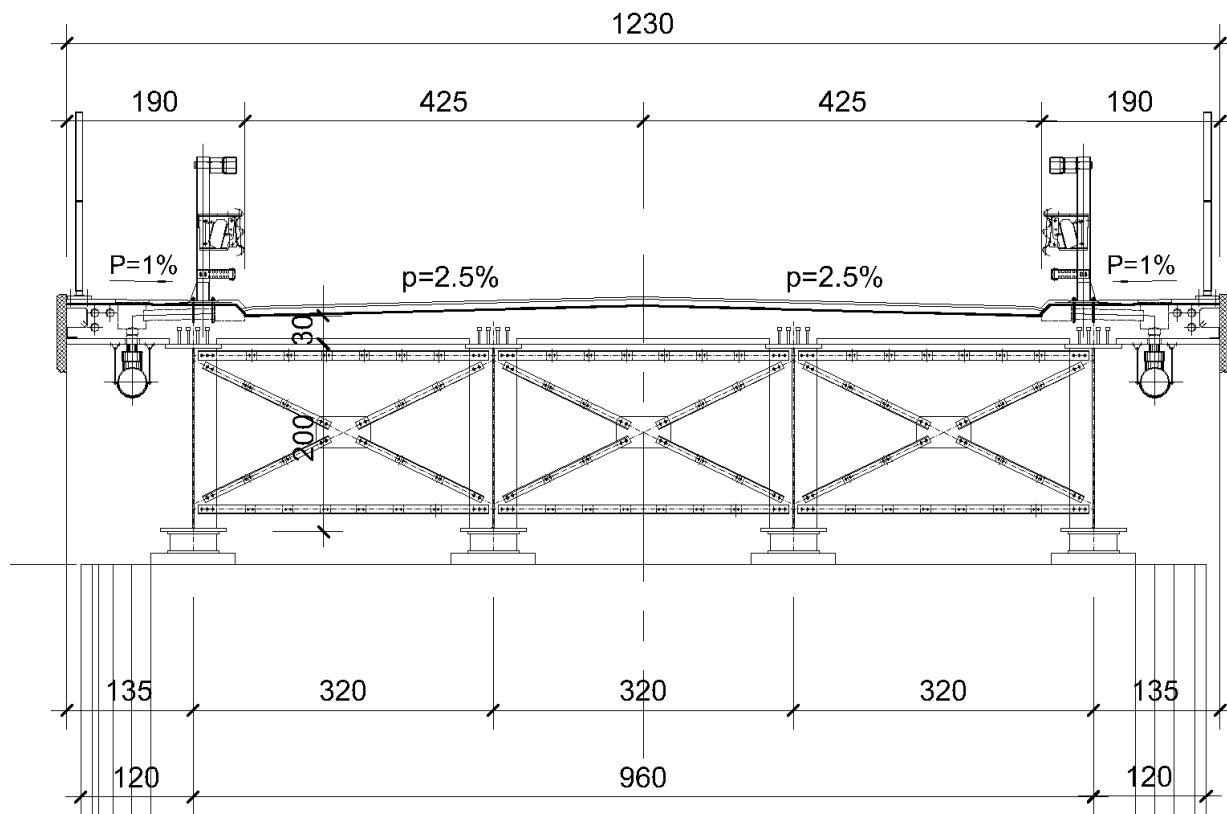


FIGURA 2- SEZIONE IMPALCATO (SU PILA)

## 1.2 NORMATIVA

Nella redazione dei calcoli statici ci si è attenuti alle prescrizioni della Normativa vigente; in particolare:

- **Legge n°1086 del 05/11/1971**  
 “Norme per la disciplina delle opere in conglomerato cementizio armato, normale e precompresso ed a struttura metallica”
- **Legge n°64 del 02/02/1974:**“Provvedimenti per le costruzioni con particolari prescrizioni per le zone sismiche”
- **Decreto Ministeriale 14/01/2008**  
 “Norme tecniche per le costruzioni”
- **Circolare Min. 02/02/2009, n° 617**  
 “Istruzioni per l’applicazione delle Norme tecniche per le costruzioni di cui al D.M. 14/01/2008”
- **UNI EN 1993-1-1:2005**  
 “Eurocodice 3 – Progettazione delle strutture in acciaio – Parte 1-1: Regole generali e regole per gli edifici”
- **UNI EN 1993-1-5:2007**  
 “Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 1-5: Elementi strutturali a lastra”
- **UNI EN 1993-1-9:2005**  
 “Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 1-9: Fatica”

**- UNI EN 1993-1-10:2005**

“Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 1-10: Resilienza del materiale e proprietà attraverso lo spessore”

**- UNI EN 1993-2:2007**

“Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 2: Ponti in acciaio”

**- UNI EN 1994-1-1:2005**

“Eurocodice 4 - Progettazione delle strutture composte acciaio-calcestruzzo – Parte 1-1: Regole generali e regole per gli edifici”

**- UNI EN 1994-2:2006**

“Eurocodice 4 - Progettazione delle strutture composte acciaio-calcestruzzo – Parte 2: Regole generali e regole per i ponti”

### 1.3 MATERIALI

I materiali di previsto impiego sono elencati nelle seguenti tabelle:

Calcestruzzo (rif. par. 11.2.10 DM 14.01.2008)		
<b>Resistenza caratteristica cubica a 28 gg del calcestruzzo <math>R_{ck}</math></b>	<b>40</b>	N/mm <sup>2</sup>
<b>Resistenza caratteristica cilindrica</b>	<b>32</b>	N/mm <sup>2</sup>
Resistenza cilindrica media <b><math>f_{cm} = f_{ck} + 8</math></b>	40.00	N/mm <sup>2</sup>
Resistenza media a trazione semplice assiale <b><math>f_{ctm} = 0,30 \cdot f_{ck}^{2/3}</math></b>	3.02	N/mm <sup>2</sup>
Resistenza caratteristica a trazione semplice assiale <b><math>f_{ctk} = 0,7 \cdot f_{ctm}</math></b>	2.12	N/mm <sup>2</sup>
Resistenza media a trazione per flessione <b><math>f_{ctm} = 1,2 \cdot f_{ctm}</math></b>	3.63	N/mm <sup>2</sup>
Modulo elastico secante tra 0 e 0,40 $f_{cm}$ <b><math>E_{cm} = 22000 (f_{cm}/10)^{0,3}</math></b>	33346	N/mm <sup>2</sup>
Coefficiente di Poisson (cls fessurato/non fessurato)	0/0,2	
Coefficiente di dilatazione termica <b><math>\alpha</math></b>	1.0E-5	1/C°
Coefficiente riduttivo per le resistenze di lunga durata <b><math>\alpha_{cc}</math></b>	0.85	
Coefficiente parziale di sicurezza relativo al calcestruzzo <b><math>\gamma_c</math></b>	1.5	
Resistenza di calcolo a compressione del calcestruzzo <b><math>f_{cd} = \alpha_{cc} f_{ck} / \gamma_c</math></b> (per spessori > 50 mm)	18.13	N/mm <sup>2</sup>
Resistenza di calcolo a compressione del calcestruzzo <b><math>f_{cd} = 0.8 \alpha_{cc} f_{ck} / \gamma_c</math></b> (per spessori ≤ 50 mm)	14.51	N/mm <sup>2</sup>
Resistenza di calcolo a trazione del calcestruzzo <b><math>f_{ctd} = f_{ctk} / \gamma_c</math></b> (per spessori > 50 mm)	1.41	N/mm <sup>2</sup>
Resistenza di calcolo a trazione del calcestruzzo <b><math>f_{ctd} = 0.8 f_{ctk} / \gamma_c</math></b> (per spessori ≤ 50 mm)	1.13	N/mm <sup>2</sup>
Stati Limite di Esercizio: Tensione massima di compressione per combinazioni caratteristiche <b><math>\sigma_c = 0.6 f_{ck}</math></b>	19.20	N/mm <sup>2</sup>
Stati Limite di Esercizio: Tensione massima di compressione per combinazioni quasi permanenti <b><math>\sigma_c = 0.45 f_{ck}</math></b>	14.40	N/mm <sup>2</sup>
Tensione tangenziale di aderenza caratteristica acciaio-calcestruzzo <b><math>f_{bk} = 2,25 \eta f_{ctk} (\eta = 1)</math></b>	4.76	N/mm <sup>2</sup>
Tensione tangenziale di aderenza di calcolo acciaio-calcestruzzo <b><math>f_{bd} = f_{bk} / \gamma_c</math></b>	3.18	N/mm <sup>2</sup>

Acciaio per cemento armato B450C (rif. par. 11.3.2.1 DM 14.01.2008)		
Valore nominale della tensione caratteristica di snervamento <b><math>f_{y,nom}</math></b>	<b>450</b>	N/mm <sup>2</sup>
Valore nominale della tensione caratteristica di rottura <b><math>f_{t,nom}</math></b>	<b>540</b>	N/mm <sup>2</sup>
Coefficiente parziale di sicurezza relativo all'acciaio <b><math>\gamma_s</math></b>	1.15	
Resistenza di calcolo dell'acciaio <b><math>f_{yd} = f_{yk} / \gamma_s</math></b>	391.3	N/mm <sup>2</sup>
Stati Limite di Esercizio: Tensione massima per combinazioni caratteristiche <b><math>\sigma_s = 0.8 f_{yk}</math></b>	360.0	N/mm <sup>2</sup>



- controventi: 3xxx, ordinati per X, Y e Z crescenti
- pile: 4xxx, ordinati per Z crescente e quindi per X crescente

Per la ripartizione trasversale dei carichi si sono inoltre schematizzati elementi trasversali fittizi rappresentanti la soletta.

La soletta considerata nelle sezioni composte è limitata alla parte gettata in opera, esclusa quindi la lastra prefabbricata inferiore, di 6 cm di spessore.

Si è assunta un'accelerazione gravitazionale pari a  $g = 9,806 \text{ m/s}^2$ .

Le unità di misura utilizzate sono coerenti con il Sistema Internazionale:

lunghezze:	m	(metri)
masse:	t	(tonnellate)
forze	kN	(kilo-Newton)

I materiali usati nella modellazione hanno le caratteristiche di calcolo seguenti:

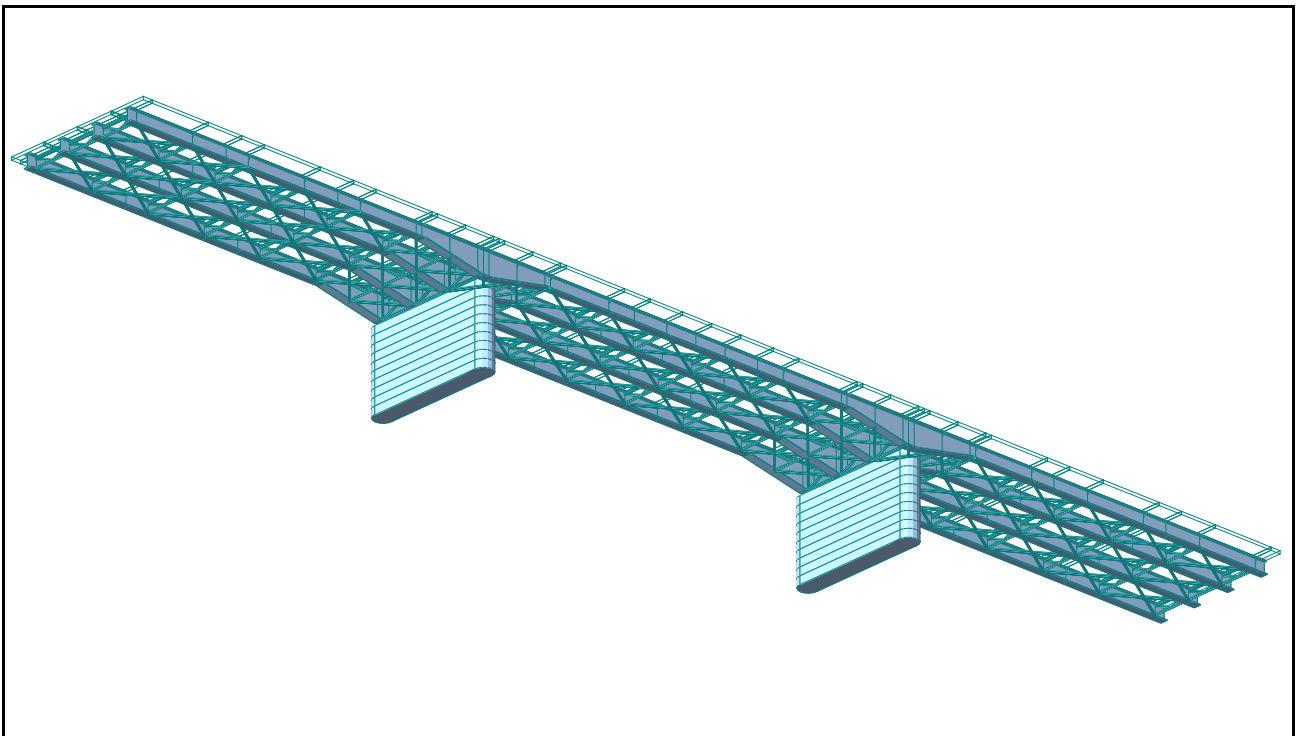
a) calcestruzzo (C32/40):

- modulo di elasticità:  $E = 33345 \text{ N/mm}^2$
- coefficiente di Poisson:  $\nu = 0,20$
- coefficiente di dilatazione termica:  $\alpha = 0.00001 \text{ C}^{-1}$
- peso specifico:  $\gamma = 25 \text{ kN/m}^3$
- massa specifica:  $m = 2.55 \text{ t/m}^3$

b) acciaio strutturale viadotto (S355W):

- modulo di elasticità:  $E = 210000 \text{ N/mm}^2$
- coefficiente di Poisson:  $\nu = 0.30$
- coefficiente di dilatazione termica:  $\alpha = 0.000012 \text{ C}^{-1}$
- peso specifico(\*):  $\gamma = 88.6 \text{ kN/m}^3$
- massa specifica(\*):  $m = 9.03 \text{ t/m}^3$

NOTA (\*): le caratteristiche volumiche dell'acciaio son aumentate del 15% per tener conto degli elementi non schematizzati nel modello (irrigidimenti, piastre, bulloni, saldature, connettori).



**FIGURA 3 - SCHEMA DI CALCOLO – ASSONOMETRIA**

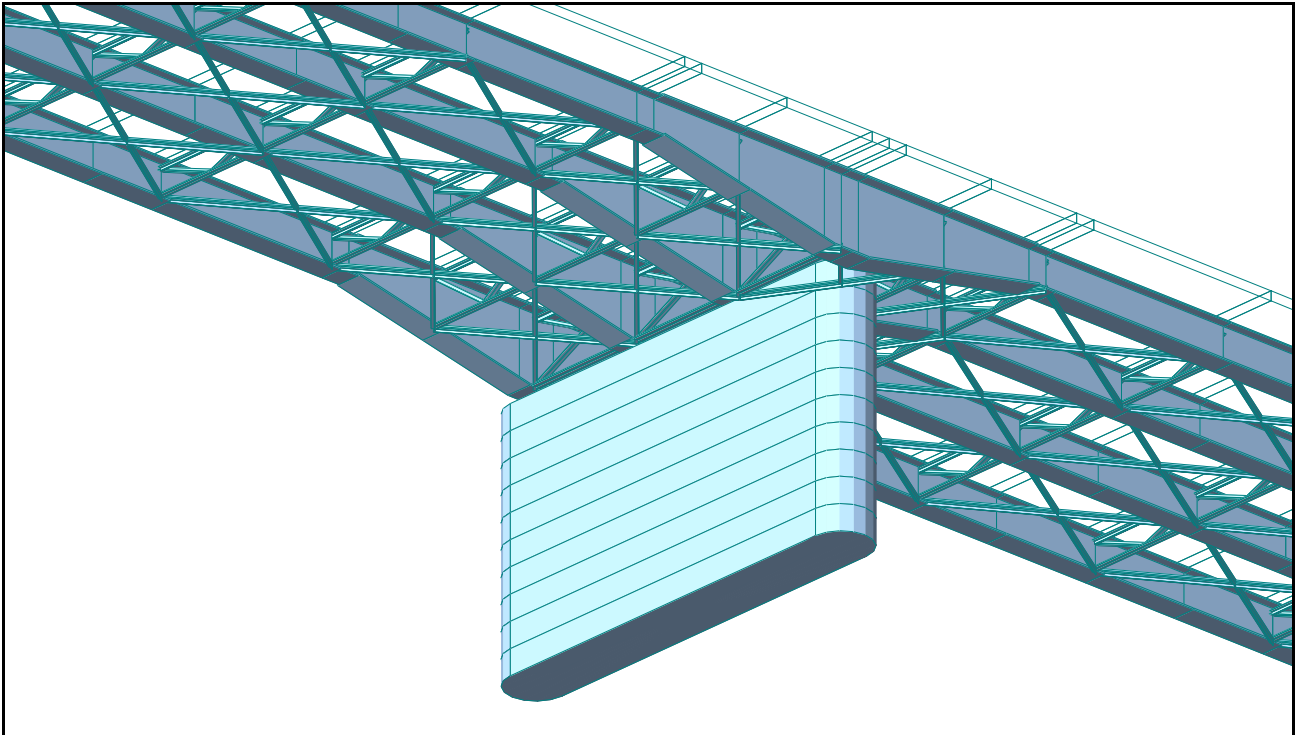
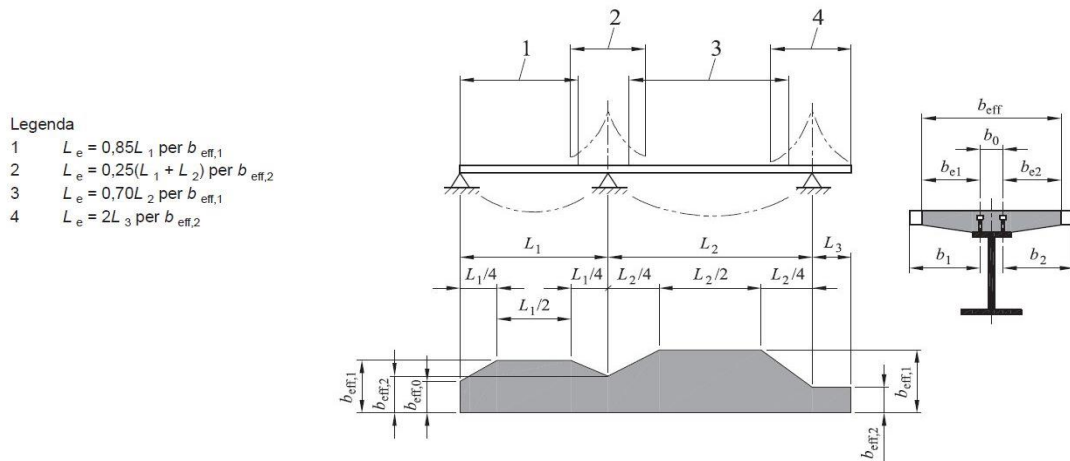


FIGURA 4 - SCHEMA DI CALCOLO – PARTICOLARE SU PILA

## 2.1 LARGHEZZE “EFFICACI” DI SOLETTA COLLABORANTE

Nel calcolo delle sezioni composte si tiene conto di una larghezza di soletta collaborante determinata secondo NTC 2008 # 4.3.2.3; il calcolo è riassunto nella seguente tabella.

TRAVE CONTINUA - CALCOLO DELLE LARGHEZZE EFFICACI DI SOLETTA COLLABORANTE - NTC 2008, § 4.3.2.3.



- Legenda
- 1  $L_e = 0,85L_1$  per  $b_{eff,1}$
  - 2  $L_e = 0,25(L_1 + L_2)$  per  $b_{eff,2}$
  - 3  $L_e = 0,70L_2$  per  $b_{eff,1}$
  - 4  $L_e = 2L_3$  per  $b_{eff,2}$

CAVALCAVIA STR. VIC. DELLE COSTE TRAVI ESTERNE	Appoggio / Campata	Estremità / Intermedio	Luce (m)	b <sub>0</sub> (m)	b <sub>1</sub> (m)	b <sub>2</sub> (m)	L <sub>0</sub> (m)	L <sub>0</sub> / 8 (m)	b <sub>e1</sub> (m)	b <sub>e2</sub> (m)	β <sub>1</sub>	β <sub>2</sub>	b <sub>eff</sub> (m)
Spalla 1	Appoggio	Estremità		0.300	1.450	1.200	26.350	3.294	1.450	1.200	1.000	1.000	2.950
	Campata	Estremità	31.000	0.300	1.450	1.200	26.350	3.294	1.450	1.200	1.000	1.000	2.950
Pila 1	Appoggio	Intermedio		0.300	1.450	1.200	17.250	2.156	1.450	1.200	1.000	1.000	2.950
	Campata	Intermedio	38.000	0.300	1.450	1.200	26.600	3.325	1.450	1.200	1.000	1.000	2.950
Pila 2	Appoggio	Intermedio		0.300	1.450	1.200	17.250	2.156	1.450	1.200	1.000	1.000	2.950
	Campata	Estremità	31.000	0.300	1.450	1.200	26.350	3.294	1.450	1.200	1.000	1.000	2.950
Spalla 2	Appoggio	Estremità		0.300	1.450	1.200	26.350	3.294	1.450	1.200	1.000	1.000	2.950

CAVALCAVIA STR. VIC. DELLE COSTE TRAVI INTERNE	Appoggio / Campata	Estremità / Intermedio	Luce (m)	b <sub>0</sub> (m)	b <sub>1</sub> (m)	b <sub>2</sub> (m)	L <sub>0</sub> (m)	L <sub>0</sub> / 8 (m)	b <sub>e1</sub> (m)	b <sub>e2</sub> (m)	β <sub>1</sub>	β <sub>2</sub>	b <sub>eff</sub> (m)
Spalla 1	Appoggio	Estremità		0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200
	Campata	Estremità	31.000	0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200
Pila 1	Appoggio	Intermedio		0.300	1.450	1.450	17.250	2.156	1.450	1.450	1.000	1.000	3.200
	Campata	Intermedio	38.000	0.300	1.450	1.450	26.600	3.325	1.450	1.450	1.000	1.000	3.200
Pila 2	Appoggio	Intermedio		0.300	1.450	1.450	17.250	2.156	1.450	1.450	1.000	1.000	3.200
	Campata	Estremità	31.000	0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200
Spalla 2	Appoggio	Estremità		0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200

### 3 DISPOSITIVI DI APOGGIO E ISOLAMENTO

Alla sommità delle pile e spalle sono disposti apparecchi di appoggio antisismici costituiti da isolatori elastomerici armati, costituiti da strati alternati di lamiere di acciaio ed elastomero, collegati mediante vulcanizzazione.

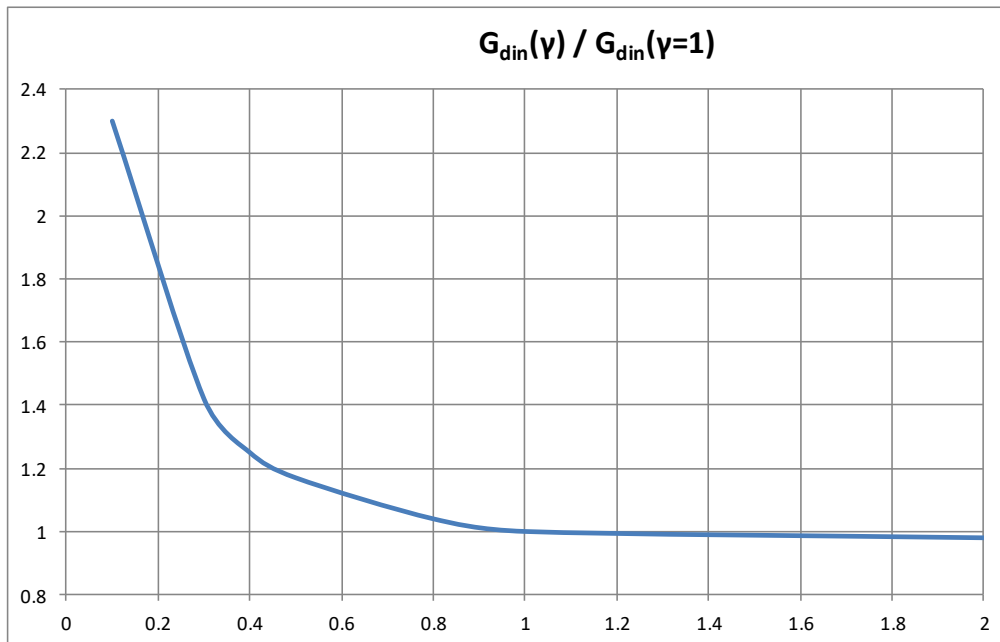
Gli isolatori sono caratterizzati da una elevata rigidità verticale, una ridotta rigidità orizzontale e una modesta capacità dissipativa; queste caratteristiche consentono, rispettivamente, di sostenere i carichi verticali senza apprezzabili cedimenti, di contenere gli spostamenti sismici orizzontali dell' impalcato e aumentare i periodi di vibrazione dell'impalcato in modo da limitare le forze dinamiche orizzontali trasmesse dall'impalcato alle pile/spalle.

Le caratteristiche di progetto degli isolatori sono riportate nella seguente tabella

Caratteristiche di progetto isolatore	SPALLE	PILE
Diametro elastomero	Øg= 300	450 mm
Spessore totale elastomero	te= 76	78 mm
Altezza totale escluse piastre di ancoraggio	h= 152	154 mm
Altezza totale comprese piastre di ancoraggio	H= 202	204 mm
Lato piastre di ancoraggio	Z= 350	500 mm
Modulo di elasticità tang. dinamico elastomero ( $\gamma = d/te = 1$ )	G(din)= 1.4	1.4 N/mmq
Deformazione massima statica di progetto SLU/SLD ( $d/te \leq 1$ )	du= 76	78 mm
Deformazione massima sismica di progetto SLC ( $d/te \leq 2$ )	dc= 152	156 mm
Rigidità orizzontale equivalente ( $\gamma = d/te = 1$ )	Ke= 1.30	2.85 kN/mm
Rigidità verticale	Kv= 768	1794 kN/mm



Il modulo tangenziale dinamico e quindi la rigidezza orizzontale equivalente sono definiti per una deformazione di taglio  $\gamma = \frac{\Delta}{t_e} = 1$ ;  $G_{din}$  varia molto nel campo  $\gamma < 1$  mentre per valori  $1 < \gamma < 2$  risulta pressoché costante; la variazione media del modulo dinamico tangenziale in funzione della deformazione di taglio è rappresentata nel seguente grafico.



## 4 ANALISI DEI CARICHI

Qui di seguito si riporta l'analisi dei carichi eseguita distinguendo tra carichi permanenti ed accidentali.

### 4.1 CARICHI PERMANENTI

a) calcestruzzo pile:

- peso specifico:  $\gamma = 25 \text{ kN/m}^3$
- massa specifica:  $m = 2.55 \text{ t/m}^3$

b) acciaio strutturale: si definisce un incremento forfetario del 15% per tener conto degli elementi non rappresentati nel modello (piastrame, bulloneria, saldature, ecc.):

- peso specifico:  $\gamma = 88.5 \text{ kN/m}^3$
- massa specifica:  $m = 9.00 \text{ t/m}^3$

c) peso soletta: il peso viene assegnato sulle singole travi (sezione reagente solo acciaio):

- travi esterne:  $g_1 = 25 \times (0.30 + 0.33) / 2 \times (1.35 + 1.60) = 23.23 \text{ kN/m}$
- travi interne:  $g_1 = 25 \times (0.33 + 0.41) / 2 \times 3.20 = 29.60 \text{ kN/m}$

### 4.2 SOVRACCARICHI PERMANENTI (FINITURE)

Le finiture vengono assegnate sulle singole travi (con soletta collaborante) con la loro eccentricità Y rispetto all'asse trave:

- trave esterna:

- pavimentazione:	$g_2 = 2.5 \times 1.05 = 2.63 \text{ kN/m}$	$e = -1.075 \text{ m}$
- cordolo:	$g_2 = 25 \times 0.15 \times 1.90 = 7.13 \text{ kN/m}$	$e = +0.40 \text{ m}$
- veletta:	$g_2 = 25 \times 0.1 \times 0.8 = 2 \text{ kN/m}$	$e = +1.375 \text{ m}$
- sicurvia:	$g_2 = 0.8 \text{ kN/m}$	$e = -0.15 \text{ m}$
- parapetto:	$g_2 = 0.5 \text{ kN/m}$	$e = +1.25 \text{ m}$
- Totale finiture:	$g_2 = 13.06 \text{ kN/m}$	$e = +0.25 \text{ m}$
- travi interne: - pavimentazione:	$g_2 = 2.5 \times 3.2 = 8 \text{ kN/m}$	$e = 0$

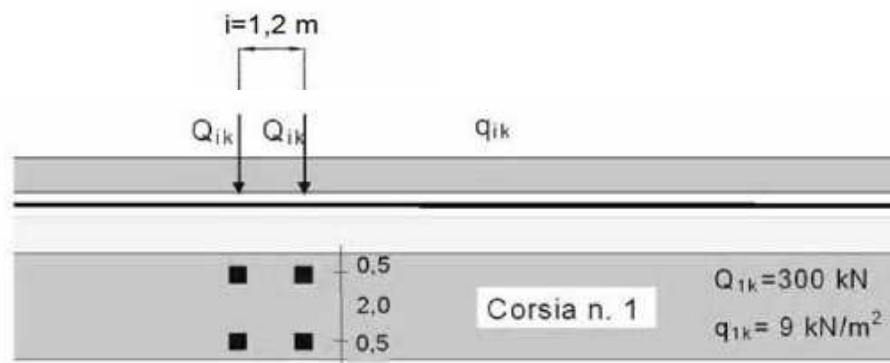
### 4.3 CARICHI MOBILI DI ESERCIZIO

L'impalcato si considera caricato secondo lo schema di carico 1 previsto dal D.M. 14/02/2008:

- una colonna di carico costituita da:

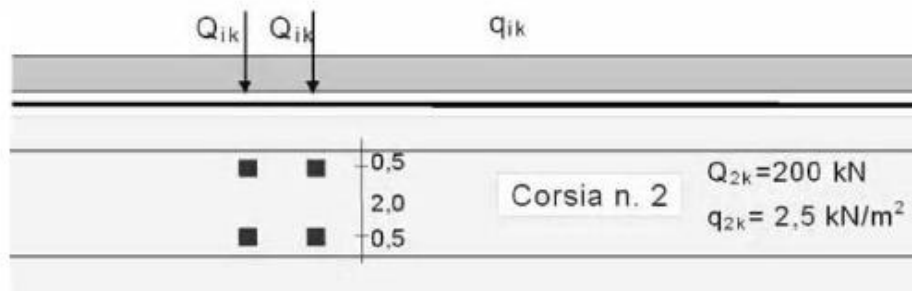
- $Q_{1k}$  mezzo convenzionale da 600 kN a due assi
- $q_{1k}$  carico ripartito pari a  $9 \text{ kN/m}^2$  ( $27 \text{ kN/m}$ )

Lo schema longitudinale della colonna è il seguente:



- una seconda colonna di carico analoga alla precedente ma con carichi pari a:

- $Q_{2k}$  mezzo convenzionale da 400 kN a due assi
- $q_{2k}$  carico ripartito pari a  $2.5 \text{ kN/m}^2$  ( $7.5 \text{ kN/m}$ )



L'area rimanente viene caricata con un carico  $q_{rk}$  pari a  $2.5 \text{ kN/m}^2$ .

Sui marciapiedi si assume lo schema di carico 5, pari a  $5.0 \text{ kN/m}^2$  che, in combinazione con i precedenti, assume il valore di  $2.5 \text{ kN/m}^2$ .

Lo schema complessivo di carico è rappresentato nelle seguenti figure, fermo restando che le corsie di carico possono essere scambiate o trascurate se più gravose per l'elemento considerato.

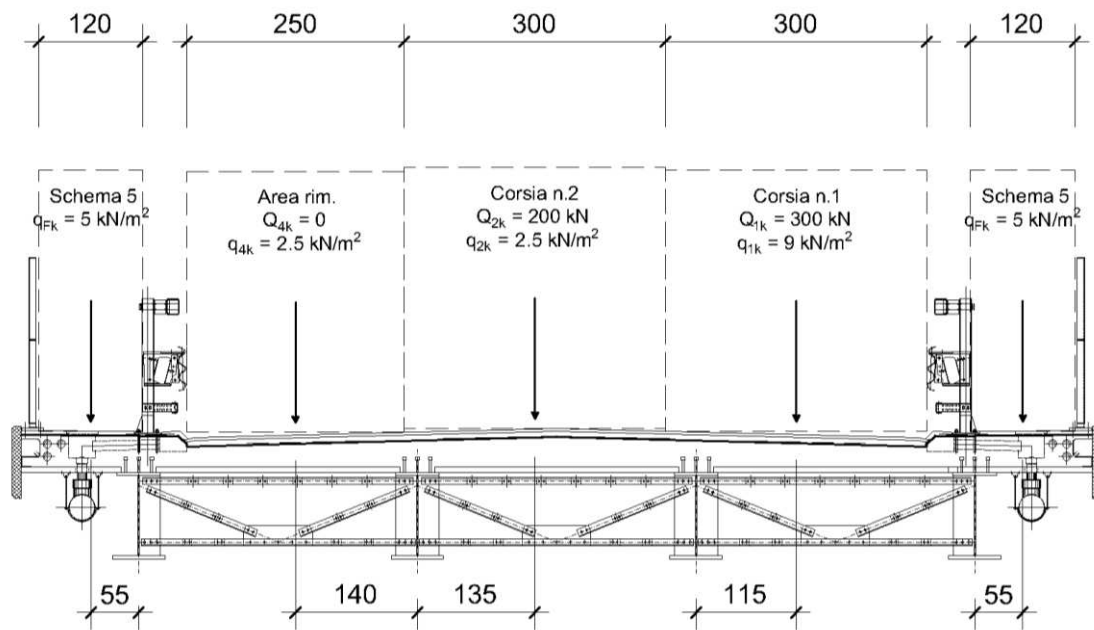


FIGURA 5 – CARICHI VARIABILI – DISPOSIZIONE A DX

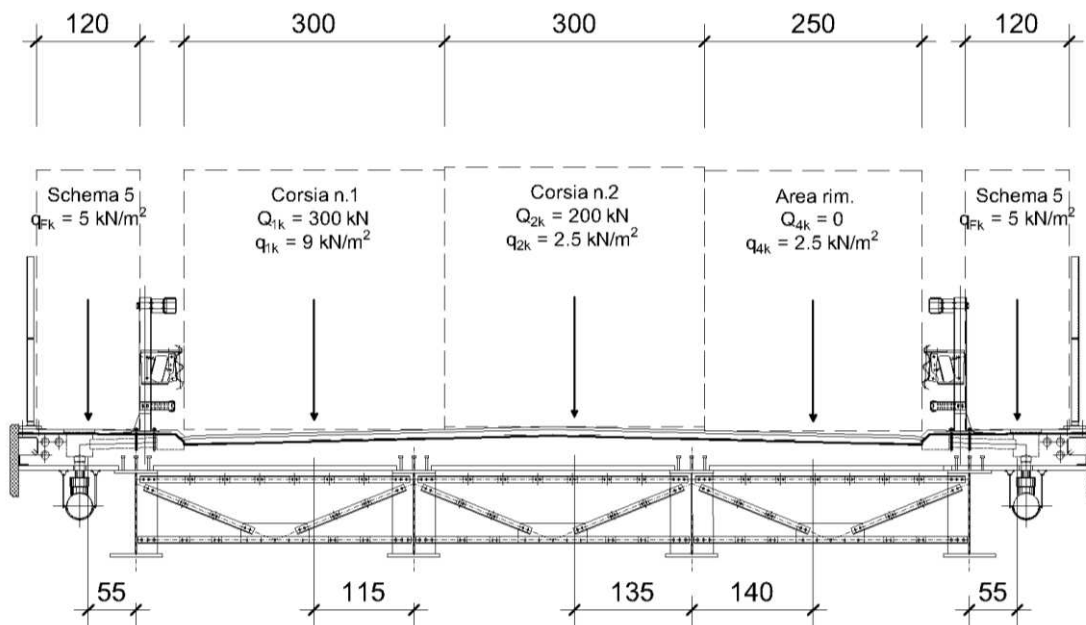


FIGURA 6 – CARICHI VARIABILI – DISPOSIZIONE A SX

Il D.M. del 2008, in accordo con quanto previsto dagli eurocodici, considera il coefficiente dinamico già compreso nel valore dei carichi mobili. La disposizione longitudinale e trasversale più gravosa dei carichi viene determinata automaticamente dal programma di calcolo per ogni sezione e componente di sollecitazione massima e minima.

Per la valutazione degli effetti dei carichi mobili agli stati limite di esercizio si applicano ai carichi i coefficienti di combinazione (NTC2008):

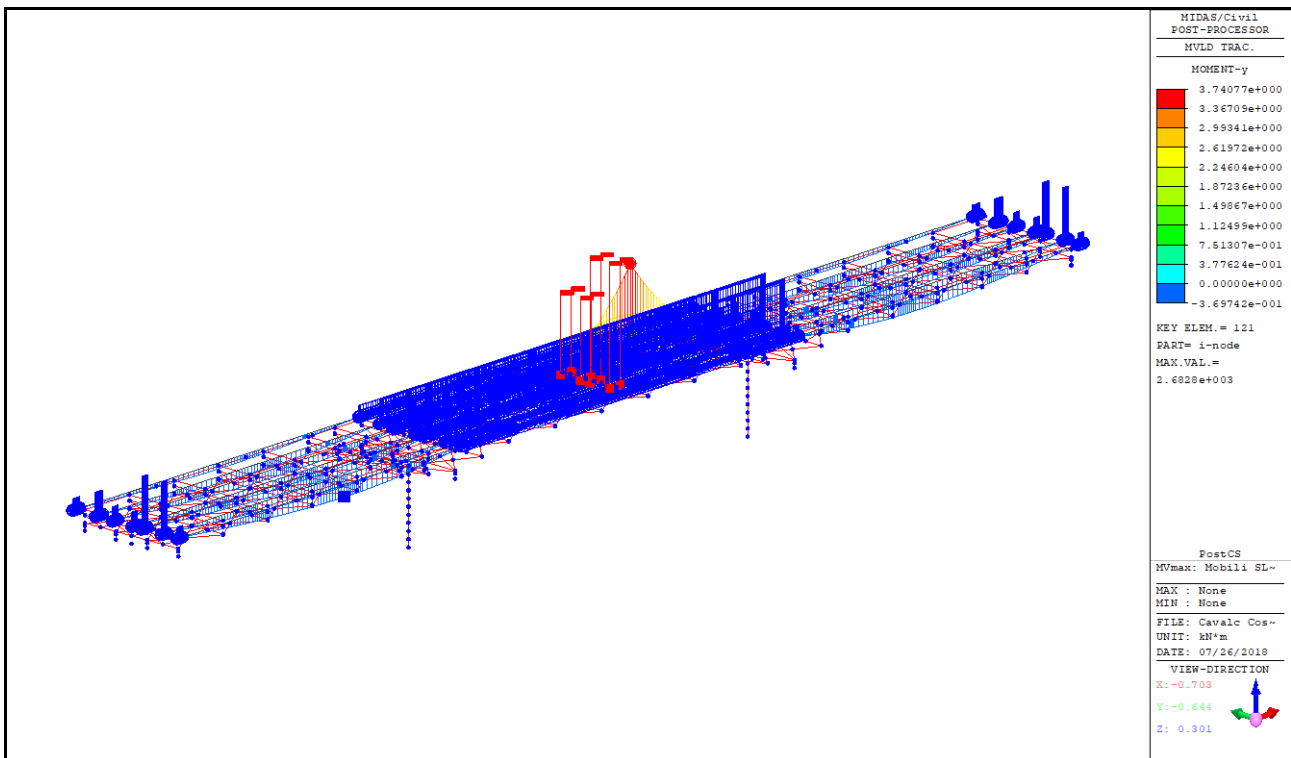
- Carichi tandem:  $\psi_0 = 0.75$        $\psi_1 = 0.75$        $\psi_2 = 0$
- Carichi distribuiti:  $\psi_0 = 0.4$        $\psi_1 = 0.4$        $\psi_2 = 0$

L'applicazione dei carichi mobili al modello strutturale è composta dei seguenti passi:

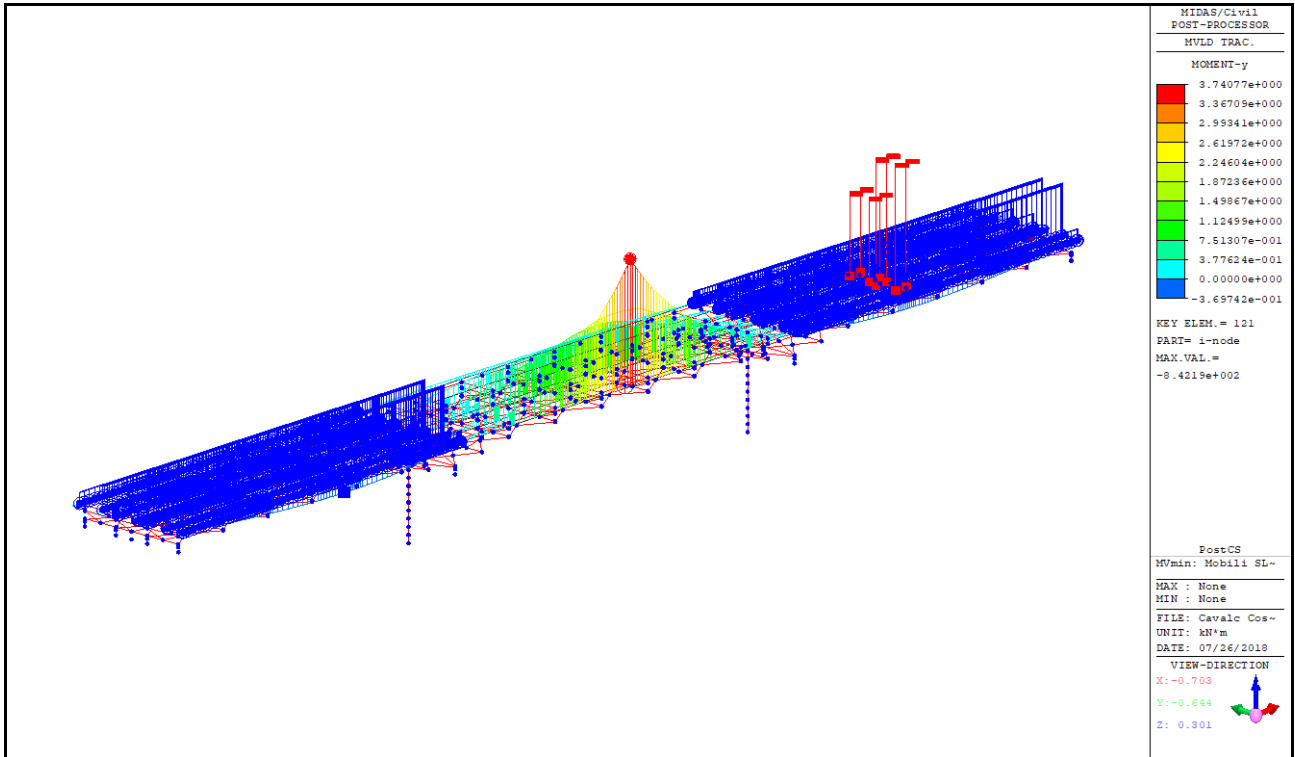
- definizione delle corsie di carico (Traffic Line Lanes) come sequenza di elementi (travi composte), caricate con la effettiva eccentricità trasversale;
- definizione degli schemi di carico transitanti sulle corsie e/o marciapiedi (Vehicles)
- definizione dei casi di carico (Moving Load Cases); per ognuno di essi si definiscono le corsie da caricare e gli schemi di carico transitanti e se considerare i coefficienti  $\psi$  (SLE) o no (SLU);
- definizione della discretizzazione dei punti di carico, assunto pari a 1/10 della lunghezza del singolo elemento caricato, nel presente caso quindi 25÷50 cm.

Nel calcolo vengono quindi generate le linee d'influenza per ogni colonna di carico, ognuna delle quali provoca il massimo o il minimo di una singola componente di sforzo (N,  $V_y$ ,  $V_z$ ,  $M_x$ ,  $M_y$ ,  $M_z$ ) per un nodo di un elemento del modello; ciò è esteso a tutti i nodi ed elementi presenti nel modello di calcolo; ovviamente se viene ricercato il massimo vengono caricati solo i tratti aventi linea d'influenza positiva e viceversa per i minimi.

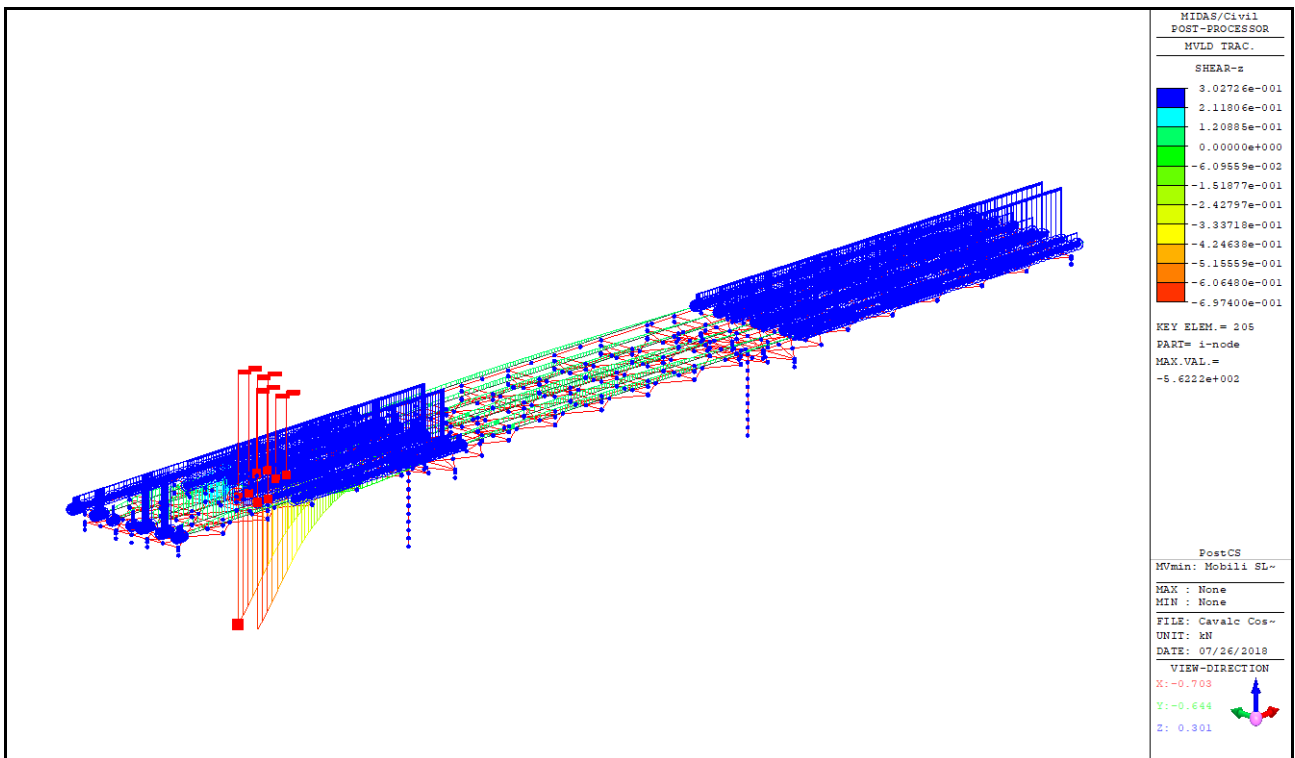
Seguono alcuni esempi grafici.



CARICHI MOBILI SLU A DX - DISPOSIZIONE MASSIMO  $M_y$  ELEMENTO 121, NODO INIZIALE (TRAVE ESTERNA MEZZERIA)



CARICHI MOBILI SLU A DX - DISPOSIZIONE MINIMO My ELEMENTO 121, NODO INIZIALE (TRAVE ESTERNA MEZZERIA)



CARICHI MOBILI SLU A DX - DISPOSIZIONE MINIMO Fz ELEMENTO 205, NODO INIZIALE (TRAVE INTERNA CAMPATA LATERALE)

#### 4.4 CARICHI MOBILI PER VERIFICHE A FATICA

##### 4.4.1 VERIFICHE PER VITA ILLIMITATA

Le verifiche a fatica per vita illimitata (NTC 5.1.4.3) vengono eseguite per le anime e le saldature delle travi. L'impalcato si considera caricato secondo il modello di carico a fatica 2, applicato sulla corsia lenta; il veicolo più gravoso è risultato essere il 3°: semirimorchio a 5 assi per un peso complessivo di 630 kN.

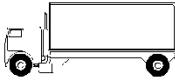




SAGOMA del VEICOLO		Distanza tra gli assi (m)	Carico frequente per asse (kN)	Tipo di ruota (Tab. 5.1.IX)
		4,5	90	A
			190	B
		4,20	80	A
		1,30	140	B
			140	B
		3,20	90	A
		5,20	180	B
		1,30	120	C
		1,30	120	C
		1,30	120	C
		3,40	90	A
		6,00	190	B
		1,80	140	B
			140	B
		4,80	90	A
		3,60	180	B
		4,40	120	C
		1,30	110	C
			110	C

FIGURA 7 – MODELLO DI CARICO AI FATICA N. 2

##### 4.4.2 VERIFICHE A DANNEGGIAMENTO

Le verifiche a danneggiamento si effettuano per i connettori trave/soletta (pioli Nelson).

L'impalcato si considera caricato secondo il modello di carico a fatica 3, applicato sulla corsia lenta; il modello consiste in 4 assi di 120 kN di peso (480 kN totali).



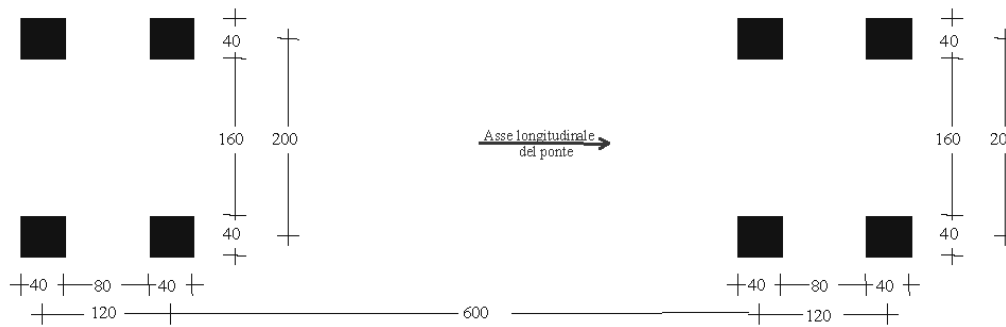


FIGURA 8 – MODELLO DI CARICO AI FATICA N.3

#### 4.5 AZIONE DEL VENTO TRASVERSALE

Nel D.M. 2008 si valuta l'azione del vento in base alla zona considerata e non ad un valore convenzionale; l'opera si trova in zona 1) Valle d'Aosta, Piemonte, Lombardia, Trentino Alto Adige, Veneto, Friuli Venezia Giulia (con l'eccezione della provincia di Trieste).



NTC 2008 - Figura 3.3.1

La pressione esterna è definita da:

$$p = q \cdot c_e \cdot c_p \cdot c_d$$

La pressione cinetica di riferimento  $q$  (in  $N/m^2$ ) in zona 1 è data dall'espressione:

$$q_b = \frac{1}{2} \cdot \rho \cdot v_b^2 = \frac{1}{2} \cdot 1.25 \cdot 25^2 = 391 \text{ N/m}^2 \text{ (0.391 kN/m}^2\text{)}$$

dove:

$\rho = 1.25 \text{ kg/m}^3$  densità dell'aria, assunta costante

$v_b(T_R)$  è la velocità di riferimento del vento (in m/s)

$v_{b,0} = 25 \text{ m/s}$  ( $a_s < a_0$ )

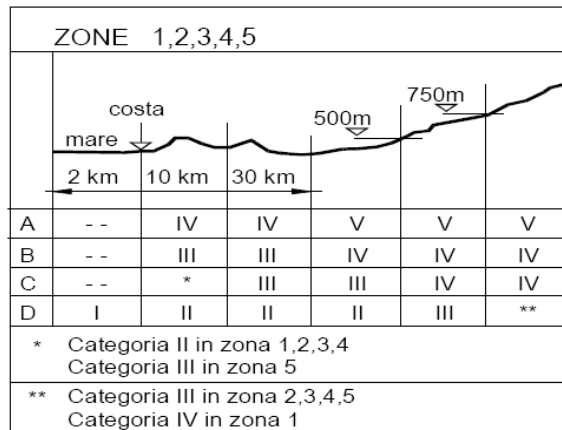
considerando un'altitudine sul livello del mare del sito:  $a_s = 70 \text{ m}$  e un periodo di ritorno di 50 anni.

$c_e$  è il coefficiente di esposizione; avendo una classe di rugosità D

*Classi di rugosità del terreno*

Classe di rugosità del terreno	Descrizione
A	Aree urbane in cui almeno il 15% della superficie sia coperto da edifici la cui altezza media superi i 15 m
B	Aree urbane (non di classe A), suburbane, industriali e boschive
C	Aree con ostacoli diffusi (alberi, case, muri, recinzioni ); aree con rugosità non riconducibile alle classi A, B, D
D	Aree prive di ostacoli (aperta campagna, aeroporti, aree agricole, pascoli, zone paludose o sabbiose, superfici innevate o ghiacciate, mare, laghi,....)

in zona 1, si ottiene una categoria di esposizione II



a cui corrispondono i seguenti parametri per la definizione del coefficiente di esposizione

*Parametri per la definizione del coefficiente di esposizione*

Categoria di esposizione del sito	$k_r$	$Z_0$ (m)	$Z_{min}$ (m)
I	0,17	0,01	2
II	0,19	0,05	4
III	0,20	0,10	5
IV	0,22	0,30	8
V	0,23	0,70	12

Con l'opera in costruzione avente altezza massima sul terreno (compresi carichi mobili):

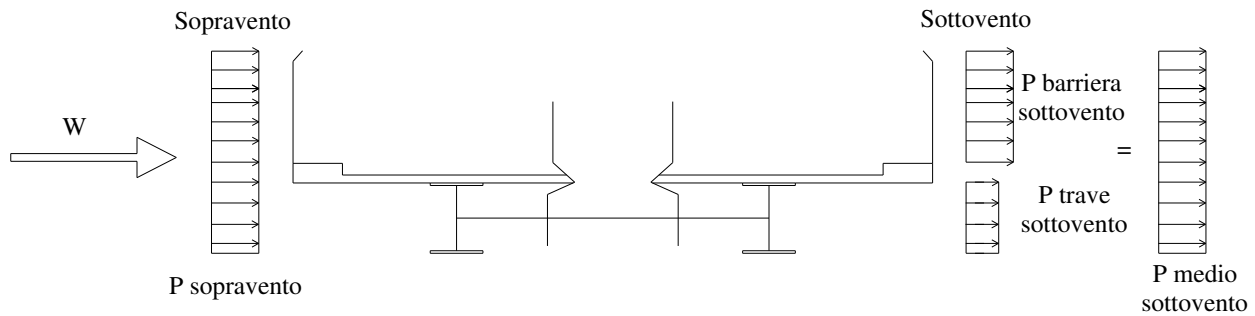
$$z \approx 12 \text{ m} > Z_{min} = 4 \text{ m}$$

$$c_e(z) = k_r^2 \cdot c_t \cdot \alpha \cdot [7 + c_t \cdot \alpha] = 0.19^2 \cdot 68.4 = 2.47$$

$$\text{dove } \alpha = \ln(z/z_0) = \ln(12/0.05) = 5.48; c_t = 1$$

$c_d$  è il coefficiente dinamico; posto pari ad uno

$c_p$  è il coefficiente di forma; nel caso di travi multiple (Circ. 02/02/2009, C3.3.10.4) si ha, per i vari elementi:



- travi principali, altezza  $h = 1$  m, distanza reciproca media  $d = 3.2$  m:

a) lato sopravvento:  $c_{p1} = 1.4$

b) lato sottovento:  $d / h = 3.2 / 1 = 3.2 \quad (2 < d/h < 5)$

$$\mu = 0.52$$

$$c_{p2} = 1.4 \times (0.52 + 0.52^2 + 0.52^3 + 0.52^4) = 1.41$$

- travi principali, altezza  $h = 2$  m, distanza reciproca media  $d = 3.2$  m:

a) lato sopravvento:  $c_{p1} = 1.4$

b) lato sottovento:  $d / h = 3.2 / 2 = 1.6 \quad (d/h < 2)$

$$\mu = 0.2$$

$$c_{p2} = 1.4 \times (0.2 + 0.2^2 + 0.2^3 + 0.2^4) = 0.35$$

- soletta, cordolo, sicurvia: per il solo lato sopravvento si ha:

$$c_p = 1.2$$

La pressione esterna sulle strutture è quindi definita dalle espressioni seguenti:

a) impalcato con travi  $h = 1$  m:

a1) lato sopravvento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (1.4 \cdot 1.0 + 1.2 \cdot 1.45) = 1.35 + 1.68 = 3.03 \text{ kN/m}$

con eccentricità rispetto al piano stradale di riferimento:

$$e_z = (-1.35 \cdot 0.90 + 1.68 \cdot 0.27) / 3.03 = -0.25 \text{ m}$$

a2) lato sottovento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (1.41 \cdot 1.0 + 1.2 \cdot 1.15) = 1.36 + 1.33 = 2.69 \text{ kN/m}$

con eccentricità rispetto al piano stradale:

$$e_z = (-1.36 \cdot 0.90 + 1.33 \cdot 1.15) / 2.69 = +0.11 \text{ m}$$

b) impalcato con travi  $h = 2$  m:

b1) lato sopravvento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (1.4 \cdot 2.0 + 1.2 \cdot 1.45) = 2.70 + 1.68 = 4.38 \text{ kN/m}$

con eccentricità rispetto al piano stradale di riferimento:

$$e_z = (-2.70 \cdot 1.40 + 1.68 \cdot 0.27) / 4.38 = -0.76 \text{ m}$$

b2) lato sottovento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (0.35 \cdot 2.0 + 1.2 \cdot 1.15) = 0.68 + 1.33 = 2.01 \text{ kN/m}$

con eccentricità rispetto al piano stradale:

$$e_z = (-0.68 \cdot 1.40 + 1.33 \cdot 1.15) / 2.01 = +0.29 \text{ m}$$

Sulle pile si ha:  $p = 0.391 \cdot 2.47 \cdot 1.50 = 1.45 \text{ kN/m}$

La pressione dovuta ai carichi mobili, per l'altezza pari a 2.0 vale:

$$p = 0.391 \cdot 2.47 \cdot 1.2 \cdot 2.00 = 2.32 \text{ kN/m}$$

con eccentricità rispetto al piano stradale:

$$e_z = 1 + 2.00 / 2 = +2.00 \text{ m}$$

Vento su pile:  $0.391 \times 1.2 \times 2.47 \times 1.5 = 1.74 \text{ kN/m}$

#### 4.6 AZIONE DI FRENAMENTO

L'azione di frenamento è funzione del carico verticale totale agente sulla corsia convenzionale n. 1 ed è uguale a:

$$F_f = 0.6 \times (2 \times Q_{1k}) + 0.10 q_{1k} \times w_1 \times L =$$

$$= 0.6 \times (2 \times 300) + 0.10 \times 9 \times 3 \times 101.60 = 634.3 \text{ kN} \quad (180 \text{ kN} \leq F_f \leq 900 \text{ kN})$$

L'azione si considera uniformemente distribuita sulle travi principali:

$$q_x = \frac{634.3}{4 \times 101.6} = 1.56 \text{ kN/m/trave}$$

#### 4.7 DILATAZIONE TERMICA DIFFERENZIALE

Si assume che le travi dell'impalcato (sezioni miste acciaio/cls.) siano soggette alla variazione termica differenziale:  $\Delta T/H = \pm 5^\circ$ .

#### 4.8 DILATAZIONE TERMICA UNIFORME

Si assume che le travi dell'impalcato (sezioni miste acciaio/cls.) siano soggette alla variazione termica uniforme:  $\Delta T = \pm 15^\circ$ .

#### 4.9 AZIONI SISMICHE

La stima dei parametri spettrali necessari per la definizione dell'azione sismica è stata effettuata utilizzando le informazioni disponibili nel reticolo di riferimento (tabella 1 – Allegato B – D.M. 14 gennaio 2008).

Considerando l'ubicazione del sito in oggetto (Lat: 45.1727; Long: 9.6368) ed ipotizzando una costruzione caratterizzata da:

- una vita nominale di 50 anni, ricadente in classe d'uso pari a IV (ponti di importanza critica per il mantenimento delle vie di comunicazione...);
- una categoria topografica T1;
- una categoria C per il sottosuolo;

Si hanno i seguenti valori dei **parametri spettrali**:

STATO LIMITE	$T_R$ [anni]	$a_g$ [g]	$F_o$ [-]	$T_C^*$ [s]
SLO	60	0.0375	2.575	0.226
SLD	101	0.0458	2.539	0.257
SLV	949	0.1003	2.541	0.297
SLC	1950	0.1273	2.528	0.305

Le espressioni dello spettro elastico  $S_e$  di risposta secondo le NTC-08 sono le seguenti:

$$\begin{aligned}
 0 \leq T < T_B & \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0 \cdot \left[ \frac{T}{T_B} + \frac{1}{\eta \cdot F_0} \left( 1 - \frac{T}{T_B} \right) \right] \\
 T_B \leq T < T_C & \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0 \\
 T_C \leq T < T_D & \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0 \cdot \left( \frac{T_C}{T} \right) \\
 T_D \leq T & \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0 \cdot \left( \frac{T_C T_D}{T^2} \right)
 \end{aligned}$$

Trattandosi di struttura sismicamente isolata si assume:

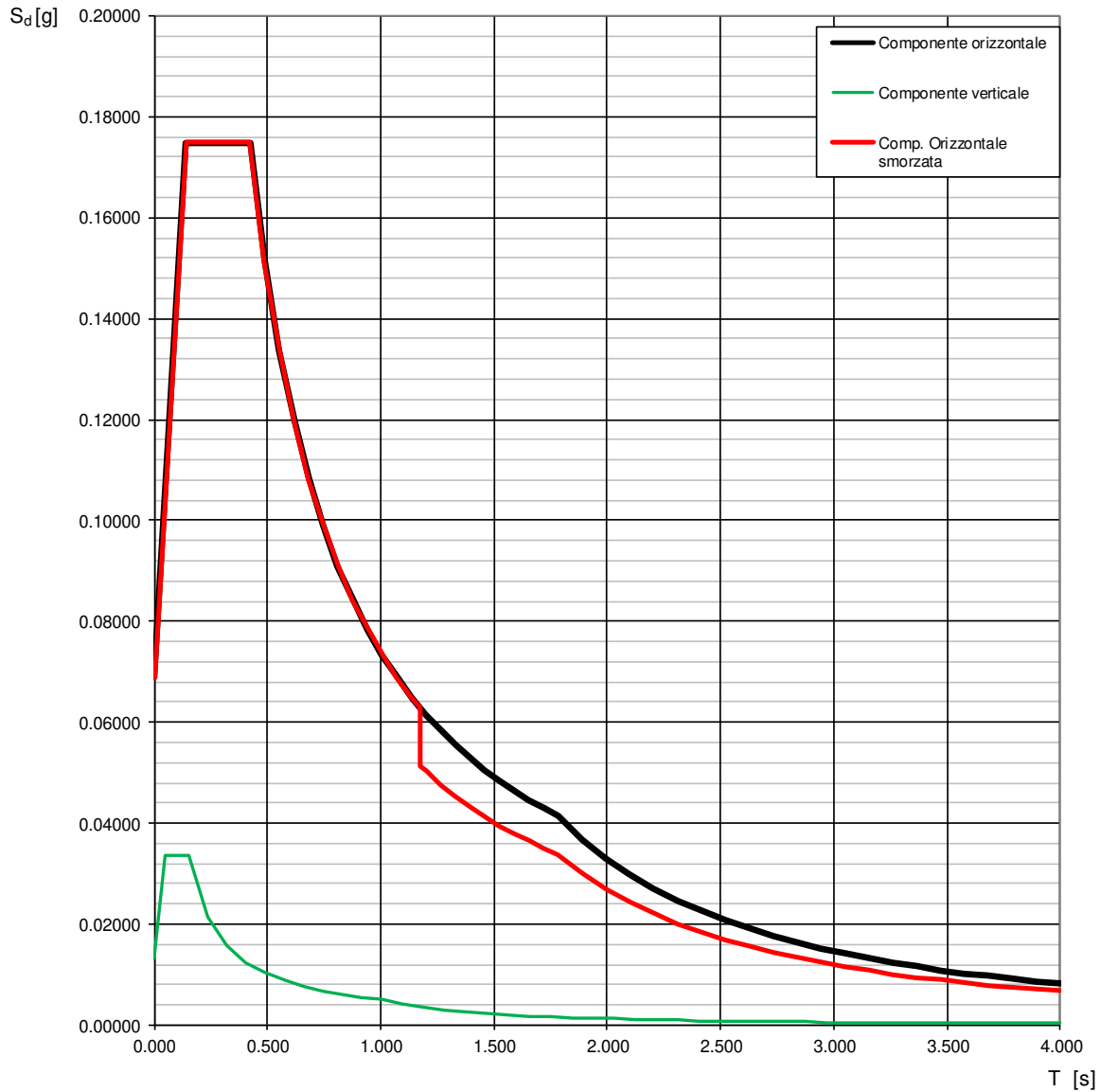
- coefficiente di smorzamento viscoso convenzionale:  $\xi = 5 \%$
- fattore di smorzamento viscoso:  $\eta = \sqrt{\frac{10}{5 + \xi}} = 1$
- fattore di struttura:  $q = 1$

Lo spettro elastico definito viene ridotto per tutto il campo di periodi  $T \geq 0,8T_{is}$ , assumendo:

- periodo di vibrazione struttura isolata:  $T_{is} = 1.464 \text{ s}$
- coefficiente di smorzamento viscoso isolatori:  $\xi = 10 \%$
- coefficiente riduttivo  $\eta = \sqrt{\frac{10}{5 + \xi}} = \sqrt{\frac{10}{5 + 10}} = 0.816 \quad (\geq 0.55)$ .

Si ottengono i seguenti andamenti degli spettri. Noto il periodo (ascissa) si ricava il relativo coefficiente sismico (ordinata).

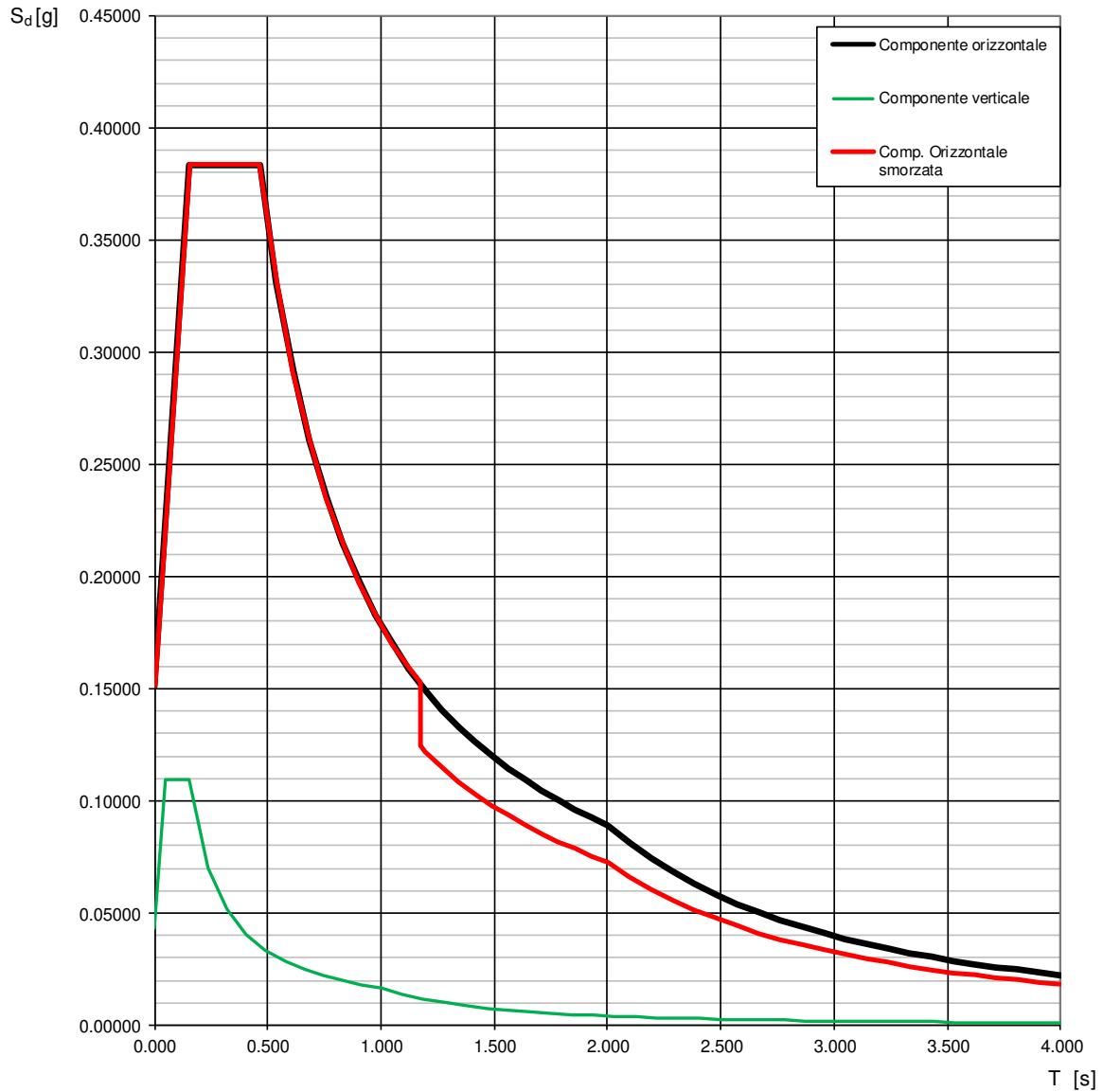
Spettri di risposta elastici (componenti orizz. e vert.) per lo stato limite: **SLD**



Spettri di risposta elastici e smorzati SLD

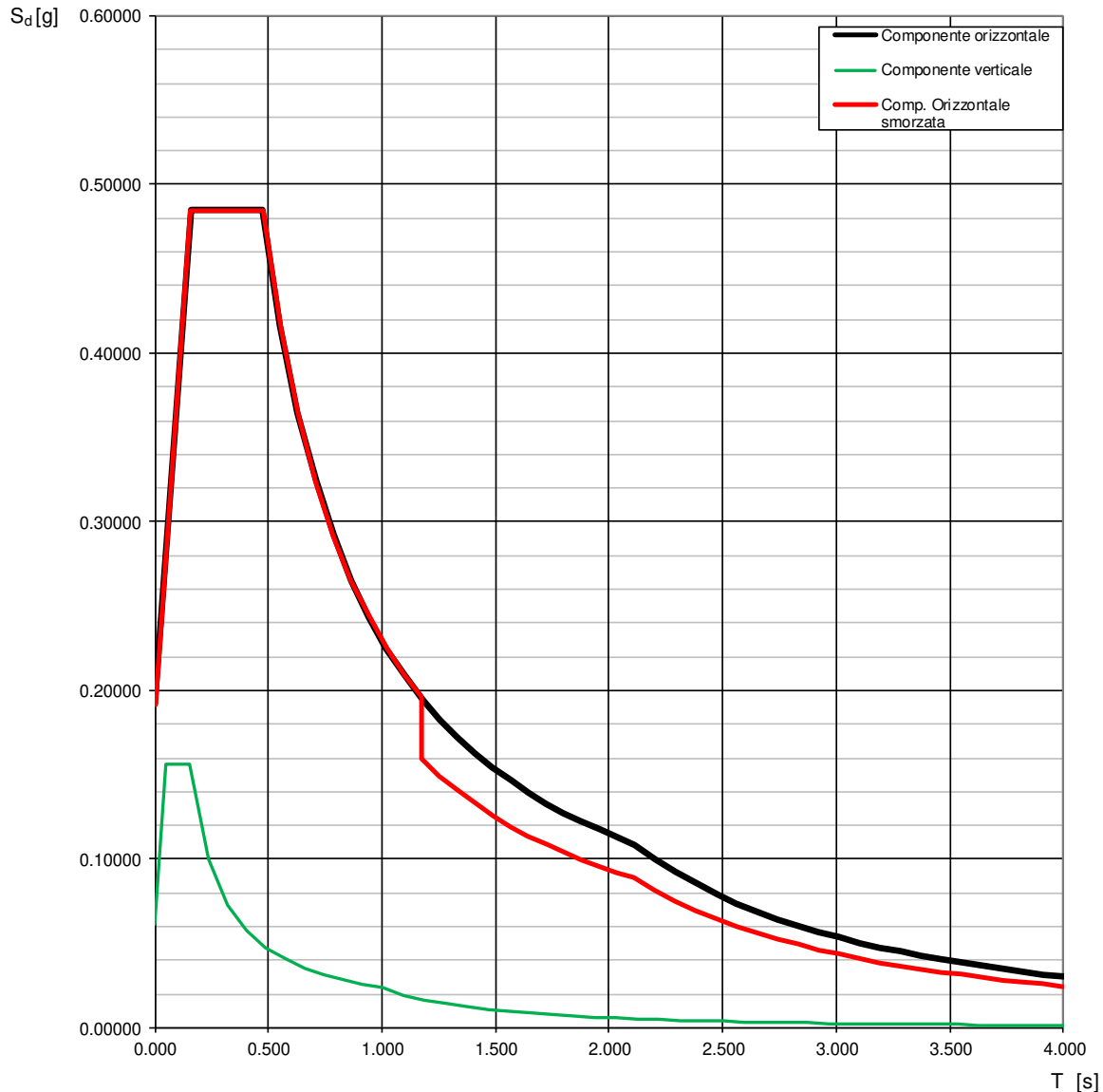


Spettri di risposta elastici (componenti orizz. e vert.) per lo stato limite: **SLV**



Spettri di risposta elastici e smorzati SLV

Spettri di risposta elastici (componenti orizz. e vert.) per lo stato limite: **SLC**



Spettri di risposta elastici e smorzati SLC

**4.10 FENOMENI DEFORMATIVI LENTI DELLA SOLETTA IN CALCESTRUZZO**

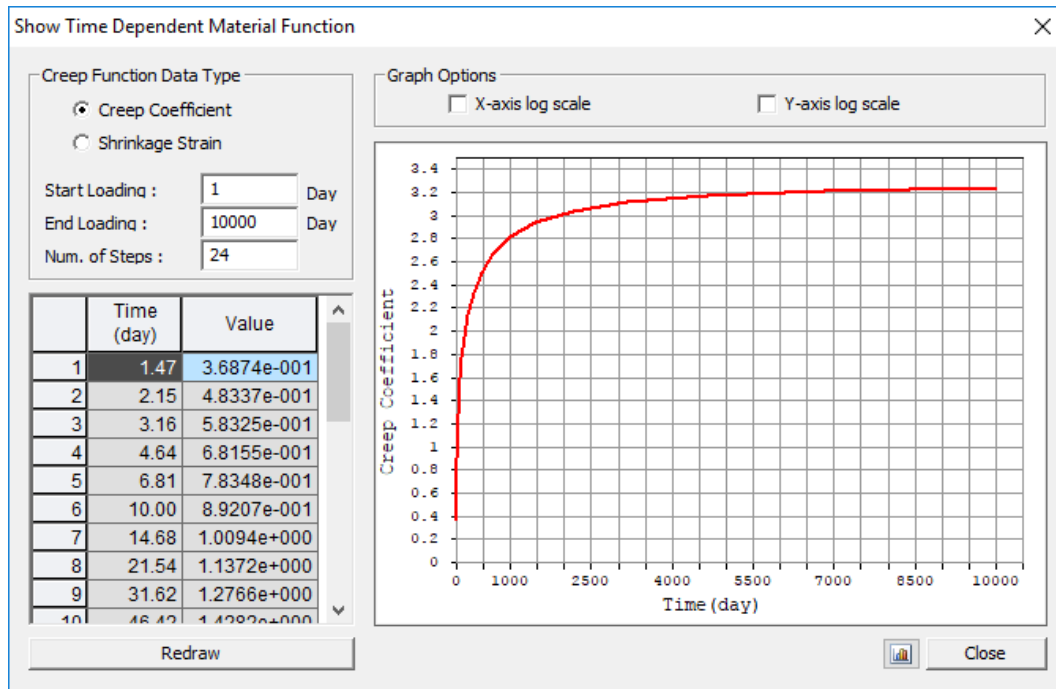
Come già detto, l'influenza di ritiro e fluage della soletta sulla struttura metallica viene considerata automaticamente dal programma di calcolo sulla base delle relazioni stabilite dal codice EN 1992-2 con i seguenti dati:

- resistenza caratteristica cilindrica a 28 gg:  $f_{ck} = 32 \text{ N/mm}^2$
- umidità relativa ambiente: 70 %
- perimetro esposto all'atmosfera:  $h = 2 \times A_c / u = 0.30 \text{ m}$
- calcestruzzo a indurimento normale: N
- età del calcestruzzo iniziale per il ritiro: 1 gg

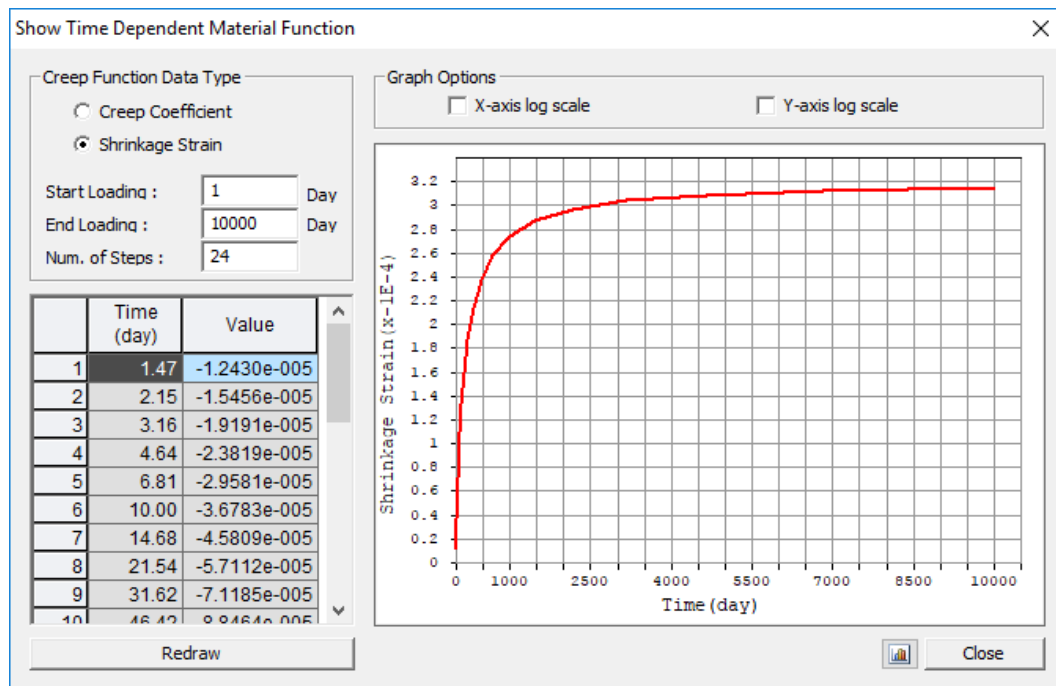
La variazione della resistenza caratteristica del calcestruzzo nel tempo viene determinata con la formula:

$$f_c(t) = f_{ck} \times e^{0,25 \times \left[ 1 - \left( \frac{28}{t} \right)^{0,5} \right]}$$

Si ottengono i seguenti diagrammi di fluage e ritiro:



CREEP (FLUAGE)



SHRINKAGE (RITIRO)

## 5 COMBINAZIONI DI CARICO

I coefficienti moltiplicativi delle singole azioni sono riassunti nelle tabelle seguenti, per le condizioni di carico statiche e sismiche.

Nelle seguenti tabelle i carichi permanenti (pesi e finiture) non appaiono esplicitamente in combinazione perché il loro contributo alle sollecitazioni deriva dagli schemi statici in fase costruttiva (diversi che nella fase di esercizio) e sono quindi accumulati nei "Dead Load" e "Erection Load 1".

Le combinazioni di carico "GEN" (general) sono utilizzate per le reazioni, gli appoggi e comunque tutto ciò che non è di acciaio e/o composto acciaio/cls.

Le combinazioni di carico "ST" sono viceversa utilizzate per le strutture in acciaio e/o acciaio/cls.

Ogni combinazione di verifica viene generata in rispetto dei par. 2.5.3, 5.1.3.12 e 7.3.5 delle NTC 2008; segue una descrizione dettagliata di esempio.

Combinazione: "ST SLU Mobili": combinazione di verifica strutturale allo SLU in cui i "Carichi Mobili" sono il carico variabile dominante mentre gli altri (Vento, Frenamento, Termici) sono assunti attraverso i coefficienti di combinazione  $\psi_0$ .

Preliminarmente per i carichi variabili che possono avere segno opposto si generano le combinazioni "negative" che vengono poi involuppate in modo da ottenere i massimi e i minimi.

- 1) "Termico unif. -15°": combinazione: "Termico unif. +15°" x (-1)
- 2) "Termico diff. -5°/H": combinazione: "Termico diff. +5°/H" x (-1)
- 6) "Termico unif. INV": involuppo delle comb. "Termico unif. +15°" e "Termico unif. -15°"
- 7) "Termico diff. INV": involuppo delle comb. "Termico diff. +5°/H" e "Termico diff. -5°/H"

Analogamente si involuppano le condizioni dei carichi mobili disposti "a sinistra" e "a destra":

- 11) "Mobili SLU INV": involuppo delle comb. "Mobili SLU Dx" e "Mobili SLU Sx"

La generazione della combinazione di cui sopra comprende quindi la somma dei seguenti casi di carico (riferimento alle tabelle NTC 2008):

- "Dead Load" x 1.35: pesi strutturali permanenti (tab. 5.1.V)
- "Erection Load 1" x 1.5: peso finiture (permanenti non strutturali), (tab. 5.1.V)
- "Creep secondary" x 1.2: parte reale (iperstatica) dell'effetto del fluage della soletta in cls. (tab. 5.1.V)
- "Shrinkage secondary" x 1.2: parte reale (iperstatica) del ritiro della soletta in cls. (tab. 5.1.V)
- "Mobili SLU INV" x 1.35: involuppo carichi mobili; carico variabile dominante (tab. 5.1.V)
- "Vento strutture" x (1.5 x 0.6) = 0.9: carico variabile (tab. 5.1.V e tab. 5.1.VI)
- "Vento Mobili" x (1.5 x 0.6) = 0.9: carico variabile (tab. 5.1.V e tab. 5.1.VI)
- "Termico unif. INV" x (1.2 x 0.6) = 0.72: carico variabile (tab. 5.1.V e tab. 5.1.VI)
- "Termico diff. INV" x (1.2 x 0.6) = 0.72: carico variabile (tab. 5.1.V e tab. 5.1.VI)

La combinazione così composta è di tipo "Strength/Stress", utilizzata per le verifiche allo SLU; le altre combinazioni dello stesso tipo sono ottenute per rotazione del carico variabile dominante.







## 6 CALCOLO DELLE SOLLECITAZIONI

Le travi composte acciaio-calcestruzzo vengono analizzate dal programma secondo le fasi di costruzione (construction stage):

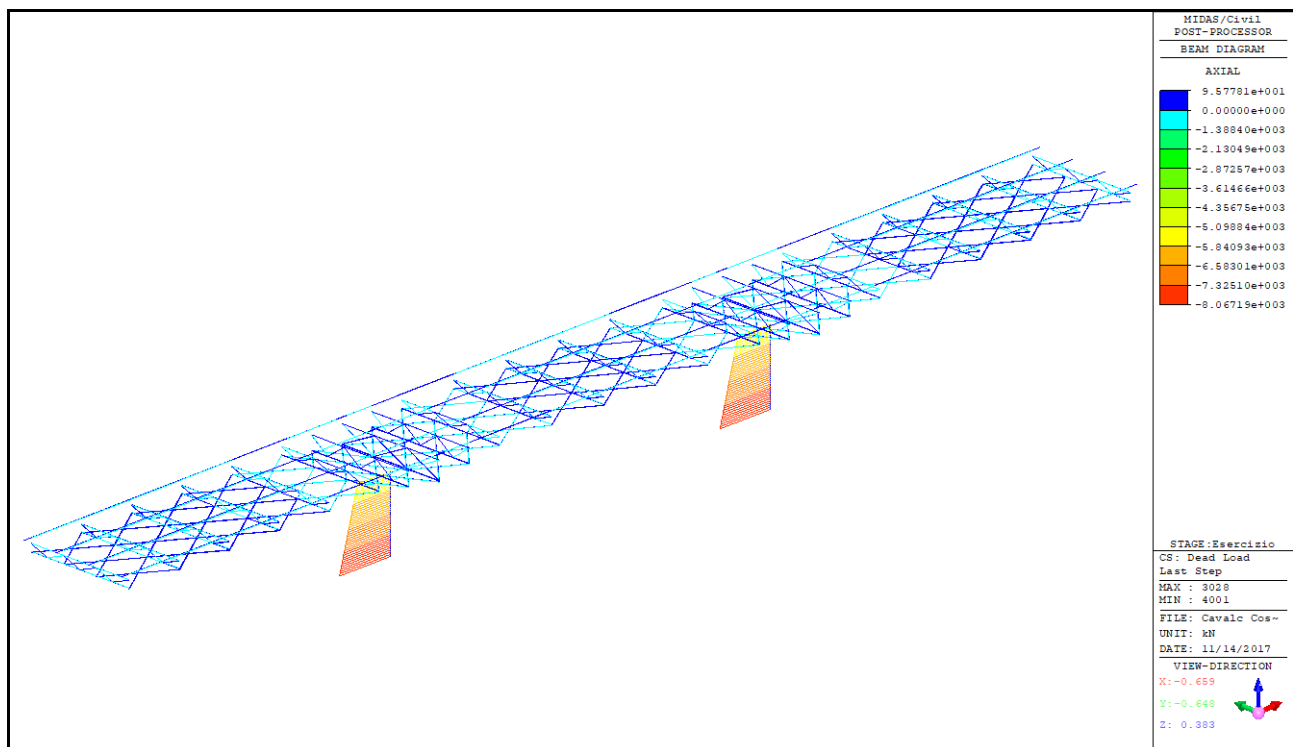
- fase 1: corrisponde alla fase costruttiva fino al getto della soletta (30 giorni):
  - sezione reagente: trave di acciaio (soletta non reagente)
  - carichi applicati: peso strutture acciaio e peso soletta
- fase 2: corrisponde a una fase costruttiva di lunga durata (10000 giorni) in cui la soletta ha caratteristiche elastiche variabili nel tempo e si tiene conto dei fenomeni di interazione lenti di ritiro e fluage, valutati secondo gli schemi EN 1992-2:
  - sezione reagente: trave di acciaio e soletta (con modulo variabile)
  - carichi applicati: finiture, ritiro, fluage della soletta
- fase 3: corrisponde alla fase di esercizio:
  - sezione reagente: trave acciaio + soletta (modulo costante)
  - carichi applicati: carichi di esercizio (vento, carichi mobili, termici, frenamento, sisma)

Si è assunta un'accelerazione gravitazionale pari a  $g = 9,806 \text{ m/s}^2$ .

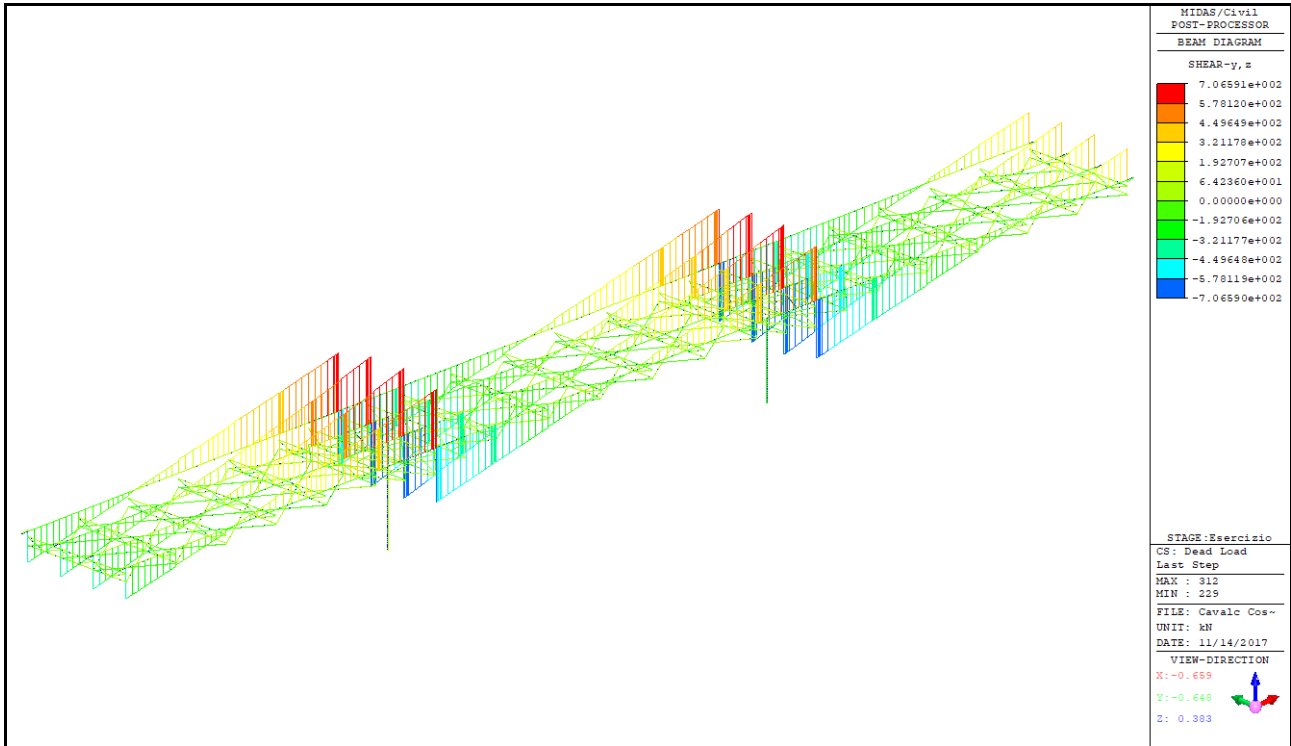
Le unità di misura utilizzate sono coerenti con il Sistema Internazionale:

lunghezze: m (metri)  
 masse: t (tonnellate)  
 forze: kN (kilo-Newton)

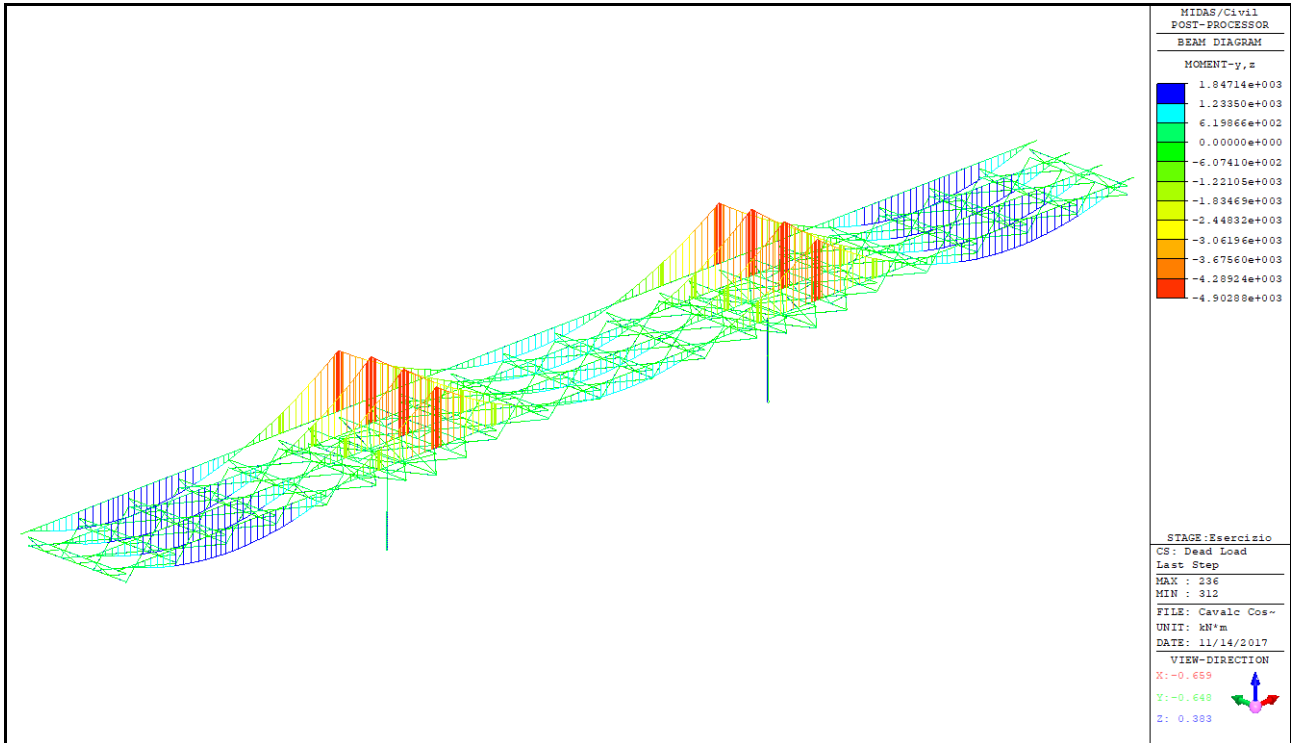
Seguono diagrammi delle azioni calcolate sulle travi composte dell'impalcato e le pile per le singole condizioni di carico (variabili involuppati  $\pm$ ).



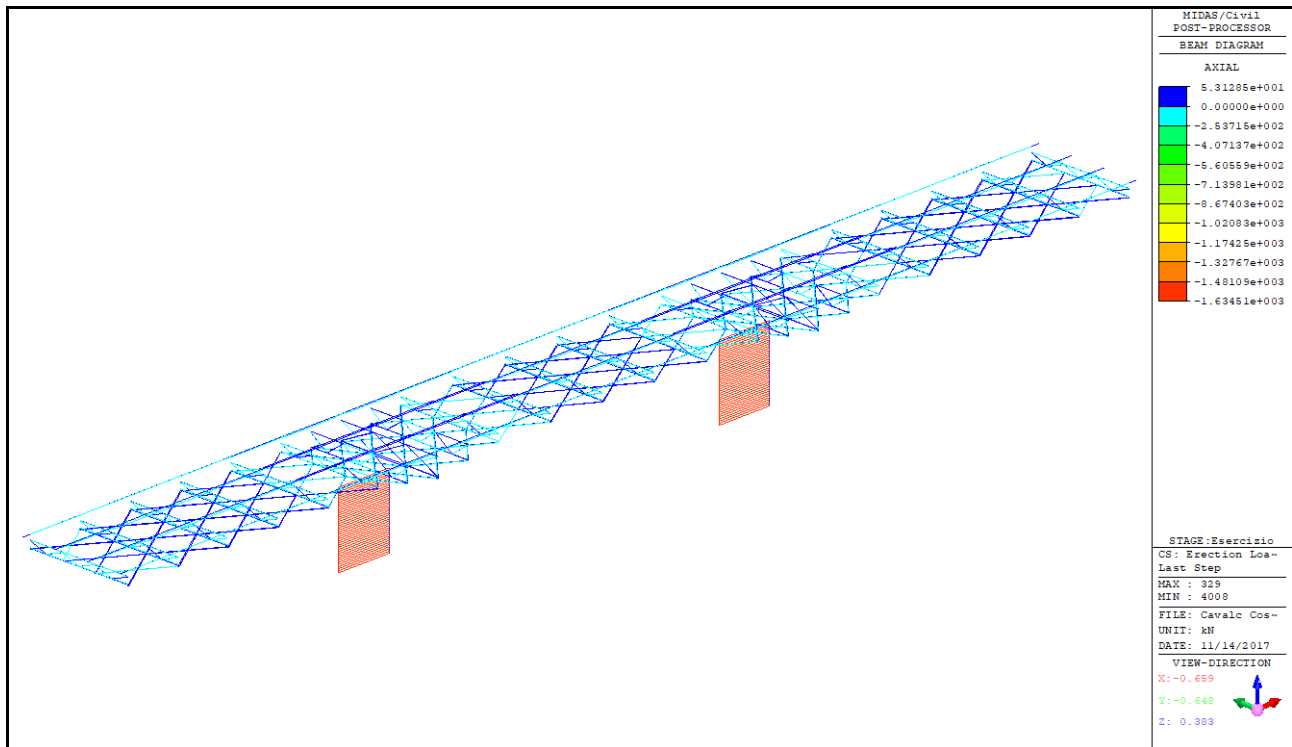
Peso travi e soletta (dead loads) - Diagramma sforzi assiali



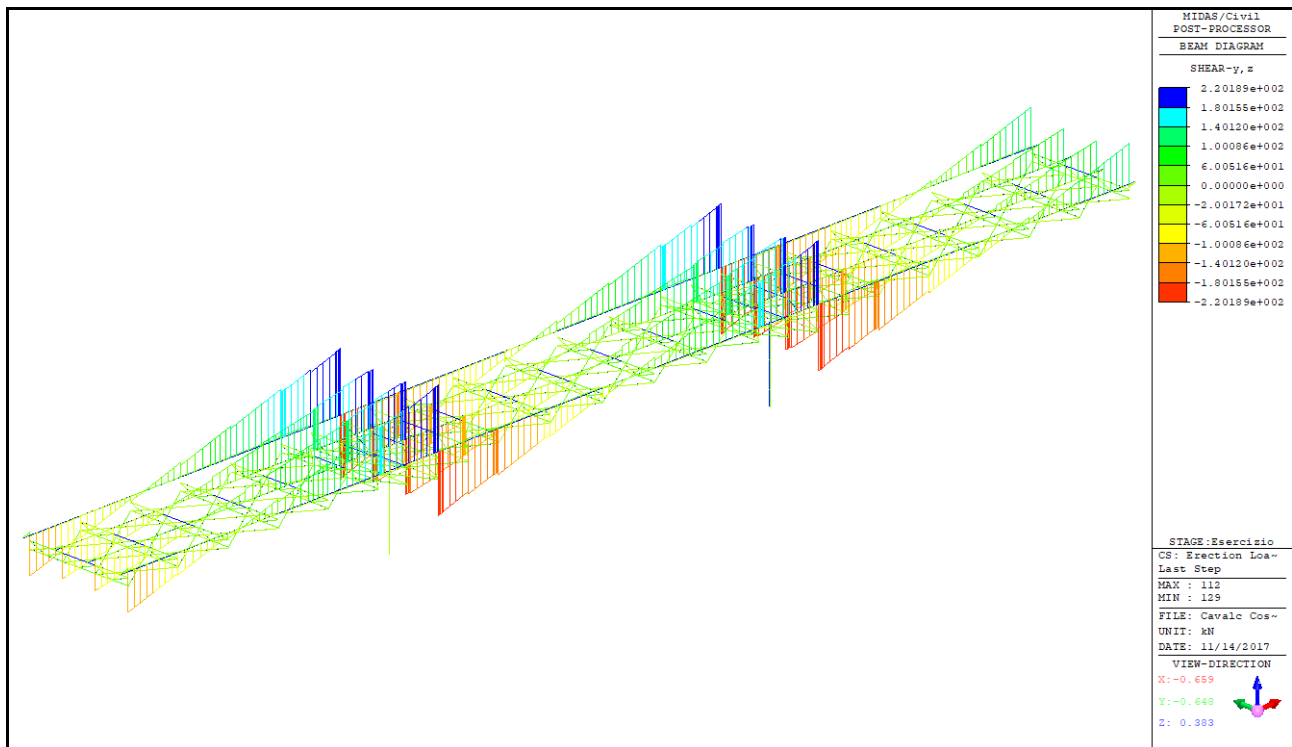
Peso travi e soletta (dead loads)- Diagramma sforzi taglianti



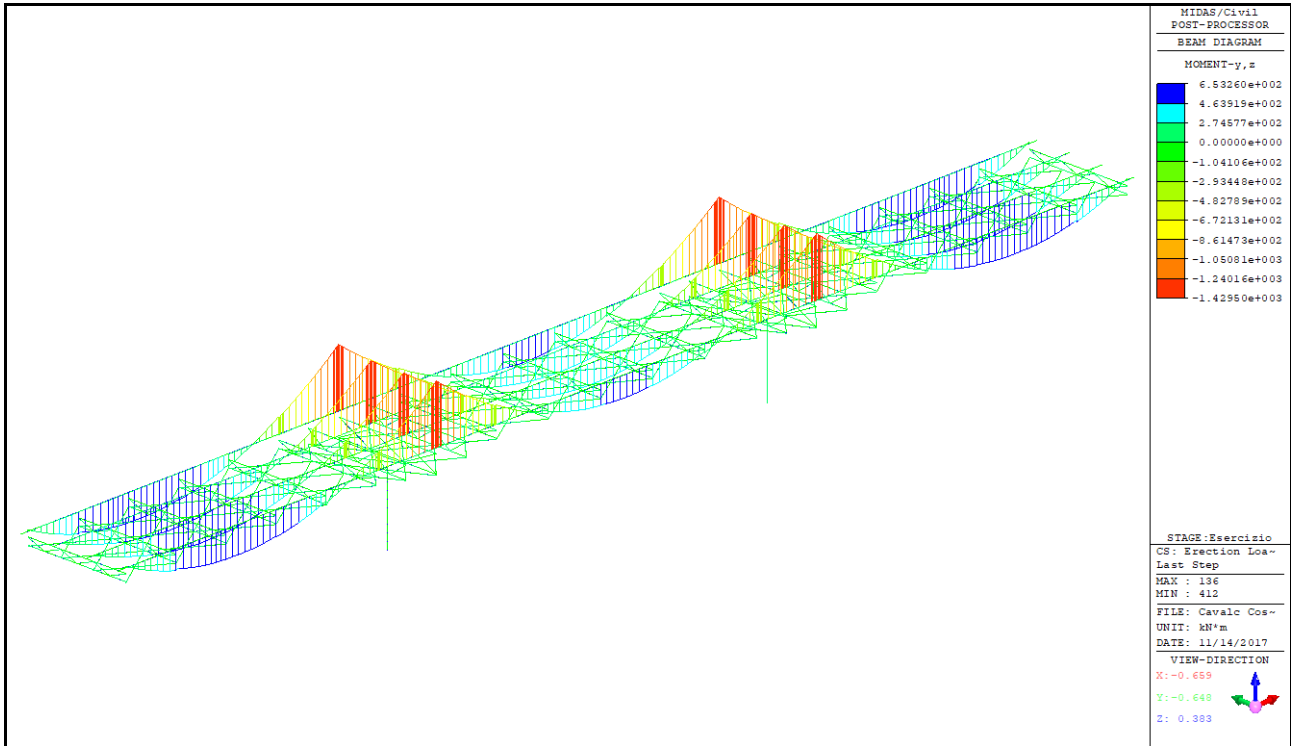
Peso travi e soletta (dead loads) - Diagramma momenti flettenti



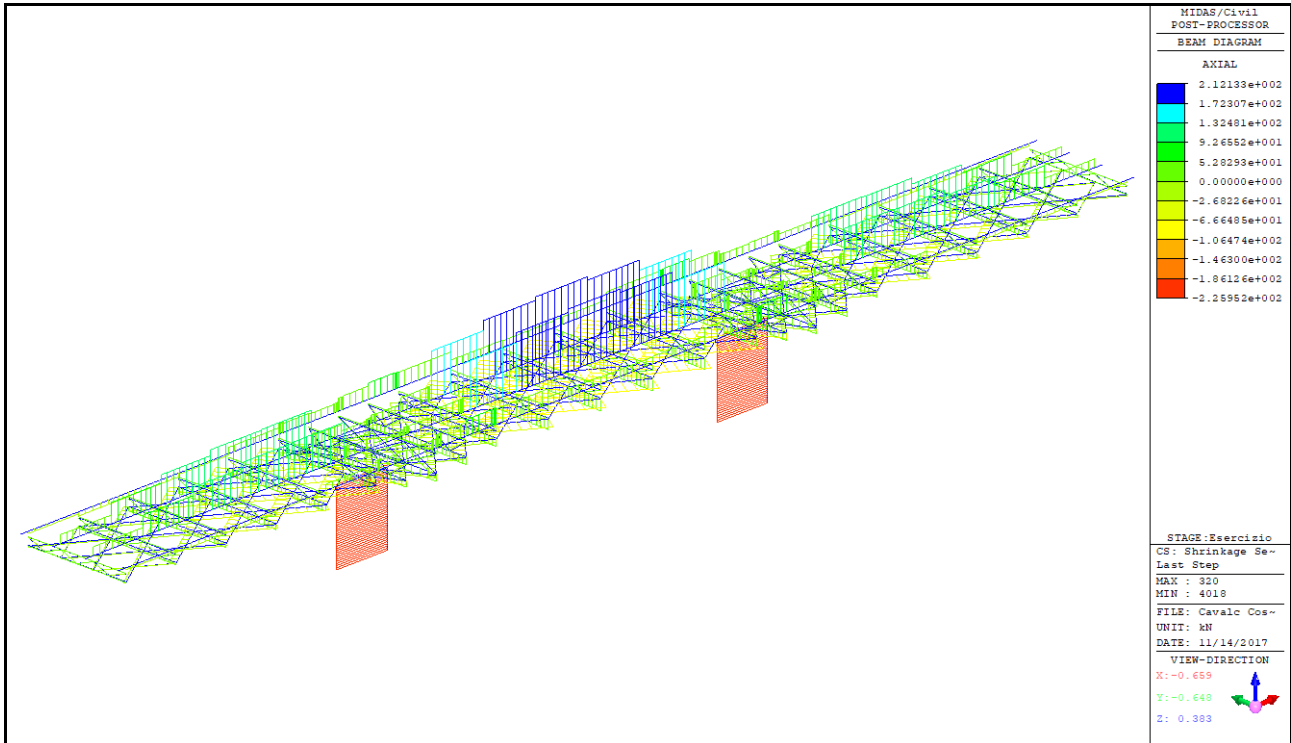
Finiture – Diagramma sforzi assiali



Finiture – Diagramma sforzi taglianti

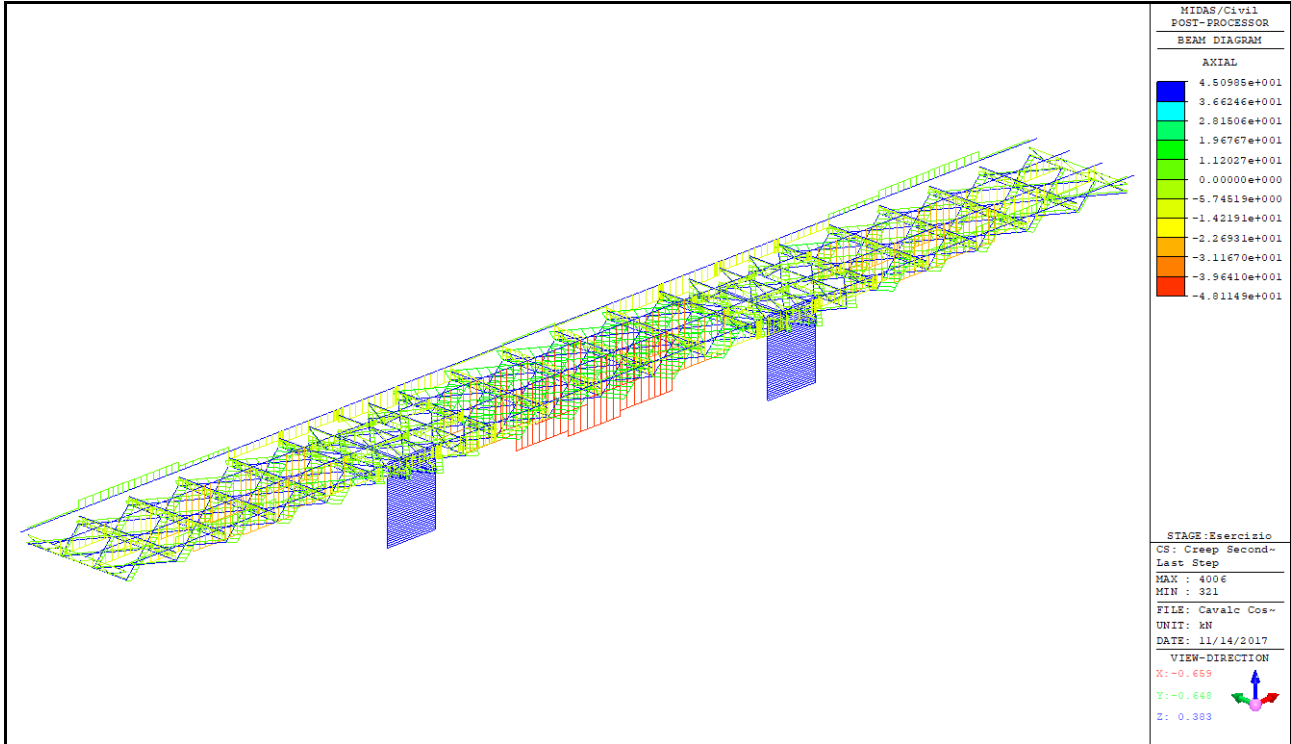


Finiture – Diagramma momenti flettenti

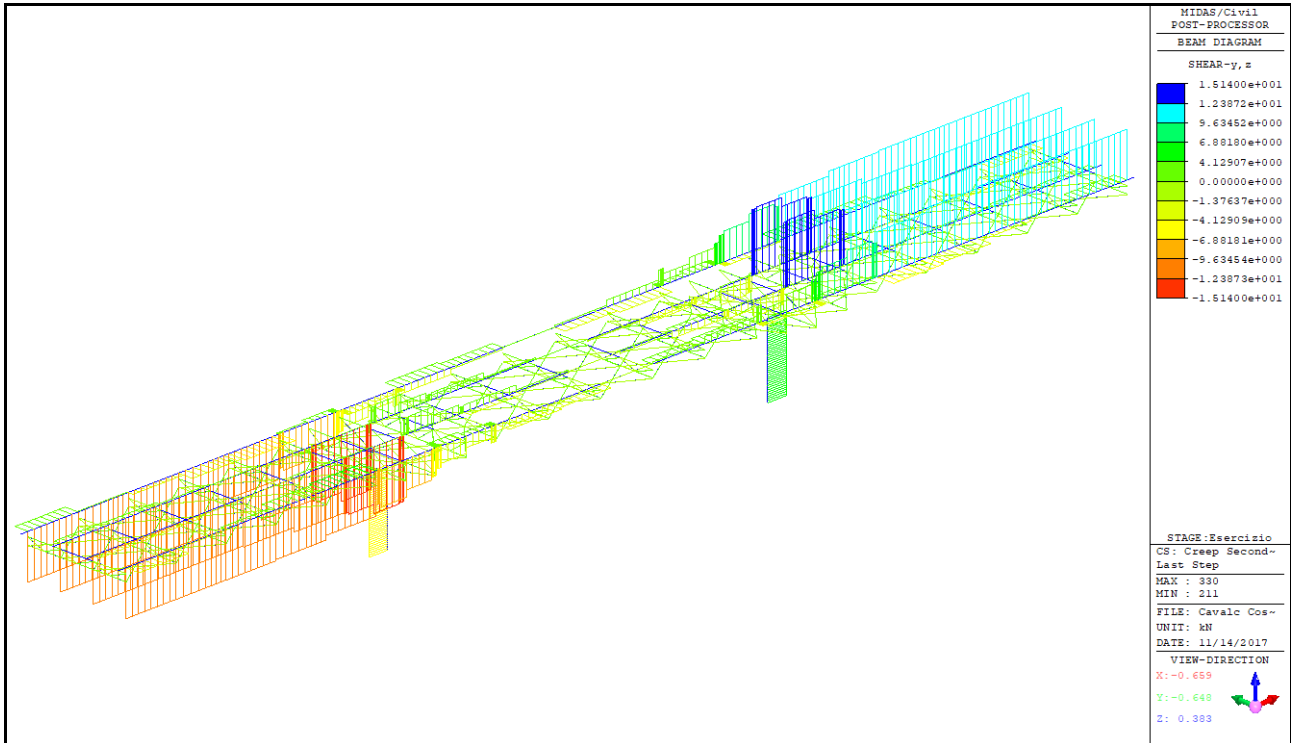


Ritiro soletta - Diagramma sforzi assiali

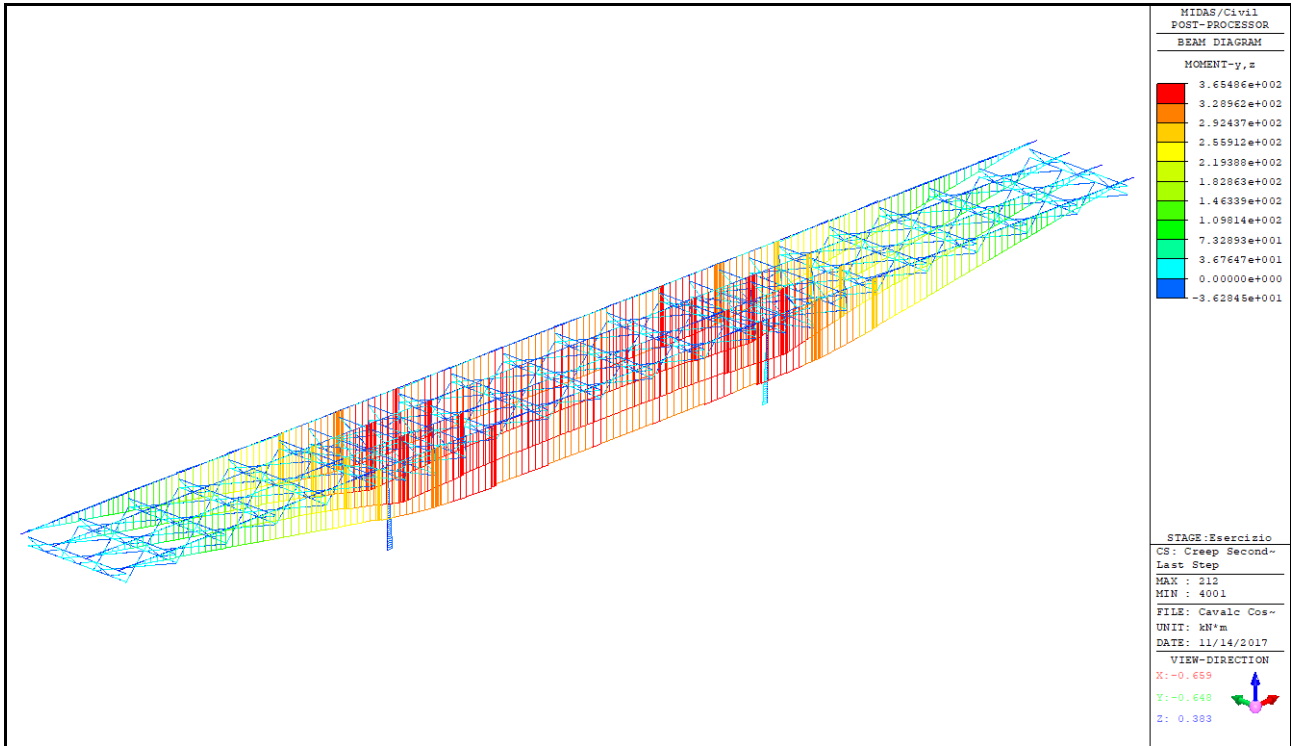




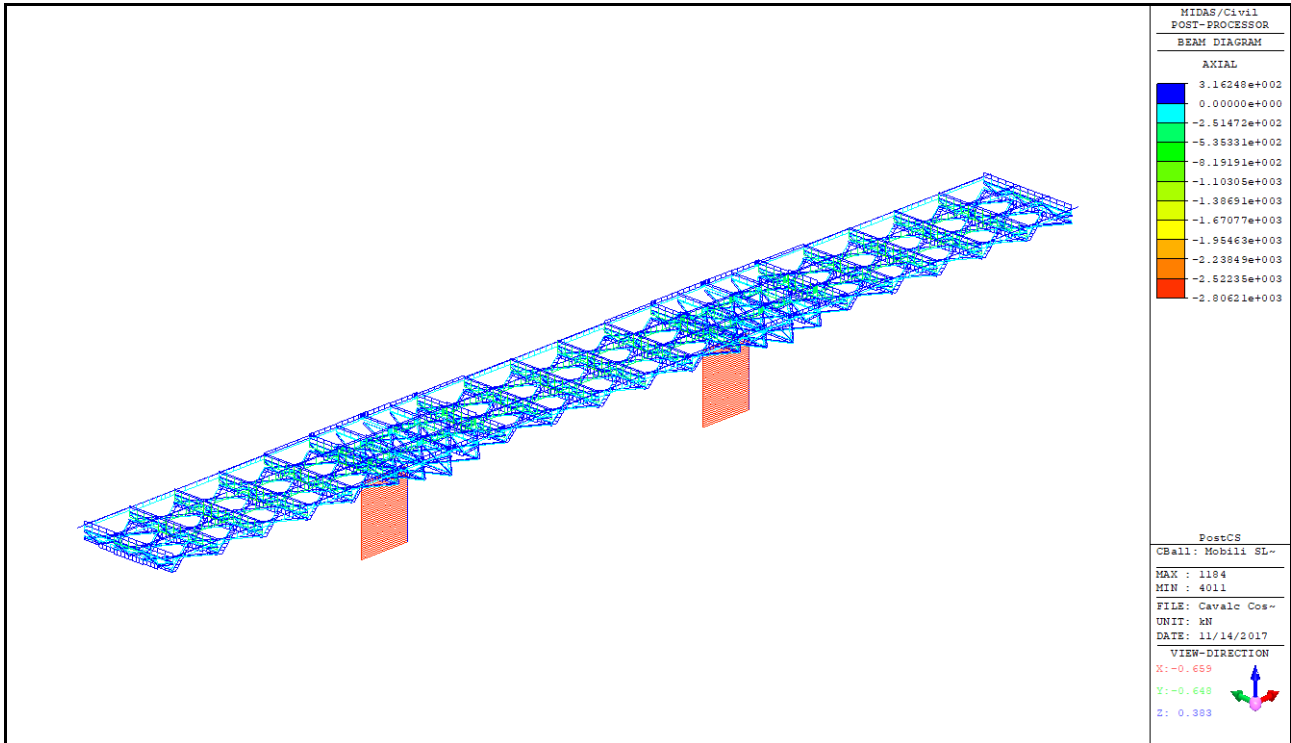
Fluage soletta - Diagramma sforzi assiali



Fluage soletta - Diagramma sforzi taglianti

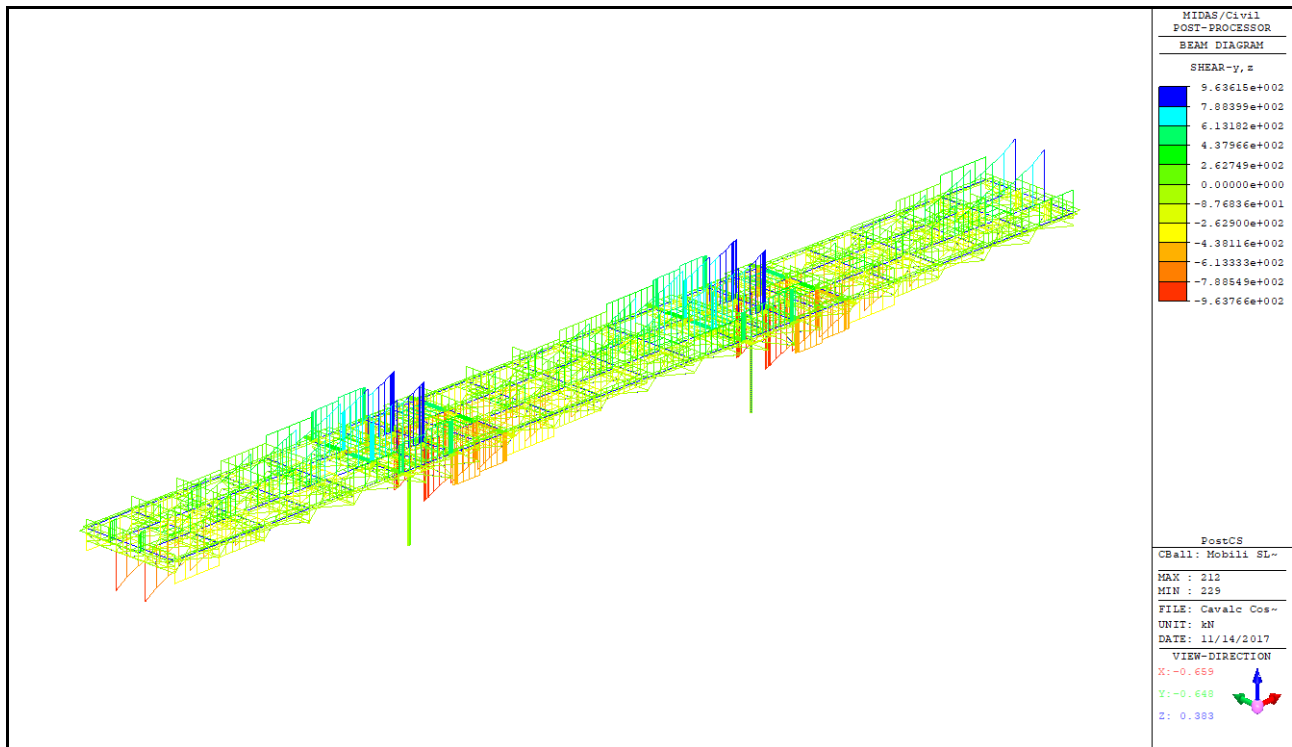


Fluage soletta - Diagramma momenti flettenti

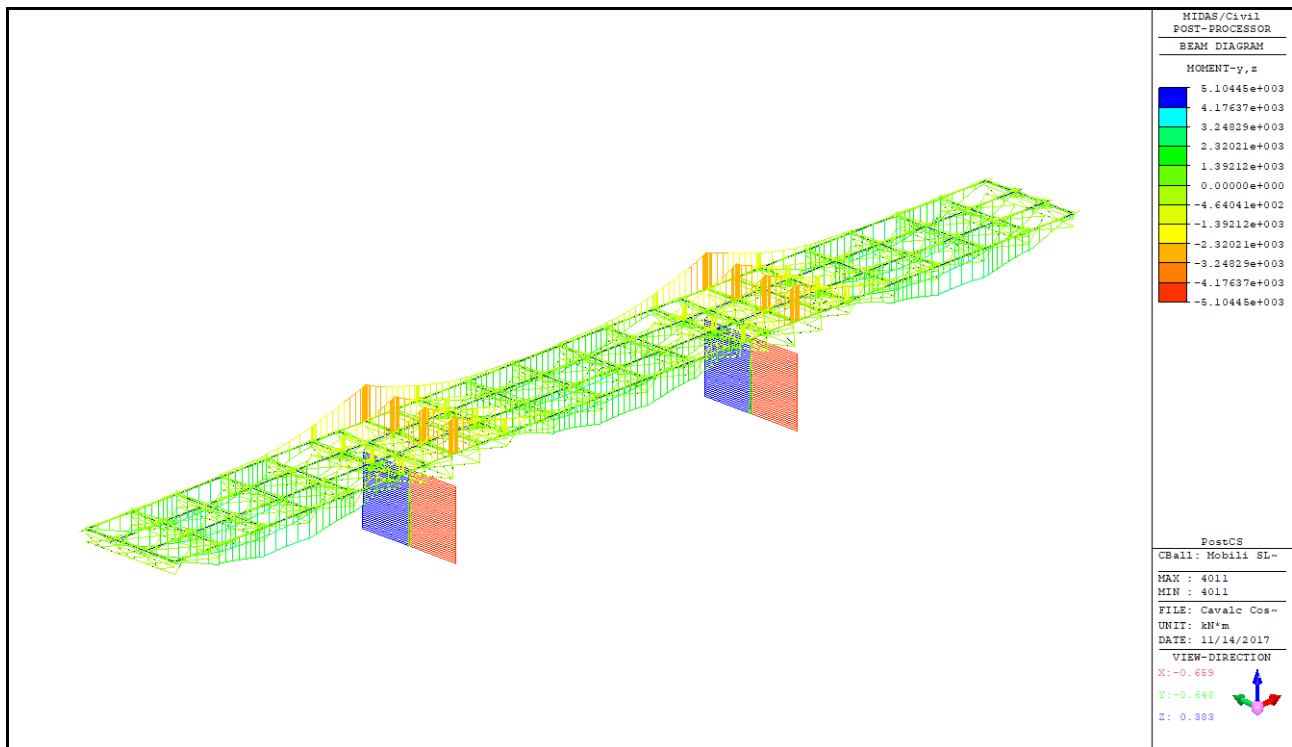


Inviluppo carichi mobili SLU - Diagramma sforzi assiali

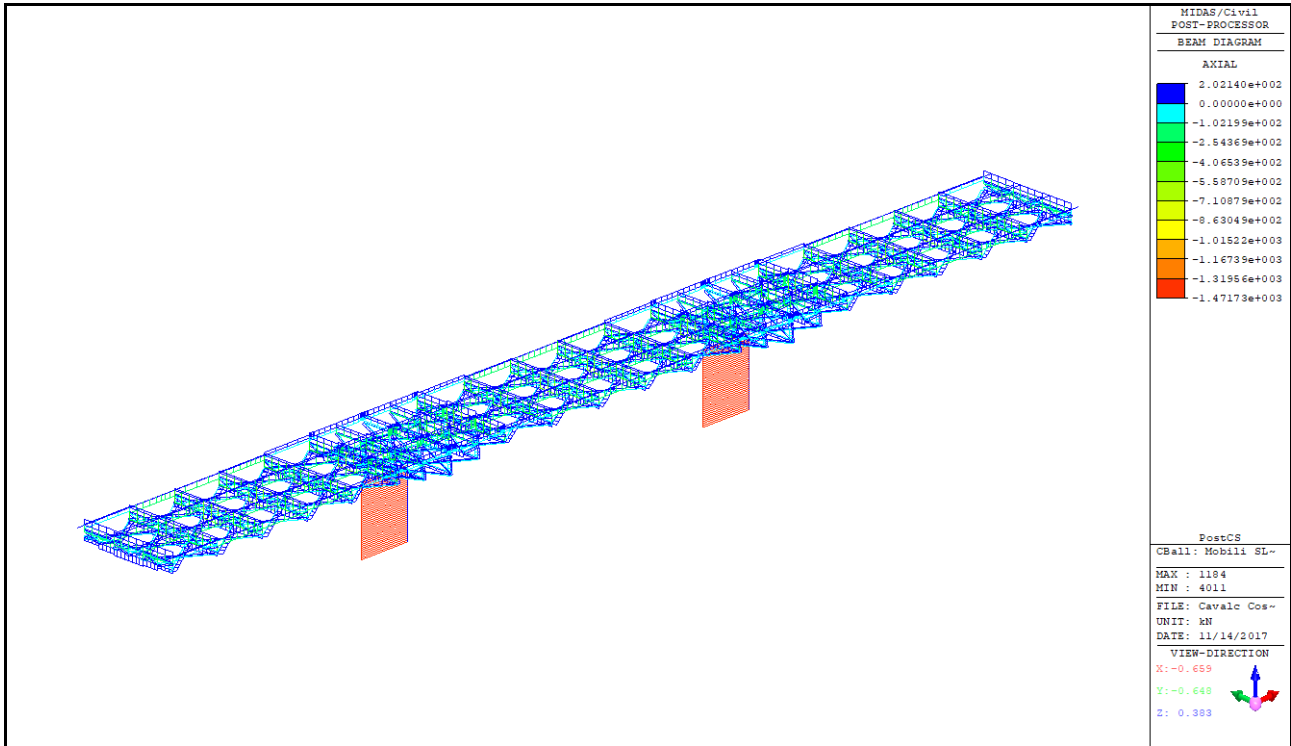




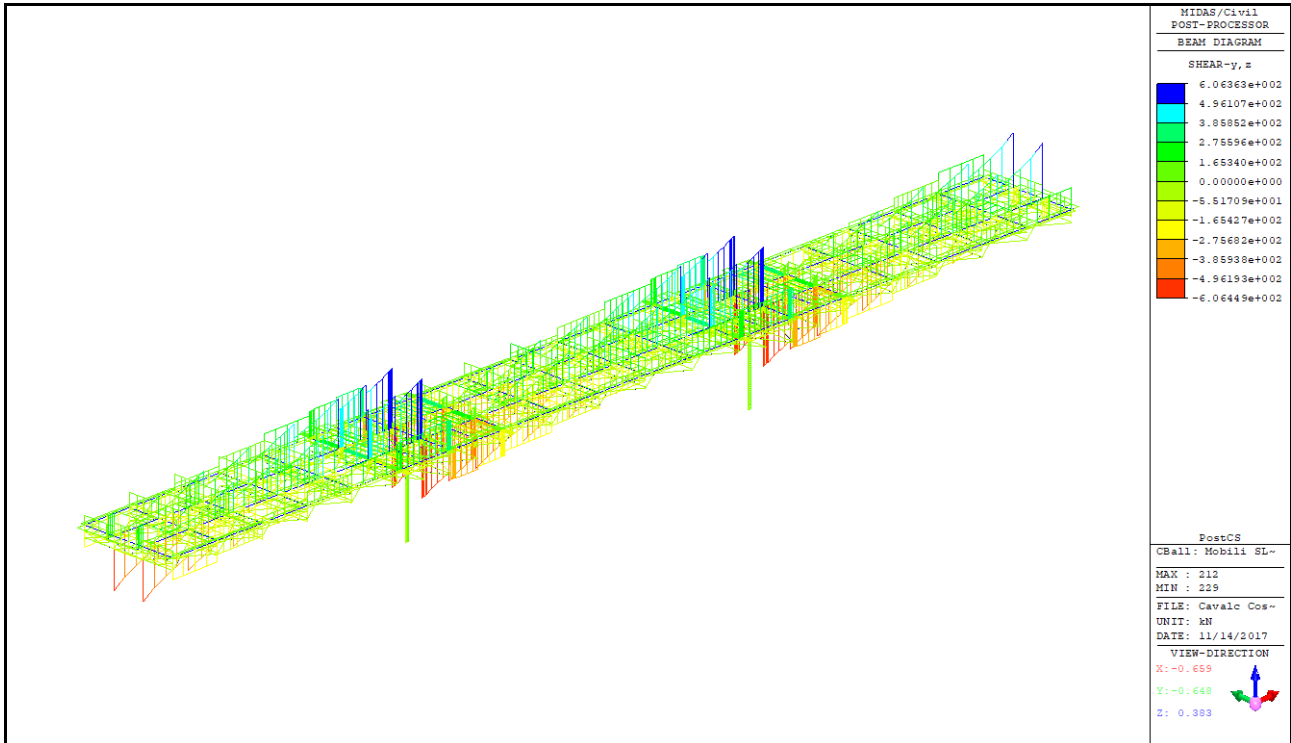
Inviluppo carichi mobili SLU - Diagramma sforzi taglianti



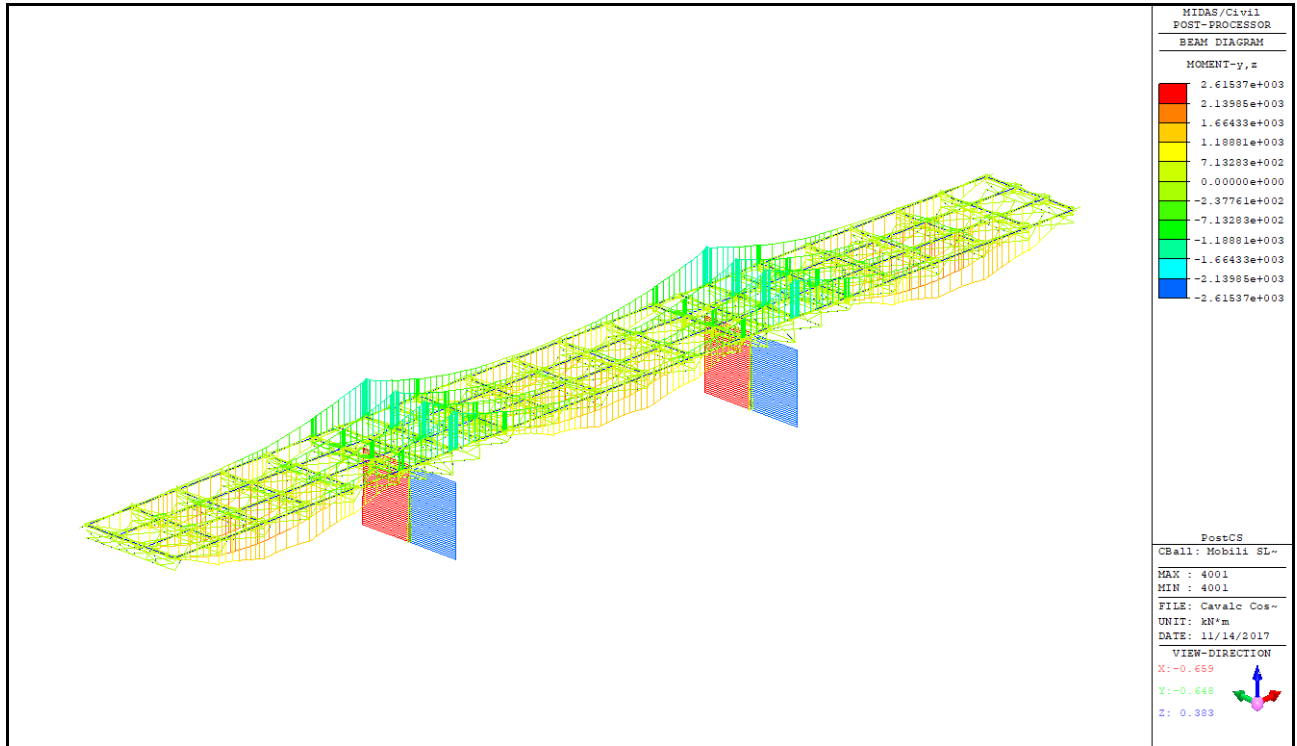
Inviluppo carichi mobili SLU - Diagramma momenti flettenti



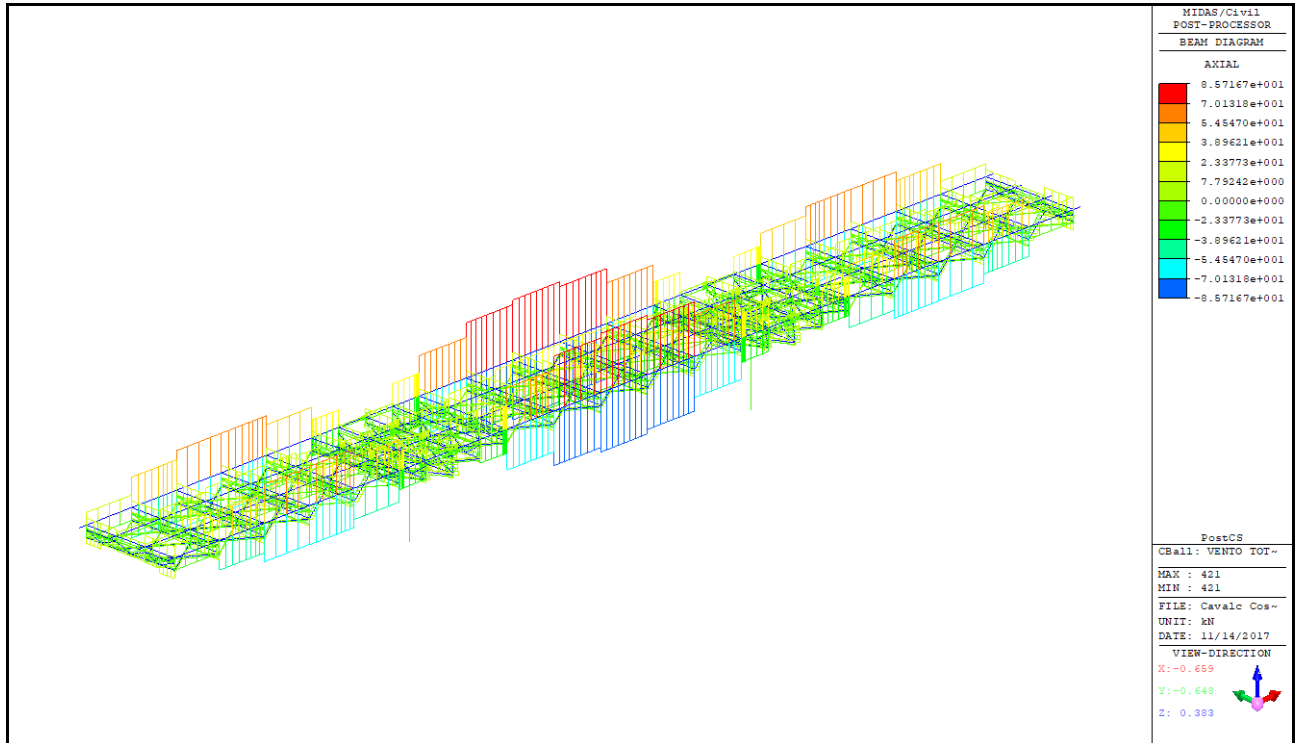
Involuppo carichi mobili SLE - Diagramma sforzi assiali



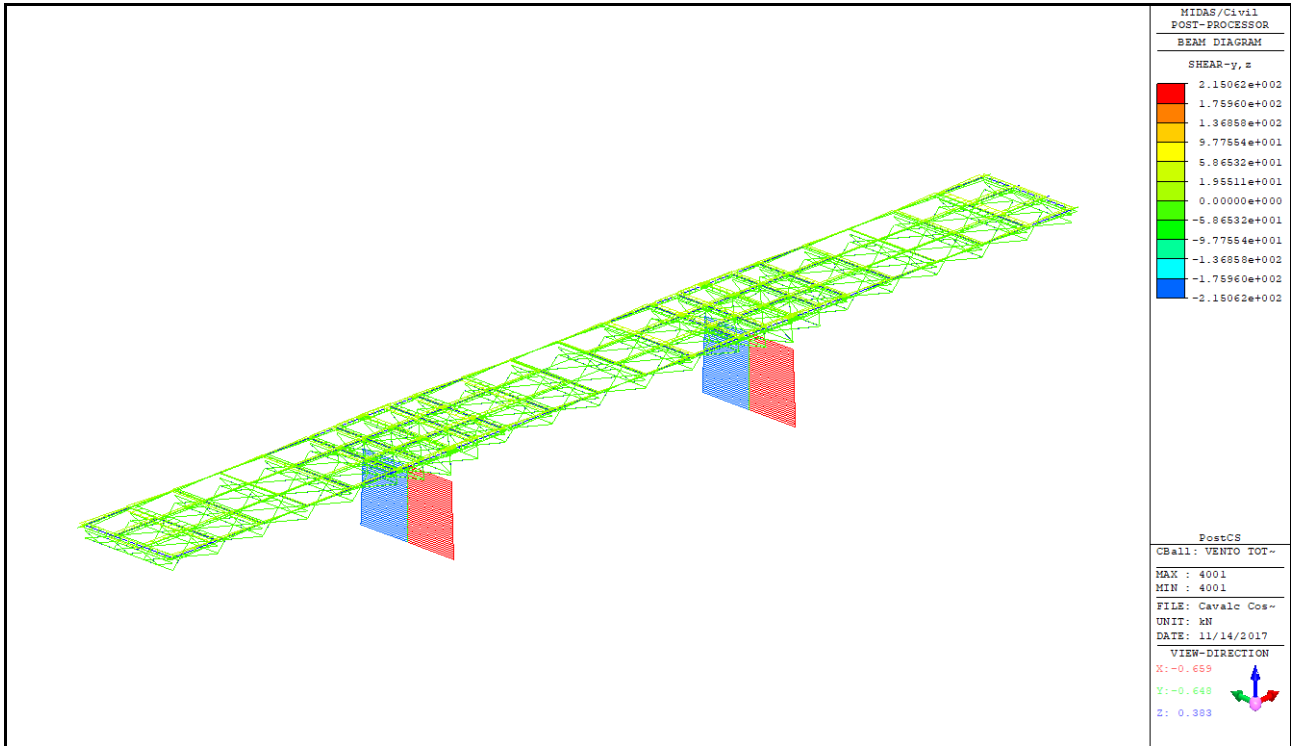
Involuppo carichi mobili SLE - Diagramma sforzi taglianti



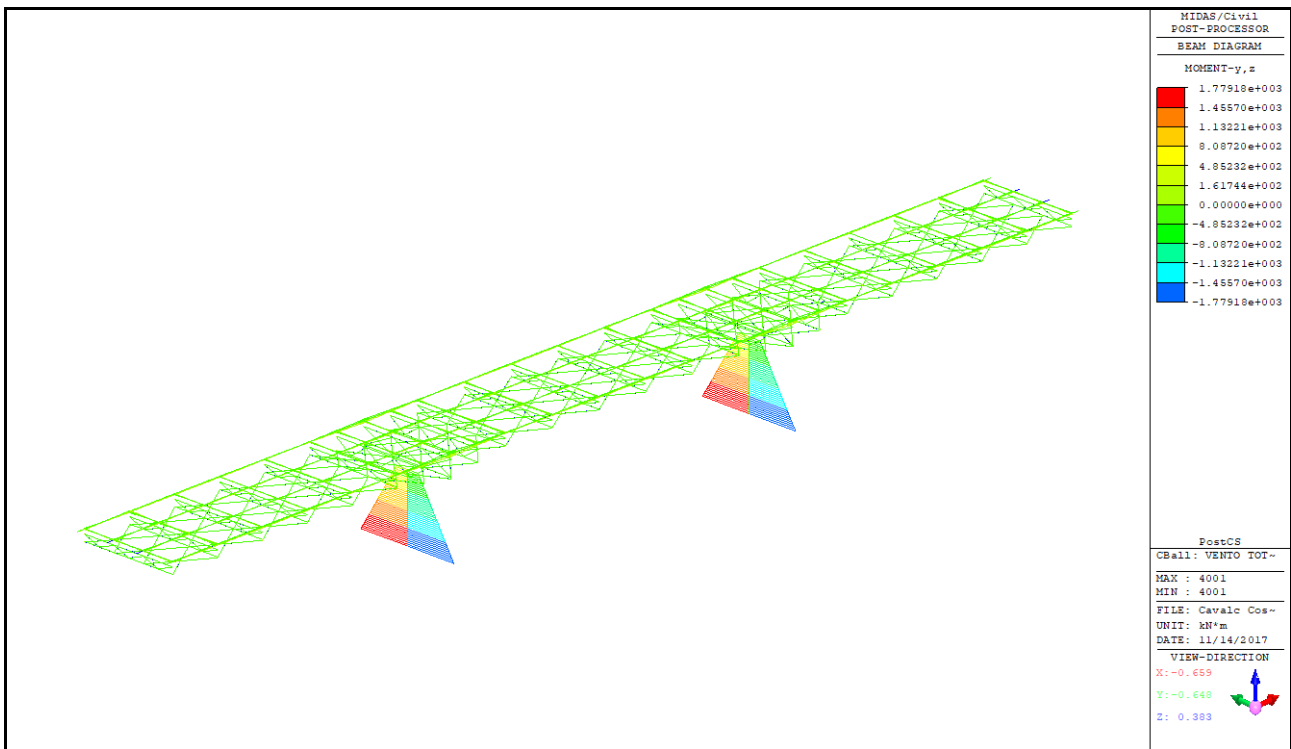
Involuppo carichi mobili SLE - Diagramma momenti flettenti



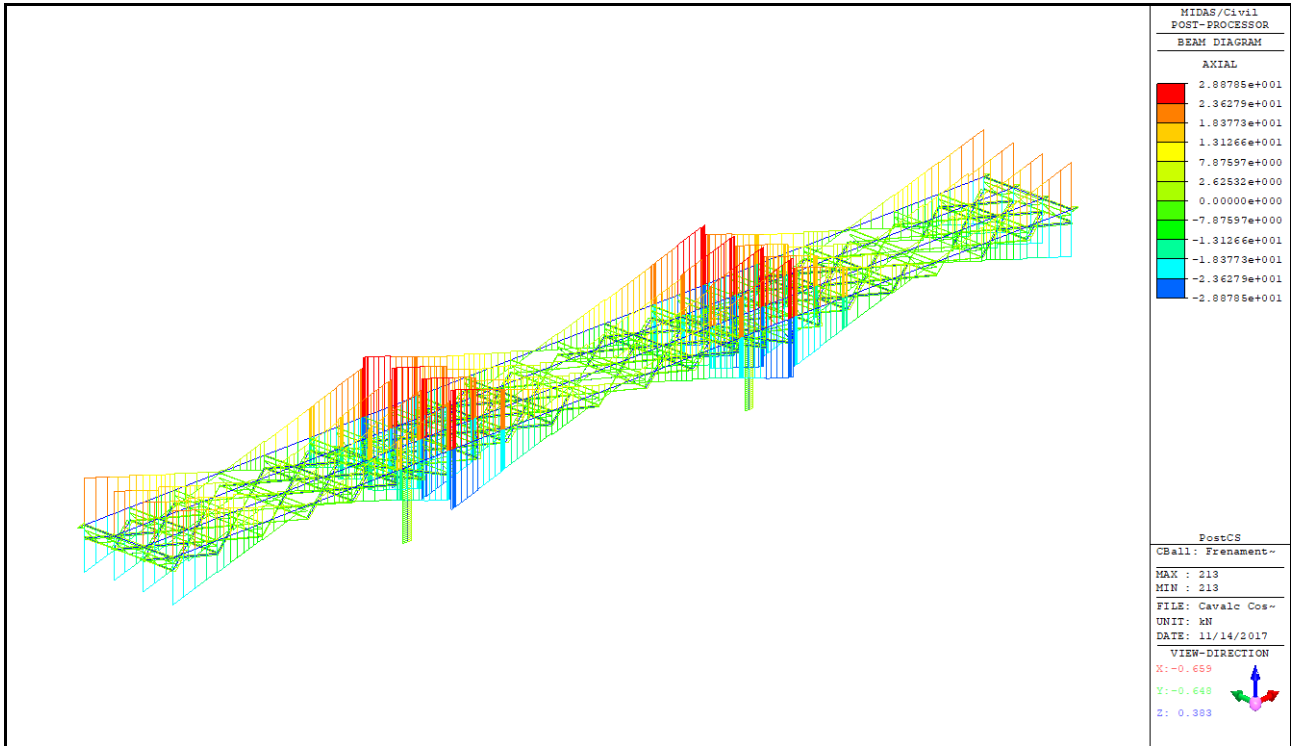
Vento trasversale (strutture e mobili) - Diagramma sforzi assiali



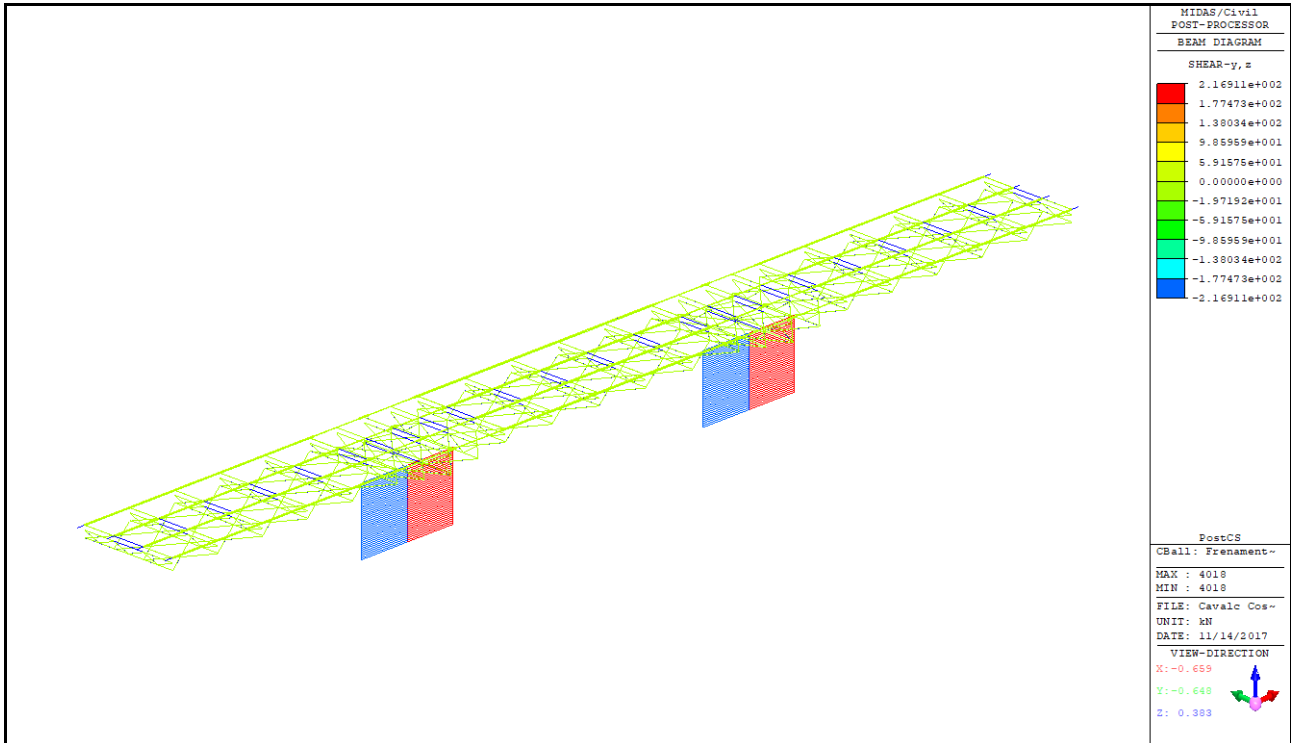
Vento trasversale (strutture e mobili) - Diagramma sforzi taglianti



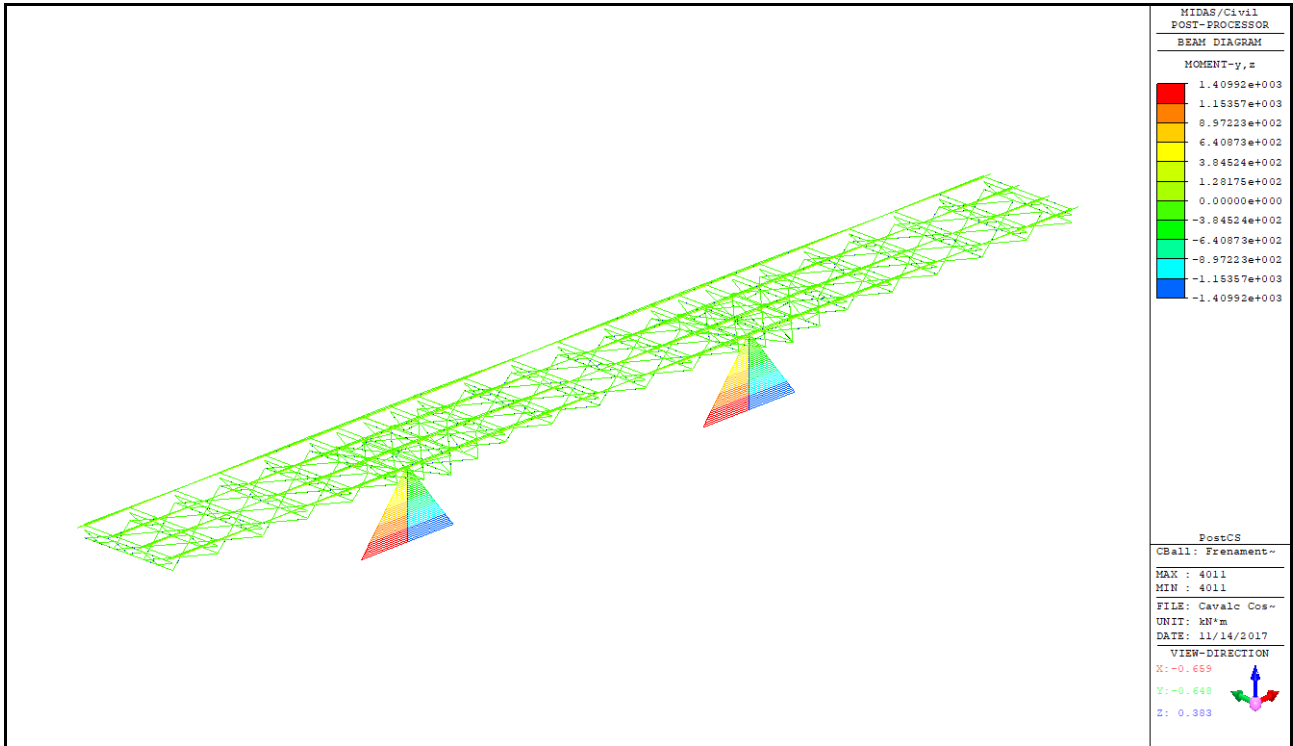
Vento trasversale (strutture e mobili) - Diagramma momenti flettenti



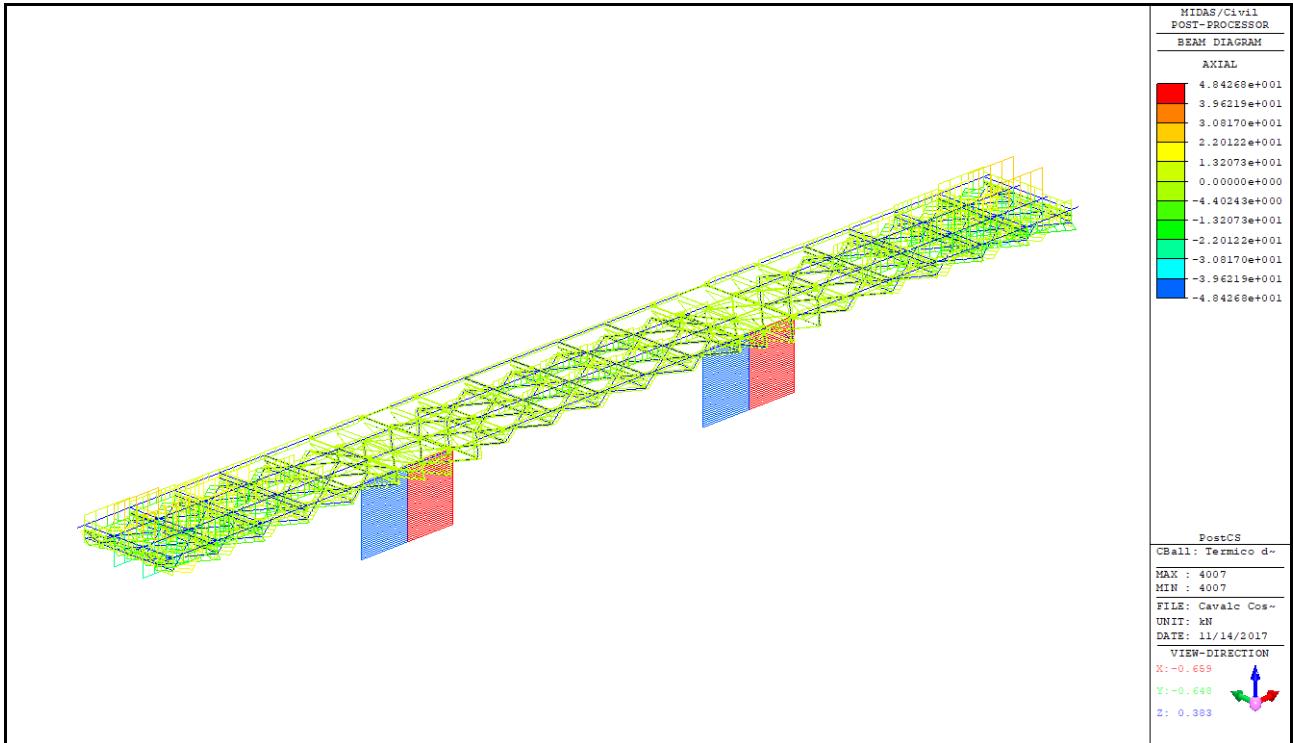
Frenamento - Diagramma sforzi assiali



Frenamento - Diagramma sforzi taglianti

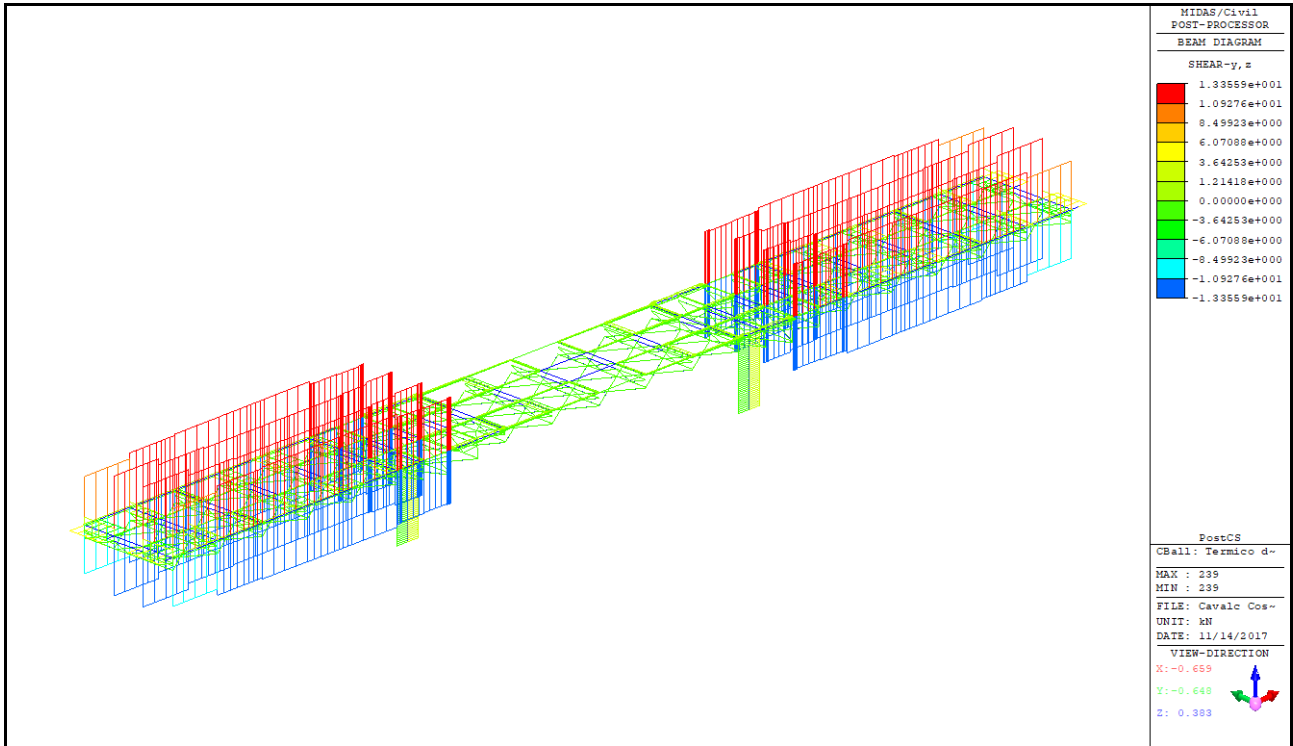


Frenamento - Diagramma momenti flettenti

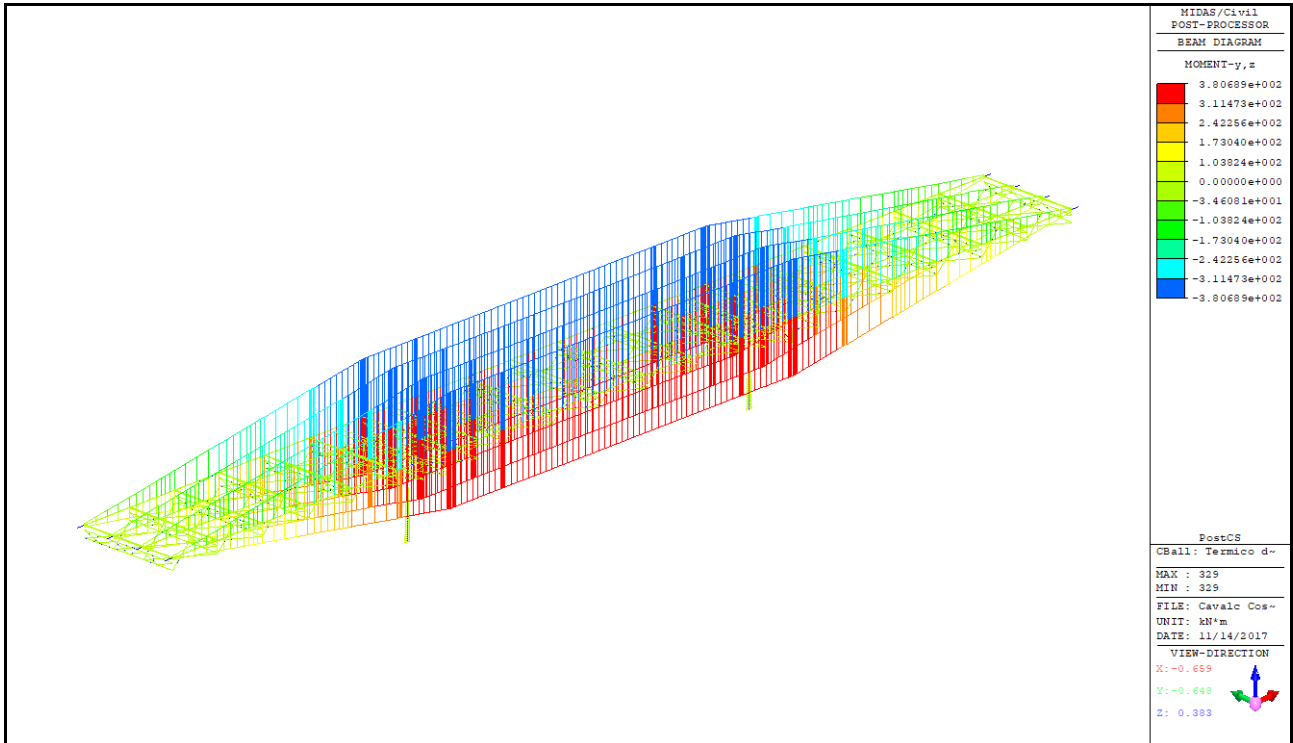


Involuppo differenziale termico  $\pm 5^\circ/H$  - Diagramma sforzi assiali



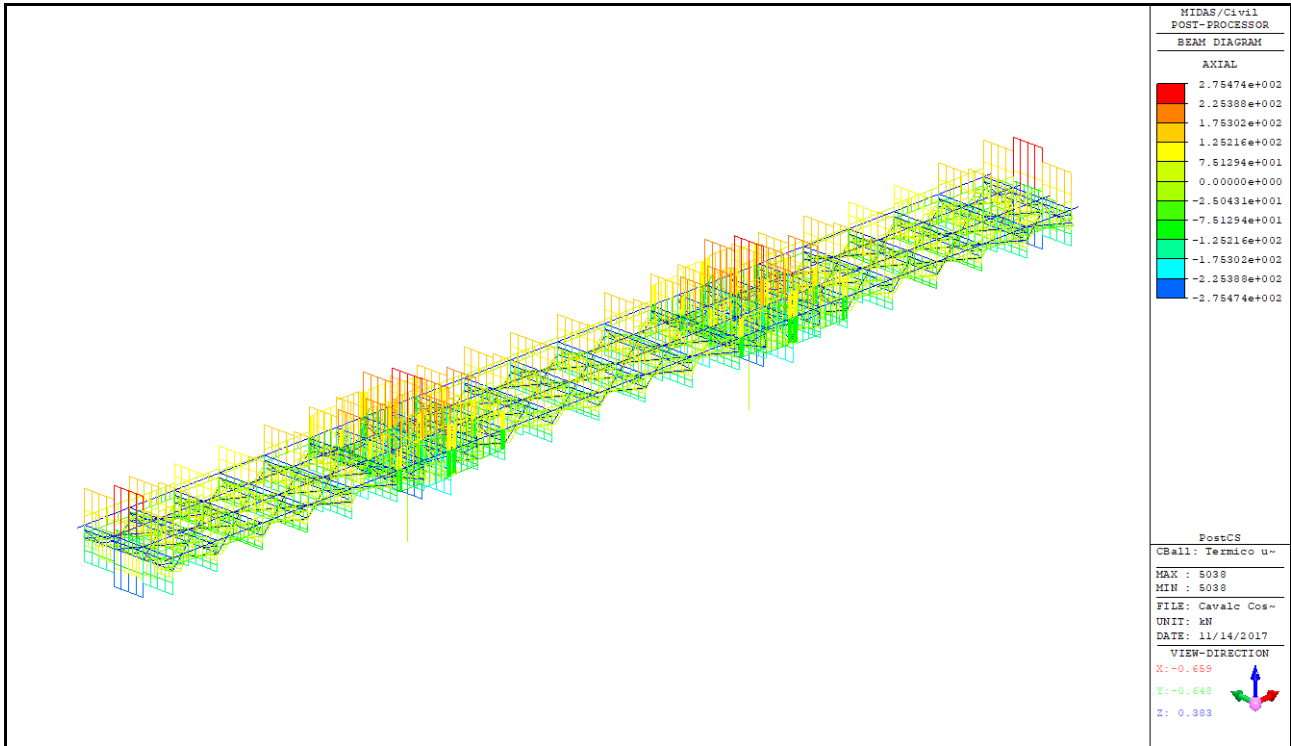


Inviluppo differenziale termico  $\pm 5^\circ/H$  - Diagramma sforzi taglianti

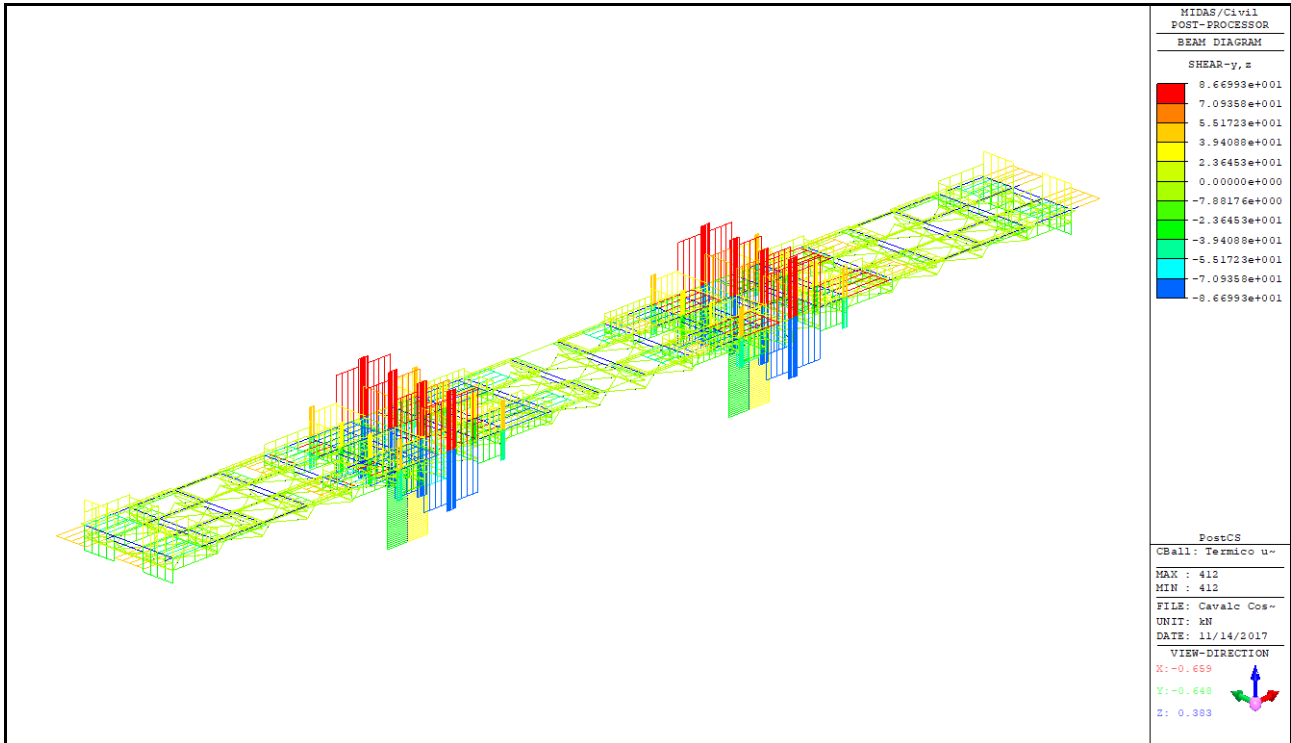


Inviluppo differenziale termico  $\pm 5^\circ/H$  - Diagramma momenti flettenti



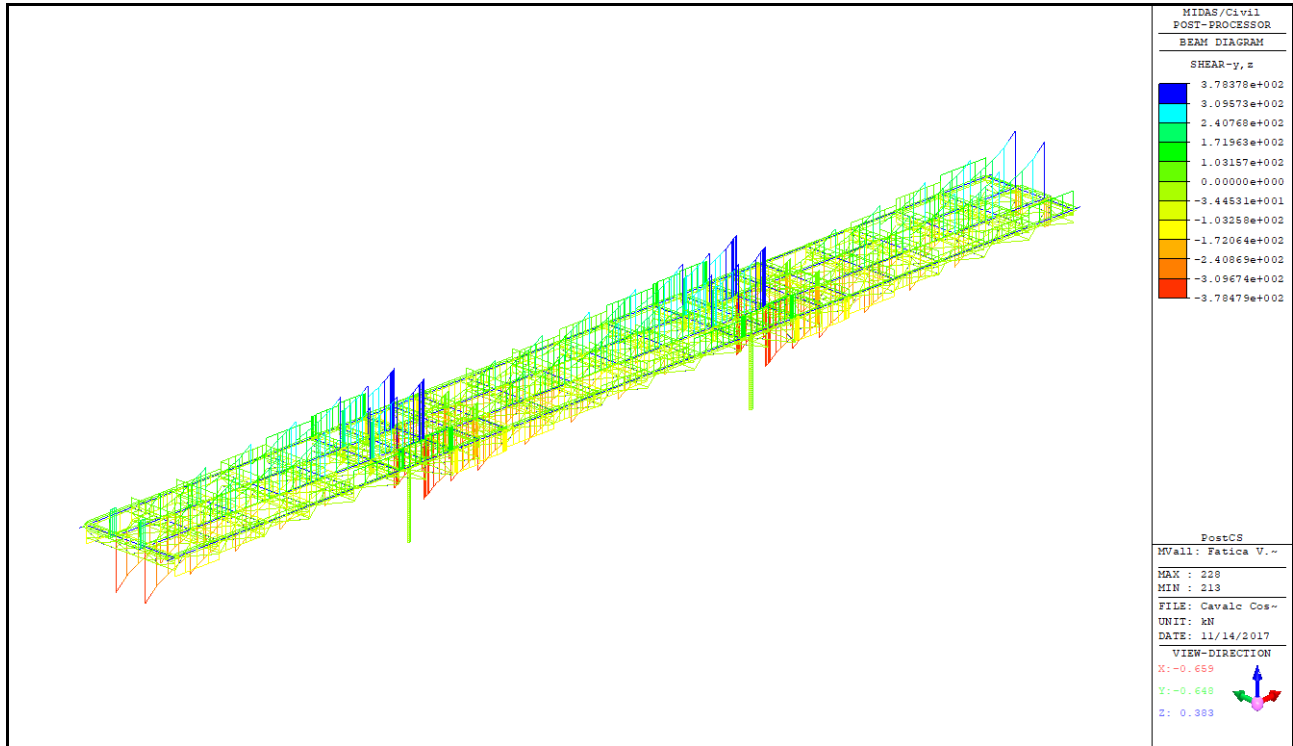


Involuppo termico uniforme  $\pm 15^\circ$ - Diagramma sforzi assiali

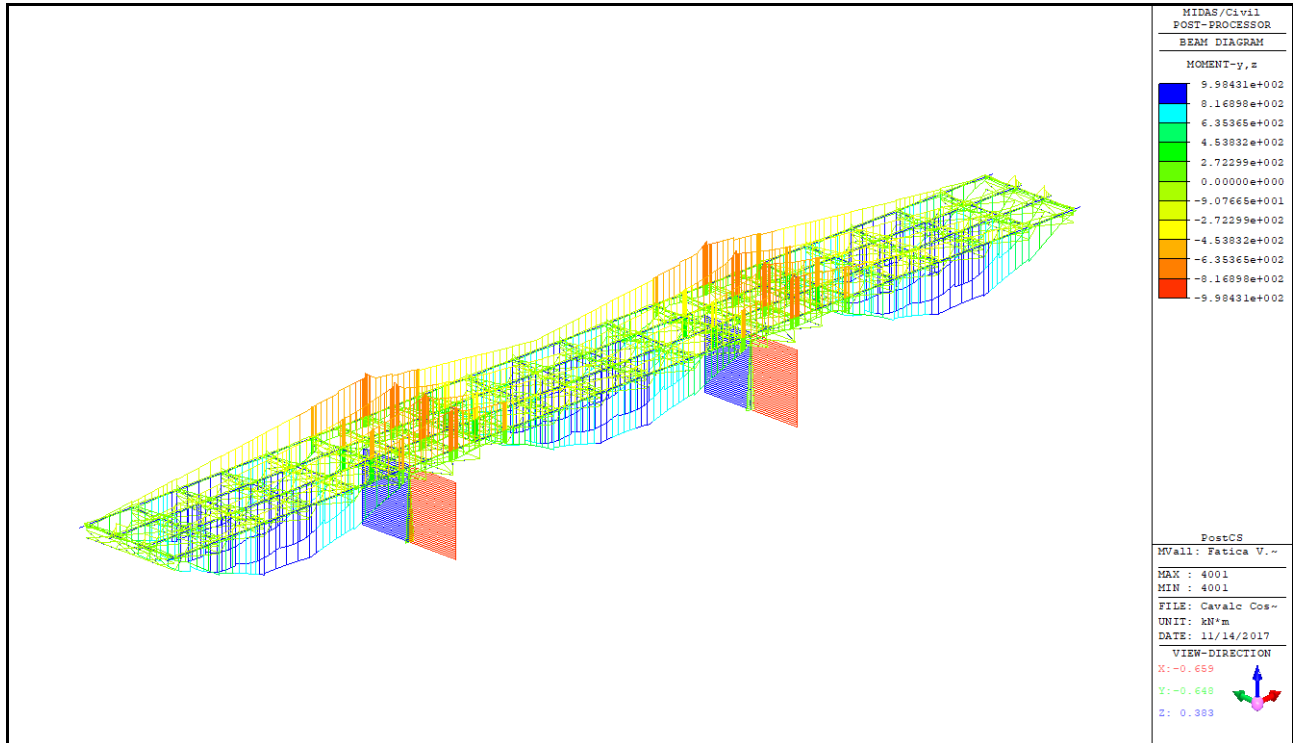


Involuppo termico uniforme  $\pm 15^\circ$ - Diagramma sforzi taglianti



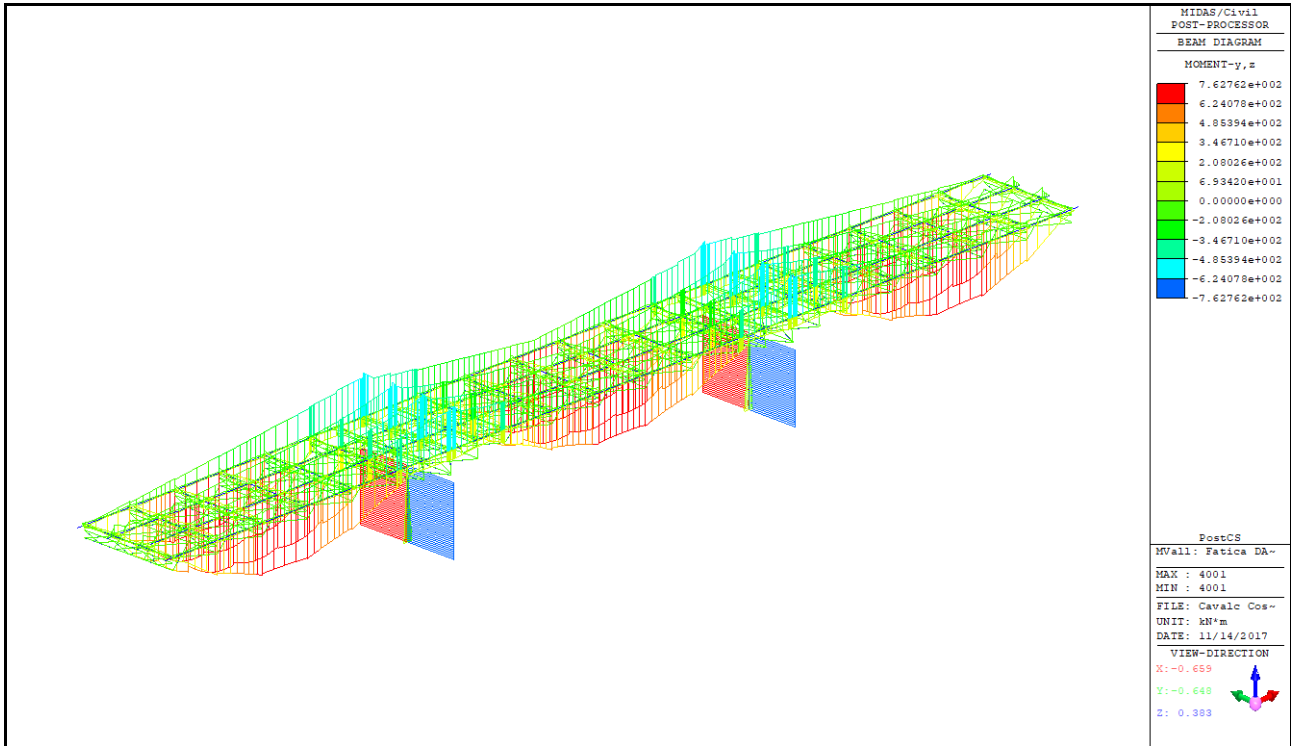


Carichi mobili fatica schema 2 (vita ill.) - Diagramma sforzi taglianti



Carichi mobili fatica schema 2 (vita ill.) - Diagramma momenti flettenti





Inviluppo carichi mobili fatica schema 3 (dann.) - Diagramma momenti flettenti

## 7 RISPOSTA SISMICA DELLA STRUTTURA

Sono stati determinati 40 modi di vibrazione con il metodo "Ritz Vectors"; i periodi determinati variano da 1.464 s a 0.0050 s; le masse eccitate risultano:

- dir. X: 98.64 %
- dir. Y: 96.31 %
- dir. Z: 97.26 %

Seguono schermate programma di calcolo.

Mode	UX	UY	UZ	RX
<b>EIGENVALUE ANALYSIS</b>				
Mode No	Frequency		Period	Tolerance
	(rad/sec)	(cycle/sec)	(sec)	
1	4.292395	0.683156	1.463795	0.0000e+000
2	4.292751	0.683212	1.463673	0.0000e+000
3	4.722273	0.751573	1.330543	0.0000e+000
4	10.895521	1.734076	0.576676	0.0000e+000
5	15.842581	2.521425	0.396601	0.0000e+000
6	16.485615	2.623767	0.381131	0.0000e+000
7	20.012120	3.185028	0.313969	0.0000e+000
8	21.257564	3.383246	0.295574	0.0000e+000
9	22.235819	3.538941	0.282570	0.0000e+000
10	26.913124	4.283357	0.233462	0.0000e+000
11	41.422767	6.592638	0.151684	0.0000e+000
12	41.797864	6.652337	0.150323	0.0000e+000
13	54.068619	8.605288	0.116208	0.0000e+000
14	56.818588	9.042959	0.110583	0.0000e+000
15	58.542898	9.317392	0.107326	0.0000e+000
16	59.801574	9.517716	0.105067	0.0000e+000
17	63.597120	10.121796	0.098797	0.0000e+000
18	65.840852	10.478897	0.095430	0.0000e+000
19	66.110041	10.521740	0.095041	0.0000e+000
20	78.893888	12.556352	0.079641	0.0000e+000
21	78.977427	12.569648	0.079557	0.0000e+000
22	88.279309	14.050088	0.071174	0.0000e+000
23	100.165410	15.941820	0.062728	0.0000e+000
24	104.792633	16.678266	0.059958	0.0000e+000
25	104.824685	16.683367	0.059940	0.0000e+000
26	107.146589	17.052909	0.058641	0.0000e+000
27	115.862130	18.440031	0.054230	0.0000e+000
28	119.077052	18.951701	0.052766	0.0000e+000
29	121.393708	19.320409	0.051759	0.0000e+000
30	123.238566	19.614027	0.050984	0.0000e+000
31	150.775182	23.996615	0.041673	0.0000e+000
32	185.205907	29.476436	0.033925	0.0000e+000
33	231.323159	36.816224	0.027162	0.0000e+000
34	235.290632	37.447667	0.026704	0.0000e+000
35	308.072793	49.031308	0.020395	0.0000e+000
36	494.647898	78.725658	0.012702	0.0000e+000
37	503.006808	80.056020	0.012491	0.0000e+000
38	656.964697	104.559179	0.009564	0.0000e+000
39	902.185816	143.587332	0.006964	0.0000e+000
40	1266.496332	201.569152	0.004961	0.0000e+000







**Element Number : 208**

**Position Informator : J**

**1 Design Condition**

**1.1 Design Parameters**

- Partial factors

$\gamma_C$ for concrete	1.50	$\gamma_V$ for headed stud	1.25
$\gamma_S$ for reinforcing steel	1.15	$\gamma_{Ff}$ for equivalent constant Amplitude stress range	1.00
$\gamma_{M0}$ for structural steel	1.05	$\gamma_{Mf}$ for fatigue strength	1.15
$\gamma_{M1}$ for structural steel	1.10	$\gamma_{Mf,s}$ for fatigue strength of studs in shear	1.00

**1.2 Material Information**

- Structural steel

$f_{sk} = 355.000 \text{ MPa}$  ,  $E_s = 210000.000 \text{ MPa}$

- Concrete

$f_{ck} = 32.000 \text{ MPa}$  ,  $E_{cm} = 33000.000 \text{ MPa}$

- Reinforcement

$f_{yk} = 450.000 \text{ MPa}$  ,  $E_r = 210000.000 \text{ MPa}$

**1.3 Sectional Information**

Slab

$B_c$	3200.000	mm	$t_c$	240.000	mm	$H_h$	60.000	mm
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Girder

$H_w$	930.000	mm	$B_1$	500.000	mm	$B_2$	600.000	mm
$t_w$	20.000	mm	$t_{f1}$	30.000	mm	$t_{f2}$	40.000	mm

Before

$A_a$	57600.000	mm <sup>2</sup>
$I_{y,a}$	10104070000.000	mm <sup>4</sup>
$I_{z,a}$	1033120000.000	mm <sup>4</sup>
$C_{y,a}$	300.000	mm
$C_{z,a}$	427.917	mm

After

$A_c$	179547.347	mm <sup>2</sup>
$I_{y,c}$	32817703285.987	mm <sup>4</sup>
$I_{z,c}$	105094855844.263	mm <sup>4</sup>
$C_{y,c}$	1600.000	mm
$C_{z,c}$	938.727	mm

Crack

$A_c$	67648.000	mm <sup>2</sup>
$I_{y,c}$	14825070227.696	mm <sup>4</sup>
$I_{z,c}$	9574171327.412	mm <sup>4</sup>
$C_{y,c}$	1600.000	mm
$C_{z,c}$	537.398	mm

## 2 Bending Resistance

### 2.1 Negative Moment

- Design load

Load combination name : ST SLU Mobili

$$N_{a,Ed} = 0.328 \text{ kN}$$

$$N_{c,Ed} = 128.913 \text{ kN}$$

$$M_{a,Ed} = -1868.615 \text{ kN} \cdot \text{m}$$

$$M_{c,Ed} = -3639.410 \text{ kN} \cdot \text{m}$$

- Stress

Top Flange

Left	y <sub>1</sub>	-250.000	mm	z <sub>1</sub>	462.602	mm	σ <sub>1</sub>	220.199	MPa
	y <sub>2</sub>	-10.000	mm	z <sub>2</sub>	462.602	mm	σ <sub>2</sub>	221.232	MPa
Right	y <sub>1</sub>	250.000	mm	z <sub>1</sub>	462.602	mm	σ <sub>1</sub>	222.350	MPa
	y <sub>2</sub>	10.000	mm	z <sub>2</sub>	462.602	mm	σ <sub>2</sub>	221.318	MPa

Bottom Flange

Left	y <sub>1</sub>	-300.000	mm	z <sub>1</sub>	-537.398	mm	σ <sub>1</sub>	-210.443	MPa
	y <sub>2</sub>	-10.000	mm	z <sub>2</sub>	-537.398	mm	σ <sub>2</sub>	-209.195	MPa
Right	y <sub>1</sub>	300.000	mm	z <sub>1</sub>	-537.398	mm	σ <sub>1</sub>	-207.862	MPa
	y <sub>2</sub>	10.000	mm	z <sub>2</sub>	-537.398	mm	σ <sub>2</sub>	-209.109	MPa

Web

Right	y <sub>1</sub>	0.000	mm	z <sub>1</sub>	432.602	mm	σ <sub>1</sub>	208.362	MPa
	y <sub>2</sub>	0.000	mm	z <sub>2</sub>	-497.398	mm	σ <sub>2</sub>	-191.935	MPa

- Classification of sections

Part	Class
Top flange	1
Web	1
Bottom flange	1
Section	1

- Plastic resistance moment,  $M_{pl,Rd}$

$$\text{Plastic NA} = 570.734 \text{ mm}$$

$$N_{slab} = 0.000 \text{ kN}$$

$$N_{rebar} = 3931.826 \text{ kN}$$

$$N_{g,top} = 7771.230 \text{ kN} \quad (\text{Upper side of PNA})$$

$$N_{g,bot} = 11703.056 \text{ kN} \quad (\text{Lower side of PNA})$$

$$M_{pl,Rd} = 10397.595 \text{ kN} \cdot \text{m}$$

$$M_{Rd} = M_{pl,Rd} = 10397.595 \text{ kN} \cdot \text{m}$$

$$M_{Rd} = 10397.595 \text{ kN} \cdot \text{m} > M_{Ed} = -5508.025 \text{ kN} \cdot \text{m} \quad \dots \text{OK}$$

### 3 Resistance to Vertical Shear

- Design load

Load combination name : ST SLU Mobili

$N_{Ed}$	=	66.553	kN
$M_{a,Ed}$	=	-1868.615	kN · m
$M_{c,Ed}$	=	-1178.987	kN · m
$V_{Ed,a}$	=	635.621	kN
$V_{Ed,c}$	=	1329.456	kN
$V_{Ed}$	=	1965.077	kN

- Stress

Top Flange

Left	$y_1$	-250.000	mm	$z_1$	462.602	mm	$\sigma_1$	136.431	MPa
	$y_2$	-10.000	mm	$z_2$	462.602	mm	$\sigma_2$	143.292	MPa
Right	$y_1$	250.000	mm	$z_1$	462.602	mm	$\sigma_1$	150.725	MPa
	$y_2$	10.000	mm	$z_2$	462.602	mm	$\sigma_2$	143.864	MPa

Bottom Flange

Left	$y_1$	-300.000	mm	$z_1$	-537.398	mm	$\sigma_1$	-129.462	MPa
	$y_2$	-10.000	mm	$z_2$	-537.398	mm	$\sigma_2$	-121.171	MPa
Right	$y_1$	300.000	mm	$z_1$	-537.398	mm	$\sigma_1$	-112.309	MPa
	$y_2$	10.000	mm	$z_2$	-537.398	mm	$\sigma_2$	-120.600	MPa

Web

Right	$y_1$	0.000	mm	$z_1$	432.602	mm	$\sigma_1$	135.644	MPa
	$y_2$	0.000	mm	$z_2$	-497.398	mm	$\sigma_2$	-110.307	MPa

- Classification of sections

Part	Class
Top flange	1
Web	1
Bottom flange	1
Section	1

- Plastic resistance moment,  $M_{pl,Rd}$

Plastic NA = 570.734 mm

$N_{slab}$	=	0.000	kN
$N_{g,top}$	=	7771.230	kN
$N_{g,bot}$	=	11703.056	kN

$M_{pl,Rd}$  = 10397.595 kN · m

- Calculation.  $V_{pl,Rd}$

Web

$$k_t = 5.34 + 4.00 \cdot (h_w/a)^2 + k_{tst} = 5.478 \quad \text{when } a/h_w = 5.376 \geq 1$$

where,  $h_w = 930.000$  mm

$$2.1/t \cdot (I_{sl}/h_w)^{1/3} = 0.000$$

$$k_{tst} = 9 \cdot (h_w/a)^2 \cdot ((I_{sl} / (t^3 \cdot h_w))^3)^{1/4} > 2.1/t \cdot (I_{sl}/h_w)^{1/3} = 0.000$$

$$I_{sl} = 0.000 \text{ mm}^4$$

$$t = 20.000 \text{ mm}$$

$$V_{pl,Rd} = A_v \cdot (f_y / \sqrt{3}) / \gamma_{MO} = 4356.850 \text{ kN}$$

$$V_{Rd} = 4356.850 \text{ kN}$$

$$V_{Edi} = V_{Ed} / \text{Num. of Web} = 1965.077 \text{ kN}$$

$$V_{Edi} / V_{Rd} = 0.451 \leq 1.0$$

... OK

#### 4 Resistance to Lateral Torsional Buckling

- Design load

Load combination name : ST SLU Mobili

$$\begin{aligned}
 N_{Ed} &= 129.241 \text{ kN} \\
 M_{Ed} &= -3639.410 \text{ kN} \cdot \text{m} \\
 V_1 &= 701.074 \text{ kN} \\
 V_2 &= 1225.615 \text{ kN} \\
 M_1 &= -590.808 \text{ kN} \cdot \text{m} \\
 M_2 &= -5508.025 \text{ kN} \cdot \text{m} \\
 M_{pl,Rd} &= 10397.595 \text{ kN} \cdot \text{m} \\
 M_{el,Rd} &= 9012.415 \text{ kN} \cdot \text{m}
 \end{aligned}$$

-  $M_{b,Rd}$  Buckling resistance moment

$$\begin{aligned}
 L &= 5000.000 \text{ m} \\
 c &= C_d / I = 0.000 \\
 \gamma &= c \cdot L^4 / (E \cdot I) = 0.000 \\
 \mu &= V_2 / V_1 = 0.572 \\
 \Phi &= 2 \cdot (1 - M_2 / M_1) / (1 + \mu) = 1.136 \\
 m_1 &= 1 + 0.44 \cdot (1 + \mu) \cdot \Phi^{1.5} + (3 + 2 \cdot \Phi) \cdot \gamma / (350 - 50 \cdot \mu) = 1.837 \\
 m_2 &= 1 + 0.44 \cdot (1 + \mu) \cdot \Phi^{1.5} + (0.195 + (0.05 + \mu / 100) \cdot \Phi) \cdot \gamma^{0.5} = 1.838 \\
 m &= \text{Min}(m_1, m_2) = 1.837 \\
 \alpha_{LT} &= 0.490 \\
 \lambda_{LT} &= 1.103 \cdot L / b \cdot \sqrt{f_y / E_m} \cdot \sqrt{1 + A_{wc} / (3 \cdot I)} = 0.297 \\
 \Phi_{LT} &= 0.5 \cdot (1 + \alpha_{LT} \cdot (\lambda_{LT} - 0.2) + \lambda_{LT}^2) = 0.568 \\
 X_{LT} &= \frac{1}{\Phi_{LT} + \sqrt{\Phi_{LT}^2 - \lambda_{LT}^2}} = 0.950447408 \\
 M_{Rd} &= 10397.595 \text{ kN} \cdot \text{m} \\
 M_{b,Rd} &= X_{LT} \cdot M_{Rd} = 9882.367 \text{ kN} \cdot \text{m}
 \end{aligned}$$

-  $N_{b,Rd}$  Buckling resistance moment

$$\begin{aligned}
 X_{LT,N} &= 0.885 \\
 N_{b,Rd} &= X_{LT} \cdot \text{Area} \cdot f_{yd} = 20231.850 \text{ kN}
 \end{aligned}$$

$$\text{Combined Ratio} = \frac{N_{Ed}}{N_{b,Rd}} + \frac{M_{Ed}}{M_{b,Rd}} = 0.374661117$$

## 5 Resistance to Longitudinal Shear

- Design load

Load combination name : ST SLU Mobili

$$\begin{aligned}
 N_{c,el} &= 0.000 \text{ kN} \\
 N_{c,f} &= 0.000 \text{ kN} \\
 M_{Ed} &= -3047.602 \text{ kN} \cdot \text{m} \\
 V_{Ed} &= 1329.456 \text{ kN} \\
 M_{pl,Rd} &= 10397.595 \text{ kN} \cdot \text{m} \\
 M_{el,Rd} &= 9012.415 \text{ kN} \cdot \text{m}
 \end{aligned}$$

- Shear resistance of a single connector

$$\begin{aligned}
 P_{Rd,1} &= 0.8 \cdot f_u \cdot \pi \cdot d^2 / 4 / \gamma_V = 111.478 \text{ kN} \\
 P_{Rd,2} &= 0.29 \cdot \alpha \cdot d^2 \cdot \sqrt{f_{ck} \cdot E_{cm}} / \gamma_V = 117.497 \text{ kN} \\
 P_{Rd} &= \text{Min}(P_{Rd,1}, P_{Rd,2}) = 111.478 \text{ kN}
 \end{aligned}$$

$$\begin{aligned}
 \text{where, } f_u &= 450.000 \text{ MPa} \\
 \alpha &= 1 \quad \text{for } h_{sc}/d > 4 \\
 \text{Num.} &= 4 \\
 d &= 22.200 \text{ mm} \\
 h_{sc} &= 175.000 \text{ mm} \\
 \text{Space} &= 200.000 \text{ mm}
 \end{aligned}$$

- Verification

$$\begin{aligned}
 v_{L,Ed} &= V_{Ed} \cdot (A \cdot z / I) = 1197.022 \text{ kN/m} \\
 v_{L,Rd} &= P_{Rd} \cdot \text{Num.}/\text{Space} = 2229.556 \text{ kN/m} \\
 v_{L,Ed} &\leq v_{L,Rd} \quad \dots \text{ OK}
 \end{aligned}$$

## 6 Resistance to Fatigue

- Design load

Load combination name : ST FATICA DANN.ACC.

$$M_y = -2606.366 \text{ kN} \cdot \text{m}$$

- Shear stress range for the connector

$$\begin{aligned}
 \Delta\tau &= F_{sc} / A_{sc} = 53.983 \text{ MPa} \\
 \text{where, } F_{sc} &= v_{L,Ed} \cdot \text{space of stud} / \text{number of stud} = 20.896 \text{ kN} \\
 A_{sc} &= 387.076 \text{ mm}^2
 \end{aligned}$$

- Damage equivalent factor

$$\begin{aligned}
 \lambda_v &= \lambda_{v,1} \cdot \lambda_{v,2} \cdot \lambda_{v,3} \cdot \lambda_{v,4} = 1.484 \\
 \text{where, } \lambda_{v,1} &= 1.550 \\
 \lambda_{v,2} &= 1.100 \\
 \lambda_{v,3} &= 0.871 \\
 \lambda_{v,4} &= 1.000
 \end{aligned}$$

- Equivalent constant amplitude range of shear stress related to 2 million cycles

$$\Delta\tau_{E,2} = \lambda_v \cdot \Delta\tau = 80.126 \text{ MPa}$$

- Verification

$$\gamma_{Ff} \cdot \Delta\tau_{E,2} / (\Delta\tau_c / \gamma_{Mf,s}) = 0.890 \leq 1$$

## 7 Stress Limitation

- In the structural steel

Characteristic load combination name : ST RARA Mobili

$$\sigma_{Ed,ser} = -168.153 \text{ MPa (Top-right fiber in the flange)}$$

$$T_{Ed,ser} = 48.246 \text{ MPa (Neutral axis in the web)}$$

$$\sigma_{Ed,ser} \leq f_y / \gamma_{M,ser}$$

$$-168.153 \text{ MPa} \leq 355.000 \text{ MPa} \quad \dots \text{ OK}$$

$$T_{Ed,ser} \leq f_y / (\sqrt{3} \cdot \gamma_{M,ser})$$

$$48.246 \text{ MPa} \leq 204.959 \text{ MPa} \quad \dots \text{ OK}$$

$$\sqrt{(\sigma_{Ed,ser})^2 + 3(T_{Ed,ser})^2} \leq f_y / \gamma_{M,ser}$$

$$187.772 \text{ MPa} \leq 355.000 \text{ MPa} \quad \dots \text{ OK}$$

- In the concrete of the slab

Characteristic load combination name : ST RARA Mobili

$$\sigma_c \leq k_1 f_{ck}$$

$$0.000 \text{ MPa} \leq 19.200 \text{ MPa} \quad \dots \text{ OK}$$

- In the reinforcement

Load combination name : ST RARA Mobili

$$\sigma_s \leq k_3 f_{yk}$$

$$141.475 \text{ MPa} \leq 360.000 \text{ MPa} \quad \dots \text{ OK}$$

## 8 Longitudinal Shear for SLS(Serviceability limit state)

- Shear resistance of a single connector

Load combination name : ST RARA Mobili

$$P_{Rd,1} = 0.8 \cdot f_u \cdot \pi \cdot d^2 / 4 / \gamma_V = 111.478 \text{ kN}$$

$$P_{Rd,2} = 0.29 \cdot \alpha \cdot d^2 \cdot \sqrt{f_{ck} \cdot E_{cm}} / \gamma_V = 117.497 \text{ kN}$$

$$P_{Rd} = \text{Min}(P_{Rd,1}, P_{Rd,2}) = 111.478 \text{ kN}$$

$$P_{Rd,ser} = k_s \cdot P_{Rd} = 66.887 \text{ kN}$$

where,  $f_u = 450.000 \text{ MPa}$

$\alpha = 1$  for  $h_{sc}/d > 4$

Num. = 4

$d = 22.200 \text{ mm}$

$h_{sc} = 175.000 \text{ mm}$

Space = 200.000 mm

$k_s = 0.600$

- Verification

$$V_{L,Ed} = V_{Ed} \cdot (A \cdot z / I) = 880.219 \text{ kN/m}$$

$$V_{L,Rd} = P_{Rd,ser} \cdot \text{Num.}/\text{Space} = 1337.733 \text{ kN/m}$$

$$V_{L,Ed} \leq V_{L,Rd} \quad \dots \text{ OK}$$

## 8.1 VERIFICHE DI RESISTENZA A FLESSIONE ALLO SLU

Si riportano le verifiche allo stato limite ultimo per flessione delle sezioni composte travi-soletta.

I coefficienti parziali per SLU assunti sono i seguenti:

- $\gamma_C = 1.5$  materiale calcestruzzo
- $\gamma_S = 1.15$  acciaio armature soletta
- $\gamma_{M0} = 1.05$  acciaio strutturale: resistenza sezioni

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi positiva e negativa più gravose.

Dati tabulati:

- Elem Property: nome delle caratteristiche geometriche dell'elemento
- Elem: numero dell'elemento
- Position: nodo iniziale (I) o finale (J) dell'elemento
- Lcom: combinazione di carico positivo o negativo più gravosa
- Type: sollecitazione (massima o minima)
- Top class: classificazione flangia superiore
- Bot class: classificazione flangia inferiore
- Web class: classificazione anima
- Sect. class: classificazione complessiva sezione
- Ma,Ed: momento di calcolo applicato alla trave in acciaio isolata
- Mc,Ed: momento di calcolo applicato alla sezione composta
- Mpl,Rd: momento resistente plastico, per sezioni di classe 1 e 2
- Mel,Rd: momento resistente elastico, per sezioni di classe 3, efficace per classe 4
- M\_Rd: momento resistente effettivo della sezione

La condizione di verifica della sezione è la seguente:

$$\text{Verification ratio: } \frac{Ma,Ed + Mc,Ed}{M\_Rd} \leq 1$$

Elem property	Elem number	Position [node]	Positive/Negative	Lcom	Type	Top Class	Bot Class	Web Class	Sect. Class	Ma,Ed (kN*m)	Mc,Ed (kN*m)	Mpl,Rd (kN*m)	Mel,Rd (kN*m)	M_Rd (kN*m)	Verif. Ratio
Concio 1	101	I[719]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.54	8011.39	6350.05	6350.05	0.000
Concio 1	101	I[719]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.54	10750.90	9086.69	10750.90	0.000
Concio 1	101	J[720]	Neg	ST SLU Mobili	FY-MIN	1	3	3	3	-11.84	-7.57	8011.39	6361.88	6361.88	0.003
Concio 1	101	J[720]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	102	I[720]	Neg	ST SLU Mobili	MY-MIN	3	3	3	3	-28.84	-219.22	8011.39	6378.89	6378.89	0.039
Concio 1	102	I[720]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-28.84	234.58	10750.90	9099.36	10750.90	0.019
Concio 1	102	J[721]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	102	J[721]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1715.27	2913.83	10750.90	8332.74	10750.90	0.431
Concio 1	103	I[721]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	103	I[721]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1698.32	2694.39	10750.90	8340.20	10750.90	0.409
Concio 1	103	J[722]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	103	J[722]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2239.98	3469.25	10750.90	8102.11	10750.90	0.531
Concio 2	104	I[722]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	104	I[722]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2245.48	3474.74	12635.70	10220.13	12635.70	0.453
Concio 2	104	J[723]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1792.77	-35.27	9000.58	4849.14	9000.58	0.195
Concio 2	104	J[723]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2420.25	3991.54	12635.70	10144.81	12635.70	0.507
Concio 2	105	I[723]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1794.66	-15.93	9000.58	4847.26	9000.58	0.198
Concio 2	105	J[723]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2422.78	3925.98	12635.70	10143.72	12635.70	0.502
Concio 2	105	J[724]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2065.20	-320.02	9000.58	4576.72	9000.58	0.194
Concio 2	105	J[724]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2065.20	3534.51	12635.70	10297.83	12635.70	0.443
Concio 2	106	I[724]	Neg	ST SLU Frenamento	FZ-MIN	2	1	1	2	2075.58	-66.99	9000.58	4566.34	9000.58	0.223
Concio 2	106	I[724]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2075.58	3628.39	12635.70	10293.36	12635.70	0.451
Concio 2	106	J[725]	Neg	ST SLU Mobili	FX-MAX	2	1	1	2	1334.18	-13.39	9000.58	5307.74	9000.58	0.147
Concio 2	106	J[725]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1334.18	2392.44	12635.70	10612.89	12635.70	0.295
Concio 3	107	I[725]	Neg	ST SLU Mobili	FX-MAX	2	1	1	2	1333.75	-14.15	10249.33	6601.91	10249.33	0.129
Concio 3	107	I[725]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1333.75	2393.75	13386.70	11130.45	13386.70	0.278
Concio 3	107	J[726]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	649.93	-1506.13	10249.33	7285.73	10249.33	0.084
Concio 3	107	J[726]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	649.93	1684.35	13386.70	11454.11	13386.70	0.174
Concio 3	108	I[726]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	672.33	-1468.17	10249.33	7263.33	10249.33	0.078
Concio 3	108	I[726]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	672.33	1894.47	13386.70	11443.51	13386.70	0.192
Concio 3	108	J[727]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1780.61	-3803.37	10249.33	8981.76	10249.33	0.545



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Concio 3	108	J[727]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	109	I[727]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1773.87	-3798.71	10249.33	8982.85	10249.33	0.544
Concio 3	109	I[727]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	109	J[728]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2069.35	-4124.14	10249.33	8935.28	10249.33	0.604
Concio 3	109	J[728]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	110	I[728]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2066.80	-4138.85	13376.13	10789.26	10789.26	0.575
Concio 4 H=var	110	I[728]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	110	J[729]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3709.10	-5890.05	21530.83	16954.86	16954.86	0.566
Concio 4 H=var	110	J[729]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	111	I[729]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3712.67	-5918.69	21530.83	16954.36	16954.36	0.568
Concio 4 H=var	111	I[729]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	111	J[730]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7955.76	30530.77	23228.13	23228.13	0.583
Concio 4 H=var	111	J[730]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	112	I[730]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7955.76	30530.77	23227.77	23227.77	0.583
Concio 4 H=200	112	I[730]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	112	J[731]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5999.98	-8469.75	30530.77	23191.04	23191.04	0.624
Concio 4 H=200	112	J[731]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	113	I[731]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5993.07	-8529.23	30530.77	23191.67	23191.67	0.626
Concio 4 H=200	113	I[731]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	113	J[732]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-8014.01	30530.77	23226.12	23226.12	0.587
Concio 4 H=200	113	J[732]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	114	I[732]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-8014.01	30530.77	23226.28	23226.28	0.587
Concio 4 H=var	114	I[732]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	114	J[733]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3854.55	-6023.03	21530.83	16934.68	16934.68	0.583
Concio 4 H=var	114	J[733]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	115	I[733]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3846.81	-5992.83	21530.83	16935.75	16935.75	0.581
Concio 4 H=var	115	I[733]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	115	J[734]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2332.48	-4477.12	13376.13	10741.68	10741.68	0.634
Concio 4 H=var	115	J[734]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	116	I[734]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2335.71	-4433.49	10249.33	8738.08	10249.33	0.660
Concio 5	116	I[734]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	116	J[735]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2066.62	-4161.98	10249.33	8935.72	10249.33	0.608
Concio 5	116	J[735]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	117	I[735]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2068.61	-4129.44	10249.33	8935.40	10249.33	0.605
Concio 5	117	I[735]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	117	J[736]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	185.45	-2299.61	10249.33	7750.21	10249.33	0.206
Concio 5	117	J[736]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	185.45	1188.90	13386.70	11673.95	13386.70	0.103
Concio 5	118	I[736]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	165.39	-2251.76	10249.33	7770.27	10249.33	0.204
Concio 5	118	I[736]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	165.39	972.99	13386.70	11683.45	13386.70	0.085
Concio 5	118	J[737]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	797.40	-1892.73	10249.33	7138.26	10249.33	0.107
Concio 5	118	J[737]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.40	1591.79	13386.70	11384.31	13386.70	0.178
Concio 6	119	I[737]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	797.64	-1893.90	8775.51	5736.30	8775.51	0.125
Concio 6	119	I[737]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.64	1589.78	12181.58	10517.11	12181.58	0.196
Concio 6	119	J[738]	Neg	ST SLU Mobili	MX-MIN	2	1	2	2	1309.89	-80.26	8775.51	5224.04	8775.51	0.140
Concio 6	119	J[738]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1309.89	2548.79	12181.58	10310.73	12181.58	0.317
Concio 6	120	I[738]	Neg	ST SLU Termico	FZ-MIN	2	1	2	2	1303.38	-151.21	8775.51	5230.56	8775.51	0.131
Concio 6	120	I[738]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1303.38	2463.61	12181.58	10313.36	12181.58	0.309
Concio 6	120	J[739]	Neg	ST SLU Frenamento	MX-MIN	2	1	2	2	1429.64	-174.19	8775.51	5104.30	8775.51	0.143
Concio 6	120	J[739]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1429.64	2385.83	12181.58	10262.49	12181.58	0.313
Concio 6	121	I[739]	Neg	ST SLU Frenamento	MX-MIN	2	1	2	2	1429.64	-174.19	8775.51	5104.30	8775.51	0.143
Concio 6	121	I[739]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1429.64	2385.83	12181.58	10262.49	12181.58	0.313
Concio 6	121	J[740]	Neg	ST SLU Frenamento	MX-MIN	2	1	2	2	1303.38	-173.38	8775.51	5230.56	8775.51	0.129
Concio 6	121	J[740]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1303.38	2463.44	12181.58	10313.36	12181.58	0.309
Concio 6	122	I[740]	Neg	ST SLU Termico	MX-MIN	2	1	2	2	1309.89	-97.72	8775.51	5224.04	8775.51	0.138
Concio 6	122	I[740]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1309.89	2548.82	12181.58	10310.73	12181.58	0.317
Concio 6	122	J[741]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	797.64	-1893.75	8775.51	5736.30	8775.51	0.125
Concio 6	122	J[741]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.64	1589.62	12181.58	10517.12	12181.58	0.196
Concio 5	123	I[741]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	797.40	-1892.60	10249.33	7138.26	10249.33	0.107
Concio 5	123	I[741]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.40	1591.63	13386.70	11384.31	13386.70	0.178
Concio 5	123	J[742]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	165.39	-2251.48	10249.33	7770.27	10249.33	0.204
Concio 5	123	J[742]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	165.39	972.68	13386.70	11683.45	13386.70	0.085
Concio 5	124	I[742]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	185.45	-2299.41	10249.33	7750.21	10249.33	0.206
Concio 5	124	I[742]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	185.45	1188.66	13386.70	11673.95	13386.70	0.103
Concio 5	124	J[743]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2068.61	-4130.24	10249.33	8935.40	10249.33	0.605
Concio 5	124	J[743]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	125	I[743]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2066.62	-4162.71	10249.33	8935.72	10249.33	0.608
Concio 5	125	I[743]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	125	J[744]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2335.71	-4434.22	10249.33	8738.08	10249.33	0.661
Concio 5	125	J[744]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	126	I[744]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2332.49	-4477.90	13376.13	10741.68	10741.68	0.634
Concio 4 H=var	126	I[744]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	126	J[745]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3846.82	-5993.57	21530.83	16935.75	16935.75	0.581
Concio 4 H=var	126	J[745]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	127	I[745]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3854.55	-6024.18	21530.83	16934.68	16934.68	0.583
Concio 4 H=var	127	I[745]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	127	J[746]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-8014.29	30530.77	23226.28	23226.28	0.587
Concio 4 H=var	127	J[746]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	128	I[746]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-8014.29	30530.77	23226.12	23226.12	0.587
Concio 4 H=200	128	I[746]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	128	J[747]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5993.07	-8529.38	30530.77	23191.67	23191.67	0.626

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Concio 4 H=200	128	J[747]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	129	I[747]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5999.98	-8469.97	30530.77	23191.04	23191.04	0.624
Concio 4 H=200	129	I[747]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	129	J[748]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7955.85	30530.77	23227.77	23227.77	0.583
Concio 4 H=200	129	J[748]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	130	I[748]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7955.85	30530.77	23228.13	23228.13	0.583
Concio 4 H=var	130	J[748]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	130	J[749]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3712.67	-5917.90	21530.83	16954.36	16954.36	0.568
Concio 4 H=var	130	J[749]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	131	I[749]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3709.1	-5889.59	21530.83	16954.86	16954.9	0.566
Concio 4 H=var	131	I[749]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	131	J[750]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2066.80	-4138.29	13376.13	10789.26	10789.26	0.575
Concio 4 H=var	131	J[750]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	132	I[750]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2069.35	-4123.62	10249.33	8935.28	10249.33	0.604
Concio 3	132	I[750]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	132	J[751]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1773.87	-3798.11	10249.33	8982.85	10249.33	0.544
Concio 3	132	J[751]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	133	I[751]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1780.61	-3802.82	10249.33	8981.76	10249.33	0.545
Concio 3	133	I[751]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	133	J[752]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	672.33	-1468.36	10249.33	7263.33	10249.33	0.078
Concio 3	133	J[752]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	672.33	1894.73	13386.70	11443.51	13386.70	0.192
Concio 3	134	I[752]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	649.93	-1506.29	10249.33	7285.73	10249.33	0.084
Concio 3	134	I[752]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	649.93	1684.75	13386.70	11454.11	13386.70	0.174
Concio 3	134	J[753]	Neg	ST SLU Termico	FZ-MAX	2	1	1	2	1333.75	-0.41	10249.33	6601.91	10249.33	0.130
Concio 3	134	J[753]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1333.75	2393.98	13386.70	11130.45	13386.70	0.278
Concio 2	135	I[753]	Neg	ST SLU Mobili	FX-MAX	2	1	1	2	1334.18	-13.26	9000.58	5307.74	9000.58	0.147
Concio 2	135	I[753]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1334.18	2392.65	12635.70	10612.89	12635.70	0.295
Concio 2	135	J[754]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2075.58	-329.72	9000.58	4566.34	9000.58	0.194
Concio 2	135	J[754]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2075.58	3628.60	12635.70	10293.36	12635.70	0.451
Concio 2	136	I[754]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2065.20	-319.93	9000.58	4576.72	9000.58	0.194
Concio 2	136	I[754]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2065.20	3534.90	12635.70	10297.83	12635.70	0.443
Concio 2	136	J[755]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1794.66	-15.93	9000.58	4847.26	9000.58	0.198
Concio 2	136	J[755]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2422.78	3925.90	12635.70	10143.72	12635.70	0.502
Concio 2	137	I[755]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1792.77	-35.27	9000.58	4849.14	9000.58	0.195
Concio 2	137	I[755]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2420.25	3991.52	12635.70	10144.81	12635.70	0.507
Concio 2	137	J[756]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	137	J[756]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2245.48	3474.88	12635.70	10220.13	12635.70	0.453
Concio 1	138	I[756]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	138	I[756]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2239.98	3469.39	10750.90	8102.11	10750.90	0.531
Concio 1	138	J[757]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	138	J[757]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1698.31	2694.45	10750.90	8340.20	10750.90	0.409
Concio 1	139	I[757]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	139	I[757]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1715.27	2913.94	10750.90	8332.74	10750.90	0.431
Concio 1	139	J[758]	Neg	ST SLU Mobili	MY-MIN	3	3	3	3	-28.84	-219.33	8011.39	6378.89	6378.89	0.039
Concio 1	139	J[758]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-28.84	234.61	10750.90	9099.36	10750.90	0.019
Concio 1	140	I[758]	Neg	ST SLU Mobili	FX-MIN	1	3	3	3	-11.84	-7.56	8011.39	6361.88	6361.88	0.003
Concio 1	140	I[758]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	140	J[759]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.54	8011.39	6350.05	6350.05	0.000
Concio 1	140	J[759]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.54	10750.90	9086.69	10750.90	0.000
Concio 1	201	I[760]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	201	I[760]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	201	J[761]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-14.59	-342.22	8093.91	6493.67	6493.67	0.055
Concio 1	201	J[761]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	202	I[761]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-43.60	-508.53	8093.91	6522.69	6522.69	0.085
Concio 1	202	I[761]	Pos	ST SLU Frenamento	MY-MAX	1	3	1	3	-43.60	109.93	10954.92	9148.80	9148.80	0.007
Concio 1	202	J[762]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1311.79	-5.85	7686.47	5167.29	7686.47	0.170
Concio 1	202	J[762]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1770.92	3205.56	10954.92	8338.97	10954.92	0.454
Concio 1	203	I[762]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	203	I[762]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1739.67	2954.33	10954.92	8352.92	10954.92	0.428
Concio 1	203	J[763]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1710.79	-7.19	7686.47	4768.30	7686.47	0.222
Concio 1	203	J[763]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2309.56	3887.82	10954.92	8098.58	10954.92	0.566
Concio 2	204	I[763]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1718.06	-6.14	9113.78	5058.62	9113.78	0.188
Concio 2	204	I[763]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2319.39	3892.39	12846.08	10223.21	12846.08	0.484
Concio 2	204	J[764]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1829.61	-69.46	9113.78	4947.07	9113.78	0.193
Concio 2	204	J[764]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2469.97	4082.61	12846.08	10157.35	12846.08	0.510
Concio 2	205	I[764]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1831.79	-46.67	9113.78	4944.89	9113.78	0.196
Concio 2	205	I[764]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2472.91	4038.81	12846.08	10156.06	12846.08	0.507
Concio 2	205	J[765]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2119.09	-205.39	9113.78	4657.59	9113.78	0.210
Concio 2	205	J[765]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2119.09	3703.25	12846.08	10310.81	12846.08	0.453
Concio 2	206	I[765]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2132.13	-190.21	9113.78	4644.55	9113.78	0.213
Concio 2	206	I[765]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2132.13	3756.67	12846.08	10305.10	12846.08	0.458
Concio 2	206	J[766]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1041.46	-150.97	9113.78	5735.22	9113.78	0.098
Concio 2	206	J[766]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.98	2943.67	12846.08	10622.68	12846.08	0.339
Concio 3	207	I[766]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1040.76	-152.91	10397.59	7099.99	10397.59	0.085
Concio 3	207	I[766]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.03	2944.20	13602.15	11144.52	13602.15	0.320
Concio 3	207	J[767]	Neg	ST SLU Vento	MY-MIN	3	1	1	3	688.37	-1345.82	10397.59	7452.38	7452.38	0.088
Concio 3	207	J[767]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	688.37	2074.43	13602.15	11488.93	13602.15	0.203
Concio 3	208	I[767]	Neg	ST SLU Mobili	MY-MIN	3	1	1	3	723.94	-1314.74	10397.59	7416.82	7416.82	0.080
Concio 3	208	I[767]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	723.94	2269.94	13602.15	11471.84	13602.15	0.220
Concio 3	208	J[768]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1868.61	-3639.41	10397.59	9012.41	10397.59	0.530

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Concio 3	208	J[768]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	209	I[768]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1856.67	-3620.74	10397.59	9014.43	10397.59	0.527
Concio 3	209	I[768]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	209	J[769]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2177.49	-3913.46	10397.59	8960.42	10397.59	0.586
Concio 3	209	J[769]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	210	I[769]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2175.02	-3941.62	13534.54	10850.42	10850.42	0.564
Concio 4 H=var	210	I[769]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	210	J[770]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3976.11	-5671.43	21870.53	17021.59	17021.59	0.567
Concio 4 H=var	210	J[770]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	211	I[770]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3971.18	-5634.42	21870.53	17022.32	17022.32	0.564
Concio 4 H=var	211	I[770]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	211	J[771]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7840.84	31054.78	23251.25	23251.25	0.601
Concio 4 H=var	211	J[771]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	212	I[771]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7840.84	31054.78	23250.97	23250.97	0.601
Concio 4 H=200	212	I[771]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	212	J[772]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6590.37	-8339.12	31054.78	23206.68	23206.68	0.643
Concio 4 H=200	212	J[772]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	213	I[772]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6588.15	-8414.24	31054.78	23206.89	23206.89	0.646
Concio 4 H=200	213	I[772]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	213	J[773]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7845.60	31054.78	23248.67	23248.67	0.602
Concio 4 H=200	213	J[773]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	214	I[773]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7845.60	31054.78	23248.84	23248.84	0.602
Concio 4 H=var	214	I[773]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	214	J[774]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4126.44	-5793.63	21870.53	16999.37	16999.37	0.584
Concio 4 H=var	214	J[774]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	215	I[774]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4132.44	-5834.60	21870.53	16998.48	16998.48	0.586
Concio 4 H=var	215	I[774]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	215	J[775]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2459.01	-4297.51	13534.54	10796.42	10796.42	0.626
Concio 4 H=var	215	J[775]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	216	I[775]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2461.48	-4290.34	10397.59	8830.17	10397.59	0.649
Concio 5	216	I[775]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	216	J[776]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2166.19	-4019.61	10397.59	8962.33	10397.59	0.595
Concio 5	216	J[776]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	217	I[776]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2173.04	-4041.37	10397.59	8961.17	10397.59	0.598
Concio 5	217	I[776]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	217	J[777]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	239.95	-2198.24	10397.59	7900.80	10397.59	0.188
Concio 5	217	J[777]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	239.95	1524.86	13602.15	11704.43	13602.15	0.130
Concio 5	218	I[777]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	204.93	-2217.43	10397.59	7935.82	10397.59	0.194
Concio 5	218	I[777]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	204.93	1324.88	13602.15	11721.26	13602.15	0.112
Concio 5	218	J[778]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	877.49	-1855.47	10397.59	7263.27	10397.59	0.094
Concio 5	218	J[778]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	877.49	2118.94	13602.15	11398.04	13602.15	0.220
Concio 6	219	I[778]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	878.84	-1853.30	8891.06	5791.23	8891.06	0.110
Concio 6	219	I[778]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	878.84	2119.14	12388.93	10524.18	12388.93	0.242
Concio 6	219	J[779]	Neg	ST SLU Frenamento	FY-MIN	2	1	2	2	1377.38	-283.10	8891.06	5292.69	8891.06	0.123
Concio 6	219	J[779]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1377.38	2643.11	12388.93	10320.42	12388.93	0.325
Concio 6	220	I[779]	Neg	ST SLU Termico	FX-MIN	2	1	2	2	1367.28	-144.26	8891.06	5302.78	8891.06	0.138
Concio 6	220	I[779]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1367.28	2588.54	12388.93	10324.55	12388.93	0.319
Concio 6	220	J[780]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1127.86	-518.39	8891.06	5542.21	8891.06	0.069
Concio 6	220	J[780]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1522.61	2800.01	12388.93	10261.07	12388.93	0.349
Concio 6	221	I[780]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1127.86	-518.39	8891.06	5542.21	8891.06	0.069
Concio 6	221	J[780]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1522.61	2800.01	12388.93	10261.07	12388.93	0.349
Concio 6	221	J[781]	Neg	ST SLU Termico	FY-MIN	2	1	2	2	1367.28	-77.49	8891.06	5302.78	8891.06	0.145
Concio 6	221	J[781]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1367.28	2588.95	12388.93	10324.55	12388.93	0.319
Concio 6	222	I[781]	Neg	ST SLU Termico	MX-MIN	2	1	2	2	1377.38	-101.87	8891.06	5292.69	8891.06	0.143
Concio 6	222	J[781]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1377.38	2642.71	12388.93	10320.42	12388.93	0.324
Concio 6	222	J[782]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	878.84	-1853.22	8891.06	5791.23	8891.06	0.110
Concio 6	222	J[782]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	878.84	2118.64	12388.93	10524.18	12388.93	0.242
Concio 5	223	I[782]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	877.49	-1855.39	10397.59	7263.27	10397.59	0.094
Concio 5	223	J[782]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	877.49	2118.44	13602.15	11398.04	13602.15	0.220
Concio 5	223	J[783]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	204.93	-2217.30	10397.59	7935.82	10397.59	0.194
Concio 5	223	J[783]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	204.93	1325.59	13602.15	11721.26	13602.15	0.113
Concio 5	224	I[783]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	239.95	-2198.33	10397.59	7900.80	10397.59	0.188
Concio 5	224	I[783]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	239.95	1524.76	13602.15	11704.43	13602.15	0.130
Concio 5	224	J[784]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2173.03	-4041.81	10397.59	8961.17	10397.59	0.598
Concio 5	224	J[784]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	225	I[784]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2166.19	-4019.92	10397.59	8962.33	10397.59	0.595
Concio 5	225	I[784]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	225	J[785]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2461.47	-4290.64	10397.59	8830.17	10397.59	0.649
Concio 5	225	J[785]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	226	I[785]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2459.00	-4297.65	13534.54	10796.42	10796.42	0.626
Concio 4 H=var	226	I[785]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	226	J[786]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4132.44	-5834.68	21870.53	16998.48	16998.48	0.586
Concio 4 H=var	226	J[786]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	227	I[786]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4126.43	-5794.19	21870.53	16999.37	16999.37	0.584
Concio 4 H=var	227	I[786]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	227	J[787]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7845.95	31054.78	23248.84	23248.84	0.602
Concio 4 H=var	227	J[787]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	228	I[787]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7845.95	31054.78	23248.67	23248.67	0.602
Concio 4 H=200	228	I[787]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	228	J[788]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6588.15	-8414.41	31054.78	23206.89	23206.89	0.646

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Concio 4 H=200	228	J[788]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	229	I[788]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6590.37	-8339.18	31054.78	23206.68	23206.68	0.643
Concio 4 H=200	229	I[788]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	229	J[789]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7840.74	31054.78	23250.97	23250.97	0.601
Concio 4 H=200	229	J[789]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	230	I[789]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7840.74	31054.78	23251.25	23251.25	0.601
Concio 4 H=var	230	I[789]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	230	J[790]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3971.19	-5634.26	21870.53	17022.32	17022.32	0.564
Concio 4 H=var	230	J[790]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	231	I[790]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3976.12	-5671.54	21870.53	17021.59	17021.59	0.567
Concio 4 H=var	231	I[790]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	231	J[791]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2175.02	-3941.58	13534.54	10850.42	10850.42	0.564
Concio 4 H=var	231	J[791]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	232	I[791]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2177.49	-3913.34	10397.59	8960.42	10397.59	0.586
Concio 3	232	I[791]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	232	J[792]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1856.67	-3620.53	10397.59	9014.43	10397.59	0.527
Concio 3	232	J[792]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	233	I[792]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1868.62	-3639.07	10397.59	9012.41	10397.59	0.530
Concio 3	233	I[792]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	233	J[793]	Neg	ST SLU Mobili	MY-MIN	3	1	1	3	723.94	-1314.57	10397.59	7416.82	7416.82	0.080
Concio 3	233	J[793]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	723.94	2270.06	13602.15	11471.84	13602.15	0.220
Concio 3	234	I[793]	Neg	ST SLU Vento	MY-MIN	3	1	1	3	688.37	-1345.97	10397.59	7452.38	7452.38	0.088
Concio 3	234	I[793]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	688.37	2073.77	13602.15	11488.93	13602.15	0.203
Concio 3	234	J[794]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1040.76	-152.91	10397.59	7099.99	10397.59	0.085
Concio 3	234	J[794]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.03	2944.82	13602.15	11144.52	13602.15	0.320
Concio 2	235	I[794]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1041.46	-150.97	9113.78	5735.22	9113.78	0.098
Concio 2	235	I[794]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.98	2944.28	12846.08	10622.68	12846.08	0.339
Concio 2	235	J[795]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2132.13	-190.23	9113.78	4644.55	9113.78	0.213
Concio 2	235	J[795]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2132.13	3757.63	12846.08	10305.10	12846.08	0.458
Concio 2	236	I[795]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2119.09	-205.43	9113.78	4657.59	9113.78	0.210
Concio 2	236	I[795]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2119.09	3703.09	12846.08	10310.81	12846.08	0.453
Concio 2	236	J[796]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1831.79	-46.67	9113.78	4944.89	9113.78	0.196
Concio 2	236	J[796]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2472.91	4039.32	12846.08	10156.06	12846.08	0.507
Concio 2	237	I[796]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1829.61	-69.46	9113.78	4947.07	9113.78	0.193
Concio 2	237	I[796]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2469.97	4082.03	12846.08	10157.35	12846.08	0.510
Concio 2	237	J[797]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1718.06	-6.14	9113.78	5058.62	9113.78	0.188
Concio 2	237	J[797]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2319.39	3892.22	12846.08	10223.21	12846.08	0.484
Concio 1	238	I[797]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1710.79	-7.19	7686.47	4768.30	7686.47	0.222
Concio 1	238	I[797]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2309.56	3888.16	10954.92	8098.58	10954.92	0.566
Concio 1	238	J[798]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	238	J[798]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1739.67	2954.47	10954.92	8352.92	10954.92	0.428
Concio 1	239	I[798]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1311.79	-5.85	7686.47	5167.29	7686.47	0.170
Concio 1	239	I[798]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1770.92	3205.45	10954.92	8338.97	10954.92	0.454
Concio 1	239	J[799]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-43.60	-508.66	8093.91	6522.69	6522.69	0.085
Concio 1	239	J[799]	Pos	ST SLU Frenamento	MY-MAX	1	3	1	3	-43.60	109.86	10954.92	9148.80	9148.80	0.007
Concio 1	240	I[799]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-14.59	-342.19	8093.91	6493.67	6493.67	0.055
Concio 1	240	I[799]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	240	J[800]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	240	J[800]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	301	I[801]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	301	I[801]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	301	J[802]	Neg	ST SLU Mobili	FY-MAX	1	3	3	3	-14.59	-342.22	8093.91	6493.67	6493.67	0.055
Concio 1	301	J[802]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	302	I[802]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-43.60	-507.62	8093.91	6522.69	6522.69	0.085
Concio 1	302	I[802]	Pos	ST SLU Frenamento	MY-MAX	1	3	1	3	-43.60	109.93	10954.92	9148.80	9148.80	0.007
Concio 1	302	J[803]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1311.79	-5.85	7686.47	5167.29	7686.47	0.170
Concio 1	302	J[803]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1770.92	3211.15	10954.92	8338.97	10954.92	0.455
Concio 1	303	I[803]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	303	I[803]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1739.67	2956.43	10954.92	8352.92	10954.92	0.429
Concio 1	303	J[804]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1710.79	-7.19	7686.47	4768.30	7686.47	0.222
Concio 1	303	J[804]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2309.56	3891.56	10954.92	8098.58	10954.92	0.566
Concio 2	304	I[804]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1718.06	-6.14	9113.78	5058.62	9113.78	0.188
Concio 2	304	I[804]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2319.39	3896.17	12846.08	10223.21	12846.08	0.484
Concio 2	304	J[805]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1829.61	-69.46	9113.78	4947.07	9113.78	0.193
Concio 2	304	J[805]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2469.97	4087.64	12846.08	10157.35	12846.08	0.510
Concio 2	305	I[805]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1831.79	-46.67	9113.78	4944.89	9113.78	0.196
Concio 2	305	I[805]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2472.91	4041.96	12846.08	10156.06	12846.08	0.507
Concio 2	305	J[806]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2119.09	-207.88	9113.78	4657.59	9113.78	0.210
Concio 2	305	J[806]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2119.09	3705.75	12846.08	10310.81	12846.08	0.453
Concio 2	306	I[806]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2132.13	-194.90	9113.78	4644.55	9113.78	0.213
Concio 2	306	I[806]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2132.13	3761.36	12846.08	10305.10	12846.08	0.459
Concio 2	306	J[807]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1041.46	-150.97	9113.78	5735.22	9113.78	0.098
Concio 2	306	J[807]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.98	2946.09	12846.08	10622.68	12846.08	0.339
Concio 3	307	I[807]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1040.76	-152.91	10397.59	7099.99	10397.59	0.085
Concio 3	307	I[807]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.03	2946.65	13602.15	11144.52	13602.15	0.320
Concio 3	307	J[808]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	688.37	-1475.23	10397.59	7452.38	10397.59	0.076
Concio 3	307	J[808]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	688.37	2075.37	13602.15	11488.93	13602.15	0.203
Concio 3	308	I[808]	Neg	ST SLU Vento	MY-MIN	3	1	1	3	723.94	-1313.97	10397.59	7416.82	7416.82	0.080
Concio 3	308	I[808]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	723.94	2272.68	13602.15	11471.84	13602.15	0.220
Concio 3	308	J[809]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1868.61	-3642.92	10397.59	9012.41	10397.59	0.530

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Concio 3	308	J[809]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	309	I[809]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1856.67	-3623.79	10397.59	9014.43	10397.59	0.527
Concio 3	309	I[809]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	309	J[810]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2177.49	-3918.52	10397.59	8960.42	10397.59	0.586
Concio 3	309	J[810]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	310	I[810]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2175.02	-3942.81	13534.54	10850.42	10850.42	0.564
Concio 4 H=var	310	I[810]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	310	J[811]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3976.11	-5680.10	21870.53	17021.59	17021.59	0.567
Concio 4 H=var	310	J[811]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	311	I[811]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3971.18	-5641.27	21870.53	17022.32	17022.32	0.565
Concio 4 H=var	311	I[811]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	311	J[812]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7855.62	31054.78	23251.25	23251.25	0.601
Concio 4 H=var	311	J[812]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	312	I[812]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7855.62	31054.78	23250.97	23250.97	0.601
Concio 4 H=200	312	I[812]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	312	J[813]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6590.37	-8355.76	31054.78	23206.68	23206.68	0.644
Concio 4 H=200	312	J[813]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	313	I[813]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6588.15	-8431.02	31054.78	23206.89	23206.89	0.647
Concio 4 H=200	313	I[813]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	313	J[814]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7860.39	31054.78	23248.67	23248.67	0.603
Concio 4 H=200	313	J[814]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	314	I[814]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7860.39	31054.78	23248.84	23248.84	0.603
Concio 4 H=var	314	I[814]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	314	J[815]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4126.44	-5799.96	21870.53	16999.37	16999.37	0.584
Concio 4 H=var	314	J[815]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	315	I[815]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4132.44	-5842.90	21870.53	16998.48	16998.48	0.587
Concio 4 H=var	315	I[815]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	315	J[816]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2459.01	-4298.17	13534.54	10796.42	10796.42	0.626
Concio 4 H=var	315	J[816]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	316	I[816]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2461.48	-4295.78	10397.59	8830.17	10397.59	0.650
Concio 5	316	I[816]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	316	J[817]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2166.19	-4022.06	10397.59	8962.33	10397.59	0.595
Concio 5	316	J[817]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	317	I[817]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2173.04	-4045.63	10397.59	8961.17	10397.59	0.598
Concio 5	317	I[817]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	317	J[818]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	239.95	-2201.47	10397.59	7900.80	10397.59	0.189
Concio 5	317	J[818]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	239.95	1528.09	13602.15	11704.43	13602.15	0.130
Concio 5	318	I[818]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	204.93	-2219.32	10397.59	7935.82	10397.59	0.194
Concio 5	318	I[818]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	204.93	1326.76	13602.15	11721.26	13602.15	0.113
Concio 5	318	J[819]	Neg	ST SLU Mobili	MY-MIN	3	1	1	3	877.49	-1614.72	10397.59	7263.27	7263.27	0.102
Concio 5	318	J[819]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	877.49	2122.36	13602.15	11398.04	13602.15	0.221
Concio 6	319	I[819]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	878.84	-1856.65	8891.06	5791.23	8891.06	0.110
Concio 6	319	I[819]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	878.84	2122.49	12388.93	10524.18	12388.93	0.242
Concio 6	319	J[820]	Neg	ST SLU Termico	MX-MIN	2	1	2	2	1377.38	-107.61	8891.06	5292.69	8891.06	0.143
Concio 6	319	J[820]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1377.38	2648.40	12388.93	10320.42	12388.93	0.325
Concio 6	320	I[820]	Neg	ST SLU Termico	FY-MIN	2	1	2	2	1367.28	-127.89	8891.06	5302.78	8891.06	0.139
Concio 6	320	I[820]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1367.28	2592.04	12388.93	10324.55	12388.93	0.320
Concio 6	320	J[821]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1127.86	-518.39	8891.06	5542.21	8891.06	0.069
Concio 6	320	J[821]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1522.61	2803.50	12388.93	10261.07	12388.93	0.349
Concio 6	321	I[821]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1127.86	-518.39	8891.06	5542.21	8891.06	0.069
Concio 6	321	I[821]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1522.61	2803.50	12388.93	10261.07	12388.93	0.349
Concio 6	321	J[822]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1012.80	-550.49	8891.06	5657.27	8891.06	0.052
Concio 6	321	J[822]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1367.28	2592.44	12388.93	10324.55	12388.93	0.320
Concio 6	322	I[822]	Neg	ST SLU Termico	MZ-MIN	2	1	2	2	1377.38	-139.32	8891.06	5292.69	8891.06	0.139
Concio 6	322	I[822]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1377.38	2648.06	12388.93	10320.42	12388.93	0.325
Concio 6	322	J[823]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	878.84	-1856.59	8891.06	5791.23	8891.06	0.110
Concio 6	322	J[823]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	878.84	2122.01	12388.93	10524.18	12388.93	0.242
Concio 5	323	I[823]	Neg	ST SLU Mobili	MY-MIN	3	1	1	3	877.49	-1614.65	10397.59	7263.27	7263.27	0.101
Concio 5	323	J[823]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	877.49	2121.88	13602.15	11398.04	13602.15	0.221
Concio 5	323	J[824]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	204.93	-2219.17	10397.59	7935.82	10397.59	0.194
Concio 5	323	J[824]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	204.93	1327.46	13602.15	11721.26	13602.15	0.113
Concio 5	324	I[824]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	239.95	-2201.33	10397.59	7900.80	10397.59	0.189
Concio 5	324	I[824]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	239.95	1527.76	13602.15	11704.43	13602.15	0.130
Concio 5	324	J[825]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2173.03	-4046.19	10397.59	8961.17	10397.59	0.598
Concio 5	324	J[825]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	325	I[825]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2166.19	-4022.46	10397.59	8962.33	10397.59	0.595
Concio 5	325	I[825]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	325	J[826]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2461.47	-4296.16	10397.59	8830.17	10397.59	0.650
Concio 5	325	J[826]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	326	I[826]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2459.00	-4298.55	13534.54	10796.42	10796.42	0.626
Concio 4 H=var	326	I[826]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	326	J[827]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4132.44	-5843.26	21870.53	16998.48	16998.48	0.587
Concio 4 H=var	326	J[827]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	327	I[827]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4126.43	-5800.88	21870.53	16999.37	16999.37	0.584
Concio 4 H=var	327	I[827]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	327	J[828]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7860.73	31054.78	23248.84	23248.84	0.603
Concio 4 H=var	327	J[828]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	328	I[828]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6149.50	-7860.73	31054.78	23248.67	23248.67	0.603
Concio 4 H=200	328	I[828]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	328	J[829]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6588.15	-8431.09	31054.78	23206.89	23206.89	0.647



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Concio 4 H=200	328	J[829]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	329	I[829]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6590.37	-8355.90	31054.78	23206.68	23206.68	0.644
Concio 4 H=200	329	I[829]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	329	J[830]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7855.52	31054.78	23250.97	23250.97	0.601
Concio 4 H=200	329	J[830]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	330	I[830]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6125.40	-7855.52	31054.78	23251.25	23251.25	0.601
Concio 4 H=var	330	I[830]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	330	J[831]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3971.19	-5640.94	21870.53	17022.32	17022.32	0.565
Concio 4 H=var	330	J[831]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	331	I[831]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3976.12	-5679.91	21870.53	17021.59	17021.59	0.567
Concio 4 H=var	331	I[831]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	331	J[832]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2175.02	-3942.53	13534.54	10850.42	10850.42	0.564
Concio 4 H=var	331	J[832]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	332	I[832]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2177.49	-3918.30	10397.59	8960.42	10397.59	0.586
Concio 3	332	I[832]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	332	J[833]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1856.67	-3623.48	10397.59	9014.43	10397.59	0.527
Concio 3	332	J[833]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	333	I[833]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1868.62	-3642.53	10397.59	9012.41	10397.59	0.530
Concio 3	333	I[833]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	333	J[834]	Neg	ST SLU Vento	MY-MIN	3	1	1	3	723.94	-1314.15	10397.59	7416.82	7416.82	0.080
Concio 3	333	J[834]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	723.94	2273.02	13602.15	11471.84	13602.15	0.220
Concio 3	334	I[834]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	688.37	-1475.34	10397.59	7452.38	10397.59	0.076
Concio 3	334	I[834]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	688.37	2074.72	13602.15	11488.93	13602.15	0.203
Concio 3	334	J[835]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1040.76	-152.91	10397.59	7099.99	10397.59	0.085
Concio 3	334	J[835]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.03	2947.25	13602.15	11144.52	13602.15	0.320
Concio 2	335	I[835]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1041.46	-150.97	9113.78	5735.22	9113.78	0.098
Concio 2	335	I[835]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1405.98	2946.67	12846.08	10622.68	12846.08	0.339
Concio 2	335	J[836]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2132.13	-194.84	9113.78	4644.55	9113.78	0.213
Concio 2	335	J[836]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2132.13	3762.24	12846.08	10305.10	12846.08	0.459
Concio 2	336	I[836]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2119.09	-207.89	9113.78	4657.59	9113.78	0.210
Concio 2	336	I[836]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2119.09	3705.55	12846.08	10310.81	12846.08	0.453
Concio 2	336	J[837]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1831.79	-46.67	9113.78	4944.89	9113.78	0.196
Concio 2	336	J[837]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2472.91	4042.28	12846.08	10156.06	12846.08	0.507
Concio 2	337	I[837]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1829.61	-69.46	9113.78	4947.07	9113.78	0.193
Concio 2	337	I[837]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2469.97	4086.89	12846.08	10157.35	12846.08	0.510
Concio 2	337	J[838]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1718.06	-6.14	9113.78	5058.62	9113.78	0.188
Concio 2	337	J[838]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2319.39	3895.98	12846.08	10223.21	12846.08	0.484
Concio 1	338	I[838]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1710.79	-7.19	7686.47	4768.30	7686.47	0.222
Concio 1	338	I[838]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2309.56	3891.88	10954.92	8098.58	10954.92	0.566
Concio 1	338	J[839]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 1	338	J[839]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1739.67	2956.55	10954.92	8352.92	10954.92	0.429
Concio 1	339	I[839]	Neg	ST SLV Vert	FX-MIN	2	1	3	2	1311.79	-5.85	7686.47	5167.29	7686.47	0.170
Concio 1	339	I[839]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1770.92	3211.07	10954.92	8338.97	10954.92	0.455
Concio 1	339	J[840]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-43.60	-507.76	8093.91	6522.69	6522.69	0.085
Concio 1	339	J[840]	Pos	ST SLU Frenamento	MY-MAX	1	3	1	3	-43.60	109.86	10954.92	9148.80	9148.80	0.007
Concio 1	340	I[840]	Neg	ST SLU Mobili	FY-MAX	1	3	3	3	-14.59	-342.19	8093.91	6493.67	6493.67	0.055
Concio 1	340	I[840]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 1	340	J[841]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	340	J[841]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	401	I[842]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.54	8011.39	6350.05	6350.05	0.000
Concio 1	401	I[842]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.54	10750.90	9086.69	10750.90	0.000
Concio 1	401	J[843]	Neg	ST SLU Mobili	FX-MAX	1	3	3	3	-11.84	-7.57	8011.39	6361.88	6361.88	0.003
Concio 1	401	J[843]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 1	402	I[843]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-28.84	-218.60	8011.39	6378.89	6378.89	0.039
Concio 1	402	I[843]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-28.84	233.97	10750.90	9099.36	10750.90	0.019
Concio 1	402	J[844]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 1	402	J[844]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1715.27	2902.81	10750.90	8332.74	10750.90	0.430
Concio 1	403	I[844]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 1	403	I[844]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1698.32	2688.33	10750.90	8340.20	10750.90	0.408
Concio 1	403	J[845]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 1	403	J[845]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2239.98	3461.94	10750.90	8102.11	10750.90	0.530
Concio 2	404	I[845]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 2	404	I[845]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2245.48	3467.56	12635.70	10220.13	12635.70	0.452
Concio 2	404	J[846]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1792.77	-35.27	9000.58	4849.14	9000.58	0.195
Concio 2	404	J[846]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2420.25	3983.52	12635.70	10144.81	12635.70	0.507
Concio 2	405	I[846]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1794.66	-15.93	9000.58	4847.26	9000.58	0.198
Concio 2	405	I[846]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2422.78	3919.39	12635.70	10143.72	12635.70	0.502
Concio 2	405	J[847]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2065.20	-314.62	9000.58	4576.72	9000.58	0.194
Concio 2	405	J[847]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2065.20	3529.10	12635.70	10297.83	12635.70	0.443
Concio 2	406	I[847]	Neg	ST SLU Frenamento	FZ-MIN	2	1	1	2	2075.58	-66.99	9000.58	4566.34	9000.58	0.223
Concio 2	406	I[847]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2075.58	3621.15	12635.70	10293.36	12635.70	0.451
Concio 2	406	J[848]	Neg	ST SLU Mobili	FX-MAX	2	1	1	2	1334.18	-17.95	9000.58	5307.74	9000.58	0.146
Concio 2	406	J[848]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1334.18	2387.88	12635.70	10612.89	12635.70	0.295
Concio 3	407	I[848]	Neg	ST SLU Mobili	FX-MAX	2	1	1	2	1333.75	-18.70	10249.33	6601.91	10249.33	0.128
Concio 3	407	I[848]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1333.75	2389.20	13386.70	11130.45	13386.70	0.278
Concio 3	407	J[849]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	649.93	-1503.36	10249.33	7285.73	10249.33	0.083
Concio 3	407	J[849]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	649.93	1681.58	13386.70	11454.11	13386.70	0.174
Concio 3	408	I[849]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	672.33	-1460.74	10249.33	7263.33	10249.33	0.077
Concio 3	408	I[849]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	672.33	1887.04	13386.70	11443.51	13386.70	0.191
Concio 3	408	J[850]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1780.61	-3794.98	10249.33	8981.76	10249.33	0.544



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Concio 3	408	J[850]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	409	I[850]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1773.87	-3794.88	10249.33	8982.85	10249.33	0.543
Concio 3	409	I[850]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 3	409	J[851]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2069.35	-4117.79	10249.33	8935.28	10249.33	0.604
Concio 3	409	J[851]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	410	I[851]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2066.80	-4135.77	13376.13	10789.26	10789.26	0.575
Concio 4 H=var	410	I[851]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	410	J[852]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3709.10	-5879.70	21530.83	16954.86	16954.86	0.566
Concio 4 H=var	410	J[852]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	411	I[852]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3712.67	-5909.27	21530.83	16954.36	16954.36	0.568
Concio 4 H=var	411	I[852]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	411	J[853]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7938.38	30530.77	23228.13	23228.13	0.583
Concio 4 H=var	411	J[853]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	412	I[853]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7938.38	30530.77	23227.77	23227.77	0.583
Concio 4 H=200	412	I[853]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	412	J[854]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5999.98	-8450.57	30530.77	23191.04	23191.04	0.623
Concio 4 H=200	412	J[854]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	413	I[854]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5993.07	-8510.08	30530.77	23191.67	23191.67	0.625
Concio 4 H=200	413	I[854]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	413	J[855]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-7996.70	30530.77	23226.12	23226.12	0.586
Concio 4 H=200	413	J[855]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	414	I[855]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-7996.70	30530.77	23226.28	23226.28	0.586
Concio 4 H=var	414	I[855]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	414	J[856]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3854.55	-6013.86	21530.83	16934.68	16934.68	0.583
Concio 4 H=var	414	J[856]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	415	I[856]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3846.81	-5982.65	21530.83	16935.75	16935.75	0.580
Concio 4 H=var	415	I[856]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	415	J[857]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2332.48	-4474.00	13376.13	10741.68	10741.68	0.634
Concio 4 H=var	415	J[857]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	416	I[857]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2335.71	-4426.71	10249.33	8738.08	10249.33	0.660
Concio 5	416	I[857]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	416	J[858]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2066.62	-4157.66	10249.33	8935.72	10249.33	0.607
Concio 5	416	J[858]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	417	I[858]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2068.61	-4120.48	10249.33	8935.40	10249.33	0.604
Concio 5	417	I[858]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	417	J[859]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	185.45	-2292.60	10249.33	7750.21	10249.33	0.206
Concio 5	417	J[859]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	185.45	1181.88	13386.70	11673.95	13386.70	0.102
Concio 5	418	I[859]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	165.39	-2249.41	10249.33	7770.27	10249.33	0.203
Concio 5	418	I[859]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	165.39	970.65	13386.70	11683.45	13386.70	0.085
Concio 5	418	J[860]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	797.40	-1888.85	10249.33	7138.26	10249.33	0.106
Concio 5	418	J[860]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.40	1587.91	13386.70	11384.31	13386.70	0.178
Concio 6	419	I[860]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	797.64	-1890.00	8775.51	5736.30	8775.51	0.124
Concio 6	419	I[860]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.64	1585.89	12181.58	10517.11	12181.58	0.196
Concio 6	419	J[861]	Neg	ST SLU Termico	MX-MIN	2	1	2	2	1309.89	-92.44	8775.51	5224.04	8775.51	0.139
Concio 6	419	J[861]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1309.89	2542.96	12181.58	10310.73	12181.58	0.316
Concio 6	420	I[861]	Neg	ST SLU Mobili	MZ-MIN	2	1	2	2	1303.38	-48.20	8775.51	5230.56	8775.51	0.143
Concio 6	420	I[861]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1303.38	2459.24	12181.58	10313.36	12181.58	0.309
Concio 6	420	J[862]	Neg	ST SLU Frenamento	MX-MIN	2	1	2	2	1429.64	-201.08	8775.51	5104.30	8775.51	0.140
Concio 6	420	J[862]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1429.64	2381.46	12181.58	10262.49	12181.58	0.313
Concio 6	421	I[862]	Neg	ST SLU Frenamento	MX-MIN	2	1	2	2	1429.64	-201.08	8775.51	5104.30	8775.51	0.140
Concio 6	421	I[862]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1429.64	2381.46	12181.58	10262.49	12181.58	0.313
Concio 6	421	J[863]	Neg	ST SLU Mobili	MZ-MIN	2	1	2	2	1303.38	-48.14	8775.51	5230.56	8775.51	0.143
Concio 6	421	J[863]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1303.38	2459.08	12181.58	10313.36	12181.58	0.309
Concio 6	422	I[863]	Neg	ST SLU Mobili	MX-MIN	2	1	2	2	1309.89	-74.44	8775.51	5224.04	8775.51	0.141
Concio 6	422	I[863]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1309.89	2542.95	12181.58	10310.73	12181.58	0.316
Concio 6	422	J[864]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	797.64	-1889.87	8775.51	5736.30	8775.51	0.124
Concio 6	422	J[864]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.64	1585.74	12181.58	10517.12	12181.58	0.196
Concio 5	423	I[864]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	797.40	-1888.73	10249.33	7138.26	10249.33	0.106
Concio 5	423	J[864]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	797.40	1587.76	13386.70	11384.31	13386.70	0.178
Concio 5	423	J[865]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	165.39	-2249.18	10249.33	7770.27	10249.33	0.203
Concio 5	423	J[865]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	165.39	970.38	13386.70	11683.45	13386.70	0.085
Concio 5	424	I[865]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	185.45	-2292.42	10249.33	7750.21	10249.33	0.206
Concio 5	424	I[865]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	185.45	1181.66	13386.70	11673.95	13386.70	0.102
Concio 5	424	J[866]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2068.61	-4121.01	10249.33	8935.40	10249.33	0.604
Concio 5	424	J[866]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	425	I[866]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2066.62	-4158.13	10249.33	8935.72	10249.33	0.607
Concio 5	425	I[866]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 5	425	J[867]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2335.71	-4427.17	10249.33	8738.08	10249.33	0.660
Concio 5	425	J[867]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	426	I[867]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2332.49	-4474.42	13376.13	10741.68	10741.68	0.634
Concio 4 H=var	426	I[867]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	426	J[868]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3846.82	-5982.96	21530.83	16935.75	16935.75	0.580
Concio 4 H=var	426	J[868]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	427	I[868]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3854.55	-6014.53	21530.83	16934.68	16934.68	0.583
Concio 4 H=var	427	I[868]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=var	427	J[869]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-7996.85	30530.77	23226.28	23226.28	0.586
Concio 4 H=var	427	J[869]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	428	I[869]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5613.22	-7996.85	30530.77	23226.12	23226.12	0.586
Concio 4 H=200	428	I[869]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.00
Concio 4 H=200	428	J[870]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5993.07	-8510.17	30530.77	23191.67	23191.67	0.625

Concio 4 H=200	428	J[870]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	429	I[870]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5999.98	-8450.85	30530.77	23191.04	23191.04	0.623
Concio 4 H=200	429	I[870]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	429	J[871]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7938.61	30530.77	23227.77	23227.77	0.583
Concio 4 H=200	429	J[871]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	430	I[871]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-5595.01	-7938.61	30530.77	23228.13	23228.13	0.583
Concio 4 H=var	430	I[871]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	430	J[872]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3712.67	-5908.96	21530.83	16954.36	16954.36	0.568
Concio 4 H=var	430	J[872]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	431	I[872]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-3709.10	-5879.67	21530.83	16954.86	16954.86	0.566
Concio 4 H=var	431	I[872]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	431	J[873]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2066.80	-4135.57	13376.13	10789.26	10789.26	0.575
Concio 4 H=var	431	J[873]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	432	I[873]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2069.35	-4117.53	10249.33	8935.28	10249.33	0.604
Concio 3	432	I[873]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	432	J[874]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1773.87	-3794.53	10249.33	8982.85	10249.33	0.543
Concio 3	432	J[874]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	433	I[874]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1780.61	-3794.70	10249.33	8981.76	10249.33	0.544
Concio 3	433	I[874]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	433	J[875]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	672.33	-1460.92	10249.33	7263.33	10249.33	0.077
Concio 3	433	J[875]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	672.33	1887.29	13386.70	11443.51	13386.70	0.191
Concio 3	434	I[875]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	649.93	-1503.49	10249.33	7285.73	10249.33	0.083
Concio 3	434	I[875]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	649.93	1681.95	13386.70	11454.11	13386.70	0.174
Concio 3	434	J[876]	Neg	ST SLU Termico	FZ-MAX	2	1	1	2	1333.75	-4.95	10249.33	6601.91	10249.33	0.130
Concio 3	434	J[876]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1333.75	2389.44	13386.70	11130.45	13386.70	0.278
Concio 2	435	I[876]	Neg	ST SLU Termico	FZ-MAX	2	1	1	2	1334.18	-4.39	9000.58	5307.74	9000.58	0.148
Concio 2	435	I[876]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1334.18	2388.10	12635.70	10612.89	12635.70	0.295
Concio 2	435	J[877]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2075.58	-322.54	9000.58	4566.34	9000.58	0.195
Concio 2	435	J[877]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2075.58	3621.42	12635.70	10293.36	12635.70	0.451
Concio 2	436	I[877]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2065.20	-314.56	9000.58	4576.72	9000.58	0.195
Concio 2	436	I[877]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2065.20	3529.52	12635.70	10297.83	12635.70	0.443
Concio 2	436	J[878]	Neg	ST SLU Vert	FX-MIN	2	1	1	2	1794.66	-15.93	9000.58	4847.26	9000.58	0.198
Concio 2	436	J[878]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2422.78	3919.35	12635.70	10143.72	12635.70	0.502
Concio 2	437	I[878]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1792.77	-35.27	9000.58	4849.14	9000.58	0.195
Concio 2	437	I[878]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2420.25	3983.53	12635.70	10144.81	12635.70	0.507
Concio 2	437	J[879]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	437	J[879]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2245.48	3467.72	12635.70	10220.13	12635.70	0.452
Concio 1	438	I[879]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	438	J[879]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2239.98	3462.10	10750.90	8102.11	10750.90	0.530
Concio 1	438	J[880]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	438	J[880]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1698.31	2688.41	10750.90	8340.20	10750.90	0.408
Concio 1	439	I[880]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	439	I[880]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1715.27	2902.93	10750.90	8332.74	10750.90	0.430
Concio 1	439	J[881]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-28.84	-218.72	8011.39	6378.89	6378.89	0.039
Concio 1	439	J[881]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-28.84	233.99	10750.90	9099.36	10750.90	0.019
Concio 1	440	I[881]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-11.84	-7.56	8011.39	6361.88	6361.88	0.003
Concio 1	440	I[881]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	440	J[882]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.54	8011.39	6350.05	6350.05	0.000
Concio 1	440	J[882]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.54	10750.90	9086.69	10750.90	0.000

## 8.2 VERIFICHE DI RESISTENZA AL TAGLIO VERTICALE ALLO SLU

Si effettuano le verifiche allo stato limite ultimo per taglio verticale delle sezioni composte travi-soletta.

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

- Elem Property: nome delle caratteristiche geometriche dell'elemento
- Elem: numero dell'elemento
- Position: nodo iniziale (I) o finale (J) dell'elemento
- Lcom: combinazione di carico My positivo o negativo più gravosa
- Type: sollecitazione (massima o minima)
- Top class: classificazione flangia superiore
- Bot class: classificazione flangia inferiore
- Web class: classificazione anima
- Sect. class: classificazione complessiva sezione
- N\_Ed: sforzo assiale di calcolo
- M\_Ed: momento di calcolo
- V\_Ed: sforzo tagliante di calcolo

Vpl,Rd: resistenza al taglio della sezione di acciaio (≥ V\_Ed)  
 Vb,Rd: resistenza all'instabilità per taglio dell'anima di acciaio (≥ V\_Ed)

La condizione di verifica della sezione è la seguente:

$$\text{Verification ratio: } \frac{V\_Ed}{\min(V_{pl,Rd}, V_{b,Rd})} \leq 1$$

Elem property	Elem number	Position	Lcom	Type	Top Class	Bot Class	Web Class	Sect. Class	N_Ed (kN)	M_Ed (kN*m)	V_Ed (kN)	Vpl,Rd (kN)	Vb,Rd (kN)	Verif. Ratio
Concio 1	101	I[719]	ST SLV Vert	FX-MAX	1	1	1	1	0.80	4.11	1.81	3302.77	2574.42	0.001
Concio 1	101	J[720]	ST SLU Mobili	MZ-MAX	1	3	3	3	0.00	25.65	48.50	3302.77	2574.41	0.019
Concio 1	102	I[720]	ST SLU Mobili	FZ-MIN	3	3	4	4	-172.65	285.67	-1107.88	3302.77	2574.22	0.430
Concio 1	102	J[721]	ST SLU Mobili	FZ-MIN	1	1	1	1	-172.65	95283.96	-804.73	3302.77	2478.97	0.325
Concio 1	103	I[721]	ST SLU Mobili	FZ-MIN	1	1	1	1	-251.07	95409.25	-603.23	3302.77	2478.97	0.243
Concio 1	103	J[722]	ST SLU Mobili	FZ-MIN	1	1	1	1	-251.07	124961.03	-421.34	3302.77	2478.97	0.170
Concio 2	104	I[722]	ST SLU Mobili	FZ-MIN	1	1	1	1	-251.07	70591.38	-421.34	3267.64	2478.30	0.170
Concio 2	104	J[723]	ST SLU Mobili	FZ-MIN	1	1	1	1	-251.07	76200.21	-298.69	3267.64	2478.30	0.121
Concio 2	105	I[723]	ST SLU Mobili	FZ-MAX	1	1	1	1	-17.09	74893.28	141.75	3267.64	2478.30	0.057
Concio 2	105	J[724]	ST SLU Mobili	FZ-MAX	1	1	1	1	-13.75	63569.64	440.77	3267.64	2478.30	0.178
Concio 2	106	I[724]	ST SLU Mobili	FZ-MAX	1	1	1	1	-50.59	64951.10	630.34	3267.64	2478.30	0.254
Concio 2	106	J[725]	ST SLU Mobili	FZ-MAX	1	1	1	1	-50.59	41883.29	814.33	3267.64	2478.30	0.329
Concio 3	107	I[725]	ST SLU Mobili	FZ-MAX	1	1	1	1	-50.59	35199.81	814.33	4356.85	4158.81	0.196
Concio 3	107	J[726]	ST SLU Mobili	FZ-MAX	1	1	1	1	-50.59	17505.76	938.10	4356.85	4158.81	0.226
Concio 3	108	I[726]	ST SLU Mobili	FZ-MAX	1	1	1	1	76.27	17736.56	1109.60	4356.85	4158.81	0.267
Concio 3	108	J[727]	ST SLU Mobili	FZ-MAX	1	1	1	1	76.27	5128.13	1419.02	4356.85	4158.81	0.341
Concio 3	109	I[727]	ST SLU Mobili	FZ-MAX	1	1	1	1	247.72	4824.28	1521.02	4356.85	4158.81	0.366
Concio 3	109	J[728]	ST SLU Mobili	FZ-MAX	1	1	1	1	247.72	5767.50	1551.96	4356.85	4158.81	0.373
Concio 4 H=var	110	I[728]	ST SLU Mobili	FZ-MAX	1	3	3	3	169.02	6495.90	1560.34	4310.00	4114.09	0.379
Concio 4 H=var	110	J[729]	ST SLU Mobili	FZ-MAX	1	3	3	3	165.50	11173.87	1725.22	6652.39	4865.83	0.355
Concio 4 H=var	111	I[729]	ST SLU Mobili	FZ-MAX	1	3	3	3	179.41	10899.91	1800.64	6652.39	4875.19	0.369
Concio 4 H=var	111	J[730]	ST SLU Mobili	FZ-MAX	1	3	4	4	175.61	14143.60	1968.43	8994.79	5364.31	0.367
Concio 4 H=200	112	I[730]	ST SLU Mobili	FZ-MAX	1	3	4	4	266.91	14297.30	1963.55	8994.79	5360.12	0.366
Concio 4 H=200	112	J[731]	ST SLU Mobili	FZ-MAX	1	3	4	4	266.91	15341.45	1997.40	8994.79	5330.47	0.375
Concio 4 H=200	113	I[731]	ST SLU Mobili	FZ-MIN	1	3	4	4	91.32	16455.92	-1863.44	8994.79	5296.51	0.352
Concio 4 H=200	113	J[732]	ST SLU Mobili	FZ-MIN	1	3	4	4	91.32	15395.96	-1829.59	8994.79	5328.86	0.343
Concio 4 H=var	114	I[732]	ST SLU Mobili	FZ-MIN	1	3	4	4	-6.49	15277.81	-1837.48	8994.79	5332.17	0.345
Concio 4 H=var	114	J[733]	ST SLU Mobili	FZ-MIN	1	3	3	3	-2.69	12058.86	-1669.70	6652.39	4833.91	0.345
Concio 4 H=var	115	I[733]	ST SLU Mobili	FZ-MIN	1	3	3	3	-22.82	12454.65	-1590.30	6652.39	4818.33	0.330
Concio 4 H=var	115	J[734]	ST SLU Mobili	FZ-MIN	1	3	3	3	-19.31	8080.62	-1425.42	4310.00	4114.09	0.346
Concio 5	116	I[734]	ST SLU Mobili	FZ-MIN	1	1	1	1	56.23	7279.81	-1412.49	4356.85	4158.81	0.340
Concio 5	116	J[735]	ST SLU Mobili	FZ-MIN	1	1	1	1	56.23	6389.18	-1381.55	4356.85	4158.81	0.332
Concio 5	117	I[735]	ST SLU Mobili	FZ-MIN	1	1	1	1	-55.75	6768.93	-1278.87	4356.85	4158.81	0.308
Concio 5	117	J[736]	ST SLU Mobili	FZ-MIN	1	1	1	1	-55.75	4541.40	-957.07	4356.85	4158.81	0.230
Concio 5	118	I[736]	ST SLU Mobili	FZ-MIN	2	1	1	2	-216.84	206.66	-785.51	4356.85	4158.81	0.189
Concio 5	118	J[737]	ST SLU Mobili	FZ-MIN	1	1	1	1	-216.84	20768.69	-643.17	4356.85	4158.81	0.155
Concio 6	119	I[737]	ST SLU Mobili	FZ-MIN	1	1	1	1	-216.84	28233.76	-643.17	2614.11	1592.23	0.404
Concio 6	119	J[738]	ST SLU Mobili	FZ-MIN	1	1	1	1	-216.84	46415.28	-466.29	2614.11	1592.23	0.293
Concio 6	120	I[738]	ST SLU Mobili	FZ-MIN	1	1	1	1	-247.92	45796.00	-277.18	2614.11	1592.23	0.174
Concio 6	120	J[739]	ST SLU Mobili	FZ-MIN	1	1	1	1	-247.92	50402.47	-118.59	2614.11	1592.23	0.074
Concio 6	121	I[740]	ST SLU Mobili	FZ-MAX	1	1	1	1	3.35	45584.57	277.17	2614.11	1592.23	0.174
Concio 6	122	I[740]	ST SLU Mobili	FZ-MAX	1	1	1	1	5.16	46327.45	466.33	2614.11	1592.23	0.293
Concio 6	122	J[741]	ST SLU Mobili	FZ-MAX	1	1	1	1	5.16	28116.97	643.21	2614.11	1592.23	0.404
Concio 5	123	I[741]	ST SLU Mobili	FZ-MAX	1	1	1	1	5.16	20830.81	643.21	4356.85	4158.81	0.155
Concio 5	123	J[742]	ST SLU Mobili	FZ-MAX	1	1	1	1	5.16	4420.62	785.55	4356.85	4158.81	0.189
Concio 5	124	I[742]	ST SLU Mobili	FZ-MAX	1	1	1	1	112.70	4785.95	957.15	4356.85	4158.81	0.230
Concio 5	124	J[743]	ST SLU Mobili	FZ-MAX	1	1	1	1	112.70	6212.96	1278.95	4356.85	4158.81	0.308
Concio 5	125	I[743]	ST SLU Mobili	FZ-MAX	1	1	1	1	262.01	5860.25	1381.55	4356.85	4158.81	0.332
Concio 5	125	J[744]	ST SLU Mobili	FZ-MAX	1	1	1	1	262.01	6730.11	1412.49	4356.85	4158.81	0.340
Concio 4 H=var	126	I[744]	ST SLU Mobili	FZ-MAX	1	3	3	3	188.70	7548.67	1425.43	4310.00	4114.09	0.346
Concio 4 H=var	126	J[745]	ST SLU Mobili	FZ-MAX	1	3	3	3	185.18	11766.75	1590.31	6652.39	4844.76	0.328
Concio 4 H=var	127	I[745]	ST SLU Mobili	FZ-MAX	1	3	3	3	185.50	11456.29	1669.45	6652.39	4855.93	0.344
Concio 4 H=var	127	J[746]	ST SLU Mobili	FZ-MAX	1	3	4	4	181.70	14293.79	1837.24	8994.79	5360.22	0.343
Concio 4 H=200	128	I[746]	ST SLU Mobili	FZ-MAX	1	3	4	4	267.92	14400.93	1829.34	8994.79	5357.27	0.341
Concio 4 H=200	128	J[747]	ST SLU Mobili	FZ-MAX	1	3	4	4	267.92	15376.11	1863.19	8994.79	5329.45	0.350
Concio 4 H=200	129	I[747]	ST SLU Mobili	FZ-MIN	1	3	4	4	77.74	16426.91	-1997.70	8994.79	5297.42	0.377
Concio 4 H=200	129	J[748]	ST SLU Mobili	FZ-MIN	1	3	4	4	77.74	15290.61	-1963.85	8994.79	5331.96	0.368
Concio 4 H=var	130	I[748]	ST SLU Mobili	FZ-MIN	1	3	4	4	-26.92	15104.90	-1968.71	8994.79	5336.70	0.369
Concio 4 H=var	130	J[749]	ST SLU Mobili	FZ-MIN	1	3	3	3	-23.12	11468.21	-1800.92	6652.39	4854.97	0.371
Concio 4 H=var	131	I[749]	ST SLU Mobili	FZ-MIN	1	3	3	3	-38.04	11827.16	-1725.25	6652.39	4841.61	0.356
Concio 4 H=var	131	J[750]	ST SLU Mobili	FZ-MIN	1	3	3	3	-34.53	6920.71	-1560.37	4310.00	4114.09	0.379
Concio 3	132	I[750]	ST SLU Mobili	FZ-MIN	1	1	1	1	48.18	6246.71	-1552.00	4356.85	4158.81	0.373
Concio 3	132	J[751]	ST SLU Mobili	FZ-MIN	1	1	1	1	48.18	5257.43	-1521.06	4356.85	4158.81	0.366

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Concio 3	133	I[751]	ST SLU Mobili	FZ-MIN	1	1	1	1	-71.96	5615.80	-1418.98	4356.85	4158.81	0.341
Concio 3	133	J[752]	ST SLU Mobili	FZ-MIN	1	1	1	1	-71.96	17650.34	-1109.55	4356.85	4158.81	0.267
Concio 3	134	I[752]	ST SLU Mobili	FZ-MIN	1	1	1	1	-210.01	17504.88	-938.08	4356.85	4158.81	0.226
Concio 3	134	J[753]	ST SLU Mobili	FZ-MIN	1	1	1	1	-210.01	35211.24	-814.32	4356.85	4158.81	0.196
Concio 2	135	I[753]	ST SLU Mobili	FZ-MIN	1	1	1	1	-210.01	41964.59	-814.32	3267.64	2478.30	0.329
Concio 2	135	J[754]	ST SLU Mobili	FZ-MIN	1	1	1	1	-210.01	65050.84	-630.33	3267.64	2478.30	0.254
Concio 2	136	I[754]	ST SLU Mobili	FZ-MIN	1	1	1	1	-193.95	63742.99	-440.76	3267.64	2478.30	0.178
Concio 2	136	J[755]	ST SLU Mobili	FZ-MIN	1	1	1	1	-197.29	75145.41	-141.73	3267.64	2478.30	0.057
Concio 2	137	I[755]	ST SLU Mobili	FZ-MAX	1	1	1	1	-81.95	76014.51	298.68	3267.64	2478.30	0.121
Concio 2	137	J[756]	ST SLU Mobili	FZ-MAX	1	1	1	1	-81.95	70355.01	421.34	3267.64	2478.30	0.170
Concio 1	138	I[756]	ST SLU Mobili	FZ-MAX	1	1	1	1	-81.95	124308.92	421.34	3302.77	2478.97	0.170
Concio 1	138	J[757]	ST SLU Mobili	FZ-MAX	1	1	1	1	-81.95	94681.12	603.23	3302.77	2478.97	0.243
Concio 1	139	I[757]	ST SLU Mobili	FZ-MAX	1	1	1	1	-55.68	94929.10	804.75	3302.77	2478.97	0.325
Concio 1	139	J[758]	ST SLU Mobili	FZ-MAX	3	3	3	3	-55.68	178.19	1107.89	3302.77	2574.34	0.430
Concio 1	140	I[758]	ST SLU Mobili	FZ-MIN	1	3	3	3	0.00	25.65	-48.50	3302.77	2574.41	0.019
Concio 1	140	J[759]	ST SLV Vert	FX-MAX	1	1	1	1	0.80	4.11	1.81	3302.77	2574.42	0.001
Concio 1	201	I[760]	ST SLV Vert	FX-MAX	1	1	1	1	0.82	6.84	1.91	3302.77	2574.42	0.001
Concio 1	201	J[761]	ST SLU Mobili	FZ-MAX	1	3	3	3	0.00	365.04	486.98	3302.77	2574.10	0.189
Concio 1	202	I[761]	ST SLU Mobili	FZ-MIN	3	3	4	4	-201.40	420.01	-1727.32	3302.77	2573.99	0.671
Concio 1	202	J[762]	ST SLU Mobili	FZ-MIN	1	1	1	1	-193.80	165228.94	-837.66	3302.77	2478.97	0.338
Concio 1	203	I[762]	ST SLU Mobili	FZ-MIN	1	1	1	1	-233.80	165729.63	-1124.82	3302.77	2478.97	0.454
Concio 1	203	J[763]	ST SLU Mobili	FZ-MIN	1	1	1	1	-207.28	217247.10	-636.97	3302.77	2478.97	0.257
Concio 2	204	I[763]	ST SLU Mobili	FZ-MIN	1	1	1	1	-207.28	94382.65	-636.97	3267.64	2478.30	0.257
Concio 2	204	J[764]	ST SLU Mobili	FZ-MAX	1	1	1	1	-10.77	98463.76	575.09	3267.64	2478.30	0.232
Concio 2	205	I[764]	ST SLU Mobili	FZ-MIN	1	1	1	1	-190.76	99567.92	-680.80	3267.64	2478.30	0.275
Concio 2	205	J[765]	ST SLU Mobili	FZ-MAX	1	1	1	1	-100.21	85928.09	1003.71	3267.64	2478.30	0.405
Concio 2	206	I[765]	ST SLU Mobili	FZ-MAX	1	1	1	1	-118.05	86329.75	667.12	3267.64	2478.30	0.269
Concio 2	206	J[766]	ST SLU Mobili	FZ-MAX	1	1	1	1	-5.71	57627.46	1132.24	3267.64	2478.30	0.457
Concio 3	207	I[766]	ST SLU Mobili	FZ-MAX	1	1	1	1	-5.71	46791.37	1132.24	4356.85	4158.81	0.272
Concio 3	207	J[767]	ST SLU Mobili	FZ-MAX	1	1	1	1	-50.49	24076.48	1452.63	4356.85	4158.81	0.349
Concio 3	208	I[767]	ST SLU Mobili	FZ-MAX	1	1	1	1	-6.95	24228.93	1173.06	4356.85	4158.81	0.282
Concio 3	208	J[768]	ST SLU Mobili	FZ-MAX	1	1	1	1	66.55	4581.62	1965.08	4356.85	4158.81	0.473
Concio 3	209	I[768]	ST SLU Mobili	FZ-MAX	1	1	1	1	45.92	4186.35	1751.12	4356.85	4158.81	0.421
Concio 3	209	J[769]	ST SLU Mobili	FZ-MAX	1	1	1	1	92.77	5172.48	1826.87	4356.85	4158.81	0.439
Concio 4 H=var	210	I[769]	ST SLU Mobili	FZ-MAX	1	3	3	3	23.18	6011.45	1820.72	4310.00	4114.09	0.443
Concio 4 H=var	210	J[770]	ST SLU Mobili	FZ-MAX	1	3	3	3	182.70	11183.69	2257.83	6652.39	4869.00	0.464
Concio 4 H=var	211	I[770]	ST SLU Mobili	FZ-MAX	1	3	3	3	97.47	10774.80	2156.38	6652.39	4882.65	0.442
Concio 4 H=var	211	J[771]	ST SLU Mobili	FZ-MAX	1	3	4	4	180.62	14325.15	2602.75	8994.79	5362.09	0.485
Concio 4 H=200	212	I[771]	ST SLU Mobili	FZ-MAX	1	3	4	4	284.44	14461.06	2598.30	8994.79	5358.39	0.485
Concio 4 H=200	212	J[772]	ST SLU Mobili	FZ-MAX	1	3	4	4	260.88	15692.94	2685.08	8994.79	5323.29	0.504
Concio 4 H=200	213	I[772]	ST SLU Mobili	FZ-MIN	1	3	4	4	53.06	16584.53	-2546.40	8994.79	5296.10	0.481
Concio 4 H=200	213	J[773]	ST SLU Mobili	FZ-MIN	1	3	4	4	66.02	15360.03	-2459.67	8994.79	5333.06	0.461
Concio 4 H=var	214	I[773]	ST SLU Mobili	FZ-MIN	1	3	4	4	-61.65	15157.70	-2464.33	8994.79	5337.36	0.462
Concio 4 H=var	214	J[774]	ST SLU Mobili	FZ-MIN	1	3	3	3	-142.09	11842.41	-2017.97	6652.39	4842.45	0.417
Concio 4 H=var	215	I[774]	ST SLU Mobili	FZ-MIN	1	3	3	3	2.24	12299.87	-2122.78	6652.39	4829.14	0.440
Concio 4 H=var	215	J[775]	ST SLU Mobili	FZ-MIN	1	3	3	3	-162.34	7593.42	-1686.61	4310.00	4114.09	0.410
Concio 5	216	I[775]	ST SLU Mobili	FZ-MIN	1	1	1	1	-77.07	6654.94	-1692.82	4356.85	4158.81	0.407
Concio 5	216	J[776]	ST SLU Mobili	FZ-MIN	1	1	1	1	-113.55	5749.55	-1617.22	4356.85	4158.81	0.389
Concio 5	217	I[776]	ST SLU Mobili	FZ-MIN	1	1	1	1	22.97	6196.83	-1839.17	4356.85	4158.81	0.442
Concio 5	217	J[777]	ST SLU Mobili	FZ-MIN	1	1	1	1	-53.82	8155.68	-1018.21	4356.85	4158.81	0.245
Concio 5	218	I[777]	ST SLU Mobili	FZ-MIN	1	1	1	1	-30.10	7803.23	-1309.68	4356.85	4158.81	0.315
Concio 5	218	J[778]	ST SLU Mobili	FZ-MIN	1	1	1	1	12.19	29144.59	-941.94	4356.85	4158.81	0.226
Concio 6	219	I[778]	ST SLU Mobili	FZ-MIN	1	1	1	1	12.19	42184.76	-941.94	2614.11	1592.23	0.592
Concio 6	219	J[779]	ST SLU Mobili	FZ-MIN	1	1	1	1	-105.54	65410.68	-497.29	2614.11	1592.23	0.312
Concio 6	220	I[779]	ST SLU Mobili	FZ-MIN	1	1	1	1	-97.57	65068.01	-846.98	2614.11	1592.23	0.532
Concio 6	220	J[780]	ST SLU Mobili	FZ-MIN	1	1	1	1	-52.49	72328.46	-431.71	2614.11	1592.23	0.271
Concio 6	221	I[780]	ST SLU Mobili	FZ-MIN	1	1	1	1	-52.49	72328.46	-431.71	2614.11	1592.23	0.271
Concio 6	221	J[781]	ST SLU Mobili	FZ-MAX	1	1	1	1	5.29	65084.43	846.80	2614.11	1592.23	0.532
Concio 6	222	I[781]	ST SLU Mobili	FZ-MAX	1	1	1	1	-19.19	65628.96	497.15	2614.11	1592.23	0.312
Concio 6	222	J[782]	ST SLU Mobili	FZ-MAX	1	1	1	1	98.55	42389.95	941.76	2614.11	1592.23	0.591
Concio 5	223	I[782]	ST SLU Mobili	FZ-MAX	1	1	1	1	98.55	29455.07	941.76	4356.85	4158.81	0.226
Concio 5	223	J[783]	ST SLU Mobili	FZ-MAX	1	1	1	1	56.34	8103.31	1309.52	4356.85	4158.81	0.315
Concio 5	224	I[783]	ST SLU Mobili	FZ-MAX	1	1	1	1	54.47	8445.54	1018.09	4356.85	4158.81	0.245
Concio 5	224	J[784]	ST SLU Mobili	FZ-MAX	1	1	1	1	131.34	5627.24	1838.93	4356.85	4158.81	0.442
Concio 5	225	I[784]	ST SLU Mobili	FZ-MAX	1	1	1	1	53.85	5231.35	1616.98	4356.85	4158.81	0.389
Concio 5	225	J[785]	ST SLU Mobili	FZ-MAX	1	1	1	1	101.03	6114.91	1692.57	4356.85	4158.81	0.407
Concio 4 H=var	226	I[785]	ST SLU Mobili	FZ-MAX	1	3	3	3	34.14	7079.50	1686.37	4310.00	4114.09	0.410
Concio 4 H=var	226	J[786]	ST SLU Mobili	FZ-MAX	1	3	3	3	194.27	11721.08	2122.60	6652.39	4850.28	0.438
Concio 4 H=var	227	I[786]	ST SLU Mobili	FZ-MAX	1	3	3	3	106.49	11314.32	2017.68	6652.39	4864.53	0.415
Concio 4 H=var	227	J[787]	ST SLU Mobili	FZ-MAX	1	3	4	4	193.75	14419.33	2464.01	8994.79	5359.53	0.460
Concio 4 H=200	228	I[787]	ST SLU Mobili	FZ-MAX	1	3	3	3	291.68	16891.65	2459.35	8994.79	5286.38	0.465
Concio 4 H=200	228	J[788]	ST SLU Mobili	FZ-MAX	1	3	4	4	270.43	15705.27	2546.08	8994.79	5322.93	0.478
Concio 4 H=200	229	I[788]	ST SLU Mobili	FZ-MIN	1	3	4	4	58.05	16561.63	-2685.40	8994.79	5296.81	0.507
Concio 4 H=200	229	J[789]	ST SLU Mobili	FZ-MIN	1	3	4	4	71.46	15255.87	-2598.63	8994.79	5336.07	0.487
Concio 4 H=var	230	I[789]	ST SLU Mobili	FZ-MIN	1	3	4	4	-60.06	15046.52	-2603.07	8994.79	5340.60	0.487
Concio 4 H=var	230	J[790]	ST SLU Mobili	FZ-MIN	1	3	3	3	-138.09	11238.58	-2156.68	6652.39	4864.08	0.443
Concio 4 H=var	231	I[790]	ST SLU Mobili	FZ-MIN	1	3	3	3	10.36	11699.59	-2258.02	6652.39	4851.05	0.465
Concio 4 H=var	231	J[791]	ST SLU Mobili	FZ-MIN	1	3	3	3	-155.75	6453.58	-1820.97	4310.00	4114.09	0.443
Concio 3	232	I[791]	ST SLU Mobili	FZ-MIN	1	1	1	1	-65.17	5596.83	-1827.13	4356.85	4158.81	0.439
Concio 3	232	J[792]	ST SLU Mobili	FZ-MIN	1	1	1	1	-103.33	4620.58	-1751.37	4356.85	4158.81	0.421



Concio 3	233	I[792]	ST SLU Mobili	FZ-MIN	1	1	1	1	-17.84	5033.37	-1965.32	4356.85	4158.81	0.473
Concio 3	233	J[793]	ST SLU Mobili	FZ-MIN	1	1	1	1	-91.27	24029.52	-1173.19	4356.85	4158.81	0.282
Concio 3	234	I[793]	ST SLU Mobili	FZ-MIN	1	1	1	1	-121.79	23889.61	-1452.83	4356.85	4158.81	0.349
Concio 3	234	J[794]	ST SLU Mobili	FZ-MIN	1	1	1	1	-76.94	46626.92	-1132.47	4356.85	4158.81	0.272
Concio 2	235	I[794]	ST SLU Mobili	FZ-MIN	1	1	1	1	-76.94	57508.59	-1132.47	3267.64	2478.30	0.457
Concio 2	235	J[795]	ST SLU Mobili	FZ-MIN	1	1	1	1	-189.23	86244.49	-667.36	3267.64	2478.30	0.269
Concio 2	236	I[795]	ST SLU Mobili	FZ-MIN	1	1	1	1	-191.02	85923.45	-1004.00	3267.64	2478.30	0.405
Concio 2	236	J[796]	ST SLU Mobili	FZ-MAX	1	1	1	1	-99.98	99493.23	680.52	3267.64	2478.30	0.275
Concio 2	237	I[796]	ST SLU Mobili	FZ-MIN	1	1	1	1	-120.75	98579.33	-575.32	3267.64	2478.30	0.232
Concio 2	237	J[797]	ST SLU Mobili	FZ-MAX	1	1	1	1	-97.32	94223.26	636.73	3267.64	2478.30	0.257
Concio 1	238	I[797]	ST SLU Mobili	FZ-MAX	1	1	1	1	-97.32	216535.65	636.73	3302.77	2478.97	0.257
Concio 1	238	J[798]	ST SLU Mobili	FZ-MAX	1	1	1	1	-123.75	164951.31	1124.54	3302.77	2478.97	0.454
Concio 1	239	I[798]	ST SLU Mobili	MY-MAX	1	1	1	1	-71.25	164428.75	837.44	3302.77	2478.97	0.338
Concio 1	239	J[799]	ST SLU Mobili	FZ-MAX	3	3	3	3	-78.72	302.14	1726.93	3302.77	2574.20	0.671
Concio 1	240	I[799]	ST SLU Mobili	MY-MIN	1	3	3	3	0.00	365.02	-486.98	3302.77	2574.10	0.189
Concio 1	240	J[800]	ST SLV Vert	FX-MAX	1	1	1	1	0.82	6.84	1.91	3302.77	2574.42	0.001
Concio 1	301	I[801]	ST SLV Vert	FX-MAX	1	1	1	1	0.82	6.84	1.91	3302.77	2574.42	0.001
Concio 1	301	J[802]	ST SLU Mobili	FY-MAX	1	3	3	3	0.00	365.04	486.98	3302.77	2574.10	0.189
Concio 1	302	I[802]	ST SLU Mobili	FZ-MIN	1	3	4	4	-198.83	418.03	-1728.26	3302.77	2573.99	0.671
Concio 1	302	J[803]	ST SLU Mobili	FZ-MIN	1	1	1	1	-191.22	165200.99	-838.60	3302.77	2478.97	0.338
Concio 1	303	I[803]	ST SLU Mobili	FZ-MIN	1	1	1	1	-232.53	165716.45	-1125.37	3302.77	2478.97	0.454
Concio 1	303	J[804]	ST SLU Mobili	FZ-MIN	1	1	1	1	-206.00	217232.27	-637.52	3302.77	2478.97	0.257
Concio 2	304	I[804]	ST SLU Mobili	FZ-MIN	1	1	1	1	-206.00	94374.23	-637.52	3267.64	2478.30	0.257
Concio 2	304	J[805]	ST SLU Mobili	FZ-MAX	1	1	1	1	-12.04	98473.27	575.61	3267.64	2478.30	0.232
Concio 2	305	I[805]	ST SLU Mobili	FZ-MIN	1	1	1	1	-189.08	99558.80	-680.90	3267.64	2478.30	0.275
Concio 2	305	J[806]	ST SLU Mobili	FZ-MAX	1	1	1	1	-101.89	85936.70	1003.81	3267.64	2478.30	0.405
Concio 2	306	I[806]	ST SLU Mobili	FZ-MAX	1	1	1	1	-122.38	86350.16	667.88	3267.64	2478.30	0.269
Concio 2	306	J[807]	ST SLU Mobili	FZ-MAX	1	1	1	1	-10.04	57645.60	1133.00	3267.64	2478.30	0.457
Concio 3	307	I[807]	ST SLU Mobili	FZ-MAX	1	1	1	1	-10.04	46806.74	1133.00	4356.85	4158.81	0.272
Concio 3	307	J[808]	ST SLU Mobili	FZ-MAX	1	1	1	1	-54.82	24090.33	1453.39	4356.85	4158.81	0.349
Concio 3	308	I[808]	ST SLU Mobili	FZ-MAX	1	1	1	1	-16.69	24260.70	1174.23	4356.85	4158.81	0.282
Concio 3	308	J[809]	ST SLU Mobili	FZ-MAX	1	1	1	1	56.82	4576.37	1966.25	4356.85	4158.81	0.473
Concio 3	309	I[809]	ST SLU Mobili	FZ-MAX	1	1	1	1	40.58	4182.35	1752.51	4356.85	4158.81	0.421
Concio 3	309	J[810]	ST SLU Mobili	FZ-MAX	1	1	1	1	87.43	5166.47	1828.27	4356.85	4158.81	0.440
Concio 4 H=var	310	I[810]	ST SLU Mobili	FZ-MAX	1	3	3	3	18.17	6008.81	1823.08	4310.00	4114.09	0.443
Concio 4 H=var	310	J[811]	ST SLU Mobili	FZ-MAX	1	3	3	3	177.69	11173.06	2260.18	6652.39	4869.36	0.464
Concio 4 H=var	311	I[811]	ST SLU Mobili	FZ-MAX	1	3	3	3	93.37	10766.34	2159.32	6652.39	4882.93	0.442
Concio 4 H=var	311	J[812]	ST SLU Mobili	FZ-MAX	1	3	4	4	176.52	14308.78	2605.69	8994.79	5362.53	0.486
Concio 4 H=200	312	I[812]	ST SLU Mobili	FZ-MAX	1	3	4	4	279.78	14444.47	2600.18	8994.79	5358.85	0.485
Concio 4 H=200	312	J[813]	ST SLU Mobili	FZ-MAX	1	3	4	4	256.23	15674.50	2686.96	8994.79	5323.84	0.505
Concio 4 H=200	313	I[813]	ST SLU Mobili	FZ-MIN	1	3	4	4	58.36	16603.35	-2548.27	8994.79	5295.50	0.481
Concio 4 H=200	313	J[814]	ST SLU Mobili	FZ-MIN	1	3	4	4	71.32	15376.87	-2461.54	8994.79	5332.57	0.462
Concio 4 H=var	314	I[814]	ST SLU Mobili	FZ-MIN	1	3	4	4	-56.92	15179.26	-2467.31	8994.79	5336.86	0.462
Concio 4 H=var	314	J[815]	ST SLU Mobili	FZ-MIN	1	3	3	3	-137.36	11850.58	-2020.95	6652.39	4842.27	0.417
Concio 4 H=var	315	I[815]	ST SLU Mobili	FZ-MIN	1	3	3	3	8.79	12310.73	-2125.27	6652.39	4828.74	0.440
Concio 4 H=var	315	J[816]	ST SLU Mobili	FZ-MIN	1	3	3	3	-155.79	7595.99	-1689.10	4310.00	4114.09	0.411
Concio 5	316	I[816]	ST SLU Mobili	FZ-MIN	1	1	1	1	-70.18	6661.60	-1694.21	4356.85	4158.81	0.407
Concio 5	316	J[817]	ST SLU Mobili	FZ-MIN	1	1	1	1	-106.66	5753.23	-1618.61	4356.85	4158.81	0.389
Concio 5	317	I[817]	ST SLU Mobili	FZ-MIN	1	1	1	1	33.19	6202.91	-1840.37	4356.85	4158.81	0.443
Concio 5	317	J[818]	ST SLU Mobili	FZ-MIN	1	1	1	1	-43.60	8121.97	-1019.41	4356.85	4158.81	0.245
Concio 5	318	I[818]	ST SLU Mobili	FZ-MIN	1	1	1	1	-25.25	7786.88	-1310.35	4356.85	4158.81	0.315
Concio 5	318	J[819]	ST SLU Mobili	FZ-MIN	1	1	1	1	17.03	29126.71	-942.61	4356.85	4158.81	0.227
Concio 6	319	I[819]	ST SLU Mobili	FZ-MIN	1	1	1	1	17.03	42160.99	-942.61	2614.11	1592.23	0.592
Concio 6	319	J[820]	ST SLU Mobili	FZ-MIN	1	1	1	1	-100.69	65384.98	-497.95	2614.11	1592.23	0.313
Concio 6	320	I[820]	ST SLU Mobili	FZ-MIN	1	1	1	1	-93.80	65048.62	-846.98	2614.11	1592.23	0.532
Concio 6	320	J[821]	ST SLU Mobili	FZ-MIN	1	1	1	1	-48.72	72309.08	-431.71	2614.11	1592.23	0.271
Concio 6	321	I[821]	ST SLU Mobili	FZ-MIN	1	1	1	1	-48.72	72309.08	-431.71	2614.11	1592.23	0.271
Concio 6	321	J[822]	ST SLU Mobili	FZ-MAX	1	1	1	1	1.51	65103.81	846.80	2614.11	1592.23	0.532
Concio 6	322	I[822]	ST SLU Mobili	FZ-MAX	1	1	1	1	-23.95	65654.36	497.83	2614.11	1592.23	0.313
Concio 6	322	J[823]	ST SLU Mobili	FZ-MAX	1	1	1	1	93.79	42413.37	942.44	2614.11	1592.23	0.592
Concio 5	323	I[823]	ST SLU Mobili	FZ-MAX	1	1	1	1	93.79	29472.71	942.44	4356.85	4158.81	0.227
Concio 5	323	J[824]	ST SLU Mobili	FZ-MAX	1	1	1	1	51.58	8119.39	1310.20	4356.85	4158.81	0.315
Concio 5	324	I[824]	ST SLU Mobili	FZ-MAX	1	1	1	1	44.59	8478.03	1019.27	4356.85	4158.81	0.245
Concio 5	324	J[825]	ST SLU Mobili	FZ-MAX	1	1	1	1	121.46	5621.09	1840.11	4356.85	4158.81	0.442
Concio 5	325	I[825]	ST SLU Mobili	FZ-MAX	1	1	1	1	47.07	5227.60	1618.36	4356.85	4158.81	0.389
Concio 5	325	J[826]	ST SLU Mobili	FZ-MAX	1	1	1	1	94.24	6108.18	1693.95	4356.85	4158.81	0.407
Concio 4 H=var	326	I[826]	ST SLU Mobili	FZ-MAX	1	3	3	3	27.69	7076.72	1688.84	4310.00	4114.09	0.411
Concio 4 H=var	326	J[827]	ST SLU Mobili	FZ-MAX	1	3	3	3	187.82	11709.99	2125.07	6652.39	4850.68	0.438
Concio 4 H=var	327	I[827]	ST SLU Mobili	FZ-MAX	1	3	3	3	101.91	11305.84	2020.73	6652.39	4864.82	0.415
Concio 4 H=var	327	J[828]	ST SLU Mobili	FZ-MAX	1	3	4	4	189.17	14402.77	2467.07	8994.79	5359.98	0.460
Concio 4 H=200	328	I[828]	ST SLU Mobili	FZ-MAX	1	3	4	4	286.52	14528.99	2461.30	8994.79	5356.53	0.459
Concio 4 H=200	328	J[829]	ST SLU Mobili	FZ-MAX	1	3	4	4	265.28	15686.60	2548.03	8994.79	5323.48	0.479
Concio 4 H=200	329	I[829]	ST SLU Mobili	FZ-MIN	1	3	4	4	62.85	16580.20	-2687.21	8994.79	5296.23	0.507
Concio 4 H=200	329	J[830]	ST SLU Mobili	FZ-MIN	1	3	4	4	76.26	15272.50	-2600.44	8994.79	5335.59	0.487
Concio 4 H=var	330	I[830]	ST SLU Mobili	FZ-MIN	1	3	4	4	-55.82	15067.36	-2605.94	8994.79	5340.11	0.488
Concio 4 H=var	330	J[831]	ST SLU Mobili	FZ-MIN	1	3	3	3	-133.85	11246.72	-2159.56	6652.39	4863.89	0.444
Concio 4 H=var	331	I[831]	ST SLU Mobili	FZ-MIN	1	3	3	3	15.47	11709.96	-2260.41	6652.39	4850.68	0.466
Concio 4 H=var	331	J[832]	ST SLU Mobili	FZ-MIN	1	3	3	3	-150.64	6456.01	-1823.36	4310.00	4114.09	0.443
Concio 3	332	I[832]	ST SLU Mobili	FZ-MIN	1	1	1	1	-59.74	5602.77	-1828.55	4356.85	4158.81	0.440

Concio 3	332	J[833]	ST SLU Mobili	FZ-MIN	1	1	1	1	-97.89	4624.50	-1752.79	4356.85	4158.81	0.421
Concio 3	333	J[833]	ST SLU Mobili	FZ-MIN	1	1	1	1	-8.04	5038.58	-1966.52	4356.85	4158.81	0.473
Concio 3	333	J[834]	ST SLU Mobili	FZ-MIN	1	1	1	1	-81.48	23997.34	-1174.39	4356.85	4158.81	0.282
Concio 3	334	J[834]	ST SLU Mobili	FZ-MIN	1	1	1	1	-117.36	23875.43	-1453.57	4356.85	4158.81	0.350
Concio 3	334	J[835]	ST SLU Mobili	FZ-MIN	1	1	1	1	-72.51	46611.26	-1133.21	4356.85	4158.81	0.272
Concio 2	335	J[835]	ST SLU Mobili	FZ-MIN	1	1	1	1	-72.51	57490.09	-1133.21	3267.64	2478.30	0.457
Concio 2	335	J[836]	ST SLU Mobili	FZ-MIN	1	1	1	1	-184.79	86223.78	-668.10	3267.64	2478.30	0.270
Concio 2	336	J[836]	ST SLU Mobili	FZ-MIN	1	1	1	1	-189.30	85914.75	-1004.09	3267.64	2478.30	0.405
Concio 2	336	J[837]	ST SLU Mobili	FZ-MAX	1	1	1	1	-101.70	99502.43	680.60	3267.64	2478.30	0.275
Concio 2	337	J[837]	ST SLU Mobili	FZ-MIN	1	1	1	1	-119.46	98569.79	-575.87	3267.64	2478.30	0.232
Concio 2	337	J[838]	ST SLU Mobili	FZ-MAX	1	1	1	1	-98.60	94231.70	637.28	3267.64	2478.30	0.257
Concio 1	338	J[838]	ST SLU Mobili	FZ-MAX	1	1	1	1	-98.60	216550.56	637.28	3302.77	2478.97	0.257
Concio 1	338	J[839]	ST SLU Mobili	FZ-MAX	1	1	1	1	-125.04	164964.57	1125.09	3302.77	2478.97	0.454
Concio 1	339	J[839]	ST SLU Mobili	MY-MAX	1	1	1	1	-73.83	164456.75	838.39	3302.77	2478.97	0.338
Concio 1	339	J[840]	ST SLU Mobili	FZ-MAX	1	3	3	3	-81.30	303.37	1727.87	3302.77	2574.20	0.671
Concio 1	340	J[840]	ST SLU Mobili	FY-MAX	1	3	3	3	0.00	365.02	-486.98	3302.77	2574.10	0.189
Concio 1	340	J[841]	ST SLV Vert	FX-MAX	1	1	1	1	0.82	6.84	1.91	3302.77	2574.42	0.001
Concio 1	401	J[842]	ST SLV Vert	FX-MAX	1	1	1	1	0.80	4.11	1.81	3302.77	2574.42	0.001
Concio 1	401	J[843]	ST SLU Mobili	MZ-MIN	1	3	3	3	0.00	25.65	48.50	3302.77	2574.41	0.019
Concio 1	402	J[843]	ST SLU Mobili	FZ-MIN	1	3	4	4	-171.29	284.49	-1105.60	3302.77	2574.22	0.429
Concio 1	402	J[844]	ST SLU Mobili	FZ-MIN	1	1	1	1	-171.29	95287.55	-802.45	3302.77	2478.97	0.324
Concio 1	403	J[844]	ST SLU Mobili	FZ-MIN	1	1	1	1	-246.70	95391.40	-602.81	3302.77	2478.97	0.243
Concio 1	403	J[845]	ST SLU Mobili	FZ-MIN	1	1	1	1	-246.70	124944.44	-420.92	3302.77	2478.97	0.170
Concio 2	404	J[845]	ST SLU Mobili	FZ-MIN	1	1	1	1	-246.70	70585.54	-420.92	3267.64	2478.30	0.170
Concio 2	404	J[846]	ST SLU Mobili	FZ-MIN	1	1	1	1	-246.70	76195.21	-298.27	3267.64	2478.30	0.120
Concio 2	405	J[846]	ST SLU Mobili	FZ-MAX	1	1	1	1	-21.62	74900.17	141.41	3267.64	2478.30	0.057
Concio 2	405	J[847]	ST SLU Mobili	FZ-MAX	1	1	1	1	-18.27	63577.71	440.43	3267.64	2478.30	0.178
Concio 2	406	J[847]	ST SLU Mobili	FZ-MAX	1	1	1	1	-51.96	64947.93	629.45	3267.64	2478.30	0.254
Concio 2	406	J[848]	ST SLU Mobili	FZ-MAX	1	1	1	1	-51.96	41882.79	813.44	3267.64	2478.30	0.328
Concio 3	407	J[848]	ST SLU Mobili	FZ-MAX	1	1	1	1	-51.96	35198.72	813.44	4356.85	4158.81	0.196
Concio 3	407	J[849]	ST SLU Mobili	FZ-MAX	1	1	1	1	-51.96	17506.45	937.21	4356.85	4158.81	0.225
Concio 3	408	J[849]	ST SLU Mobili	FZ-MAX	1	1	1	1	82.24	17714.03	1106.81	4356.85	4158.81	0.266
Concio 3	408	J[850]	ST SLU Mobili	FZ-MAX	1	1	1	1	82.24	5137.61	1416.23	4356.85	4158.81	0.341
Concio 3	409	J[850]	ST SLU Mobili	FZ-MAX	1	1	1	1	253.93	4829.25	1517.55	4356.85	4158.81	0.365
Concio 3	409	J[851]	ST SLU Mobili	FZ-MAX	1	1	1	1	253.93	5774.99	1548.49	4356.85	4158.81	0.372
Concio 4 H=var	410	J[851]	ST SLU Mobili	FZ-MAX	1	3	3	3	175.52	6500.86	1557.44	4310.00	4114.09	0.379
Concio 4 H=var	410	J[852]	ST SLU Mobili	FZ-MAX	1	3	3	3	172.00	11186.76	1722.32	6652.39	4865.38	0.354
Concio 4 H=var	411	J[852]	ST SLU Mobili	FZ-MAX	1	3	3	3	173.68	10907.09	1797.47	6652.39	4874.95	0.369
Concio 4 H=var	411	J[853]	ST SLU Mobili	FZ-MAX	1	3	4	4	169.87	14158.71	1965.25	8994.79	5363.90	0.366
Concio 4 H=200	412	J[853]	ST SLU Mobili	FZ-MAX	1	3	4	4	263.30	14313.25	1959.96	8994.79	5359.69	0.366
Concio 4 H=200	412	J[854]	ST SLU Mobili	FZ-MAX	1	3	4	4	263.30	15359.20	1993.81	8994.79	5329.94	0.374
Concio 4 H=200	413	J[854]	ST SLU Mobili	FZ-MIN	1	3	4	4	93.34	16437.58	-1859.76	8994.79	5297.09	0.351
Concio 4 H=200	413	J[855]	ST SLU Mobili	FZ-MIN	1	3	4	4	93.34	15379.46	-1825.91	8994.79	5329.35	0.343
Concio 4 H=var	414	J[855]	ST SLU Mobili	FZ-MIN	1	3	4	4	-2.22	15266.43	-1834.24	8994.79	5332.61	0.344
Concio 4 H=var	414	J[856]	ST SLU Mobili	FZ-MIN	1	3	3	3	1.58	12051.36	-1666.46	6652.39	4834.26	0.345
Concio 4 H=var	415	J[856]	ST SLU Mobili	FZ-MIN	1	3	3	3	-29.64	12441.81	-1587.49	6652.39	4818.64	0.329
Concio 4 H=var	415	J[857]	ST SLU Mobili	FZ-MIN	1	3	3	3	-26.13	8075.54	-1422.61	4310.00	4114.09	0.346
Concio 5	416	J[857]	ST SLU Mobili	FZ-MIN	1	1	1	1	49.69	7271.83	-1409.07	4356.85	4158.81	0.339
Concio 5	416	J[858]	ST SLU Mobili	FZ-MIN	1	1	1	1	49.69	6383.67	-1378.13	4356.85	4158.81	0.331
Concio 5	417	J[858]	ST SLU Mobili	FZ-MIN	1	1	1	1	-62.10	6758.81	-1276.22	4356.85	4158.81	0.307
Concio 5	417	J[859]	ST SLU Mobili	FZ-MIN	1	1	1	1	-62.10	4564.49	-954.42	4356.85	4158.81	0.229
Concio 5	418	J[859]	ST SLU Mobili	FZ-MIN	2	1	1	2	-215.89	204.16	-784.84	4356.85	4158.81	0.189
Concio 5	418	J[860]	ST SLU Mobili	FZ-MIN	1	1	1	1	-215.89	20770.17	-642.51	4356.85	4158.81	0.154
Concio 6	419	J[860]	ST SLU Mobili	FZ-MIN	1	1	1	1	-215.89	28234.47	-642.51	2614.11	1592.23	0.404
Concio 6	419	J[861]	ST SLU Mobili	FZ-MIN	1	1	1	1	-215.89	46417.94	-465.62	2614.11	1592.23	0.292
Concio 6	420	J[861]	ST SLU Mobili	FZ-MIN	1	1	1	1	-245.45	45792.11	-277.18	2614.11	1592.23	0.174
Concio 6	420	J[862]	ST SLU Mobili	FZ-MIN	1	1	1	1	-245.45	50398.57	-118.59	2614.11	1592.23	0.074
Concio 6	421	J[862]	ST SLU Mobili	FZ-MIN	1	1	1	1	-245.45	50398.57	-118.59	2614.11	1592.23	0.074
Concio 6	421	J[863]	ST SLU Mobili	FZ-MAX	1	1	1	1	0.88	45588.47	277.17	2614.11	1592.23	0.174
Concio 6	422	J[863]	ST SLU Mobili	FZ-MAX	1	1	1	1	4.15	46324.98	465.64	2614.11	1592.23	0.292
Concio 6	422	J[864]	ST SLU Mobili	FZ-MAX	1	1	1	1	4.15	28116.48	642.53	2614.11	1592.23	0.404
Concio 5	423	J[864]	ST SLU Mobili	FZ-MAX	1	1	1	1	4.15	20829.49	642.53	4356.85	4158.81	0.154
Concio 5	423	J[865]	ST SLU Mobili	FZ-MAX	1	1	1	1	4.15	4420.88	784.87	4356.85	4158.81	0.189
Concio 5	424	J[865]	ST SLU Mobili	FZ-MAX	1	1	1	1	118.78	4763.56	954.45	4356.85	4158.81	0.230
Concio 5	424	J[866]	ST SLU Mobili	FZ-MAX	1	1	1	1	118.78	6223.30	1276.25	4356.85	4158.81	0.307
Concio 5	425	J[866]	ST SLU Mobili	FZ-MAX	1	1	1	1	268.28	5865.98	1378.12	4356.85	4158.81	0.331
Concio 5	425	J[867]	ST SLU Mobili	FZ-MAX	1	1	1	1	268.28	6738.32	1409.07	4356.85	4158.81	0.339
Concio 4 H=var	426	J[867]	ST SLU Mobili	FZ-MAX	1	3	3	3	195.25	7554.03	1422.59	4310.00	4114.09	0.346
Concio 4 H=var	426	J[868]	ST SLU Mobili	FZ-MAX	1	3	3	3	191.74	11779.92	1587.47	6652.39	4844.28	0.328
Concio 4 H=var	427	J[868]	ST SLU Mobili	FZ-MAX	1	3	3	3	181.22	11464.27	1666.35	6652.39	4855.64	0.343
Concio 4 H=var	427	J[869]	ST SLU Mobili	FZ-MAX	1	3	4	4	177.42	14309.54	1834.14	8994.79	5359.79	0.342
Concio 4 H=200	428	J[869]	ST SLU Mobili	FZ-MAX	1	3	4	4	265.86	14417.55	1825.81	8994.79	5356.82	0.341
Concio 4 H=200	428	J[870]	ST SLU Mobili	FZ-MAX	1	3	4	4	265.86	15394.50	1859.66	8994.79	5328.90	0.349
Concio 4 H=200	429	J[870]	ST SLU Mobili	FZ-MIN	1	3	4	4	81.32	16409.21	-1993.95	8994.79	5297.98	0.376
Concio 4 H=200	429	J[871]	ST SLU Mobili	FZ-MIN	1	3	4	4	81.32	15274.78	-1960.10	8994.79	5332.42	0.368
Concio 4 H=var	430	J[871]	ST SLU Mobili	FZ-MIN	1	3	4	4	-21.18	15095.62	-1965.40	8994.79	5337.11	0.368
Concio 4 H=var	430	J[872]	ST SLU Mobili	FZ-MIN	1	3	3	3	-17.38	11461.51	-1797.61	6652.39	4855.34	0.370
Concio 4 H=var	431	J[872]	ST SLU Mobili	FZ-MIN	1	3	3	3	-44.81	11814.60	-1722.39	6652.39	4841.91	0.356
Concio 4 H=var	431	J[873]	ST SLU Mobili	FZ-MIN	1	3	3	3	-41.29	6916.04	-1557.51	4310.00	4114.09	0.379

Concio 3	432	J[873]	ST SLU Mobili	FZ-MIN	1	1	1	1	41.70	6239.44	-1548.54	4356.85	4158.81	0.372
Concio 3	432	J[874]	ST SLU Mobili	FZ-MIN	1	1	1	1	41.70	5252.66	-1517.59	4356.85	4158.81	0.365
Concio 3	433	J[874]	ST SLU Mobili	FZ-MIN	1	1	1	1	-78.20	5606.53	-1416.24	4356.85	4158.81	0.341
Concio 3	433	J[875]	ST SLU Mobili	FZ-MIN	1	1	1	1	-78.20	17673.58	-1106.82	4356.85	4158.81	0.266
Concio 3	434	J[875]	ST SLU Mobili	FZ-MIN	1	1	1	1	-208.72	17504.41	-937.21	4356.85	4158.81	0.225
Concio 3	434	J[876]	ST SLU Mobili	FZ-MIN	1	1	1	1	-208.72	35212.52	-813.44	4356.85	4158.81	0.196
Concio 2	435	J[876]	ST SLU Mobili	FZ-MIN	1	1	1	1	-208.72	41965.30	-813.44	3267.64	2478.30	0.328
Concio 2	435	J[877]	ST SLU Mobili	FZ-MIN	1	1	1	1	-208.72	65054.18	-629.46	3267.64	2478.30	0.254
Concio 2	436	J[877]	ST SLU Mobili	FZ-MIN	1	1	1	1	-189.45	63734.96	-440.42	3267.64	2478.30	0.178
Concio 2	436	J[878]	ST SLU Mobili	FZ-MIN	1	1	1	1	-192.79	75138.55	-141.39	3267.64	2478.30	0.057
Concio 2	437	J[878]	ST SLU Mobili	FZ-MAX	1	1	1	1	-86.32	76019.51	298.26	3267.64	2478.30	0.120
Concio 2	437	J[879]	ST SLU Mobili	FZ-MAX	1	1	1	1	-86.32	70360.84	420.92	3267.64	2478.30	0.170
Concio 1	438	J[879]	ST SLU Mobili	FZ-MAX	1	1	1	1	-86.32	124325.48	420.92	3302.77	2478.97	0.170
Concio 1	438	J[880]	ST SLU Mobili	FZ-MAX	1	1	1	1	-86.32	94698.93	602.81	3302.77	2478.97	0.243
Concio 1	439	J[880]	ST SLU Mobili	FZ-MAX	1	1	1	1	-57.04	94925.50	802.47	3302.77	2478.97	0.324
Concio 1	439	J[881]	ST SLU Mobili	FZ-MAX	1	3	3	3	-57.04	178.98	1105.61	3302.77	2574.34	0.429
Concio 1	440	J[881]	ST SLU Mobili	FZ-MIN	1	3	3	3	0.00	25.65	-48.50	3302.77	2574.41	0.019
Concio 1	440	J[882]	ST SLV Vert	FX-MAX	1	1	1	1	0.80	4.11	1.81	3302.77	2574.42	0.001

### 8.3 VERIFICHE ALL'INSTABILITÀ FLESSO-TORSIONALE DELL'ANIMA

Si effettuano le verifiche allo stato limite ultimo per instabilità flessio-torsionale delle sezioni composte travi-soletta secondo lo schema ad U invertita (Circ. 02/02/2009, C4.3.4.4).

I coefficienti parziali per SLU assunti sono i seguenti:

$$\gamma_{M1} = 1.10 \quad \text{acciaio strutturale: resistenza all'instabilità (ponti)}$$

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

- Elem Property: nome delle caratteristiche geometriche dell'elemento
- Elem: numero dell'elemento
- Position: nodo iniziale (I) o finale (J) dell'elemento
- Lcom: combinazione di carico positivo o negativo più gravosa
- Type: sollecitazione (massima o minima)
- Sect. class: classificazione complessiva sezione
- N\_Ed: sforzo assiale di calcolo
- M\_Ed: momento di calcolo
- Nb,Rd: sforzo assiale resistente all'instabilità del corrente compresso
- Mb,Rd: momento resistente all'instabilità
- Mcr: momento critico

Interaction ratio: condizione di verifica: 
$$\frac{N_{Ed}}{N_{b,Rd}} + \frac{M_{Ed}}{M_{b,Rd}} \leq 1$$

Elem property	Elem number	Position	Lcom	Type	Sect. Class	N_Ed (kN)	M_Ed (kN*m)	Nb,Rd (kN)	Mb,Rd (kN*m)	Mcr (kN*m)	Interaction Ratio
Concio 1	101	I[719]	ST SLV Long	FX-MIN	4	-1.71	-0.54	17963.00	6350.05	109.91	0.000
Concio 1	101	J[720]	ST SLU Frenamento	FY-MIN	3	-1.68	-7.34	17963.00	6361.88	112.41	0.001
Concio 1	102	I[720]	ST SLU Mobili	MY-MIN	3	-155.91	-219.22	15885.38	6119.92	21.89	0.046
Concio 1	102	J[721]	ST SLU Mobili	MY-MAX	1	-119.62	2913.83	46848.01	10026.33	45.33	0.293
Concio 1	103	I[721]	ST SLU Mobili	MY-MAX	1	-298.75	2694.39	51956.43	10426.89	45.48	0.264
Concio 1	103	J[722]	ST SLU Mobili	MY-MAX	1	-291.05	3469.25	51956.43	10426.89	45.48	0.338
Concio 2	104	I[722]	ST SLU Mobili	MY-MAX	1	-291.05	3474.74	55693.51	12635.70	90.26	0.280
Concio 2	104	J[723]	ST SLU Mobili	MY-MAX	1	-249.55	3991.54	55693.51	12635.70	90.26	0.320
Concio 2	105	I[723]	ST SLU Mobili	MY-MAX	1	-261.35	3925.98	46533.18	11024.13	38.77	0.362
Concio 2	105	J[724]	ST SLU Mobili	MY-MAX	1	-258.54	3534.51	46533.18	11024.13	38.77	0.326
Concio 2	106	I[724]	ST SLU Mobili	MY-MAX	1	-180.16	3628.39	52741.05	12306.57	69.87	0.298
Concio 2	106	J[725]	ST SLU Mobili	MY-MAX	1	-229.70	2392.44	52741.05	12306.57	69.87	0.199
Concio 3	107	I[725]	ST SLU Mobili	MY-MAX	1	-229.70	2393.75	57024.39	13386.70	112.30	0.183
Concio 3	107	J[726]	ST SLU Termico	MY-MIN	1	-171.49	-1506.13	22659.14	10249.33	91.81	0.155
Concio 3	108	I[726]	ST SLU Termico	MY-MIN	1	-49.95	-1468.17	20044.03	9701.81	30.63	0.154
Concio 3	108	J[727]	ST SLU Mobili	MY-MIN	1	-33.75	-3803.37	20044.03	9727.55	30.93	0.393
Concio 3	109	I[727]	ST SLU Mobili	MY-MIN	1	43.44	-3798.71	22659.14	10249.33	169.80	0.373
Concio 3	109	J[728]	ST SLU Mobili	MY-MIN	1	42.37	-4124.14	22659.14	10249.33	169.80	0.404
Concio 4 H=var	110	I[728]	ST SLU Mobili	MY-MIN	3	-75.50	-4138.85	29959.30	10789.26	78.05	0.386
Concio 4 H=var	110	J[729]	ST SLU Mobili	MY-MIN	3	-87.18	-5890.05	33340.25	16954.86	77.94	0.350



Concio 4 H=var	111	I[729]	ST SLU Mobili	MY-MIN	3	-76.41	-5918.69	33340.25	16954.36	75.02	0.351
Concio 4 H=var	111	J[730]	ST SLU Mobili	MY-MIN	4	-92.30	-7955.76	36721.20	23228.13	75.71	0.345
Concio 4 H=200	112	I[730]	ST SLU Mobili	MY-MIN	4	66.11	-7955.76	36721.20	23227.77	330.21	0.344
Concio 4 H=200	112	J[731]	ST SLU Mobili	MY-MIN	4	94.04	-8469.75	36721.20	23191.04	330.21	0.368
Concio 4 H=200	113	I[731]	ST SLU Mobili	MY-MIN	4	106.73	-8529.23	36721.20	23191.67	329.87	0.371
Concio 4 H=200	113	J[732]	ST SLU Mobili	MY-MIN	4	101.60	-8014.01	36721.20	23226.12	329.87	0.348
Concio 4 H=var	114	I[732]	ST SLU Mobili	MY-MIN	4	-67.67	-8014.01	36721.20	23226.28	75.06	0.347
Concio 4 H=var	114	J[733]	ST SLU Mobili	MY-MIN	3	-23.72	-6023.03	33340.25	16934.68	74.38	0.356
Concio 4 H=var	115	I[733]	ST SLU Mobili	MY-MIN	3	-29.64	-5992.83	33340.25	16935.75	75.87	0.355
Concio 4 H=var	115	J[734]	ST SLU Mobili	MY-MIN	3	-17.03	-4477.12	29959.30	10741.68	75.98	0.417
Concio 5	116	I[734]	ST SLU Mobili	MY-MIN	1	86.69	-4433.49	22659.14	10249.33	167.90	0.436
Concio 5	116	J[735]	ST SLU Mobili	MY-MIN	1	88.22	-4161.98	22659.14	10249.33	167.90	0.410
Concio 5	117	I[735]	ST SLU Mobili	MY-MIN	1	28.43	-4129.44	19837.42	9545.33	26.84	0.434
Concio 5	117	J[736]	ST SLU Termico	MY-MIN	1	40.19	-2299.61	19837.42	9502.16	26.18	0.244
Concio 5	118	I[736]	ST SLU Termico	MY-MIN	1	-130.76	-2251.76	22659.14	10249.33	47.21	0.226
Concio 5	118	J[737]	ST SLU Termico	MY-MIN	1	-102.44	-1892.73	22659.14	10249.33	47.21	0.189
Concio 6	119	I[737]	ST SLU Termico	MY-MIN	2	-102.44	-1893.90	18653.73	8775.51	35.03	0.221
Concio 6	119	J[738]	ST SLU Mobili	MY-MAX	1	-98.68	2548.79	52344.97	11938.70	70.87	0.215
Concio 6	120	I[738]	ST SLU Mobili	MY-MAX	1	-161.07	2463.61	53194.79	11955.87	64.12	0.209
Concio 6	120	J[739]	ST SLU Mobili	MY-MAX	1	-198.24	2385.83	53194.79	11955.87	64.12	0.203
Concio 6	121	I[739]	ST SLU Mobili	MY-MAX	1	-198.24	2385.83	53194.79	11955.88	64.12	0.203
Concio 6	121	J[740]	ST SLU Mobili	MY-MAX	1	-161.11	2463.44	53194.79	11955.88	64.12	0.209
Concio 6	122	I[740]	ST SLU Mobili	MY-MAX	1	-98.56	2548.82	52344.97	11938.60	70.87	0.215
Concio 6	122	J[741]	ST SLU Termico	MY-MIN	2	-102.47	-1893.75	18653.73	8775.51	35.03	0.221
Concio 5	123	I[741]	ST SLU Termico	MY-MIN	1	-102.47	-1892.60	22659.14	10249.33	47.21	0.189
Concio 5	123	J[742]	ST SLU Termico	MY-MIN	1	-130.83	-2251.48	22659.14	10249.33	47.21	0.225
Concio 5	124	I[742]	ST SLU Termico	MY-MIN	1	40.07	-2299.41	19837.42	9504.35	26.18	0.244
Concio 5	124	J[743]	ST SLU Mobili	MY-MIN	1	28.32	-4130.24	19837.42	9547.34	26.84	0.434
Concio 5	125	I[743]	ST SLU Mobili	MY-MIN	1	88.19	-4162.71	22659.14	10249.33	167.90	0.410
Concio 5	125	J[744]	ST SLU Mobili	MY-MIN	1	86.66	-4434.22	22659.14	10249.33	167.90	0.437
Concio 4 H=var	126	I[744]	ST SLU Mobili	MY-MIN	3	-17.05	-4477.90	29959.30	10741.68	75.98	0.417
Concio 4 H=var	126	J[745]	ST SLU Mobili	MY-MIN	3	-29.66	-5993.57	33340.25	16935.75	75.87	0.355
Concio 4 H=var	127	I[745]	ST SLU Mobili	MY-MIN	3	-23.60	-6024.18	33340.25	16934.68	74.38	0.356
Concio 4 H=var	127	J[746]	ST SLU Mobili	MY-MIN	4	-67.59	-8014.29	36721.20	23226.28	75.06	0.347
Concio 4 H=200	128	I[746]	ST SLU Mobili	MY-MIN	4	101.65	-8014.29	36721.20	23226.12	329.86	0.348
Concio 4 H=200	128	J[747]	ST SLU Mobili	MY-MIN	4	106.77	-8529.38	36721.20	23191.67	329.86	0.371
Concio 4 H=200	129	I[747]	ST SLU Mobili	MY-MIN	4	94.11	-8469.97	36721.20	23191.04	330.21	0.368
Concio 4 H=200	129	J[748]	ST SLU Mobili	MY-MIN	4	66.00	-7955.85	36721.20	23227.77	330.21	0.344
Concio 4 H=var	130	I[748]	ST SLU Mobili	MY-MIN	4	-92.43	-7955.85	36721.20	23228.13	75.71	0.345
Concio 4 H=var	130	J[749]	ST SLU Mobili	MY-MIN	3	-76.53	-5917.90	33340.25	16954.36	75.03	0.351
Concio 4 H=var	131	I[749]	ST SLU Mobili	MY-MIN	3	-87.14	-5889.59	33340.25	16954.86	77.94	0.350
Concio 4 H=var	131	J[750]	ST SLU Mobili	MY-MIN	3	-75.45	-4138.29	29959.30	10789.26	78.05	0.386
Concio 3	132	I[750]	ST SLU Mobili	MY-MIN	1	42.41	-4123.62	22659.14	10249.33	169.80	0.404
Concio 3	132	J[751]	ST SLU Mobili	MY-MIN	1	43.49	-3798.11	22659.14	10249.33	169.80	0.373
Concio 3	133	I[751]	ST SLU Mobili	MY-MIN	1	-33.62	-3802.82	20044.03	9725.08	30.93	0.393
Concio 3	133	J[752]	ST SLU Termico	MY-MIN	1	-49.82	-1468.36	20044.03	9699.27	30.63	0.154
Concio 3	134	I[752]	ST SLU Termico	MY-MIN	1	-171.45	-1506.29	22659.14	10249.33	91.80	0.155
Concio 3	134	J[753]	ST SLU Mobili	MY-MAX	1	-229.75	2393.98	57024.39	13386.70	112.29	0.183
Concio 2	135	I[753]	ST SLU Mobili	MY-MAX	1	-229.75	2392.65	52741.05	12306.77	69.87	0.199
Concio 2	135	J[754]	ST SLU Mobili	MY-MAX	1	-180.35	3628.60	52741.05	12306.77	69.87	0.298
Concio 2	136	I[754]	ST SLU Mobili	MY-MAX	1	-258.47	3534.90	46533.18	11025.43	38.77	0.326
Concio 2	136	J[755]	ST SLU Mobili	MY-MAX	1	-261.42	3925.90	46533.18	11025.43	38.77	0.362
Concio 2	137	I[755]	ST SLU Mobili	MY-MAX	1	-249.48	3991.52	55693.51	12635.70	90.26	0.320
Concio 2	137	J[756]	ST SLU Mobili	MY-MAX	1	-291.04	3474.88	55693.51	12635.70	90.26	0.280
Concio 1	138	I[756]	ST SLU Mobili	MY-MAX	1	-291.04	3469.39	51956.43	10426.75	45.48	0.338
Concio 1	138	J[757]	ST SLU Mobili	MY-MAX	1	-298.83	2694.45	51956.43	10426.75	45.48	0.264
Concio 1	139	I[757]	ST SLU Mobili	MY-MAX	1	-119.58	2913.94	46848.01	10024.84	45.33	0.293
Concio 1	139	J[758]	ST SLU Mobili	MY-MIN	3	-156.02	-218.27	15885.38	6093.55	21.22	0.046
Concio 1	140	I[758]	ST SLU Frenamento	MY-MIN	3	-1.68	-7.34	17963.00	6361.88	112.41	0.001
Concio 1	140	J[759]	ST SLV Long	FX-MIN	4	-1.71	-0.54	17963.00	6350.05	109.91	0.000
Concio 1	201	I[760]	ST SLV Long	FX-MIN	4	-1.76	-0.53	18098.91	6479.08	110.09	0.000
Concio 1	201	J[761]	ST SLU Mobili	MY-MIN	3	0.00	-342.22	18098.91	6493.67	112.51	0.053
Concio 1	202	I[761]	ST SLU Mobili	FY-MIN	3	-361.82	-467.20	16005.57	6252.66	22.39	0.097
Concio 1	202	J[762]	ST SLU Mobili	MY-MAX	1	-170.15	3205.56	49645.95	10274.39	46.44	0.315
Concio 1	203	I[762]	ST SLU Mobili	MY-MAX	1	-346.83	2954.33	55059.46	10642.35	45.95	0.284
Concio 1	203	J[763]	ST SLU Mobili	MY-MAX	1	-330.74	3887.82	55059.46	10642.35	45.95	0.371
Concio 2	204	I[763]	ST SLU Mobili	MY-MAX	1	-330.74	3892.39	58902.07	12846.08	87.85	0.309
Concio 2	204	J[764]	ST SLU Mobili	MY-MAX	1	-391.05	4082.61	58902.07	12846.08	87.85	0.324
Concio 2	205	I[764]	ST SLU Mobili	MY-MAX	1	-409.46	4038.81	49214.00	11204.67	38.67	0.369
Concio 2	205	J[765]	ST SLU Mobili	MY-MAX	1	-397.44	3703.25	49214.00	11204.67	38.67	0.339
Concio 2	206	I[765]	ST SLU Mobili	MY-MAX	1	-276.23	3756.67	55779.51	12475.36	66.65	0.306
Concio 2	206	J[766]	ST SLU Mobili	MY-MAX	1	-204.11	2943.67	55779.51	12475.36	66.65	0.240
Concio 3	207	I[766]	ST SLU Mobili	MY-MAX	1	-204.11	2944.20	60219.77	13602.15	112.19	0.220
Concio 3	207	J[767]	ST SLU Vento	MY-MIN	3	-112.92	-1345.82	22871.47	7452.38	112.47	0.186
Concio 3	208	I[767]	ST SLU Mobili	MY-MIN	3	156.95	-1314.74	20231.85	7049.29	31.64	0.194
Concio 3	208	J[768]	ST SLU Mobili	MY-MIN	1	129.24	-3639.41	20231.85	9882.37	31.64	0.375
Concio 3	209	I[768]	ST SLU Mobili	MY-MIN	1	21.07	-3620.74	22871.47	10397.59	170.21	0.349
Concio 3	209	J[769]	ST SLU Mobili	MY-MIN	1	18.70	-3913.46	22871.47	10397.59	170.21	0.377
Concio 4 H=var	210	I[769]	ST SLU Mobili	MY-MIN	3	-99.68	-3941.62	30596.27	10850.42	79.59	0.367
Concio 4 H=var	210	J[770]	ST SLU Mobili	MY-MIN	3	-120.80	-5671.43	33977.22	17021.59	79.18	0.337

Concio 4 H=var	211	I[770]	ST SLU Mobili	MY-MIN	3	-218.23	-5634.42	33977.22	17022.32	76.72	0.337
Concio 4 H=var	211	J[771]	ST SLU Mobili	MY-MIN	4	-248.76	-7840.84	37358.17	23251.25	77.24	0.344
Concio 4 H=200	212	I[771]	ST SLU Mobili	MY-MIN	4	-68.56	-7840.84	37358.17	23250.97	332.45	0.339
Concio 4 H=200	212	J[772]	ST SLU Mobili	MY-MIN	4	-141.65	-8339.12	37358.17	23206.68	332.45	0.363
Concio 4 H=200	213	I[772]	ST SLU Mobili	MY-MIN	4	-154.27	-8414.24	37358.17	23206.89	332.94	0.367
Concio 4 H=200	213	J[773]	ST SLU Mobili	MY-MIN	4	-128.09	-7845.60	37358.17	23248.67	332.94	0.341
Concio 4 H=var	214	I[773]	ST SLU Mobili	MY-MIN	4	-315.38	-7845.60	37358.17	23248.84	76.27	0.346
Concio 4 H=var	214	J[774]	ST SLU Mobili	MY-MIN	3	-213.65	-5793.63	33977.22	16999.37	75.76	0.347
Concio 4 H=var	215	I[774]	ST SLU Mobili	MY-MIN	3	-117.65	-5834.60	33977.22	16998.48	77.17	0.347
Concio 4 H=var	215	J[775]	ST SLU Mobili	MY-MIN	3	-96.55	-4297.51	30596.27	10796.42	77.58	0.401
Concio 5	216	I[775]	ST SLU Mobili	MY-MIN	1	9.72	-4290.34	22871.47	10397.59	168.63	0.413
Concio 5	216	J[776]	ST SLU Mobili	MY-MIN	1	12.55	-4019.61	22871.47	10397.59	168.63	0.387
Concio 5	217	I[776]	ST SLU Mobili	MY-MIN	1	183.81	-4041.37	20023.30	9704.32	27.49	0.426
Concio 5	217	J[777]	ST SLU Termico	MY-MIN	1	170.75	-2198.24	20023.30	9661.76	26.70	0.236
Concio 5	218	I[777]	ST SLU Termico	MY-MIN	1	-2.25	-2217.43	22871.47	10397.59	48.38	0.213
Concio 5	218	J[778]	ST SLU Termico	MY-MIN	1	4.34	-1855.47	22871.47	10397.59	48.38	0.179
Concio 6	219	I[778]	ST SLU Termico	MY-MIN	2	4.34	-1853.30	18787.18	8891.06	38.25	0.209
Concio 6	219	J[779]	ST SLU Mobili	MY-MAX	1	-154.38	2643.11	55412.36	12098.88	65.96	0.221
Concio 6	220	I[779]	ST SLU Mobili	MY-MAX	1	-251.55	2588.54	56311.98	12178.14	67.04	0.217
Concio 6	220	J[780]	ST SLU Mobili	MY-MAX	1	-182.96	2800.01	56311.98	12178.14	67.04	0.233
Concio 6	221	I[780]	ST SLU Mobili	MY-MAX	1	-182.96	2800.01	56311.98	12181.87	67.03	0.233
Concio 6	221	J[781]	ST SLU Mobili	MY-MAX	1	-251.57	2588.95	56311.98	12181.87	67.03	0.217
Concio 6	222	I[781]	ST SLU Mobili	MY-MAX	1	-154.39	2642.71	55412.36	12085.70	65.97	0.222
Concio 6	222	J[782]	ST SLU Termico	MY-MIN	2	4.39	-1853.22	18787.18	8891.06	38.25	0.209
Concio 5	223	I[782]	ST SLU Termico	MY-MIN	1	4.39	-1855.39	22871.47	10397.59	48.38	0.179
Concio 5	223	J[783]	ST SLU Termico	MY-MIN	1	-2.19	-2217.30	22871.47	10397.59	48.38	0.213
Concio 5	224	I[783]	ST SLU Termico	MY-MIN	1	170.95	-2198.33	20023.30	9663.47	26.70	0.236
Concio 5	224	J[784]	ST SLU Mobili	MY-MIN	1	183.99	-4041.81	20023.30	9705.43	27.49	0.426
Concio 5	225	I[784]	ST SLU Mobili	MY-MIN	1	12.82	-4019.92	22871.47	10397.59	168.63	0.387
Concio 5	225	J[785]	ST SLU Mobili	MY-MIN	1	9.98	-4290.64	22871.47	10397.59	168.63	0.413
Concio 4 H=var	226	I[785]	ST SLU Mobili	MY-MIN	3	-96.29	-4297.65	30596.27	10796.42	77.58	0.401
Concio 4 H=var	226	J[786]	ST SLU Mobili	MY-MIN	3	-117.38	-5834.68	33977.22	16998.48	77.17	0.347
Concio 4 H=var	227	I[786]	ST SLU Mobili	MY-MIN	3	-213.42	-5794.19	33977.22	16999.37	75.76	0.347
Concio 4 H=var	227	J[787]	ST SLU Mobili	MY-MIN	4	-315.08	-7845.95	37358.17	23248.84	76.27	0.346
Concio 4 H=200	228	I[787]	ST SLU Mobili	MY-MIN	4	-127.82	-7845.95	37358.17	23248.67	332.94	0.341
Concio 4 H=200	228	J[788]	ST SLU Mobili	MY-MIN	4	-154.00	-8414.41	37358.17	23206.89	332.94	0.367
Concio 4 H=200	229	I[788]	ST SLU Mobili	MY-MIN	4	-141.88	-8339.18	37358.17	23206.68	332.45	0.363
Concio 4 H=200	229	J[789]	ST SLU Mobili	MY-MIN	4	-68.75	-7840.74	37358.17	23250.97	332.45	0.339
Concio 4 H=var	230	I[789]	ST SLU Mobili	MY-MIN	4	-248.97	-7840.74	37358.17	23251.25	77.24	0.344
Concio 4 H=var	230	J[790]	ST SLU Mobili	MY-MIN	3	-218.45	-5634.26	33977.22	17022.32	76.72	0.337
Concio 4 H=var	231	I[790]	ST SLU Mobili	MY-MIN	3	-121.08	-5671.54	33977.22	17021.59	79.18	0.337
Concio 4 H=var	231	J[791]	ST SLU Mobili	MY-MIN	3	-99.95	-3941.58	30596.27	10850.42	79.60	0.367
Concio 3	232	I[791]	ST SLU Mobili	MY-MIN	1	18.44	-3913.34	22871.47	10397.59	170.21	0.377
Concio 3	232	J[792]	ST SLU Mobili	MY-MIN	1	20.81	-3620.53	22871.47	10397.59	170.21	0.349
Concio 3	233	I[792]	ST SLU Mobili	MY-MIN	1	129.04	-3639.07	20231.85	9880.80	31.64	0.375
Concio 3	233	J[793]	ST SLU Mobili	MY-MIN	3	156.75	-1314.57	20231.85	7048.18	31.64	0.194
Concio 3	234	I[793]	ST SLU Vento	MY-MIN	3	-113.03	-1345.97	22871.47	7452.38	112.46	0.186
Concio 3	234	J[794]	ST SLU Mobili	MY-MAX	1	-204.02	2944.82	60219.77	13602.15	112.21	0.220
Concio 2	235	I[794]	ST SLU Mobili	MY-MAX	1	-204.02	2944.28	55779.51	12483.91	66.65	0.240
Concio 2	235	J[795]	ST SLU Mobili	MY-MAX	1	-276.20	3757.63	55779.51	12483.91	66.65	0.306
Concio 2	236	I[795]	ST SLU Mobili	MY-MAX	1	-397.38	3703.09	49214.00	11201.96	38.67	0.339
Concio 2	236	J[796]	ST SLU Mobili	MY-MAX	1	-409.46	4039.32	49214.00	11201.96	38.67	0.369
Concio 2	237	I[796]	ST SLU Mobili	MY-MAX	1	-391.07	4082.03	58902.07	12846.08	87.85	0.324
Concio 2	237	J[797]	ST SLU Mobili	MY-MAX	1	-330.77	3892.22	58902.07	12846.08	87.85	0.309
Concio 1	238	I[797]	ST SLU Mobili	MY-MAX	1	-330.77	3888.16	55059.46	10634.94	45.95	0.372
Concio 1	238	J[798]	ST SLU Mobili	MY-MAX	1	-346.81	2954.47	55059.46	10634.94	45.95	0.284
Concio 1	239	I[798]	ST SLU Mobili	MY-MAX	1	-170.16	3205.45	49645.95	10231.78	46.44	0.317
Concio 1	239	J[799]	ST SLU Mobili	MZ-MIN	3	-361.65	-467.40	16005.57	6256.54	22.59	0.097
Concio 1	240	I[799]	ST SLU Mobili	MY-MIN	3	0.00	-342.19	18098.91	6493.67	112.51	0.053
Concio 1	240	J[800]	ST SLV Long	FX-MIN	4	-1.76	-0.53	18098.91	6479.08	110.09	0.000
Concio 1	301	I[801]	ST SLV Long	FX-MIN	4	-1.76	-0.53	18098.91	6479.08	110.09	0.000
Concio 1	301	J[802]	ST SLU Mobili	FY-MAX	3	0.00	-342.22	18098.91	6493.67	112.51	0.053
Concio 1	302	I[802]	ST SLU Mobili	MY-MIN	3	-229.04	-507.62	16005.57	6263.81	23.82	0.095
Concio 1	302	J[803]	ST SLU Mobili	MY-MAX	1	-172.73	3211.15	49645.95	10274.61	46.45	0.316
Concio 1	303	I[803]	ST SLU Mobili	MY-MAX	1	-348.11	2956.43	55059.46	10642.43	45.95	0.284
Concio 1	303	J[804]	ST SLU Mobili	MY-MAX	1	-332.02	3891.56	55059.46	10642.43	45.95	0.372
Concio 2	304	I[804]	ST SLU Mobili	MY-MAX	1	-332.02	3896.17	58902.07	12846.08	87.86	0.309
Concio 2	304	J[805]	ST SLU Mobili	MY-MAX	1	-392.33	4087.64	58902.07	12846.08	87.86	0.325
Concio 2	305	I[805]	ST SLU Mobili	MY-MAX	1	-411.15	4041.96	49214.00	11204.71	38.67	0.369
Concio 2	305	J[806]	ST SLU Mobili	MY-MAX	1	-399.12	3705.75	49214.00	11204.71	38.67	0.339
Concio 2	306	I[806]	ST SLU Mobili	MY-MAX	1	-280.56	3761.36	55779.51	12475.45	66.66	0.307
Concio 2	306	J[807]	ST SLU Mobili	MY-MAX	1	-208.44	2946.09	55779.51	12475.45	66.66	0.240
Concio 3	307	I[807]	ST SLU Mobili	MY-MAX	1	-208.44	2946.65	60219.77	13602.15	112.20	0.220
Concio 3	307	J[808]	ST SLU Mobili	MY-MAX	1	-289.94	2075.37	60219.77	13602.15	112.20	0.157
Concio 3	308	I[808]	ST SLU Vento	MY-MIN	3	135.61	-1313.97	20231.85	7039.09	31.38	0.193
Concio 3	308	J[809]	ST SLU Mobili	MY-MIN	1	138.97	-3642.92	20231.85	9882.15	31.63	0.376
Concio 3	309	I[809]	ST SLU Mobili	MY-MIN	1	26.41	-3623.79	22871.47	10397.59	170.23	0.350
Concio 3	309	J[810]	ST SLU Mobili	MY-MIN	1	24.04	-3918.52	22871.47	10397.59	170.23	0.378
Concio 4 H=var	310	I[810]	ST SLU Mobili	MY-MIN	3	-94.67	-3942.81	30596.27	10850.42	79.62	0.367
Concio 4 H=var	310	J[811]	ST SLU Mobili	MY-MIN	3	-115.79	-5680.10	33977.22	17021.59	79.20	0.337



Concio 4 H=var	411	J[852]	ST SLU Mobili	MY-MIN	3	-70.68	-5909.27	33340.25	16954.36	75.01	0.351
Concio 4 H=var	411	J[853]	ST SLU Mobili	MY-MIN	4	-86.57	-7938.38	36721.20	23228.13	75.70	0.344
Concio 4 H=200	412	J[853]	ST SLU Mobili	MY-MIN	4	69.72	-7938.38	36721.20	23227.77	330.20	0.344
Concio 4 H=200	412	J[854]	ST SLU Mobili	MY-MIN	4	97.65	-8450.57	36721.20	23191.04	330.20	0.367
Concio 4 H=200	413	J[854]	ST SLU Mobili	MY-MIN	4	108.76	-8510.08	36721.20	23191.67	329.86	0.370
Concio 4 H=200	413	J[855]	ST SLU Mobili	MY-MIN	4	103.63	-7996.70	36721.20	23226.12	329.86	0.347
Concio 4 H=var	414	J[855]	ST SLU Mobili	MY-MIN	4	-63.40	-7996.70	36721.20	23226.28	75.05	0.346
Concio 4 H=var	414	J[856]	ST SLU Mobili	MY-MIN	3	-19.46	-6013.86	33340.25	16934.68	74.37	0.356
Concio 4 H=var	415	J[856]	ST SLU Mobili	MY-MIN	3	-36.45	-5982.65	33340.25	16935.75	75.85	0.354
Concio 4 H=var	415	J[857]	ST SLU Mobili	MY-MIN	3	-23.84	-4474.00	29959.30	10741.68	75.96	0.417
Concio 5	416	J[857]	ST SLU Mobili	MY-MIN	1	80.16	-4426.71	22659.14	10249.33	167.88	0.435
Concio 5	416	J[858]	ST SLU Mobili	MY-MIN	1	81.68	-4157.66	22659.14	10249.33	167.88	0.409
Concio 5	417	J[858]	ST SLU Mobili	MY-MIN	1	22.09	-4120.48	19837.42	9545.94	26.85	0.433
Concio 5	417	J[859]	ST SLU Termico	MY-MIN	1	33.84	-2292.60	19837.42	9502.73	26.19	0.243
Concio 5	418	J[859]	ST SLU Termico	MY-MIN	1	-129.81	-2249.41	22659.14	10249.33	47.24	0.225
Concio 5	418	J[860]	ST SLU Termico	MY-MIN	1	-101.49	-1888.85	22659.14	10249.33	47.24	0.189
Concio 6	419	J[860]	ST SLU Termico	MY-MIN	2	-101.49	-1890.00	18653.73	8775.51	35.11	0.221
Concio 6	419	J[861]	ST SLU Mobili	MY-MAX	1	-99.63	2542.96	52344.97	11938.73	70.87	0.215
Concio 6	420	J[861]	ST SLU Mobili	MY-MAX	1	-163.53	2459.24	53194.79	11955.87	64.12	0.209
Concio 6	420	J[862]	ST SLU Mobili	MY-MAX	1	-200.71	2381.46	53194.79	11955.87	64.12	0.203
Concio 6	421	J[862]	ST SLU Mobili	MY-MAX	1	-200.71	2381.46	53194.79	11955.88	64.12	0.203
Concio 6	421	J[863]	ST SLU Mobili	MY-MAX	1	-163.58	2459.08	53194.79	11955.88	64.12	0.209
Concio 6	422	J[863]	ST SLU Mobili	MY-MAX	1	-99.57	2542.95	52344.97	11938.64	70.87	0.215
Concio 6	422	J[864]	ST SLU Termico	MY-MIN	2	-101.46	-1889.87	18653.73	8775.51	35.11	0.221
Concio 5	423	J[864]	ST SLU Termico	MY-MIN	1	-101.46	-1888.73	22659.14	10249.33	47.24	0.189
Concio 5	423	J[865]	ST SLU Termico	MY-MIN	1	-129.82	-2249.18	22659.14	10249.33	47.24	0.225
Concio 5	424	J[865]	ST SLU Termico	MY-MIN	1	33.99	-2292.42	19837.42	9504.67	26.19	0.243
Concio 5	424	J[866]	ST SLU Mobili	MY-MIN	1	22.24	-4121.01	19837.42	9547.64	26.85	0.433
Concio 5	425	J[866]	ST SLU Mobili	MY-MIN	1	81.92	-4158.13	22659.14	10249.33	167.87	0.409
Concio 5	425	J[867]	ST SLU Mobili	MY-MIN	1	80.39	-4427.17	22659.14	10249.33	167.87	0.436
Concio 4 H=var	426	J[867]	ST SLU Mobili	MY-MIN	3	-23.61	-4474.42	29959.30	10741.68	75.96	0.417
Concio 4 H=var	426	J[868]	ST SLU Mobili	MY-MIN	3	-36.22	-5982.96	33340.25	16935.75	75.85	0.354
Concio 4 H=var	427	J[868]	ST SLU Mobili	MY-MIN	3	-19.33	-6014.53	33340.25	16934.68	74.37	0.356
Concio 4 H=var	427	J[869]	ST SLU Mobili	MY-MIN	4	-63.32	-7996.85	36721.20	23226.28	75.05	0.346
Concio 4 H=200	428	J[869]	ST SLU Mobili	MY-MIN	4	103.71	-7996.85	36721.20	23226.12	329.86	0.347
Concio 4 H=200	428	J[870]	ST SLU Mobili	MY-MIN	4	108.84	-8510.17	36721.20	23191.67	329.86	0.370
Concio 4 H=200	429	J[870]	ST SLU Mobili	MY-MIN	4	97.70	-8450.85	36721.20	23191.04	330.21	0.367
Concio 4 H=200	429	J[871]	ST SLU Mobili	MY-MIN	4	69.59	-7938.61	36721.20	23227.77	330.21	0.344
Concio 4 H=var	430	J[871]	ST SLU Mobili	MY-MIN	4	-86.69	-7938.61	36721.20	23228.13	75.70	0.344
Concio 4 H=var	430	J[872]	ST SLU Mobili	MY-MIN	3	-70.79	-5908.96	33340.25	16954.36	75.01	0.351
Concio 4 H=var	431	J[872]	ST SLU Mobili	MY-MIN	3	-93.90	-5879.67	33340.25	16954.86	77.92	0.350
Concio 4 H=var	431	J[873]	ST SLU Mobili	MY-MIN	3	-82.22	-4135.57	29959.30	10789.26	78.04	0.386
Concio 3	432	J[873]	ST SLU Mobili	MY-MIN	1	35.94	-4117.53	22659.14	10249.33	169.77	0.403
Concio 3	432	J[874]	ST SLU Mobili	MY-MIN	1	37.01	-3794.53	22659.14	10249.33	169.77	0.372
Concio 3	433	J[874]	ST SLU Mobili	MY-MIN	1	-39.86	-3794.70	20044.03	9725.89	30.95	0.392
Concio 3	433	J[875]	ST SLU Termico	MY-MIN	1	-56.06	-1460.92	20044.03	9700.08	30.65	0.153
Concio 3	434	J[875]	ST SLU Termico	MY-MIN	1	-170.16	-1503.49	22659.14	10249.33	92.12	0.154
Concio 3	434	J[876]	ST SLU Mobili	MY-MAX	1	-231.03	2389.44	57024.39	13386.70	112.29	0.183
Concio 2	435	J[876]	ST SLU Mobili	MY-MAX	1	-231.03	2388.10	52741.05	12306.75	69.87	0.198
Concio 2	435	J[877]	ST SLU Mobili	MY-MAX	1	-181.64	3621.42	52741.05	12306.75	69.87	0.298
Concio 2	436	J[877]	ST SLU Mobili	MY-MAX	1	-262.97	3529.52	46533.18	11025.35	38.77	0.326
Concio 2	436	J[878]	ST SLU Mobili	MY-MAX	1	-265.93	3919.35	46533.18	11025.35	38.77	0.361
Concio 2	437	J[878]	ST SLU Mobili	MY-MAX	1	-253.84	3983.53	55693.51	12635.70	90.26	0.320
Concio 2	437	J[879]	ST SLU Mobili	MY-MAX	1	-295.40	3467.72	55693.51	12635.70	90.26	0.280
Concio 1	438	J[879]	ST SLU Mobili	MY-MAX	1	-295.40	3462.10	51956.43	10426.78	45.48	0.338
Concio 1	438	J[880]	ST SLU Mobili	MY-MAX	1	-303.20	2688.41	51956.43	10426.78	45.48	0.264
Concio 1	439	J[880]	ST SLU Mobili	MY-MAX	1	-120.93	2902.93	46848.01	10024.92	45.33	0.292
Concio 1	439	J[881]	ST SLU Mobili	MY-MIN	3	-154.56	-218.72	15885.38	6123.53	21.88	0.045
Concio 1	440	J[881]	ST SLU Frenamento	MY-MIN	3	-1.68	-7.34	17963.00	6361.88	112.41	0.001
Concio 1	440	J[882]	ST SLV Long	FX-MIN	4	-1.71	-0.54	17963.00	6350.05	109.91	0.000

## 8.4 VERIFICA DELLE TENSIONI IN ESERCIZIO

Si effettuano le verifiche allo stato limite di esercizio (SLE) delle tensioni massime nell'acciaio delle travi, nel calcestruzzo della soletta e nelle armature della soletta.

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem: numero dell'elemento  
Position: nodo iniziale (I) o finale (J) dell'elemento

Lcom: combinazione di carico più gravosa  
 Type: tipo combinazione (caratteristica, frequente, quasi permanente)

Flange travi in acciaio:

Sigma\_Ed\_ser: tensione assiale massima  
 Tau\_Ed\_ser: tensione tangenziale massima  
 SQRT(sigma^2+3 tau^2): tensione ideale

ALW =  $f_{yk} / \gamma_{M,ser}$ : tensione limite

Soletta in calcestruzzo:

Sigma\_c: tensione assiale massima  
 k\*fck: tensione limite (comb. caratteristica:  $k_1 = 0.6$ ; quasi perm.:  $k_2 = 0.45$ )

Armatura soletta:

Sigma\_s: tensione assiale massima  
 k\*fsk: tensione limite (comb. caratteristica:  $k_3 = 0.8$ )



Elem property	Elem number	Position	Top and Bottom Flange of Structural Steel								Concrete Deck				Reinforcement in Deck				VERIFICATIO
			Loom	Type	Sigma_Ed_ser (kN/m <sup>2</sup> )	ALW (kN/m <sup>2</sup> )	Tau_Ed_ser (kN/m <sup>2</sup> )	ALW (kN/m <sup>2</sup> )	Sort (sig <sup>2</sup> /sigma <sup>2</sup> )	ALW (kN/m <sup>2</sup> )	Loom	Type	Sigma_c (kN/m <sup>2</sup> )	k*ick (kN/m <sup>2</sup> )	Loom	Type	Sigma_s (kN/m <sup>2</sup> )	k*isk (kN/m <sup>2</sup> )	
Concio 1	101	J1791	ST RARA Frenamento	Characteristic	0,00	355000	0,00	20499,35	0,00	355000	ST RARA Frenamento	Characteristic	0,00	9200	ST RARA Frenamento	Characteristic	0,00	360000	Verified
Concio 1	101	J17201	ST RARA Vento	Characteristic	792,46	355000	2363,67	20499,35	4,70	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	728,78	360000	Verified
Concio 1	102	J17201	ST RARA Mobil	Characteristic	23223,86	355000	53060,00	20499,35	94,79155	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	6414,65	360000	Verified
Concio 1	102	J1721	ST RARA Mobil	Characteristic	144332,8	355000	38215,79	20499,35	16933194	355000	ST RARA Mobil	Characteristic	4792,11	9200	ST RARA Mobil	Characteristic	26415,77	360000	Verified
Concio 1	103	J1721	ST RARA Mobil	Characteristic	156637,02	355000	2176,78	20499,35	14776,90	355000	ST RARA Mobil	Characteristic	4217,23	9200	ST RARA Mobil	Characteristic	23057,00	360000	Verified
Concio 1	103	J1722	ST RARA Mobil	Characteristic	178558,64	355000	166118,3	20499,35	181445,28	355000	ST RARA Mobil	Characteristic	5099,72	9200	ST RARA Mobil	Characteristic	27694,11	360000	Verified
Concio 2	104	J1723	ST RARA Mobil	Characteristic	144621,41	355000	198119,9	20499,35	48246,49	355000	ST RARA Mobil	Characteristic	4734,72	9200	ST RARA Mobil	Characteristic	26160,31	360000	Verified
Concio 2	104	J1723	ST RARA Mobil	Characteristic	151458,00	355000	13673,80	20499,35	16385,80	355000	ST RARA Mobil	Characteristic	5786,11	9200	ST RARA Mobil	Characteristic	32084,77	360000	Verified
Concio 2	105	J1723	ST RARA Mobil	Characteristic	160234,95	355000	5645,02	20499,35	160522,53	355000	ST RARA Mobil	Characteristic	5403,79	9200	ST RARA Mobil	Characteristic	29828,75	360000	Verified
Concio 2	105	J1724	ST RARA Mobil	Characteristic	189538,07	355000	13647,04	20499,35	141525,96	355000	ST RARA Mobil	Characteristic	5044,54	9200	ST RARA Mobil	Characteristic	31027,63	360000	Verified
Concio 2	106	J1724	ST RARA Mobil	Characteristic	142770,52	355000	32440,81	20499,35	15376,55	355000	ST RARA Mobil	Characteristic	5187,49	9200	ST RARA Mobil	Characteristic	37323,28	360000	Verified
Concio 2	106	J1725	ST RARA Mobil	Characteristic	91295,12	355000	37178,44	20499,35	111720,67	355000	ST RARA Mobil	Characteristic	3187,96	9200	ST RARA Mobil	Characteristic	54761,89	360000	Verified
Concio 3	107	J1725	ST RARA Mobil	Characteristic	88080,16	355000	27883,83	20499,35	100452,17	355000	ST RARA Mobil	Characteristic	3170,40	9200	ST RARA Mobil	Characteristic	44658,51	360000	Verified
Concio 3	107	J1726	ST RARA Mobil	Characteristic	51933,67	355000	29796,96	20499,35	73216,68	355000	ST RARA Mobil	Characteristic	2396,98	9200	ST RARA Mobil	Characteristic	63431,61	360000	Verified
Concio 3	108	J1726	ST RARA Mobil	Characteristic	59352,96	355000	43114,89	20499,35	95391,06	355000	ST RARA Mobil	Characteristic	2953,62	9200	ST RARA Mobil	Characteristic	75419,88	360000	Verified
Concio 3	108	J1727	ST RARA Mobil	Characteristic	169154,85	355000	4457,44	20499,35	159899,89	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	144221,85	360000	Verified
Concio 3	109	J1727	ST RARA Mobil	Characteristic	169864,80	355000	44012,64	20499,35	186186,43	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	148819,94	360000	Verified
Concio 3	109	J1728	ST RARA Mobil	Characteristic	190110,32	355000	48215,54	20499,35	205609,50	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	157885,88	360000	Verified
Concio 4 Hava	110	J1728	ST RARA Mobil	Characteristic	152207,18	355000	46244,43	20499,35	177996,13	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	199820,03	360000	Verified
Concio 4 Hava	110	J1729	ST RARA Mobil	Characteristic	148834,53	355000	34140,52	20499,35	159872,55	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	88749,97	360000	Verified
Concio 4 Hava	111	J1729	ST RARA Mobil	Characteristic	148892,01	355000	33394,62	20499,35	159732,37	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	83094,97	360000	Verified
Concio 4 Hava	111	J1730	ST RARA Mobil	Characteristic	148250,59	355000	13682,66	20499,35	158081,03	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	81829,42	360000	Verified
Concio 4 H20	112	J1730	ST RARA Mobil	Characteristic	146789,81	355000	2754,25	20499,35	154331,95	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	81458,01	360000	Verified
Concio 4 H20	112	J1731	ST RARA Mobil	Characteristic	59865,06	355000	32211,79	20499,35	69321,10	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	92455,62	360000	Verified
Concio 4 H20	113	J1731	ST RARA Mobil	Characteristic	160490,56	355000	34361,24	20499,35	17170,40	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	94054,63	360000	Verified
Concio 4 H20	113	J1732	ST RARA Mobil	Characteristic	149435,19	355000	33606,33	20499,35	160371,55	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	85011,80	360000	Verified
Concio 4 Hava	114	J1732	ST RARA Mobil	Characteristic	150688,63	355000	33661,84	20499,35	161984,14	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	84477,29	360000	Verified
Concio 4 Hava	114	J1733	ST RARA Mobil	Characteristic	152451,10	355000	34445,14	20499,35	163709,32	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	88265,37	360000	Verified
Concio 4 Hava	115	J1733	ST RARA Mobil	Characteristic	156533,69	355000	33134,01	20499,35	162149,40	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	93857,88	360000	Verified
Concio 4 Hava	115	J1734	ST RARA Mobil	Characteristic	167113,32	355000	44680,94	20499,35	184163,03	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	109455,24	360000	Verified
Concio 5	116	J1734	ST RARA Mobil	Characteristic	210049,18	355000	43810,54	20499,35	223336,07	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	17280,54	360000	Verified
Concio 5	116	J1735	ST RARA Mobil	Characteristic	192367,54	355000	42600,51	20499,35	200093,20	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	161967,54	360000	Verified
Concio 5	117	J1735	ST RARA Mobil	Characteristic	190298,67	355000	40776,85	20499,35	202906,31	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	157479,95	360000	Verified
Concio 5	117	J1736	ST RARA Termino	Characteristic	63818,34	355000	21247,46	20499,35	12474,46	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	10514,99	360000	Verified
Concio 5	118	J1736	ST RARA Termino	Characteristic	63776,40	355000	20765,61	20499,35	14508,67	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	9466,99	360000	Verified
Concio 5	118	J1737	ST RARA Mobil	Characteristic	53767,77	355000	20726,20	20499,35	64825,65	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	79727,73	360000	Verified
Concio 6	119	J1737	ST RARA Mobil	Characteristic	56062,84	355000	34543,67	20499,35	82610,90	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	96280,05	360000	Verified
Concio 6	119	J1738	ST RARA Mobil	Characteristic	95437,57	355000	26391,65	20499,35	10761,95	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	166892,70	360000	Verified
Concio 6	120	J1738	ST RARA Mobil	Characteristic	92622,33	355000	4878,92	20499,35	93006,72	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	89066,57	360000	Verified
Concio 6	120	J1739	ST RARA Mobil	Characteristic	94934,10	355000	56,18	20499,35	94934,10	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	81238,31	360000	Verified
Concio 6	121	J1739	ST RARA Mobil	Characteristic	94934,10	355000	56,18	20499,35	94934,10	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	81238,31	360000	Verified
Concio 6	121	J1740	ST RARA Mobil	Characteristic	95438,66	355000	4882,11	20499,35	93003,87	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	88062,27	360000	Verified
Concio 6	122	J1740	ST RARA Mobil	Characteristic	95438,66	355000	29050,37	20499,35	107890,17	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	96687,54	360000	Verified
Concio 6	122	J1741	ST RARA Mobil	Characteristic	95699,77	355000	35202,12	20499,35	104338,50	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	96926,73	360000	Verified
Concio 5	123	J1741	ST RARA Mobil	Characteristic	53734,96	355000	21121,27	20499,35	69005,92	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	79725,88	360000	Verified
Concio 5	123	J1742	ST RARA Termino	Characteristic	63568,61	355000	20245,54	20499,35	74180,20	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	94604,41	360000	Verified
Concio 5	124	J1742	ST RARA Termino	Characteristic	65212,59	355000	19185,67	20499,35	79191,19	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	106103,03	360000	Verified
Concio 5	124	J1743	ST RARA Mobil	Characteristic	190285,25	355000	39268,27	20499,35	202075,40	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	157482,20	360000	Verified
Concio 5	125	J1743	ST RARA Mobil	Characteristic	192401,53	355000	38732,55	20499,35	203762,07	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	161900,11	360000	Verified
Concio 5	125	J1744	ST RARA Mobil	Characteristic	210080,80	355000	39942,59	20499,35	221179,05	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	171385,42	360000	Verified
Concio 4 Hava	126	J1744	ST RARA Mobil	Characteristic	16740,36	355000	41065,31	20499,35	181645,20	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	109202,58	360000	Verified
Concio 4 Hava	126	J1745	ST RARA Mobil	Characteristic	15663,69	355000	30791,49	20499,35	160767,60	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	93885,74	360000	Verified
Concio 4 Hava	127	J1745	ST RARA Mobil	Characteristic	152457,87	355000	30146,42	20499,35	161151,54	355000	ST RARA Mobil	Characteristic	0,00	9200	ST RARA Mobil	Characteristic	85246,41	360000	Verified
Concio 4 Hava	127	J174																	

Concio 1	140	J758	ST RARA Vento	Characteristic	-792.43	355000	2363.67	204959.35	4.169.99	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	728.73	360000	Verified
Concio 1	140	J759	ST RARA Frenamento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Frenamento	Characteristic	0.00	19200	ST RARA Frenamento	Characteristic	0.00	360000	Verified
Concio 1	201	J760	ST RARA Frenamento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Frenamento	Characteristic	0.00	19200	ST RARA Frenamento	Characteristic	0.00	360000	Verified
Concio 1	201	J761	ST RARA Mobili	Characteristic	1270.100	355000	25533.04	204959.35	460.12.21	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	15292.59	360000	Verified
Concio 1	202	J761	ST RARA Mobili	Characteristic	39954.87	355000	71477.95	204959.35	13009105	355000	ST RARA Mobili	Characteristic	6.16.71	19200	ST RARA Mobili	Characteristic	11202.70	360000	Verified
Concio 1	202	J762	ST RARA Mobili	Characteristic	154374.55	355000	30066.78	204959.35	57863.82	355000	ST RARA Mobili	Characteristic	4648.39	19200	ST RARA Mobili	Characteristic	29233.25	360000	Verified
Concio 1	203	J762	ST RARA Mobili	Characteristic	144687.81	355000	30041.20	204959.35	163759.50	355000	ST RARA Mobili	Characteristic	4652.03	19200	ST RARA Mobili	Characteristic	25639.35	360000	Verified
Concio 1	203	J763	ST RARA Mobili	Characteristic	192299.49	355000	9676.21	204959.35	192032.24	355000	ST RARA Mobili	Characteristic	5399.17	19200	ST RARA Mobili	Characteristic	29070.77	360000	Verified
Concio 2	204	J763	ST RARA Mobili	Characteristic	15017.78	355000	9780.25	204959.35	155940.60	355000	ST RARA Mobili	Characteristic	4997.07	19200	ST RARA Mobili	Characteristic	2735190	360000	Verified
Concio 2	204	J764	ST RARA Mobili	Characteristic	163463.43	355000	5644.22	204959.35	163755.50	355000	ST RARA Mobili	Characteristic	5351.30	19200	ST RARA Mobili	Characteristic	29367.68	360000	Verified
Concio 2	205	J764	ST RARA Mobili	Characteristic	162948.33	355000	1121.82	204959.35	164083.03	355000	ST RARA Mobili	Characteristic	547.83	19200	ST RARA Mobili	Characteristic	29802.66	360000	Verified
Concio 2	205	J765	ST RARA Mobili	Characteristic	143661.19	355000	28772.68	204959.35	152059.66	355000	ST RARA Mobili	Characteristic	4896.41	19200	ST RARA Mobili	Characteristic	26923.73	360000	Verified
Concio 2	206	J765	ST RARA Mobili	Characteristic	146244.16	355000	35522.02	204959.35	158659.37	355000	ST RARA Mobili	Characteristic	4794.50	19200	ST RARA Mobili	Characteristic	34773.79	360000	Verified
Concio 2	206	J766	ST RARA Mobili	Characteristic	105196.06	355000	55938.08	204959.35	143015.45	355000	ST RARA Mobili	Characteristic	3695.24	19200	ST RARA Mobili	Characteristic	52530.82	360000	Verified
Concio 3	207	J766	ST RARA Mobili	Characteristic	10344.24	355000	4193.56	204959.35	124703.48	355000	ST RARA Mobili	Characteristic	3673.91	19200	ST RARA Mobili	Characteristic	43663.81	360000	Verified
Concio 3	207	J767	ST RARA Mobili	Characteristic	60559.05	355000	39474.06	204959.35	91334.56	355000	ST RARA Mobili	Characteristic	2872.76	19200	ST RARA Mobili	Characteristic	65785.95	360000	Verified
Concio 3	208	J767	ST RARA Mobili	Characteristic	76959.90	355000	46562.09	204959.35	105463.89	355000	ST RARA Mobili	Characteristic	2846.58	19200	ST RARA Mobili	Characteristic	70917.87	360000	Verified
Concio 3	208	J768	ST RARA Mobili	Characteristic	168152.53	355000	48245.82	204959.35	187718.1	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	141475.27	360000	Verified
Concio 3	209	J768	ST RARA Mobili	Characteristic	167499.32	355000	46106.36	204959.35	185562.42	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	221777.31	360000	Verified
Concio 3	209	J769	ST RARA Mobili	Characteristic	186625.01	355000	48009.58	204959.35	204312.64	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	188934.08	360000	Verified
Concio 4 Hvar	210	J769	ST RARA Mobili	Characteristic	150159.91	355000	48857.32	204959.35	172363.31	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	107766.44	360000	Verified
Concio 4 Hvar	210	J770	ST RARA Mobili	Characteristic	149349.10	355000	38107.81	204959.35	163284.32	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	89844.43	360000	Verified
Concio 4 Hvar	211	J770	ST RARA Mobili	Characteristic	184623.80	355000	38004.96	204959.35	162548.97	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	78703.62	360000	Verified
Concio 4 Hvar	211	J771	ST RARA Mobili	Characteristic	154219.45	355000	32967.25	204959.35	16445108	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	90812.23	360000	Verified
Concio 4 H20	212	J771	ST RARA Mobili	Characteristic	152919.15	355000	32879.92	204959.35	163179.46	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	93448.63	360000	Verified
Concio 4 H20	212	J772	ST RARA Mobili	Characteristic	163559.25	355000	37780.23	204959.35	176163.75	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	105763.79	360000	Verified
Concio 4 H20	213	J772	ST RARA Mobili	Characteristic	16458183	355000	39432.32	204959.35	178190.63	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	107690.83	360000	Verified
Concio 4 H20	213	J773	ST RARA Mobili	Characteristic	153045.45	355000	37562.83	204959.35	163300.35	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	94807.71	360000	Verified
Concio 4 Hvar	214	J773	ST RARA Mobili	Characteristic	154488.96	355000	37174.19	204959.35	167369.65	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	92209.11	360000	Verified
Concio 4 Hvar	214	J774	ST RARA Mobili	Characteristic	153736.78	355000	37957.80	204959.35	167204.61	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	814313.1	360000	Verified
Concio 4 Hvar	215	J774	ST RARA Mobili	Characteristic	154323.75	355000	36264.34	204959.35	166161.71	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	93718.93	360000	Verified
Concio 4 Hvar	215	J775	ST RARA Mobili	Characteristic	16638108	355000	46013.49	204959.35	184484.12	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	181130.9	360000	Verified
Concio 5	216	J775	ST RARA Mobili	Characteristic	208115.60	355000	45686.00	204959.35	222651.60	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	213635.25	360000	Verified
Concio 5	216	J776	ST RARA Mobili	Characteristic	180639.08	355000	43777.58	204959.35	205855.01	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	244308.86	360000	Verified
Concio 5	217	J776	ST RARA Mobili	Characteristic	19193.09	355000	44546.81	204959.35	208664.93	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	186918.8	360000	Verified
Concio 5	217	J777	ST RARA Termco	Characteristic	58243.61	355000	21841.10	204959.35	69450.84	355000	ST RARA Mobili	Characteristic	837.16	19200	ST RARA Mobili	Characteristic	100743.34	360000	Verified
Concio 5	218	J777	ST RARA Termco	Characteristic	61615.13	355000	21871.04	204959.35	72328.77	355000	ST RARA Mobili	Characteristic	1746.68	19200	ST RARA Mobili	Characteristic	95726.40	360000	Verified
Concio 5	218	J778	ST RARA Mobili	Characteristic	66770.07	355000	17926.71	204959.35	73636.56	355000	ST RARA Mobili	Characteristic	2462.29	19200	ST RARA Mobili	Characteristic	78909.59	360000	Verified
Concio 6	219	J778	ST RARA Mobili	Characteristic	71005.74	355000	29877.86	204959.35	87862.82	355000	ST RARA Mobili	Characteristic	2484.67	19200	ST RARA Mobili	Characteristic	96246.60	360000	Verified
Concio 6	219	J779	ST RARA Mobili	Characteristic	98613.51	355000	52633.89	204959.35	99032.49	355000	ST RARA Mobili	Characteristic	3174.38	19200	ST RARA Mobili	Characteristic	94028.41	360000	Verified
Concio 6	220	J779	ST RARA Mobili	Characteristic	96364.87	355000	1916.49	204959.35	96422.03	355000	ST RARA Mobili	Characteristic	3328.13	19200	ST RARA Mobili	Characteristic	85949.73	360000	Verified
Concio 6	220	J780	ST RARA Mobili	Characteristic	106332.98	355000	2652.25	204959.35	106432.17	355000	ST RARA Mobili	Characteristic	3436.79	19200	ST RARA Mobili	Characteristic	78282.16	360000	Verified
Concio 6	221	J780	ST RARA Mobili	Characteristic	106332.98	355000	2652.25	204959.35	106432.17	355000	ST RARA Mobili	Characteristic	3436.79	19200	ST RARA Mobili	Characteristic	78282.16	360000	Verified
Concio 6	221	J781	ST RARA Mobili	Characteristic	96374.45	355000	24958.05	204959.35	105225.50	355000	ST RARA Mobili	Characteristic	3328.85	19200	ST RARA Mobili	Characteristic	85355.26	360000	Verified
Concio 6	222	J781	ST RARA Mobili	Characteristic	98604.20	355000	32476.23	204959.35	119320.50	355000	ST RARA Mobili	Characteristic	3173.72	19200	ST RARA Mobili	Characteristic	94033.81	360000	Verified
Concio 6	222	J782	ST RARA Mobili	Characteristic	70994.23	355000	57099.75	204959.35	121742.86	355000	ST RARA Mobili	Characteristic	2484.18	19200	ST RARA Mobili	Characteristic	96248.97	360000	Verified
Concio 5	223	J782	ST RARA Mobili	Characteristic	66759.53	355000	34259.85	204959.35	89319.91	355000	ST RARA Mobili	Characteristic	2461.81	19200	ST RARA Mobili	Characteristic	78910.89	360000	Verified
Concio 5	223	J783	ST RARA Termco	Characteristic	61611.52	355000	21542.36	204959.35	72029.15	355000	ST RARA Mobili	Characteristic	1748.03	19200	ST RARA Mobili	Characteristic	95722.77	360000	Verified
Concio 5	224	J783	ST RARA Termco	Characteristic	58246.26	355000	20182.99	204959.35	67931.48	355000	ST RARA Mobili	Characteristic	1841.68	19200	ST RARA Mobili	Characteristic	100761.16	360000	Verified
Concio 5	224	J784	ST RARA Mobili	Characteristic	119147.64	355000	43502.77	204959.35	202607.10	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	158604.42	360000	Verified
Concio 5	225	J784	ST RARA Mobili	Characteristic	180647.84	355000	41211.66	204959.35	203572.59	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	238020.39	360000	Verified
Concio 5	225	J785	ST RARA Mobili	Characteristic	208124.72	355000	43120.18	204959.35	212119.76	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	205177.74	360000	Verified
Concio 4 Hvar	226	J785	ST RARA Mobili	Characteristic	166382.10	355000	43961.68	204959.35	182977.84	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	180015.57	360000	Verified
Concio 4 Hvar	226	J786	ST RARA Mobili	Characteristic	154326.26	355000	34934.98	204959.35	165764.75	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili				



Concio 1	240	J(799)	ST RARA Mobili	Characteristic	12700.09	355000	25533.03	204959.35	46011.94	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	15291.44	360000	Verified
Concio 1	240	J(800)	ST RARA Vento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Vento	Characteristic	0.00	19200	ST RARA Vento	Characteristic	0.00	360000	Verified
Concio 1	301	J(801)	ST RARA Frenamento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Frenamento	Characteristic	0.00	19200	ST RARA Frenamento	Characteristic	0.00	360000	Verified
Concio 1	301	J(802)	ST RARA Mobili	Characteristic	12701.00	355000	25533.04	204959.35	46012.01	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	15292.50	360000	Verified
Concio 1	302	J(802)	ST RARA Mobili	Characteristic	36641.66	355000	68457.50	204959.35	124104.39	355000	ST RARA Mobili	Characteristic	3954.11	19200	ST RARA Mobili	Characteristic	11203.10	360000	Verified
Concio 1	302	J(803)	ST RARA Mobili	Characteristic	154340.77	355000	30012.52	204959.35	157902.79	355000	ST RARA Mobili	Characteristic	4419.88	19200	ST RARA Mobili	Characteristic	23750.66	360000	Verified
Concio 1	303	J(803)	ST RARA Mobili	Characteristic	144900.07	355000	30015.21	204959.35	153944.05	355000	ST RARA Mobili	Characteristic	4794.20	19200	ST RARA Mobili	Characteristic	26219.66	360000	Verified
Concio 1	303	J(804)	ST RARA Mobili	Characteristic	191458.17	355000	9650.22	204959.35	192186.39	355000	ST RARA Mobili	Characteristic	5344.95	19200	ST RARA Mobili	Characteristic	28744.78	360000	Verified
Concio 2	304	J(804)	ST RARA Mobili	Characteristic	155159.91	355000	9733.99	204959.35	156076.91	355000	ST RARA Mobili	Characteristic	4942.60	19200	ST RARA Mobili	Characteristic	27025.04	360000	Verified
Concio 2	304	J(805)	ST RARA Mobili	Characteristic	163659.58	355000	5670.48	204959.35	163964.01	355000	ST RARA Mobili	Characteristic	5229.10	19200	ST RARA Mobili	Characteristic	28635.91	360000	Verified
Concio 2	305	J(805)	ST RARA Mobili	Characteristic	163035.14	355000	1126.44	204959.35	164170.18	355000	ST RARA Mobili	Characteristic	5447.40	19200	ST RARA Mobili	Characteristic	29977.59	360000	Verified
Concio 2	305	J(806)	ST RARA Mobili	Characteristic	143816.48	355000	2877.30	204959.35	146700.30	355000	ST RARA Mobili	Characteristic	4993.91	19200	ST RARA Mobili	Characteristic	37665.19	360000	Verified
Concio 2	306	J(806)	ST RARA Mobili	Characteristic	146423.90	355000	3556.21	204959.35	149980.35	355000	ST RARA Mobili	Characteristic	4888.17	19200	ST RARA Mobili	Characteristic	28252.83	360000	Verified
Concio 2	306	J(807)	ST RARA Mobili	Characteristic	105272.54	355000	55974.27	204959.35	143114.16	355000	ST RARA Mobili	Characteristic	3737.92	19200	ST RARA Mobili	Characteristic	57912.3	360000	Verified
Concio 3	307	J(807)	ST RARA Mobili	Characteristic	101419.39	355000	41980.70	204959.35	124791.95	355000	ST RARA Mobili	Characteristic	376.55	19200	ST RARA Mobili	Characteristic	4724.17	360000	Verified
Concio 3	307	J(808)	ST RARA Mobili	Characteristic	60632.02	355000	39501.20	204959.35	91418.13	355000	ST RARA Mobili	Characteristic	2936.01	19200	ST RARA Mobili	Characteristic	60944.40	360000	Verified
Concio 3	308	J(808)	ST RARA Mobili	Characteristic	68032.90	355000	46604.06	204959.35	105666.51	355000	ST RARA Mobili	Characteristic	2818.24	19200	ST RARA Mobili	Characteristic	69927.27	360000	Verified
Concio 3	308	J(809)	ST RARA Mobili	Characteristic	188924.81	355000	48203.86	204959.35	184314.99	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	143609.01	360000	Verified
Concio 3	309	J(809)	ST RARA Mobili	Characteristic	166087.98	355000	46056.39	204959.35	184251.97	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	235864.33	360000	Verified
Concio 3	309	J(810)	ST RARA Mobili	Characteristic	185760.23	355000	47959.61	204959.35	203487.68	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	205348.69	360000	Verified
Concio 4 Hvar	310	J(810)	ST RARA Mobili	Characteristic	149428.65	355000	48772.06	204959.35	176653.91	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	13086.73	360000	Verified
Concio 4 Hvar	310	J(811)	ST RARA Mobili	Characteristic	150496.71	355000	38052.57	204959.35	164296.24	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	8518.20	360000	Verified
Concio 4 Hvar	311	J(811)	ST RARA Mobili	Characteristic	148893.24	355000	37936.06	204959.35	162747.14	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	7550.56	360000	Verified
Concio 4 Hvar	311	J(812)	ST RARA Mobili	Characteristic	154672.24	355000	32916.29	204959.35	164845.22	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	88130.25	360000	Verified
Concio 4 H20	312	J(812)	ST RARA Mobili	Characteristic	153371.13	355000	32847.27	204959.35	163583.42	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	92053.06	360000	Verified
Concio 4 H20	312	J(813)	ST RARA Mobili	Characteristic	163137.69	355000	37747.58	204959.35	175751.38	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	104298.63	360000	Verified
Concio 4 H20	313	J(813)	ST RARA Mobili	Characteristic	163882.22	355000	39464.73	204959.35	177566.25	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	100113.34	360000	Verified
Concio 4 H20	313	J(814)	ST RARA Mobili	Characteristic	152618.11	355000	37955.24	204959.35	169291.18	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	90688.15	360000	Verified
Concio 4 Hvar	314	J(814)	ST RARA Mobili	Characteristic	154066.46	355000	37225.95	204959.35	167014.33	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	87891.58	360000	Verified
Concio 4 Hvar	314	J(815)	ST RARA Mobili	Characteristic	154087.08	355000	38027.78	204959.35	167574.36	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	78956.21	360000	Verified
Concio 4 Hvar	315	J(815)	ST RARA Mobili	Characteristic	155695.36	355000	36322.87	204959.35	167925.88	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	87353.72	360000	Verified
Concio 4 Hvar	315	J(816)	ST RARA Mobili	Characteristic	165463.38	355000	46103.82	204959.35	183724.84	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	123687.13	360000	Verified
Concio 5	316	J(816)	ST RARA Mobili	Characteristic	207040.03	355000	45735.81	204959.35	221677.39	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	21431.14	360000	Verified
Concio 5	316	J(817)	ST RARA Mobili	Characteristic	188885.09	355000	43827.39	204959.35	203568.41	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	24637.81	360000	Verified
Concio 5	317	J(817)	ST RARA Mobili	Characteristic	192901.66	355000	44589.81	204959.35	207787.89	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	16325.58	360000	Verified
Concio 5	317	J(818)	ST RARA Termino	Characteristic	59160.69	355000	21884.09	204959.35	70261.15	355000	ST RARA Mobili	Characteristic	1769.11	19200	ST RARA Mobili	Characteristic	10102.14	360000	Verified
Concio 5	318	J(818)	ST RARA Termino	Characteristic	60976.29	355000	21894.96	204959.35	71807.21	355000	ST RARA Mobili	Characteristic	1844.03	19200	ST RARA Mobili	Characteristic	92407.54	360000	Verified
Concio 5	318	J(819)	ST RARA Mobili	Characteristic	66864.16	355000	17902.79	204959.35	73704.45	355000	ST RARA Mobili	Characteristic	2508.28	19200	ST RARA Mobili	Characteristic	82599.67	360000	Verified
Concio 6	319	J(819)	ST RARA Mobili	Characteristic	71103.20	355000	29837.99	204959.35	87900.98	355000	ST RARA Mobili	Characteristic	2530.73	19200	ST RARA Mobili	Characteristic	101284.71	360000	Verified
Concio 6	319	J(820)	ST RARA Mobili	Characteristic	98887.70	355000	52104.02	204959.35	192999.22	355000	ST RARA Mobili	Characteristic	3337.22	19200	ST RARA Mobili	Characteristic	88479.51	360000	Verified
Concio 6	320	J(820)	ST RARA Mobili	Characteristic	96481.32	355000	1916.61	204959.35	96538.41	355000	ST RARA Mobili	Characteristic	3383.66	19200	ST RARA Mobili	Characteristic	91035.56	360000	Verified
Concio 6	320	J(821)	ST RARA Mobili	Characteristic	106449.20	355000	2652.36	204959.35	106548.29	355000	ST RARA Mobili	Characteristic	3492.22	19200	ST RARA Mobili	Characteristic	83958.33	360000	Verified
Concio 6	321	J(821)	ST RARA Mobili	Characteristic	106449.20	355000	2652.36	204959.35	106548.29	355000	ST RARA Mobili	Characteristic	3492.22	19200	ST RARA Mobili	Characteristic	83958.33	360000	Verified
Concio 6	321	J(822)	ST RARA Mobili	Characteristic	96490.46	355000	24958.16	204959.35	105731.44	355000	ST RARA Mobili	Characteristic	3358.18	19200	ST RARA Mobili	Characteristic	91021.78	360000	Verified
Concio 6	322	J(822)	ST RARA Mobili	Characteristic	98880.20	355000	32516.96	204959.35	113795.22	355000	ST RARA Mobili	Characteristic	3336.82	19200	ST RARA Mobili	Characteristic	88494.90	360000	Verified
Concio 6	322	J(823)	ST RARA Mobili	Characteristic	70922.29	355000	5740.48	204959.35	121857.36	355000	ST RARA Mobili	Characteristic	2530.17	19200	ST RARA Mobili	Characteristic	101822.77	360000	Verified
Concio 5	323	J(823)	ST RARA Mobili	Characteristic	66854.16	355000	34284.29	204959.35	89418.76	355000	ST RARA Mobili	Characteristic	2507.72	19200	ST RARA Mobili	Characteristic	82357.93	360000	Verified
Concio 5	323	J(824)	ST RARA Termino	Characteristic	60974.33	355000	21617.92	204959.35	71462.80	355000	ST RARA Mobili	Characteristic	1845.41	19200	ST RARA Mobili	Characteristic	92409.51	360000	Verified
Concio 5	324	J(824)	ST RARA Termino	Characteristic	59161.14	355000	20140.74	204959.35	68680.33	355000	ST RARA Mobili	Characteristic	1771.03	19200	ST RARA Mobili	Characteristic	10110.37	360000	Verified
Concio 5	324	J(825)	ST RARA Mobili	Characteristic	182916.41	355000	43460.82	204959.35	207082.57	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	161864.61	360000	Verified
Concio 5	325	J(825)	ST RARA Mobili	Characteristic	188898.96	355000	41862.35	204959.35	201905.51	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	258062.52	360000	Verified
Concio 5	325	J(826)	ST RARA Mobili	Characteristic	207052.48	355000	43070.87	204959.35	220081.87	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	223796.10	360000	Verified
Concio 4 Hvar	326	J(826)	ST RARA Mobili	Characteristic	165471.79	355000	43872.18	204959.35	182095.74	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	123904.70	360000	Verified
Concio 4 Hvar	326	J(827)	ST RARA Mobili	Characteristic	155698.67	355000	34876.99	204959.35	167006.86	3									

Concio 1	340	[840]	ST RARA Mobili	Characteristic	12700.09	355000	25933.003	204959.35	4601194	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	1529144	360000	Verified
Concio 1	340	[841]	ST RARA Vento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	0.00	360000	Verified
Concio 1	401	[842]	ST RARA Frenamento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Frenamento	Characteristic	0.00	19200	ST RARA Frenamento	Characteristic	0.00	360000	Verified
Concio 1	401	[843]	ST RARA Mobili	Characteristic	799.52	355000	2485.80	204959.35	4337.90	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	506.25	360000	Verified
Concio 1	402	[843]	ST RARA Mobili	Characteristic	19775.09	355000	50040.44	204959.35	88989.89	355000	ST RARA Mobili	Characteristic	1232.56	19200	ST RARA Mobili	Characteristic	62606.08	360000	Verified
Concio 1	402	[844]	ST RARA Mobili	Characteristic	144395.25	355000	38323.66	204959.35	58922.58	355000	ST RARA Mobili	Characteristic	4586.92	19200	ST RARA Mobili	Characteristic	25143.73	360000	Verified
Concio 1	403	[844]	ST RARA Mobili	Characteristic	136476.02	355000	21736.93	204959.35	41573.77	355000	ST RARA Mobili	Characteristic	4019.79	19200	ST RARA Mobili	Characteristic	2192.46	360000	Verified
Concio 1	403	[845]	ST RARA Mobili	Characteristic	78479.66	355000	18631.60	204959.35	18173.64	355000	ST RARA Mobili	Characteristic	50814.33	19200	ST RARA Mobili	Characteristic	2759.75	360000	Verified
Concio 2	404	[845]	ST RARA Mobili	Characteristic	141579.64	355000	189119.24	204959.35	14321.35	355000	ST RARA Mobili	Characteristic	4772.84	19200	ST RARA Mobili	Characteristic	2361.51	360000	Verified
Concio 2	404	[846]	ST RARA Mobili	Characteristic	516333.31	355000	58833.78	204959.35	15371.10	355000	ST RARA Mobili	Characteristic	5275.58	19200	ST RARA Mobili	Characteristic	30520.03	360000	Verified
Concio 2	405	[846]	ST RARA Mobili	Characteristic	160143.22	355000	5528.31	204959.35	160429.25	355000	ST RARA Mobili	Characteristic	3532.75	19200	ST RARA Mobili	Characteristic	29256.54	360000	Verified
Concio 2	405	[847]	ST RARA Mobili	Characteristic	139287.99	355000	13630.83	204959.35	14274.64	355000	ST RARA Mobili	Characteristic	4545.83	19200	ST RARA Mobili	Characteristic	4159.27	360000	Verified
Concio 2	406	[847]	ST RARA Mobili	Characteristic	142478.46	355000	92398.23	204959.35	16319.82	355000	ST RARA Mobili	Characteristic	514.75	19200	ST RARA Mobili	Characteristic	2957.75	360000	Verified
Concio 2	406	[848]	ST RARA Mobili	Characteristic	1928133	355000	37155.86	204959.35	11866.93	355000	ST RARA Mobili	Characteristic	3203.40	19200	ST RARA Mobili	Characteristic	60422.58	360000	Verified
Concio 3	407	[848]	ST RARA Mobili	Characteristic	1807123	355000	27861.90	204959.35	100417.81	355000	ST RARA Mobili	Characteristic	3858.86	19200	ST RARA Mobili	Characteristic	48579.00	360000	Verified
Concio 3	407	[849]	ST RARA Mobili	Characteristic	15763.94	355000	29765.02	204959.35	73057.34	355000	ST RARA Mobili	Characteristic	2308.45	19200	ST RARA Mobili	Characteristic	6780.70	360000	Verified
Concio 3	408	[849]	ST RARA Mobili	Characteristic	15033.07	355000	3084.93	204959.35	35151.65	355000	ST RARA Mobili	Characteristic	2791.43	19200	ST RARA Mobili	Characteristic	6783.97	360000	Verified
Concio 3	408	[850]	ST RARA Mobili	Characteristic	17260.74	355000	44617.30	204959.35	18789.20	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	1570.93	360000	Verified
Concio 3	409	[850]	ST RARA Mobili	Characteristic	17425.94	355000	44197.13	204959.35	18789.53	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	15578.83	360000	Verified
Concio 3	409	[851]	ST RARA Mobili	Characteristic	19276.22	355000	45840.04	204959.35	206768.87	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	65247.24	360000	Verified
Concio 4 Hava	410	[851]	ST RARA Mobili	Characteristic	153250.47	355000	46349.49	204959.35	173004.43	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	103933.38	360000	Verified
Concio 4 Hava	410	[852]	ST RARA Mobili	Characteristic	1492.91	355000	34208.59	204959.35	160552.24	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	85963.16	360000	Verified
Concio 4 Hava	411	[852]	ST RARA Mobili	Characteristic	14938.71	355000	33469.09	204959.35	160754.96	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	87843.90	360000	Verified
Concio 4 Hava	411	[853]	ST RARA Mobili	Characteristic	147032.40	355000	27915.57	204959.35	154778.37	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	80643.20	360000	Verified
Concio 4 H20	412	[853]	ST RARA Mobili	Characteristic	145861.91	355000	27576.70	204959.35	153483.28	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	81136.38	360000	Verified
Concio 4 H20	412	[854]	ST RARA Mobili	Characteristic	158606.89	355000	32274.24	204959.35	168169.63	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	88074.65	360000	Verified
Concio 4 H20	413	[854]	ST RARA Mobili	Characteristic	158976.45	355000	34297.29	204959.35	16972.77	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	88600.29	360000	Verified
Concio 4 H20	413	[855]	ST RARA Mobili	Characteristic	148369.74	355000	33542.39	204959.35	159338.81	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	81310.18	360000	Verified
Concio 4 Hava	414	[855]	ST RARA Mobili	Characteristic	149629.86	355000	33605.56	204959.35	160552.47	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	80821.17	360000	Verified
Concio 4 Hava	414	[856]	ST RARA Mobili	Characteristic	153777.81	355000	34369.05	204959.35	164897.87	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	91075.09	360000	Verified
Concio 4 Hava	415	[856]	ST RARA Mobili	Characteristic	152550.35	355000	33068.03	204959.35	162948.13	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	87835.09	360000	Verified
Concio 4 Hava	415	[857]	ST RARA Mobili	Characteristic	18304.21	355000	44579.09	204959.35	16570.71	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	113839.22	360000	Verified
Concio 5	416	[857]	ST RARA Mobili	Characteristic	21193.32	355000	43688.18	204959.35	224528.63	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	79694.53	360000	Verified
Concio 5	416	[858]	ST RARA Mobili	Characteristic	19480.99	355000	42478.14	204959.35	207652.20	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	172398.03	360000	Verified
Concio 5	417	[858]	ST RARA Mobili	Characteristic	192809.31	355000	40618.04	204959.35	202843.52	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	17183.23	360000	Verified
Concio 5	417	[859]	ST RARA Termico	Characteristic	64204.21	355000	21852.65	204959.35	73922.15	355000	ST RARA Mobili	Characteristic	1777.68	19200	ST RARA Mobili	Characteristic	99131.08	360000	Verified
Concio 5	418	[859]	ST RARA Termico	Characteristic	65215.81	355000	20732.66	204959.35	74448.85	355000	ST RARA Mobili	Characteristic	152.22	19200	ST RARA Mobili	Characteristic	98273.71	360000	Verified
Concio 5	418	[860]	ST RARA Mobili	Characteristic	53749.45	355000	20750.15	204959.35	64658.40	355000	ST RARA Mobili	Characteristic	2022.62	19200	ST RARA Mobili	Characteristic	84774.05	360000	Verified
Concio 6	419	[860]	ST RARA Mobili	Characteristic	56967.72	355000	34583.58	204959.35	82664.34	355000	ST RARA Mobili	Characteristic	2039.96	19200	ST RARA Mobili	Characteristic	104311.00	360000	Verified
Concio 6	419	[861]	ST RARA Mobili	Characteristic	95166.81	355000	28431.56	204959.35	10753.08	355000	ST RARA Mobili	Characteristic	3563.97	19200	ST RARA Mobili	Characteristic	89627.27	360000	Verified
Concio 6	420	[861]	ST RARA Mobili	Characteristic	92580.29	355000	48770.05	204959.35	32964.87	355000	ST RARA Mobili	Characteristic	3351.36	19200	ST RARA Mobili	Characteristic	93388.65	360000	Verified
Concio 6	420	[862]	ST RARA Mobili	Characteristic	94930.85	355000	56.28	204959.35	94930.90	355000	ST RARA Mobili	Characteristic	3114.98	19200	ST RARA Mobili	Characteristic	87672.24	360000	Verified
Concio 6	421	[862]	ST RARA Mobili	Characteristic	94930.85	355000	56.28	204959.35	94930.90	355000	ST RARA Mobili	Characteristic	3114.98	19200	ST RARA Mobili	Characteristic	87672.24	360000	Verified
Concio 6	421	[863]	ST RARA Mobili	Characteristic	92576.71	355000	48819.8	204959.35	92962.08	355000	ST RARA Mobili	Characteristic	3351.13	19200	ST RARA Mobili	Characteristic	93375.02	360000	Verified
Concio 6	422	[863]	ST RARA Mobili	Characteristic	95166.82	355000	29009.57	204959.35	10761.68	355000	ST RARA Mobili	Characteristic	3555.36	19200	ST RARA Mobili	Characteristic	89639.95	360000	Verified
Concio 6	422	[864]	ST RARA Mobili	Characteristic	95964.59	355000	35813.22	204959.35	83390.16	355000	ST RARA Mobili	Characteristic	2039.81	19200	ST RARA Mobili	Characteristic	104305.23	360000	Verified
Concio 5	423	[864]	ST RARA Mobili	Characteristic	53746.59	355000	21096.79	204959.35	64991.69	355000	ST RARA Mobili	Characteristic	2022.47	19200	ST RARA Mobili	Characteristic	84769.67	360000	Verified
Concio 5	423	[865]	ST RARA Termico	Characteristic	65211.34	355000	20270.02	204959.35	74061.73	355000	ST RARA Mobili	Characteristic	1515.8	19200	ST RARA Mobili	Characteristic	95217.4	360000	Verified
Concio 5	424	[865]	ST RARA Termico	Characteristic	64200.85	355000	19282.21	204959.35	73368.23	355000	ST RARA Mobili	Characteristic	1777.23	19200	ST RARA Mobili	Characteristic	99136.91	360000	Verified
Concio 5	424	[866]	ST RARA Mobili	Characteristic	92826.05	355000	39364.82	204959.35	204525.43	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	17218.30	360000	Verified
Concio 5	425	[866]	ST RARA Mobili	Characteristic	194192.25	355000	38855.21	204959.35	20592.26	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	172404.45	360000	Verified
Concio 5	425	[867]	ST RARA Mobili	Characteristic	21405.06	355000	40065.26	204959.35	222503.43	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	17903.25	360000	Verified
Concio 4 Hava	426	[867]	ST RARA Mobili	Characteristic	168310.46	355000	41168.33	204959.35	182791.98	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	118844.95	360000	Verified
Concio 4 Hava	426	[868]																	

## 8.5 VERIFICHE A FATICA

Si effettuano le verifiche allo stato limite di fatica per vita illimitata dei profilati metallici e delle saldature anima/piattabanda.

L'impalcato si considera caricato secondo il modello di carico a fatica 2, applicato sulla corsia lenta; il veicolo più gravoso è risultato essere il 3°: semirimorchio a 5 assi per un peso complessivo di 630 kN (ved. 3.4.1).

Si assume che le strutture siano sensibili alla rottura per fatica e che essa produca conseguenze significative; il coefficiente parziale di sicurezza per le verifiche è quindi pari a:  $\gamma_{Mf} = 1.35$ .

Le caratteristiche resistenti (elastiche) tengono conto del contributo del calcestruzzo della soletta se compressa, se tesa tengono conto solamente dell'armatura.

La verifica viene eseguita per ogni tipologia di trave (concio) per:

- a) profilato metallico: tensioni normali:  $\Delta\sigma_C = 112 \text{ N/mm}^2$
- b) profilato metallico: tensioni tangenziali:  $\Delta\tau_C = 100 \text{ N/mm}^2$
- c) saldature: tensioni tangenziali:  $\Delta\tau_C = 80 \text{ N/mm}^2$

Seguono le verifiche eseguite per ogni tipologia di sezione all'interno della quale si sono individuate le condizioni:

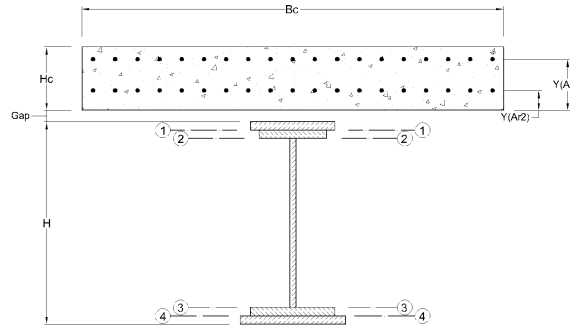
- 1)  $\Delta M$  massimo e  $\Delta T$  associato (per verifica a)
- 2)  $\Delta T$  massimo e  $\Delta M$  associato (per verifiche b, c)

**VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):**

**CONCI C1**

- Legenda**
- As Area profilo metallico
  - Yg\_inf(s) Baricentro profilo metallico
  - H Altezza profilo metallico
  - Yg\_inf Baricentro della sezione composta
  - Jc Momento d'inerzia della sezione composta
  - Sc\_max Momento statico massimo della sezione del profilo
  - Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
  - b Larghezza dei piatti (piattabande, anime, ecc.)
  - t Spessore dei piatti (piattabande, anime, ecc.)
  - A Sezione resistente dei piatti (piattabande, anime, ecc.)
  - Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
  - S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
  - $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
  - C.ANG Saldatura a cordone d'angolo
  - a Larghezza della gola del cordone d'angolo (somma)
  - P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
  - $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
  - Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
  - Bc 320 cm (base efficace soletta in clc.)
  - Hc 24 cm (altezza soletta in clc.)
  - Gap 6 cm (distanza tra profilo metallico e soletta)
  - Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
  - Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
  - Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
  - Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
  - Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



**Caratteristiche geometriche**

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jo/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	471.0	1.5	46.91	875818	100.0	98.88	2678605	-2396436	27089	24421.6	1.097	CLS in compressione
Teso	471.0	1.5	46.91	875818	100.0	55.33	1159041	-25948	20947	11743.6	0.987	CLS in trazione

**Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	94.0	1.5	141.0	48.88					ANIMA
sez1	0.0	0.0	0.0	-1.12	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	0.38	24421.2	1.097	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	3.0	180.0	97.38	17528.8	1.528	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	98.88	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

**Caratteristiche con sezione avente il CLS totalmente in trazione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	94.0	1.5	141.0	5.33					ANIMA
sez1	0.0	0.0	0.0	-44.67	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-43.17	10441.4	1.110	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	3.0	180.0	53.83	9689.7	1.196	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	55.33	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

**Sollecitazioni flettenti e taglianti associate (con segno)**

TIPO DI COMBINAZIONE	M(max)	M(min)	V(max)	V(min)	Elemento
DM(max) - DT(ass)	2810.8	1763.5	-56.5	-58.6	203
DM(ass) - DT(max)	-131.7	-28.7	768.5	392.0	239

**VERIFICA (tensioni normali) DEL PROFILATO METALLICO**

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
sez. max	103.8	65.1	38.66	112.0	61.14	VERIFICATO

<- Tab. C.4.2.XIV (3)  $\Delta\sigma c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma c=112$

**VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI**

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
sez. max	51.91	26.48	25.43	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	48.96	24.97	23.98	80.0	27.08	VERIFICATO
sez3	45.43	23.17	22.26	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<- Tab. C.4.2.XIIIb (6)  $\Delta\tau c=100$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$   
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 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$



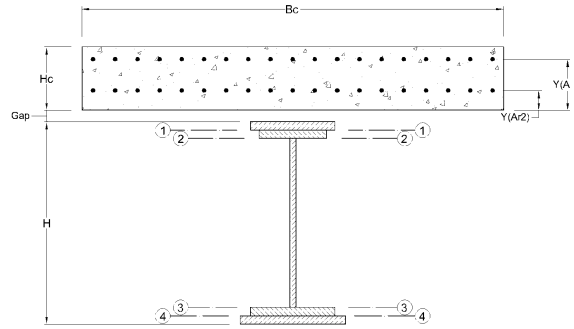


**VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):**

**CONCI C3**

- Legenda**
- As Area profilo metallico
  - Yg\_inf(s) Baricentro profilo metallico
  - H Altezza profilo metallico
  - Yg\_inf Baricentro della sezione composta
  - Jc Momento d'inerzia della sezione composta
  - Sc\_max Momento statico massimo della sezione del profilo
  - Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
  - b Larghezza dei piatti (piattabande, anime, ecc.)
  - t Spessore dei piatti (piattabande, anime, ecc.)
  - A Sezione resistente dei piatti (piattabande, anime, ecc.)
  - Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
  - S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
  - $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
  - C.ANG Saldatura a cordone d'angolo
  - a Larghezza della gola del cordone d'angolo (somma)
  - P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
  - $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
  - Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
  - Bc 320 cm (base efficace soletta in cls.)
  - Hc 24 cm (altezza soletta in cls.)
  - Gap 6 cm (distanza tra profilo metallico e soletta)
  - Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
  - Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
  - Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
  - Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
  - Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



**Caratteristiche geometriche**

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jo/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	576.0	2.0	42.79	1010400	100.0	94.67	3320287	-623177	35071	30462.7	1.090	CLS in compressione
Teso	576.0	2.0	42.79	1010400	100.0	50.24	1334247	-26816	26555	13717.2	0.973	CLS in trazione

**Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	2.0	186.0	44.17					ANIMA
sez1	0.0	0.0	0.0	-5.33	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-3.83	30457.3	1.090	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	4.0	240.0	92.67	22241.3	1.493	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	94.67	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

**Caratteristiche con sezione avente il CLS totalmente in trazione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	2.0	186.0	-0.26					ANIMA
sez1	0.0	0.0	0.0	-49.76	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-48.26	11531.1	1.157	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	4.0	240.0	48.24	11578.7	1.152	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	50.24	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

**Sollecitazioni flettenti e taglianti associate (con segno)**

TIPO DI COMBINAZIONE	M(max)	M(min)	V(max)	V(min)	Elemento
	kNm	kNm	kN	kN	n°
DM(max) - DT(ass)	1535.5	288.3	478.1	365.4	207
DM(ass) - DT(max)	-2550.6	-2398.7	-613.0	-956.4	233

**VERIFICA (tensioni normali) DEL PROFILATO METALLICO**

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	43.8	8.2	35.56	112.0	61.14	VERIFICATO

<- Tab. C.4.2.XIV (3)  $\Delta\sigma c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma c=112$

**VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI**

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	-31.51	-49.16	17.65	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	-37.46	-58.44	20.98	80.0	27.08	VERIFICATO
sez3	-37.61	-58.69	21.07	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

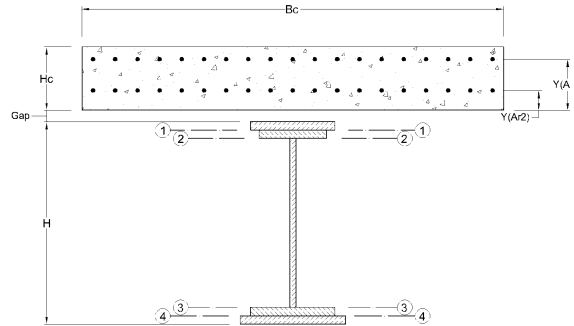
<- Tab. C.4.2.XIIIb (6)  $\Delta\tau c=100$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$

**VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):**

**CONCI C4**

- Legenda**
- As Area profilo metallico
  - Yg\_inf(s) Baricentro profilo metallico
  - H Altezza profilo metallico
  - Yg\_inf Baricentro della sezione composta
  - Jc Momento d'inerzia della sezione composta
  - Sc\_max Momento statico massimo della sezione del profilo
  - Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
  - b Larghezza dei piatti (piattabande, anime, ecc.)
  - t Spessore dei piatti (piattabande, anime, ecc.)
  - A Sezione resistente dei piatti (piattabande, anime, ecc.)
  - Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
  - S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
  - $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
  - C.ANG Saldatura a cordone d'angolo
  - a Larghezza della gola del cordone d'angolo (somma)
  - P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
  - $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
  - Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
  - Bc 320 cm (base efficace soletta in cls.)
  - Hc 24 cm (altezza soletta in cls.)
  - Gap 6 cm (distanza tra profilo metallico e soletta)
  - Ar1 100.48 cm<sup>2</sup> (area di armatura soletta)
  - Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
  - Ar2 100.48 cm<sup>2</sup> (area di armatura soletta)
  - Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
  - Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



**Caratteristiche geometriche**

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jo/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	904.0	2.0	95.66	6157420	200.0	170.33	14481460	-488151	85018	74800.6	1.936	CLS in compressione
Teso	904.0	2.0	95.66	6157420	200.0	117.73	8594389	-104463	73002	45338.0	1.896	CLS in trazione

**Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	192.0	2.0	384.0	70.33					ANIMA
sez1	0.0	0.0	0.0	-29.67	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	60.0	4.0	240.0	-27.67	74141.8	1.953	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	70.0	4.0	280.0	168.33	47133.5	3.072	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	170.33	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

**Caratteristiche con sezione avente il CLS totalmente in trazione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	192.0	2.0	384.0	17.73					ANIMA
sez1	0.0	0.0	0.0	-82.27	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	60.0	4.0	240.0	-80.27	39211.5	2.192	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	70.0	4.0	280.0	115.73	32403.9	2.652	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	117.73	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

**Sollecitazioni flettenti e taglianti associate (con segno)**

TIPO DI COMBINAZIONE	M(max)	M(min)	V(max)	V(min)	Elemento
DM(max) - DT(ass)	-7553.4	-8609.2	-846.4	-1105.7	213
DM(ass) - DT(max)	-8177.8	-7533.9	1242.7	844.1	228

**VERIFICA (tensioni normali) DEL PROFILATO METALLICO**

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
sez. max	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	VERIFICATO
	-103.5	-117.9	14.46	112.0	61.14	

<- Tab. C.4.2.XV (3)  $\Delta\sigma c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma c=112$

**VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI**

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
sez. max	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	VERIFICATO
	32.78	22.27	10.51	100.0	33.85	
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	40.09	27.23	12.86	80.0	27.08	VERIFICATO
sez3	33.13	22.51	10.63	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<- Tab. C.4.2.XIIIb (6)  $\Delta\tau c=100$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau c=80$   
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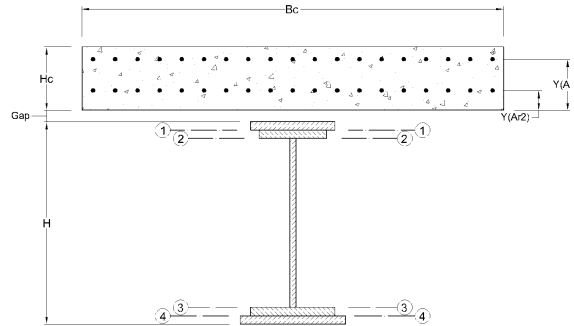
**VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):**

**CONCI C5**

**Legenda**

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i-esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in clc.)
- Hc 24 cm (altezza soletta in clc.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

**SCHEMA SEZIONE COMPOSTA**



**Caratteristiche geometriche**

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jo/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	CLS in compressione
Compr.	576.0	2.0	42.79	1010407	100.0	94.67	3320294	-623178	35072	30462.7	1.090	CLS in trazione
Teso	576.0	2.0	42.79	1010407	100.0	50.24	1334254	-26816	26555	13717.2	0.973	CLS in trazione

**Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	93.0	2.0	186.0	44.17					ANIMA
sez 1	0.0	0.0	0.0	-5.33	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez 2	50.0	3.0	150.0	-3.83	30457.3	1.090	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez 3	60.0	4.0	240.0	92.67	22241.3	1.493	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez 4	0.0	0.0	0.0	94.67	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

**Caratteristiche con sezione avente il CLS totalmente in trazione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	93.0	2.0	186.0	-0.26					ANIMA
sez 1	0.0	0.0	0.0	-49.76	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez 2	50.0	3.0	150.0	-48.26	11531.1	1.157	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez 3	60.0	4.0	240.0	48.24	11578.7	1.152	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez 4	0.0	0.0	0.0	50.24	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

**Sollecitazioni flettenti e taglianti associate (con segno)**

TIPO DI COMBINAZIONE	M(max) kNm	M(min) kNm	V(max) kN	V(min) kN	Elemento
DM(max) - DT(ass)	504.4	-659.1	361.4	254.6	223
DM(ass) - DT(max)	-3081.5	-2972.1	-526.3	-872.1	217

**VERIFICA (tensioni normali) DEL PROFILATO METALLICO**

POSIZ.	$\sigma_{inf}(+)$ N/mm <sup>2</sup>	$\sigma_{inf}(-)$ N/mm <sup>2</sup>	$\Delta\sigma$ N/mm <sup>2</sup>	$\Delta\sigma_c$ N/mm <sup>2</sup>	$\Delta\sigma D/\gamma M_f s$ N/mm <sup>2</sup>	$\Delta\sigma < \Delta\sigma D/\gamma M_f s$
sez. max	14.4	-24.8	39.20	112.0	61.14	VERIFICATO

<- Tab. C.4.2.XV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

**VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI**

POSIZ.	$\tau(+)$ N/mm <sup>2</sup>	$\tau(-)$ N/mm <sup>2</sup>	$\Delta\tau$ N/mm <sup>2</sup>	$\Delta\tau_c$ N/mm <sup>2</sup>	$\Delta\tau D/\gamma M_f s$ N/mm <sup>2</sup>	$\Delta\tau < \Delta\tau D/\gamma M_f s$
sez. max	-27.05	-44.83	17.78	100.0	33.85	VERIFICATO
sez 1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez 2	-32.16	-53.30	21.14	80.0	27.08	VERIFICATO
sez 3	-32.29	-53.52	21.22	80.0	27.08	VERIFICATO
sez 4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<- Tab. C.4.2.XIib (6)  $\Delta\tau_c=100$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
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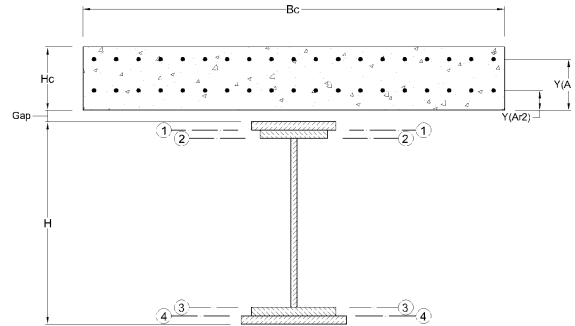
**VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):**

**CONCI C6**

**Legenda**

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i-esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in clc.)
- Hc 24 cm (altezza soletta in clc.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

**SCHEMA SEZIONE COMPOSTA**



**Caratteristiche geometriche**

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jo/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	501.6	1.2	41.65	951707	100.0	96.51	3115259	-893577	32278	27818.6	1.120	CLS in compressione
Teso	501.6	1.2	41.65	951707	100.0	50.21	1280596	-25722	25503	12852.8	0.996	CLS in trazione

**Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	1.2	111.6	46.01					ANIMA
sez1	0.0	0.0	0.0	-3.49	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-1.99	27818.4	1.120	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	4.0	240.0	94.51	22683.3	1.373	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	96.51	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

**Caratteristiche con sezione avente il CLS totalmente in trazione**

POSIZ.	b	t	A	Z	S_i	Jo/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	1.2	111.6	-0.29					ANIMA
sez1	0.0	0.0	0.0	-49.79	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-48.29	11539.4	1.110	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	4.0	240.0	48.21	11571.3	1.107	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	50.21	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

**Sollecitazioni flettenti e taglianti associate (con segno)**

TIPO DI COMBINAZIONE	M(max)	M(min)	V(max)	V(min)	Elemento
	kNm	kNm	kN	kN	n°
DM(max) - DT(ass)	507.0	-655.8	361.4	254.6	222
DM(ass) - DT(max)	907.3	1134.1	-14.1	-329.3	220

**VERIFICA (tensioni normali) DEL PROFILATO METALLICO**

POSIZ.	$\sigma_{inf}(+)$	$\sigma_{inf}(-)$	$\Delta\sigma$	$\Delta\sigma_c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	15.7	-25.7	41.42	112.0	61.14	VERIFICATO

<- Tab. C.4.2.XV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

**VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI**

POSIZ.	$\tau(+)$	$\tau(-)$	$\Delta\tau$	$\Delta\tau_c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	-1.05	-24.51	23.46	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	-0.89	-20.80	19.91	80.0	27.08	VERIFICATO
sez3	-0.72	-16.96	16.23	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<- Tab. C.4.2.XIib (6)  $\Delta\tau_c=100$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <- Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

## 9 VERIFICHE DEI CONNETTORI "NELSON"

Dati connettori:

numero per fila: 3 (C2, C6); 4 (C1, C3, C4, C5)  
 interasse file: 0.20 m  
 diametro: 22 mm (7/8")  
 altezza: 175 mm  
 resistenza a rottura: 450 N/mm<sup>2</sup>

### 9.1 STATO LIMITE ULTIMO – RESISTENZA AL TAGLIO LONGITUDINALE

Si effettuano le verifiche allo stato limite ultimo per taglio longitudinale dei connettori trave-soletta. I coefficienti parziali per SLU assunti sono i seguenti:

$\gamma_V = 1.25$  : connettori: resistenza allo SLU

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem property: nome delle caratteristiche geometriche dell'elemento  
 Elem: numero dell'elemento  
 Position: nodo iniziale (I) o finale (J) dell'elemento  
 Lcom: combinazione di carico più gravosa  
 Type: sollecitazione (massima o minima)  
 V\_L,Ed: sforzo di taglio longitudinale agente sulla larghezza della regione inelastica  
 v\_L,Ed: sforzo di taglio longitudinale di calcolo per unità di lunghezza (m) all'interfaccia trave-soletta  
 P\_Rd: resistenza al taglio del singolo connettore "Nelson"  
 v\_L,Rd: resistenza al taglio longitudinale per trave e per unità di lunghezza

$$v_{Ed} = \frac{v_{L,Ed}}{2 \times t_c} \quad (t_c = \text{spessore della soletta})$$

Condizione di verifica: Verification Ratio:  $\frac{v_{L,Ed}}{v_{L,Rd}} \leq 1$

Elem property	Elem number	Position	Lcom	Type	V_L,Ed (kN)	v_L,Ed (kN/m)	P_Rd (kN)	v_L,Rd (kN/m)	Verif. Ratio
Concio 1	101	I[719]	ST SLV Vert	FX-MAX	1.81	1.64	111.48	2229.56	0.001
Concio 1	101	J[720]	ST SLU Mobili	MZ-MAX	18.91	17.14	111.48	2229.56	0.008
Concio 1	102	I[720]	ST SLU Mobili	FZ-MIN	-666.59	604.27	111.48	2229.56	0.271
Concio 1	102	J[721]	ST SLU Mobili	FZ-MIN	-548.39	497.12	111.48	2229.56	0.223
Concio 1	103	I[721]	ST SLU Mobili	FZ-MIN	-367.19	332.86	111.48	2229.56	0.149
Concio 1	103	J[722]	ST SLU Mobili	FZ-MIN	-296.27	268.57	111.48	2229.56	0.120
Concio 2	104	I[722]	ST SLU Mobili	FZ-MIN	-296.27	262.49	111.48	1672.17	0.157
Concio 2	104	J[723]	ST SLU Mobili	FZ-MIN	-248.99	220.60	111.48	1672.17	0.132
Concio 2	105	I[723]	ST SLU Mobili	FZ-MAX	164.46	145.71	111.48	1672.17	0.087
Concio 2	105	J[724]	ST SLU Mobili	FZ-MAX	275.03	243.67	111.48	1672.17	0.146
Concio 2	106	I[724]	ST SLU Mobili	FZ-MAX	439.74	389.60	111.48	1672.17	0.233
Concio 2	106	J[725]	ST SLU Mobili	FZ-MAX	510.66	452.43	111.48	1672.17	0.271
Concio 3	107	I[725]	ST SLU Mobili	FZ-MAX	510.66	457.60	111.48	2229.56	0.205
Concio 3	107	J[726]	ST SLU Mobili	FZ-MAX	557.94	499.96	111.48	2229.56	0.224
Concio 3	108	I[726]	ST SLU Mobili	FZ-MAX	714.62	640.36	111.48	2229.56	0.287
Concio 3	108	J[727]	ST SLU Mobili	FZ-MAX	832.82	746.28	111.48	2229.56	0.335
Concio 3	109	I[727]	ST SLU Mobili	FZ-MAX	939.63	841.99	111.48	2229.56	0.378
Concio 3	109	J[728]	ST SLU Mobili	FZ-MAX	951.45	852.58	111.48	2229.56	0.382
Concio 4 H=var	110	I[728]	ST SLU Mobili	FZ-MAX	957.18	837.37	111.48	2229.56	0.376
Concio 4 H=var	110	J[729]	ST SLU Mobili	FZ-MAX	1021.05	615.85	111.48	2229.56	0.276
Concio 4 H=var	111	I[729]	ST SLU Mobili	FZ-MAX	1103.38	665.51	111.48	2229.56	0.298

Concio 4 H=var	111	J[730]	ST SLU Mobili	FZ-MAX	1167.22	535.50	111.48	2229.56	0.240
Concio 4 H=200	112	I[730]	ST SLU Mobili	FZ-MAX	1164.15	534.09	111.48	2229.56	0.240
Concio 4 H=200	112	J[731]	ST SLU Mobili	FZ-MAX	1176.91	539.95	111.48	2229.56	0.242
Concio 4 H=200	113	I[731]	ST SLU Mobili	FZ-MIN	-1093.20	501.54	111.48	2229.56	0.225
Concio 4 H=200	113	J[732]	ST SLU Mobili	FZ-MIN	-1080.44	495.68	111.48	2229.56	0.222
Concio 4 H=var	114	I[732]	ST SLU Mobili	FZ-MIN	-1085.49	498.00	111.48	2229.56	0.223
Concio 4 H=var	114	J[733]	ST SLU Mobili	FZ-MIN	-1021.64	616.21	111.48	2229.56	0.276
Concio 4 H=var	115	I[733]	ST SLU Mobili	FZ-MIN	-937.04	565.18	111.48	2229.56	0.253
Concio 4 H=var	115	J[734]	ST SLU Mobili	FZ-MIN	-873.17	763.88	111.48	2229.56	0.343
Concio 5	116	I[734]	ST SLU Mobili	FZ-MIN	-864.75	774.88	111.48	2229.56	0.348
Concio 5	116	J[735]	ST SLU Mobili	FZ-MIN	-852.93	764.29	111.48	2229.56	0.343
Concio 5	117	I[735]	ST SLU Mobili	FZ-MIN	-745.96	668.44	111.48	2229.56	0.300
Concio 5	117	J[736]	ST SLU Mobili	FZ-MIN	-623.03	558.29	111.48	2229.56	0.250
Concio 5	118	I[736]	ST SLU Mobili	FZ-MIN	-466.74	418.24	111.48	2229.56	0.188
Concio 5	118	J[737]	ST SLU Mobili	FZ-MIN	-412.37	369.52	111.48	2229.56	0.166
Concio 6	119	I[737]	ST SLU Mobili	FZ-MIN	-412.37	362.45	111.48	1672.17	0.217
Concio 6	119	J[738]	ST SLU Mobili	FZ-MIN	-343.81	302.20	111.48	1672.17	0.181
Concio 6	120	I[738]	ST SLU Mobili	FZ-MIN	-180.06	158.26	111.48	1672.17	0.095
Concio 6	120	J[739]	ST SLU Mobili	FZ-MIN	-118.59	104.24	111.48	1672.17	0.062
Concio 6	121	I[739]	ST SLU Mobili	FZ-MIN	-118.59	104.24	111.48	1672.17	0.062
Concio 6	121	J[740]	ST SLU Mobili	FZ-MAX	180.04	158.25	111.48	1672.17	0.095
Concio 6	122	I[740]	ST SLU Mobili	FZ-MAX	343.85	302.23	111.48	1672.17	0.181
Concio 6	122	J[741]	ST SLU Mobili	FZ-MAX	412.41	362.49	111.48	1672.17	0.217
Concio 5	123	I[741]	ST SLU Mobili	FZ-MAX	412.41	369.55	111.48	2229.56	0.166
Concio 5	123	J[742]	ST SLU Mobili	FZ-MAX	466.78	418.27	111.48	2229.56	0.188
Concio 5	124	I[742]	ST SLU Mobili	FZ-MAX	623.11	558.36	111.48	2229.56	0.250
Concio 5	124	J[743]	ST SLU Mobili	FZ-MAX	746.04	668.51	111.48	2229.56	0.300
Concio 5	125	I[743]	ST SLU Mobili	FZ-MAX	852.93	764.29	111.48	2229.56	0.343
Concio 5	125	J[744]	ST SLU Mobili	FZ-MAX	864.75	774.88	111.48	2229.56	0.348
Concio 4 H=var	126	I[744]	ST SLU Mobili	FZ-MAX	873.18	763.89	111.48	2229.56	0.343
Concio 4 H=var	126	J[745]	ST SLU Mobili	FZ-MAX	937.05	565.19	111.48	2229.56	0.253
Concio 4 H=var	127	I[745]	ST SLU Mobili	FZ-MAX	1021.40	616.07	111.48	2229.56	0.276
Concio 4 H=var	127	J[746]	ST SLU Mobili	FZ-MAX	1085.25	497.89	111.48	2229.56	0.223
Concio 4 H=200	128	I[746]	ST SLU Mobili	FZ-MAX	1080.19	495.57	111.48	2229.56	0.222
Concio 4 H=200	128	J[747]	ST SLU Mobili	FZ-MAX	1092.95	501.43	111.48	2229.56	0.225
Concio 4 H=200	129	I[747]	ST SLU Mobili	FZ-MIN	-1177.21	540.08	111.48	2229.56	0.242
Concio 4 H=200	129	J[748]	ST SLU Mobili	FZ-MIN	-1164.44	534.22	111.48	2229.56	0.240
Concio 4 H=var	130	I[748]	ST SLU Mobili	FZ-MIN	-1167.50	535.63	111.48	2229.56	0.240
Concio 4 H=var	130	J[749]	ST SLU Mobili	FZ-MIN	-1103.66	665.68	111.48	2229.56	0.299
Concio 4 H=var	131	I[749]	ST SLU Mobili	FZ-MIN	-1021.07	615.87	111.48	2229.56	0.276
Concio 4 H=var	131	J[750]	ST SLU Mobili	FZ-MIN	-957.21	837.39	111.48	2229.56	0.376
Concio 3	132	I[750]	ST SLU Mobili	FZ-MIN	-951.49	852.61	111.48	2229.56	0.382
Concio 3	132	J[751]	ST SLU Mobili	FZ-MIN	-939.67	842.02	111.48	2229.56	0.378
Concio 3	133	I[751]	ST SLU Mobili	FZ-MIN	-832.78	746.24	111.48	2229.56	0.335
Concio 3	133	J[752]	ST SLU Mobili	FZ-MIN	-714.58	640.32	111.48	2229.56	0.287
Concio 3	134	I[752]	ST SLU Mobili	FZ-MIN	-557.93	499.95	111.48	2229.56	0.224
Concio 3	134	J[753]	ST SLU Mobili	FZ-MIN	-510.65	457.58	111.48	2229.56	0.205
Concio 2	135	I[753]	ST SLU Mobili	FZ-MIN	-510.65	452.42	111.48	1672.17	0.271
Concio 2	135	J[754]	ST SLU Mobili	FZ-MIN	-439.73	389.59	111.48	1672.17	0.233
Concio 2	136	I[754]	ST SLU Mobili	FZ-MIN	-275.02	243.66	111.48	1672.17	0.146
Concio 2	136	J[755]	ST SLU Mobili	FZ-MIN	-164.43	145.68	111.48	1672.17	0.087
Concio 2	137	I[755]	ST SLU Mobili	FZ-MAX	248.99	220.60	111.48	1672.17	0.132
Concio 2	137	J[756]	ST SLU Mobili	FZ-MAX	296.27	262.49	111.48	1672.17	0.157
Concio 1	138	I[756]	ST SLU Mobili	FZ-MAX	296.27	268.57	111.48	2229.56	0.120
Concio 1	138	J[757]	ST SLU Mobili	FZ-MAX	367.19	332.86	111.48	2229.56	0.149
Concio 1	139	I[757]	ST SLU Mobili	FZ-MAX	548.40	497.13	111.48	2229.56	0.223
Concio 1	139	J[758]	ST SLU Mobili	FZ-MAX	666.60	604.28	111.48	2229.56	0.271
Concio 1	140	I[758]	ST SLU Mobili	FZ-MIN	-18.91	17.14	111.48	2229.56	0.008
Concio 1	140	J[759]	ST SLV Vert	FX-MAX	1.81	1.64	111.48	2229.56	0.001

Concio 1	201	I[760]	ST SLV Vert	FX-MAX	1.91	1.74	111.48	2229.56	0.001
Concio 1	201	J[761]	ST SLU Mobili	FZ-MAX	450.51	410.01	111.48	2229.56	0.184
Concio 1	202	I[761]	ST SLU Mobili	FZ-MIN	-1250.44	1138.02	111.48	2229.56	0.510
Concio 1	202	J[762]	ST SLU Mobili	FZ-MIN	-588.73	535.80	111.48	2229.56	0.240
Concio 1	203	I[762]	ST SLU Mobili	FZ-MIN	-866.47	788.57	111.48	2229.56	0.354
Concio 1	203	J[763]	ST SLU Mobili	FZ-MIN	-515.39	469.05	111.48	2229.56	0.210
Concio 2	204	I[763]	ST SLU Mobili	FZ-MIN	-515.39	458.57	111.48	1672.17	0.274
Concio 2	204	J[764]	ST SLU Mobili	FZ-MAX	604.09	537.49	111.48	1672.17	0.321
Concio 2	205	I[764]	ST SLU Mobili	FZ-MIN	-635.85	565.75	111.48	1672.17	0.338
Concio 2	205	J[765]	ST SLU Mobili	FZ-MAX	817.23	727.13	111.48	1672.17	0.435
Concio 2	206	I[765]	ST SLU Mobili	FZ-MAX	494.50	439.99	111.48	1672.17	0.263
Concio 2	206	J[766]	ST SLU Mobili	FZ-MAX	820.76	730.27	111.48	1672.17	0.437
Concio 3	207	I[766]	ST SLU Mobili	FZ-MAX	820.76	739.00	111.48	2229.56	0.331
Concio 3	207	J[767]	ST SLU Mobili	FZ-MAX	1047.46	943.12	111.48	2229.56	0.423
Concio 3	208	I[767]	ST SLU Mobili	FZ-MAX	771.66	694.79	111.48	2229.56	0.312
Concio 3	208	J[768]	ST SLU Mobili	FZ-MAX	1329.46	1197.02	111.48	2229.56	0.537
Concio 3	209	I[768]	ST SLU Mobili	FZ-MAX	1121.19	1009.50	111.48	2229.56	0.453
Concio 3	209	J[769]	ST SLU Mobili	FZ-MAX	1173.52	1056.62	111.48	2229.56	0.474
Concio 4 H=var	210	I[769]	ST SLU Mobili	FZ-MAX	1165.17	1026.93	111.48	2229.56	0.461
Concio 4 H=var	210	J[770]	ST SLU Mobili	FZ-MAX	1479.69	901.67	111.48	2229.56	0.404
Concio 4 H=var	211	I[770]	ST SLU Mobili	FZ-MAX	1361.72	829.79	111.48	2229.56	0.372
Concio 4 H=var	211	J[771]	ST SLU Mobili	FZ-MAX	1682.58	781.42	111.48	2229.56	0.350
Concio 4 H=200	212	I[771]	ST SLU Mobili	FZ-MAX	1681.05	780.71	111.48	2229.56	0.350
Concio 4 H=200	212	J[772]	ST SLU Mobili	FZ-MAX	1742.45	809.22	111.48	2229.56	0.363
Concio 4 H=200	213	I[772]	ST SLU Mobili	FZ-MIN	-1656.42	769.27	111.48	2229.56	0.345
Concio 4 H=200	213	J[773]	ST SLU Mobili	FZ-MIN	-1595.07	740.78	111.48	2229.56	0.332
Concio 4 H=var	214	I[773]	ST SLU Mobili	FZ-MIN	-1596.34	741.37	111.48	2229.56	0.333
Concio 4 H=var	214	J[774]	ST SLU Mobili	FZ-MIN	-1275.50	777.25	111.48	2229.56	0.349
Concio 4 H=var	215	I[774]	ST SLU Mobili	FZ-MIN	-1395.43	850.33	111.48	2229.56	0.381
Concio 4 H=var	215	J[775]	ST SLU Mobili	FZ-MIN	-1081.84	953.49	111.48	2229.56	0.428
Concio 5	216	I[775]	ST SLU Mobili	FZ-MIN	-1090.53	981.89	111.48	2229.56	0.440
Concio 5	216	J[776]	ST SLU Mobili	FZ-MIN	-1038.36	934.92	111.48	2229.56	0.419
Concio 5	217	I[776]	ST SLU Mobili	FZ-MIN	-1253.34	1128.49	111.48	2229.56	0.506
Concio 5	217	J[777]	ST SLU Mobili	FZ-MIN	-675.97	608.63	111.48	2229.56	0.273
Concio 5	218	I[777]	ST SLU Mobili	FZ-MIN	-963.39	867.42	111.48	2229.56	0.389
Concio 5	218	J[778]	ST SLU Mobili	FZ-MIN	-703.40	633.33	111.48	2229.56	0.284
Concio 6	219	I[778]	ST SLU Mobili	FZ-MIN	-703.40	620.69	111.48	1672.17	0.371
Concio 6	219	J[779]	ST SLU Mobili	FZ-MAX	511.85	451.67	111.48	1672.17	0.270
Concio 6	220	I[779]	ST SLU Mobili	FZ-MIN	-727.50	641.96	111.48	1672.17	0.384
Concio 6	220	J[780]	ST SLU Mobili	FZ-MIN	-431.71	380.95	111.48	1672.17	0.228
Concio 6	221	I[780]	ST SLU Mobili	FZ-MIN	-431.71	380.95	111.48	1672.17	0.228
Concio 6	221	J[781]	ST SLU Mobili	FZ-MAX	727.32	641.80	111.48	1672.17	0.384
Concio 6	222	I[781]	ST SLU Mobili	FZ-MIN	-512.05	451.84	111.48	1672.17	0.270
Concio 6	222	J[782]	ST SLU Mobili	FZ-MAX	703.21	620.53	111.48	1672.17	0.371
Concio 5	223	I[782]	ST SLU Mobili	FZ-MAX	703.21	633.16	111.48	2229.56	0.284
Concio 5	223	J[783]	ST SLU Mobili	FZ-MAX	963.23	867.28	111.48	2229.56	0.389
Concio 5	224	I[783]	ST SLU Mobili	FZ-MAX	675.85	608.52	111.48	2229.56	0.273
Concio 5	224	J[784]	ST SLU Mobili	FZ-MAX	1253.10	1128.27	111.48	2229.56	0.506
Concio 5	225	I[784]	ST SLU Mobili	FZ-MAX	1038.12	934.70	111.48	2229.56	0.419
Concio 5	225	J[785]	ST SLU Mobili	FZ-MAX	1090.28	981.67	111.48	2229.56	0.440
Concio 4 H=var	226	I[785]	ST SLU Mobili	FZ-MAX	1081.61	953.28	111.48	2229.56	0.428
Concio 4 H=var	226	J[786]	ST SLU Mobili	FZ-MAX	1395.25	850.22	111.48	2229.56	0.381
Concio 4 H=var	227	I[786]	ST SLU Mobili	FZ-MAX	1275.20	777.07	111.48	2229.56	0.349
Concio 4 H=var	227	J[787]	ST SLU Mobili	FZ-MAX	1596.03	741.22	111.48	2229.56	0.332
Concio 4 H=200	228	I[787]	ST SLU Mobili	FZ-MAX	1594.74	740.63	111.48	2229.56	0.332
Concio 4 H=200	228	J[788]	ST SLU Mobili	FZ-MAX	1656.09	769.12	111.48	2229.56	0.345
Concio 4 H=200	229	I[788]	ST SLU Mobili	FZ-MIN	-1742.77	809.37	111.48	2229.56	0.363
Concio 4 H=200	229	J[789]	ST SLU Mobili	FZ-MIN	-1681.38	780.86	111.48	2229.56	0.350
Concio 4 H=var	230	I[789]	ST SLU Mobili	FZ-MIN	-1682.90	781.56	111.48	2229.56	0.351



Concio 4 H=var	230	J[790]	ST SLU Mobili	FZ-MIN	-1362.02	829.97	111.48	2229.56	0.372
Concio 4 H=var	231	I[790]	ST SLU Mobili	FZ-MIN	-1479.88	901.79	111.48	2229.56	0.404
Concio 4 H=var	231	J[791]	ST SLU Mobili	FZ-MIN	-1165.42	1027.15	111.48	2229.56	0.461
Concio 3	232	I[791]	ST SLU Mobili	FZ-MIN	-1173.78	1056.85	111.48	2229.56	0.474
Concio 3	232	J[792]	ST SLU Mobili	FZ-MIN	-1121.44	1009.73	111.48	2229.56	0.453
Concio 3	233	I[792]	ST SLU Mobili	FZ-MIN	-1329.70	1197.24	111.48	2229.56	0.537
Concio 3	233	J[793]	ST SLU Mobili	FZ-MIN	-771.79	694.91	111.48	2229.56	0.312
Concio 3	234	I[793]	ST SLU Mobili	FZ-MIN	-1047.65	943.29	111.48	2229.56	0.423
Concio 3	234	J[794]	ST SLU Mobili	FZ-MIN	-820.99	739.20	111.48	2229.56	0.332
Concio 2	235	I[794]	ST SLU Mobili	FZ-MIN	-820.99	730.48	111.48	1672.17	0.437
Concio 2	235	J[795]	ST SLU Mobili	FZ-MIN	-494.74	440.19	111.48	1672.17	0.263
Concio 2	236	I[795]	ST SLU Mobili	FZ-MIN	-817.52	727.39	111.48	1672.17	0.435
Concio 2	236	J[796]	ST SLU Mobili	FZ-MAX	635.56	565.49	111.48	1672.17	0.338
Concio 2	237	I[796]	ST SLU Mobili	FZ-MIN	-604.32	537.70	111.48	1672.17	0.322
Concio 2	237	J[797]	ST SLU Mobili	FZ-MAX	515.15	458.36	111.48	1672.17	0.274
Concio 1	238	I[797]	ST SLU Mobili	FZ-MAX	515.15	468.84	111.48	2229.56	0.210
Concio 1	238	J[798]	ST SLU Mobili	FZ-MAX	866.20	788.32	111.48	2229.56	0.354
Concio 1	239	I[798]	ST SLU Mobili	MY-MAX	588.51	535.60	111.48	2229.56	0.240
Concio 1	239	J[799]	ST SLU Mobili	FZ-MAX	1250.06	1137.67	111.48	2229.56	0.510
Concio 1	240	I[799]	ST SLU Mobili	FZ-MIN	-450.51	410.01	111.48	2229.56	0.184
Concio 1	240	J[800]	ST SLV Vert	FX-MAX	1.91	1.74	111.48	2229.56	0.001
Concio 1	301	I[801]	ST SLV Vert	FX-MAX	1.91	1.74	111.48	2229.56	0.001
Concio 1	301	J[802]	ST SLU Mobili	FZ-MAX	450.51	410.01	111.48	2229.56	0.184
Concio 1	302	I[802]	ST SLU Mobili	FZ-MIN	-1251.38	1138.88	111.48	2229.56	0.511
Concio 1	302	J[803]	ST SLU Mobili	FZ-MIN	-589.67	536.65	111.48	2229.56	0.241
Concio 1	303	I[803]	ST SLU Mobili	FZ-MIN	-867.02	789.07	111.48	2229.56	0.354
Concio 1	303	J[804]	ST SLU Mobili	FZ-MIN	-515.94	469.55	111.48	2229.56	0.211
Concio 2	304	I[804]	ST SLU Mobili	FZ-MIN	-515.94	459.06	111.48	1672.17	0.275
Concio 2	304	J[805]	ST SLU Mobili	FZ-MAX	604.61	537.95	111.48	1672.17	0.322
Concio 2	305	I[805]	ST SLU Mobili	FZ-MIN	-635.94	565.83	111.48	1672.17	0.338
Concio 2	305	J[806]	ST SLU Mobili	FZ-MAX	817.32	727.22	111.48	1672.17	0.435
Concio 2	306	I[806]	ST SLU Mobili	FZ-MAX	495.26	440.66	111.48	1672.17	0.264
Concio 2	306	J[807]	ST SLU Mobili	FZ-MAX	821.51	730.95	111.48	1672.17	0.437
Concio 3	307	I[807]	ST SLU Mobili	FZ-MAX	821.51	739.68	111.48	2229.56	0.332
Concio 3	307	J[808]	ST SLU Mobili	FZ-MAX	1048.22	943.80	111.48	2229.56	0.423
Concio 3	308	I[808]	ST SLU Mobili	FZ-MAX	772.83	695.85	111.48	2229.56	0.312
Concio 3	308	J[809]	ST SLU Mobili	FZ-MAX	1330.63	1198.08	111.48	2229.56	0.537
Concio 3	309	I[809]	ST SLU Mobili	FZ-MAX	1122.58	1010.76	111.48	2229.56	0.453
Concio 3	309	J[810]	ST SLU Mobili	FZ-MAX	1174.91	1057.87	111.48	2229.56	0.474
Concio 4 H=var	310	I[810]	ST SLU Mobili	FZ-MAX	1167.52	1029.00	111.48	2229.56	0.462
Concio 4 H=var	310	J[811]	ST SLU Mobili	FZ-MAX	1482.04	903.11	111.48	2229.56	0.405
Concio 4 H=var	311	I[811]	ST SLU Mobili	FZ-MAX	1364.66	831.58	111.48	2229.56	0.373
Concio 4 H=var	311	J[812]	ST SLU Mobili	FZ-MAX	1685.52	782.78	111.48	2229.56	0.351
Concio 4 H=200	312	I[812]	ST SLU Mobili	FZ-MAX	1682.93	781.58	111.48	2229.56	0.351
Concio 4 H=200	312	J[813]	ST SLU Mobili	FZ-MAX	1744.33	810.09	111.48	2229.56	0.363
Concio 4 H=200	313	I[813]	ST SLU Mobili	FZ-MIN	-1658.28	770.13	111.48	2229.56	0.345
Concio 4 H=200	313	J[814]	ST SLU Mobili	FZ-MIN	-1596.94	741.64	111.48	2229.56	0.333
Concio 4 H=var	314	I[814]	ST SLU Mobili	FZ-MIN	-1599.32	742.75	111.48	2229.56	0.333
Concio 4 H=var	314	J[815]	ST SLU Mobili	FZ-MIN	-1278.48	779.06	111.48	2229.56	0.349
Concio 4 H=var	315	I[815]	ST SLU Mobili	FZ-MIN	-1397.92	851.85	111.48	2229.56	0.382
Concio 4 H=var	315	J[816]	ST SLU Mobili	FZ-MIN	-1084.34	955.69	111.48	2229.56	0.429
Concio 5	316	I[816]	ST SLU Mobili	FZ-MIN	-1091.92	983.15	111.48	2229.56	0.441
Concio 5	316	J[817]	ST SLU Mobili	FZ-MIN	-1039.75	936.17	111.48	2229.56	0.420
Concio 5	317	I[817]	ST SLU Mobili	FZ-MIN	-1254.54	1129.57	111.48	2229.56	0.507
Concio 5	317	J[818]	ST SLU Mobili	FZ-MIN	-677.17	609.71	111.48	2229.56	0.273
Concio 5	318	I[818]	ST SLU Mobili	FZ-MIN	-964.06	868.02	111.48	2229.56	0.389
Concio 5	318	J[819]	ST SLU Mobili	FZ-MIN	-704.06	633.93	111.48	2229.56	0.284
Concio 6	319	I[819]	ST SLU Mobili	FZ-MIN	-704.06	621.28	111.48	1672.17	0.372
Concio 6	319	J[820]	ST SLU Mobili	FZ-MAX	512.52	452.25	111.48	1672.17	0.270



Concio 6	320	I[820]	ST SLU Mobili	FZ-MIN	-727.50	641.96	111.48	1672.17	0.384
Concio 6	320	J[821]	ST SLU Mobili	FZ-MIN	-431.71	380.95	111.48	1672.17	0.228
Concio 6	321	I[821]	ST SLU Mobili	FZ-MIN	-431.71	380.95	111.48	1672.17	0.228
Concio 6	321	J[822]	ST SLU Mobili	FZ-MAX	727.32	641.80	111.48	1672.17	0.384
Concio 6	322	I[822]	ST SLU Mobili	FZ-MIN	-512.73	452.44	111.48	1672.17	0.271
Concio 6	322	J[823]	ST SLU Mobili	FZ-MAX	703.89	621.13	111.48	1672.17	0.371
Concio 5	323	I[823]	ST SLU Mobili	FZ-MAX	703.89	633.77	111.48	2229.56	0.284
Concio 5	323	J[824]	ST SLU Mobili	FZ-MAX	963.91	867.89	111.48	2229.56	0.389
Concio 5	324	I[824]	ST SLU Mobili	FZ-MAX	677.03	609.58	111.48	2229.56	0.273
Concio 5	324	J[825]	ST SLU Mobili	FZ-MAX	1254.28	1129.33	111.48	2229.56	0.507
Concio 5	325	I[825]	ST SLU Mobili	FZ-MAX	1039.49	935.94	111.48	2229.56	0.420
Concio 5	325	J[826]	ST SLU Mobili	FZ-MAX	1091.66	982.91	111.48	2229.56	0.441
Concio 4 H=var	326	I[826]	ST SLU Mobili	FZ-MAX	1084.08	955.46	111.48	2229.56	0.429
Concio 4 H=var	326	J[827]	ST SLU Mobili	FZ-MAX	1397.72	851.72	111.48	2229.56	0.382
Concio 4 H=var	327	I[827]	ST SLU Mobili	FZ-MAX	1278.26	778.93	111.48	2229.56	0.349
Concio 4 H=var	327	J[828]	ST SLU Mobili	FZ-MAX	1599.08	742.64	111.48	2229.56	0.333
Concio 4 H=200	328	I[828]	ST SLU Mobili	FZ-MAX	1596.70	741.53	111.48	2229.56	0.333
Concio 4 H=200	328	J[829]	ST SLU Mobili	FZ-MAX	1658.04	770.02	111.48	2229.56	0.345
Concio 4 H=200	329	I[829]	ST SLU Mobili	FZ-MIN	-1744.59	810.21	111.48	2229.56	0.363
Concio 4 H=200	329	J[830]	ST SLU Mobili	FZ-MIN	-1683.20	781.70	111.48	2229.56	0.351
Concio 4 H=var	330	I[830]	ST SLU Mobili	FZ-MIN	-1685.78	782.90	111.48	2229.56	0.351
Concio 4 H=var	330	J[831]	ST SLU Mobili	FZ-MIN	-1364.90	831.72	111.48	2229.56	0.373
Concio 4 H=var	331	I[831]	ST SLU Mobili	FZ-MIN	-1482.27	903.25	111.48	2229.56	0.405
Concio 4 H=var	331	J[832]	ST SLU Mobili	FZ-MIN	-1167.81	1029.25	111.48	2229.56	0.462
Concio 3	332	I[832]	ST SLU Mobili	FZ-MIN	-1175.20	1058.13	111.48	2229.56	0.475
Concio 3	332	J[833]	ST SLU Mobili	FZ-MIN	-1122.86	1011.01	111.48	2229.56	0.453
Concio 3	333	I[833]	ST SLU Mobili	FZ-MIN	-1330.90	1198.32	111.48	2229.56	0.537
Concio 3	333	J[834]	ST SLU Mobili	FZ-MIN	-772.99	695.99	111.48	2229.56	0.312
Concio 3	334	I[834]	ST SLU Mobili	FZ-MIN	-1048.39	943.96	111.48	2229.56	0.423
Concio 3	334	J[835]	ST SLU Mobili	FZ-MIN	-821.73	739.87	111.48	2229.56	0.332
Concio 2	335	I[835]	ST SLU Mobili	FZ-MIN	-821.73	731.13	111.48	1672.17	0.437
Concio 2	335	J[836]	ST SLU Mobili	FZ-MIN	-495.48	440.85	111.48	1672.17	0.264
Concio 2	336	I[836]	ST SLU Mobili	FZ-MIN	-817.60	727.47	111.48	1672.17	0.435
Concio 2	336	J[837]	ST SLU Mobili	FZ-MAX	635.65	565.57	111.48	1672.17	0.338
Concio 2	337	I[837]	ST SLU Mobili	FZ-MIN	-604.87	538.19	111.48	1672.17	0.322
Concio 2	337	J[838]	ST SLU Mobili	FZ-MAX	515.70	458.85	111.48	1672.17	0.274
Concio 1	338	I[838]	ST SLU Mobili	FZ-MAX	515.70	469.34	111.48	2229.56	0.211
Concio 1	338	J[839]	ST SLU Mobili	FZ-MAX	866.74	788.82	111.48	2229.56	0.354
Concio 1	339	I[839]	ST SLU Mobili	MY-MAX	589.45	536.46	111.48	2229.56	0.241
Concio 1	339	J[840]	ST SLU Mobili	FZ-MAX	1251.00	1138.53	111.48	2229.56	0.511
Concio 1	340	I[840]	ST SLU Mobili	FZ-MIN	-450.51	410.01	111.48	2229.56	0.184
Concio 1	340	J[841]	ST SLV Vert	FX-MAX	1.91	1.74	111.48	2229.56	0.001
Concio 1	401	I[842]	ST SLV Vert	FX-MAX	1.81	1.64	111.48	2229.56	0.001
Concio 1	401	J[843]	ST SLU Mobili	MZ-MIN	18.91	17.14	111.48	2229.56	0.008
Concio 1	402	I[843]	ST SLU Mobili	FZ-MIN	-664.30	602.20	111.48	2229.56	0.270
Concio 1	402	J[844]	ST SLU Mobili	FZ-MIN	-546.10	495.05	111.48	2229.56	0.222
Concio 1	403	I[844]	ST SLU Mobili	FZ-MIN	-366.77	332.48	111.48	2229.56	0.149
Concio 1	403	J[845]	ST SLU Mobili	FZ-MIN	-295.85	268.20	111.48	2229.56	0.120
Concio 2	404	I[845]	ST SLU Mobili	FZ-MIN	-295.85	262.12	111.48	1672.17	0.157
Concio 2	404	J[846]	ST SLU Mobili	FZ-MIN	-248.57	220.23	111.48	1672.17	0.132
Concio 2	405	I[846]	ST SLU Mobili	FZ-MAX	164.12	145.40	111.48	1672.17	0.087
Concio 2	405	J[847]	ST SLU Mobili	FZ-MAX	274.69	243.37	111.48	1672.17	0.146
Concio 2	406	I[847]	ST SLU Mobili	FZ-MAX	438.85	388.81	111.48	1672.17	0.233
Concio 2	406	J[848]	ST SLU Mobili	FZ-MAX	509.77	451.64	111.48	1672.17	0.270
Concio 3	407	I[848]	ST SLU Mobili	FZ-MAX	509.77	456.80	111.48	2229.56	0.205
Concio 3	407	J[849]	ST SLU Mobili	FZ-MAX	557.05	499.17	111.48	2229.56	0.224
Concio 3	408	I[849]	ST SLU Mobili	FZ-MAX	711.83	637.86	111.48	2229.56	0.286
Concio 3	408	J[850]	ST SLU Mobili	FZ-MAX	830.03	743.78	111.48	2229.56	0.334
Concio 3	409	I[850]	ST SLU Mobili	FZ-MAX	936.16	838.88	111.48	2229.56	0.376

Concio 3	409	J[851]	ST SLU Mobili	FZ-MAX	947.98	849.47	111.48	2229.56	0.381
Concio 4 H=var	410	I[851]	ST SLU Mobili	FZ-MAX	954.28	834.83	111.48	2229.56	0.374
Concio 4 H=var	410	J[852]	ST SLU Mobili	FZ-MAX	1018.15	614.10	111.48	2229.56	0.275
Concio 4 H=var	411	I[852]	ST SLU Mobili	FZ-MAX	1100.21	663.60	111.48	2229.56	0.298
Concio 4 H=var	411	J[853]	ST SLU Mobili	FZ-MAX	1164.05	534.04	111.48	2229.56	0.240
Concio 4 H=200	412	I[853]	ST SLU Mobili	FZ-MAX	1160.55	532.44	111.48	2229.56	0.239
Concio 4 H=200	412	J[854]	ST SLU Mobili	FZ-MAX	1173.32	538.30	111.48	2229.56	0.241
Concio 4 H=200	413	I[854]	ST SLU Mobili	FZ-MIN	-1089.52	499.85	111.48	2229.56	0.224
Concio 4 H=200	413	J[855]	ST SLU Mobili	FZ-MIN	-1076.75	493.99	111.48	2229.56	0.222
Concio 4 H=var	414	I[855]	ST SLU Mobili	FZ-MIN	-1082.25	496.51	111.48	2229.56	0.223
Concio 4 H=var	414	J[856]	ST SLU Mobili	FZ-MIN	-1018.40	614.26	111.48	2229.56	0.276
Concio 4 H=var	415	I[856]	ST SLU Mobili	FZ-MIN	-934.23	563.49	111.48	2229.56	0.253
Concio 4 H=var	415	J[857]	ST SLU Mobili	FZ-MIN	-870.36	761.42	111.48	2229.56	0.342
Concio 5	416	I[857]	ST SLU Mobili	FZ-MIN	-861.33	771.82	111.48	2229.56	0.346
Concio 5	416	J[858]	ST SLU Mobili	FZ-MIN	-849.51	761.23	111.48	2229.56	0.341
Concio 5	417	I[858]	ST SLU Mobili	FZ-MIN	-743.31	666.07	111.48	2229.56	0.299
Concio 5	417	J[859]	ST SLU Mobili	FZ-MIN	-620.38	555.91	111.48	2229.56	0.249
Concio 5	418	I[859]	ST SLU Mobili	FZ-MIN	-466.07	417.64	111.48	2229.56	0.187
Concio 5	418	J[860]	ST SLU Mobili	FZ-MIN	-411.70	368.92	111.48	2229.56	0.165
Concio 6	419	I[860]	ST SLU Mobili	FZ-MIN	-411.70	361.87	111.48	1672.17	0.216
Concio 6	419	J[861]	ST SLU Mobili	FZ-MIN	-343.14	301.61	111.48	1672.17	0.180
Concio 6	420	I[861]	ST SLU Mobili	FZ-MIN	-180.05	158.26	111.48	1672.17	0.095
Concio 6	420	J[862]	ST SLU Mobili	FZ-MIN	-118.59	104.24	111.48	1672.17	0.062
Concio 6	421	I[862]	ST SLU Mobili	FZ-MIN	-118.59	104.24	111.48	1672.17	0.062
Concio 6	421	J[863]	ST SLU Mobili	FZ-MAX	180.04	158.25	111.48	1672.17	0.095
Concio 6	422	I[863]	ST SLU Mobili	FZ-MAX	343.17	301.63	111.48	1672.17	0.180
Concio 6	422	J[864]	ST SLU Mobili	FZ-MAX	411.73	361.89	111.48	1672.17	0.216
Concio 5	423	I[864]	ST SLU Mobili	FZ-MAX	411.73	368.94	111.48	2229.56	0.165
Concio 5	423	J[865]	ST SLU Mobili	FZ-MAX	466.10	417.66	111.48	2229.56	0.187
Concio 5	424	I[865]	ST SLU Mobili	FZ-MAX	620.42	555.94	111.48	2229.56	0.249
Concio 5	424	J[866]	ST SLU Mobili	FZ-MAX	743.34	666.10	111.48	2229.56	0.299
Concio 5	425	I[866]	ST SLU Mobili	FZ-MAX	849.50	761.23	111.48	2229.56	0.341
Concio 5	425	J[867]	ST SLU Mobili	FZ-MAX	861.32	771.82	111.48	2229.56	0.346
Concio 4 H=var	426	I[867]	ST SLU Mobili	FZ-MAX	870.34	761.40	111.48	2229.56	0.342
Concio 4 H=var	426	J[868]	ST SLU Mobili	FZ-MAX	934.21	563.47	111.48	2229.56	0.253
Concio 4 H=var	427	I[868]	ST SLU Mobili	FZ-MAX	1018.30	614.19	111.48	2229.56	0.275
Concio 4 H=var	427	J[869]	ST SLU Mobili	FZ-MAX	1082.14	496.47	111.48	2229.56	0.223
Concio 4 H=200	428	I[869]	ST SLU Mobili	FZ-MAX	1076.65	493.95	111.48	2229.56	0.222
Concio 4 H=200	428	J[870]	ST SLU Mobili	FZ-MAX	1089.42	499.81	111.48	2229.56	0.224
Concio 4 H=200	429	I[870]	ST SLU Mobili	FZ-MIN	-1173.46	538.36	111.48	2229.56	0.241
Concio 4 H=200	429	J[871]	ST SLU Mobili	FZ-MIN	-1160.69	532.50	111.48	2229.56	0.239
Concio 4 H=var	430	I[871]	ST SLU Mobili	FZ-MIN	-1164.19	534.11	111.48	2229.56	0.240
Concio 4 H=var	430	J[872]	ST SLU Mobili	FZ-MIN	-1100.35	663.69	111.48	2229.56	0.298
Concio 4 H=var	431	I[872]	ST SLU Mobili	FZ-MIN	-1018.21	614.14	111.48	2229.56	0.275
Concio 4 H=var	431	J[873]	ST SLU Mobili	FZ-MIN	-954.34	834.89	111.48	2229.56	0.374
Concio 3	432	I[873]	ST SLU Mobili	FZ-MIN	-948.03	849.51	111.48	2229.56	0.381
Concio 3	432	J[874]	ST SLU Mobili	FZ-MIN	-936.21	838.92	111.48	2229.56	0.376
Concio 3	433	I[874]	ST SLU Mobili	FZ-MIN	-830.04	743.79	111.48	2229.56	0.334
Concio 3	433	J[875]	ST SLU Mobili	FZ-MIN	-711.84	637.87	111.48	2229.56	0.286
Concio 3	434	I[875]	ST SLU Mobili	FZ-MIN	-557.06	499.17	111.48	2229.56	0.224
Concio 3	434	J[876]	ST SLU Mobili	FZ-MIN	-509.78	456.80	111.48	2229.56	0.205
Concio 2	435	I[876]	ST SLU Mobili	FZ-MIN	-509.78	451.65	111.48	1672.17	0.270
Concio 2	435	J[877]	ST SLU Mobili	FZ-MIN	-438.86	388.81	111.48	1672.17	0.233
Concio 2	436	I[877]	ST SLU Mobili	FZ-MIN	-274.68	243.36	111.48	1672.17	0.146
Concio 2	436	J[878]	ST SLU Mobili	FZ-MIN	-164.09	145.38	111.48	1672.17	0.087
Concio 2	437	I[878]	ST SLU Mobili	FZ-MAX	248.57	220.23	111.48	1672.17	0.132
Concio 2	437	J[879]	ST SLU Mobili	FZ-MAX	295.85	262.12	111.48	1672.17	0.157
Concio 1	438	I[879]	ST SLU Mobili	FZ-MAX	295.85	268.19	111.48	2229.56	0.120
Concio 1	438	J[880]	ST SLU Mobili	FZ-MAX	366.77	332.48	111.48	2229.56	0.149

Concio 1	439	I[880]	ST SLU Mobili	FZ-MAX	546.12	495.06	111.48	2229.56	0.222
Concio 1	439	J[881]	ST SLU Mobili	FZ-MAX	664.32	602.21	111.48	2229.56	0.270
Concio 1	440	I[881]	ST SLU Mobili	FZ-MIN	-18.91	17.14	111.48	2229.56	0.008
Concio 1	440	J[882]	ST SLV Vert	FX-MAX	1.81	1.64	111.48	2229.56	0.001

## 9.2 STATO LIMITE DI ESERCIZIO – RESISTENZA AL TAGLIO LONGITUDINALE

Si effettuano le verifiche allo stato limite di esercizio per taglio longitudinale dei connettori trave-soletta. I coefficienti parziali per SLE assunti sono i seguenti:

$$k_s = 0.60 \quad \text{connettori: resistenza allo SLE}$$

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem property: nome delle caratteristiche geometriche dell'elemento

Elem: numero dell'elemento

Position: nodo iniziale (I) o finale (J) dell'elemento

Lcom: combinazione di carico più gravosa

Type: tipo combinazione (caratteristica, frequente, quasi permanente)

V\_c,Ed: sforzo di taglio longitudinale agente sulla larghezza della regione inelastica

v\_L,Ed: sforzo di taglio longitudinale di calcolo per unità di lunghezza (m) all'interfaccia trave-soletta

P\_Rd,ser: resistenza al taglio (SLE) del singolo connettore "Nelson"

v\_L,Rd: resistenza al taglio longitudinale per trave e per unità di lunghezza

Condizione di verifica: Verification Ratio:  $\frac{v_{L,Ed}}{v_{L,Rd}} \leq 1$

Elem property	Elem number	Position	Lcom	Type	V_c,Ed (kN)	v_L,Ed (kN/m)	P_Rd_ser (kN)	v_L,Rd (kN/m)	Verif. Ratio
Concio 1	101	I[719]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	101	J[720]	ST RARA Mobili	Characteristic	12.85	11.65	66.89	1337.73	0.009
Concio 1	102	I[720]	ST RARA Mobili	Characteristic	-475.23	430.80	66.89	1337.73	0.322
Concio 1	102	J[721]	ST RARA Mobili	Characteristic	-394.93	358.01	66.89	1337.73	0.268
Concio 1	103	I[721]	ST RARA Mobili	Characteristic	-259.37	235.12	66.89	1337.73	0.176
Concio 1	103	J[722]	ST RARA Mobili	Characteristic	-211.19	191.45	66.89	1337.73	0.143
Concio 2	104	I[722]	ST RARA Mobili	Characteristic	-211.19	187.11	66.89	1003.30	0.186
Concio 2	104	J[723]	ST RARA Mobili	Characteristic	-179.07	158.65	66.89	1003.30	0.158
Concio 2	105	I[723]	ST RARA Mobili	Characteristic	129.83	115.02	66.89	1003.30	0.115
Concio 2	105	J[724]	ST RARA Mobili	Characteristic	204.48	181.16	66.89	1003.30	0.181
Concio 2	106	I[724]	ST RARA Mobili	Characteristic	327.41	290.08	66.89	1003.30	0.289
Concio 2	106	J[725]	ST RARA Mobili	Characteristic	375.59	332.77	66.89	1003.30	0.332
Concio 3	107	I[725]	ST RARA Mobili	Characteristic	375.59	336.56	66.89	1337.73	0.252
Concio 3	107	J[726]	ST RARA Mobili	Characteristic	407.71	365.35	66.89	1337.73	0.273
Concio 3	108	I[726]	ST RARA Mobili	Characteristic	524.57	470.06	66.89	1337.73	0.351
Concio 3	108	J[727]	ST RARA Mobili	Characteristic	604.87	542.02	66.89	1337.73	0.405
Concio 3	109	I[727]	ST RARA Mobili	Characteristic	685.57	614.33	66.89	1337.73	0.459
Concio 3	109	J[728]	ST RARA Mobili	Characteristic	693.60	621.52	66.89	1337.73	0.465
Concio 4 H=var	110	I[728]	ST RARA Mobili	Characteristic	697.56	610.25	66.89	1337.73	0.456
Concio 4 H=var	110	J[729]	ST RARA Mobili	Characteristic	741.24	447.08	66.89	1337.73	0.334
Concio 4 H=var	111	I[729]	ST RARA Mobili	Characteristic	804.77	485.40	66.89	1337.73	0.363
Concio 4 H=var	111	J[730]	ST RARA Mobili	Characteristic	848.43	389.24	66.89	1337.73	0.291
Concio 4 H=200	112	I[730]	ST RARA Mobili	Characteristic	846.42	388.32	66.89	1337.73	0.290
Concio 4 H=200	112	J[731]	ST RARA Mobili	Characteristic	855.15	392.33	66.89	1337.73	0.293
Concio 4 H=200	113	I[731]	ST RARA Mobili	Characteristic	-788.12	361.57	66.89	1337.73	0.270
Concio 4 H=200	113	J[732]	ST RARA Mobili	Characteristic	-779.39	357.57	66.89	1337.73	0.267

Concio 4 H=var	114	I[732]	ST RARA Mobili	Characteristic	-783.04	359.25	66.89	1337.73	0.269
Concio 4 H=var	114	J[733]	ST RARA Mobili	Characteristic	-739.38	445.97	66.89	1337.73	0.333
Concio 4 H=var	115	I[733]	ST RARA Mobili	Characteristic	-674.03	406.55	66.89	1337.73	0.304
Concio 4 H=var	115	J[734]	ST RARA Mobili	Characteristic	-630.36	551.45	66.89	1337.73	0.412
Concio 5	116	I[734]	ST RARA Mobili	Characteristic	-624.22	559.36	66.89	1337.73	0.418
Concio 5	116	J[735]	ST RARA Mobili	Characteristic	-616.19	552.16	66.89	1337.73	0.413
Concio 5	117	I[735]	ST RARA Mobili	Characteristic	-535.29	479.67	66.89	1337.73	0.359
Concio 5	117	J[736]	ST RARA Mobili	Characteristic	-451.78	404.83	66.89	1337.73	0.303
Concio 5	118	I[736]	ST RARA Mobili	Characteristic	-335.25	300.42	66.89	1337.73	0.225
Concio 5	118	J[737]	ST RARA Mobili	Characteristic	-298.32	267.32	66.89	1337.73	0.200
Concio 6	119	I[737]	ST RARA Mobili	Characteristic	-298.32	262.21	66.89	1003.30	0.261
Concio 6	119	J[738]	ST RARA Mobili	Characteristic	-251.74	221.27	66.89	1003.30	0.221
Concio 6	120	I[738]	ST RARA Mobili	Characteristic	-129.60	113.91	66.89	1003.30	0.114
Concio 6	120	J[739]	ST RARA Mobili	Characteristic	-87.84	77.21	66.89	1003.30	0.077
Concio 6	121	I[739]	ST RARA Mobili	Characteristic	-87.84	77.21	66.89	1003.30	0.077
Concio 6	121	J[740]	ST RARA Mobili	Characteristic	129.59	113.90	66.89	1003.30	0.114
Concio 6	122	I[740]	ST RARA Mobili	Characteristic	251.77	221.29	66.89	1003.30	0.221
Concio 6	122	J[741]	ST RARA Mobili	Characteristic	298.34	262.23	66.89	1003.30	0.261
Concio 5	123	I[741]	ST RARA Mobili	Characteristic	298.34	267.34	66.89	1337.73	0.200
Concio 5	123	J[742]	ST RARA Mobili	Characteristic	335.28	300.44	66.89	1337.73	0.225
Concio 5	124	I[742]	ST RARA Mobili	Characteristic	451.84	404.88	66.89	1337.73	0.303
Concio 5	124	J[743]	ST RARA Mobili	Characteristic	535.35	479.72	66.89	1337.73	0.359
Concio 5	125	I[743]	ST RARA Mobili	Characteristic	616.19	552.16	66.89	1337.73	0.413
Concio 5	125	J[744]	ST RARA Mobili	Characteristic	624.22	559.36	66.89	1337.73	0.418
Concio 4 H=var	126	I[744]	ST RARA Mobili	Characteristic	630.36	551.46	66.89	1337.73	0.412
Concio 4 H=var	126	J[745]	ST RARA Mobili	Characteristic	674.04	406.55	66.89	1337.73	0.304
Concio 4 H=var	127	I[745]	ST RARA Mobili	Characteristic	739.22	445.87	66.89	1337.73	0.333
Concio 4 H=var	127	J[746]	ST RARA Mobili	Characteristic	782.88	359.17	66.89	1337.73	0.268
Concio 4 H=200	128	I[746]	ST RARA Mobili	Characteristic	779.22	357.49	66.89	1337.73	0.267
Concio 4 H=200	128	J[747]	ST RARA Mobili	Characteristic	787.95	361.50	66.89	1337.73	0.270
Concio 4 H=200	129	I[747]	ST RARA Mobili	Characteristic	-855.35	392.42	66.89	1337.73	0.293
Concio 4 H=200	129	J[748]	ST RARA Mobili	Characteristic	-846.62	388.41	66.89	1337.73	0.290
Concio 4 H=var	130	I[748]	ST RARA Mobili	Characteristic	-848.62	389.33	66.89	1337.73	0.291
Concio 4 H=var	130	J[749]	ST RARA Mobili	Characteristic	-804.96	485.52	66.89	1337.73	0.363
Concio 4 H=var	131	I[749]	ST RARA Mobili	Characteristic	-741.26	447.10	66.89	1337.73	0.334
Concio 4 H=var	131	J[750]	ST RARA Mobili	Characteristic	-697.58	610.27	66.89	1337.73	0.456
Concio 3	132	I[750]	ST RARA Mobili	Characteristic	-693.62	621.54	66.89	1337.73	0.465
Concio 3	132	J[751]	ST RARA Mobili	Characteristic	-685.59	614.35	66.89	1337.73	0.459
Concio 3	133	I[751]	ST RARA Mobili	Characteristic	-604.84	541.99	66.89	1337.73	0.405
Concio 3	133	J[752]	ST RARA Mobili	Characteristic	-524.54	470.04	66.89	1337.73	0.351
Concio 3	134	I[752]	ST RARA Mobili	Characteristic	-407.71	365.34	66.89	1337.73	0.273
Concio 3	134	J[753]	ST RARA Mobili	Characteristic	-375.59	336.56	66.89	1337.73	0.252
Concio 2	135	I[753]	ST RARA Mobili	Characteristic	-375.59	332.76	66.89	1003.30	0.332
Concio 2	135	J[754]	ST RARA Mobili	Characteristic	-327.41	290.07	66.89	1003.30	0.289
Concio 2	136	I[754]	ST RARA Mobili	Characteristic	-204.47	181.16	66.89	1003.30	0.181
Concio 2	136	J[755]	ST RARA Mobili	Characteristic	-129.81	115.01	66.89	1003.30	0.115
Concio 2	137	I[755]	ST RARA Mobili	Characteristic	179.07	158.65	66.89	1003.30	0.158
Concio 2	137	J[756]	ST RARA Mobili	Characteristic	211.19	187.11	66.89	1003.30	0.186
Concio 1	138	I[756]	ST RARA Mobili	Characteristic	211.19	191.44	66.89	1337.73	0.143
Concio 1	138	J[757]	ST RARA Mobili	Characteristic	259.37	235.12	66.89	1337.73	0.176
Concio 1	139	I[757]	ST RARA Mobili	Characteristic	394.94	358.01	66.89	1337.73	0.268
Concio 1	139	J[758]	ST RARA Mobili	Characteristic	475.24	430.81	66.89	1337.73	0.322
Concio 1	140	I[758]	ST RARA Mobili	Characteristic	-12.85	11.65	66.89	1337.73	0.009
Concio 1	140	J[759]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	201	I[760]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	201	J[761]	ST RARA Mobili	Characteristic	333.00	303.06	66.89	1337.73	0.227
Concio 1	202	I[761]	ST RARA Mobili	Characteristic	-911.98	829.99	66.89	1337.73	0.620
Concio 1	202	J[762]	ST RARA Mobili	Characteristic	-426.27	387.95	66.89	1337.73	0.290
Concio 1	203	I[762]	ST RARA Mobili	Characteristic	-631.53	574.75	66.89	1337.73	0.430



Concio 1	203	J[763]	ST RARA Mobili	Characteristic	-374.14	340.50	66.89	1337.73	0.255
Concio 2	204	I[763]	ST RARA Mobili	Characteristic	-374.14	332.89	66.89	1003.30	0.332
Concio 2	204	J[764]	ST RARA Mobili	Characteristic	455.17	404.99	66.89	1003.30	0.404
Concio 2	205	I[764]	ST RARA Mobili	Characteristic	-466.06	414.68	66.89	1003.30	0.413
Concio 2	205	J[765]	ST RARA Mobili	Characteristic	607.50	540.53	66.89	1003.30	0.539
Concio 2	206	I[765]	ST RARA Mobili	Characteristic	367.67	327.13	66.89	1003.30	0.326
Concio 2	206	J[766]	ST RARA Mobili	Characteristic	606.67	539.79	66.89	1003.30	0.538
Concio 3	207	I[766]	ST RARA Mobili	Characteristic	606.67	546.23	66.89	1337.73	0.408
Concio 3	207	J[767]	ST RARA Mobili	Characteristic	772.82	695.83	66.89	1337.73	0.520
Concio 3	208	I[767]	ST RARA Mobili	Characteristic	568.87	512.20	66.89	1337.73	0.383
Concio 3	208	J[768]	ST RARA Mobili	Characteristic	977.60	880.22	66.89	1337.73	0.658
Concio 3	209	I[768]	ST RARA Mobili	Characteristic	824.18	742.08	66.89	1337.73	0.555
Concio 3	209	J[769]	ST RARA Mobili	Characteristic	862.50	776.58	66.89	1337.73	0.581
Concio 4 H=var	210	I[769]	ST RARA Mobili	Characteristic	855.75	754.22	66.89	1337.73	0.564
Concio 4 H=var	210	J[770]	ST RARA Mobili	Characteristic	1086.50	662.07	66.89	1337.73	0.495
Concio 4 H=var	211	I[770]	ST RARA Mobili	Characteristic	1001.82	610.48	66.89	1337.73	0.456
Concio 4 H=var	211	J[771]	ST RARA Mobili	Characteristic	1237.27	574.61	66.89	1337.73	0.430
Concio 4 H=200	212	I[771]	ST RARA Mobili	Characteristic	1237.18	574.57	66.89	1337.73	0.430
Concio 4 H=200	212	J[772]	ST RARA Mobili	Characteristic	1282.21	595.48	66.89	1337.73	0.445
Concio 4 H=200	213	I[772]	ST RARA Mobili	Characteristic	-1213.54	563.59	66.89	1337.73	0.421
Concio 4 H=200	213	J[773]	ST RARA Mobili	Characteristic	-1168.54	542.69	66.89	1337.73	0.406
Concio 4 H=var	214	I[773]	ST RARA Mobili	Characteristic	-1168.48	542.66	66.89	1337.73	0.406
Concio 4 H=var	214	J[774]	ST RARA Mobili	Characteristic	-933.04	568.56	66.89	1337.73	0.425
Concio 4 H=var	215	I[774]	ST RARA Mobili	Characteristic	-1019.25	621.10	66.89	1337.73	0.464
Concio 4 H=var	215	J[775]	ST RARA Mobili	Characteristic	-789.19	695.56	66.89	1337.73	0.520
Concio 5	216	I[775]	ST RARA Mobili	Characteristic	-796.18	716.86	66.89	1337.73	0.536
Concio 5	216	J[776]	ST RARA Mobili	Characteristic	-757.97	682.47	66.89	1337.73	0.510
Concio 5	217	I[776]	ST RARA Mobili	Characteristic	-916.51	825.21	66.89	1337.73	0.617
Concio 5	217	J[777]	ST RARA Mobili	Characteristic	-493.45	444.29	66.89	1337.73	0.332
Concio 5	218	I[777]	ST RARA Mobili	Characteristic	-705.96	635.64	66.89	1337.73	0.475
Concio 5	218	J[778]	ST RARA Mobili	Characteristic	-515.42	464.08	66.89	1337.73	0.347
Concio 6	219	I[778]	ST RARA Mobili	Characteristic	-515.42	454.82	66.89	1003.30	0.453
Concio 6	219	J[779]	ST RARA Mobili	Characteristic	382.48	337.51	66.89	1003.30	0.336
Concio 6	220	I[779]	ST RARA Mobili	Characteristic	-536.58	473.48	66.89	1003.30	0.472
Concio 6	220	J[780]	ST RARA Mobili	Characteristic	-319.78	282.18	66.89	1003.30	0.281
Concio 6	221	I[780]	ST RARA Mobili	Characteristic	-319.78	282.18	66.89	1003.30	0.281
Concio 6	221	J[781]	ST RARA Mobili	Characteristic	536.44	473.37	66.89	1003.30	0.472
Concio 6	222	I[781]	ST RARA Mobili	Characteristic	-382.63	337.64	66.89	1003.30	0.337
Concio 6	222	J[782]	ST RARA Mobili	Characteristic	515.28	454.69	66.89	1003.30	0.453
Concio 5	223	I[782]	ST RARA Mobili	Characteristic	515.28	463.95	66.89	1337.73	0.347
Concio 5	223	J[783]	ST RARA Mobili	Characteristic	705.84	635.53	66.89	1337.73	0.475
Concio 5	224	I[783]	ST RARA Mobili	Characteristic	493.36	444.21	66.89	1337.73	0.332
Concio 5	224	J[784]	ST RARA Mobili	Characteristic	916.33	825.05	66.89	1337.73	0.617
Concio 5	225	I[784]	ST RARA Mobili	Characteristic	757.80	682.31	66.89	1337.73	0.510
Concio 5	225	J[785]	ST RARA Mobili	Characteristic	796.00	716.70	66.89	1337.73	0.536
Concio 4 H=var	226	I[785]	ST RARA Mobili	Characteristic	789.02	695.40	66.89	1337.73	0.520
Concio 4 H=var	226	J[786]	ST RARA Mobili	Characteristic	1019.12	621.02	66.89	1337.73	0.464
Concio 4 H=var	227	I[786]	ST RARA Mobili	Characteristic	932.83	568.44	66.89	1337.73	0.425
Concio 4 H=var	227	J[787]	ST RARA Mobili	Characteristic	1168.25	542.56	66.89	1337.73	0.406
Concio 4 H=200	228	I[787]	ST RARA Mobili	Characteristic	1168.31	542.58	66.89	1337.73	0.406
Concio 4 H=200	228	J[788]	ST RARA Mobili	Characteristic	1213.31	563.48	66.89	1337.73	0.421
Concio 4 H=200	229	I[788]	ST RARA Mobili	Characteristic	-1282.45	595.59	66.89	1337.73	0.445
Concio 4 H=200	229	J[789]	ST RARA Mobili	Characteristic	-1237.42	574.68	66.89	1337.73	0.430
Concio 4 H=var	230	I[789]	ST RARA Mobili	Characteristic	-1237.50	574.71	66.89	1337.73	0.430
Concio 4 H=var	230	J[790]	ST RARA Mobili	Characteristic	-1002.03	610.61	66.89	1337.73	0.456
Concio 4 H=var	231	I[790]	ST RARA Mobili	Characteristic	-1086.64	662.16	66.89	1337.73	0.495
Concio 4 H=var	231	J[791]	ST RARA Mobili	Characteristic	-855.93	754.38	66.89	1337.73	0.564
Concio 3	232	I[791]	ST RARA Mobili	Characteristic	-862.69	776.75	66.89	1337.73	0.581
Concio 3	232	J[792]	ST RARA Mobili	Characteristic	-824.37	742.25	66.89	1337.73	0.555

Concio 3	233	I[792]	ST RARA Mobili	Characteristic	-977.79	880.38	66.89	1337.73	0.658
Concio 3	233	J[793]	ST RARA Mobili	Characteristic	-568.96	512.29	66.89	1337.73	0.383
Concio 3	234	I[793]	ST RARA Mobili	Characteristic	-772.96	695.96	66.89	1337.73	0.520
Concio 3	234	J[794]	ST RARA Mobili	Characteristic	-606.84	546.39	66.89	1337.73	0.408
Concio 2	235	I[794]	ST RARA Mobili	Characteristic	-606.84	539.94	66.89	1003.30	0.538
Concio 2	235	J[795]	ST RARA Mobili	Characteristic	-367.84	327.29	66.89	1003.30	0.326
Concio 2	236	I[795]	ST RARA Mobili	Characteristic	-607.72	540.72	66.89	1003.30	0.539
Concio 2	236	J[796]	ST RARA Mobili	Characteristic	465.85	414.49	66.89	1003.30	0.413
Concio 2	237	I[796]	ST RARA Mobili	Characteristic	-455.35	405.15	66.89	1003.30	0.404
Concio 2	237	J[797]	ST RARA Mobili	Characteristic	373.96	332.73	66.89	1003.30	0.332
Concio 1	238	I[797]	ST RARA Mobili	Characteristic	373.96	340.34	66.89	1337.73	0.254
Concio 1	238	J[798]	ST RARA Mobili	Characteristic	631.33	574.57	66.89	1337.73	0.430
Concio 1	239	I[798]	ST RARA Mobili	Characteristic	426.11	387.80	66.89	1337.73	0.290
Concio 1	239	J[799]	ST RARA Mobili	Characteristic	911.70	829.73	66.89	1337.73	0.620
Concio 1	240	I[799]	ST RARA Mobili	Characteristic	-333.00	303.06	66.89	1337.73	0.227
Concio 1	240	J[800]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	301	I[801]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	301	J[802]	ST RARA Mobili	Characteristic	333.00	303.06	66.89	1337.73	0.227
Concio 1	302	I[802]	ST RARA Mobili	Characteristic	-912.61	830.56	66.89	1337.73	0.621
Concio 1	302	J[803]	ST RARA Mobili	Characteristic	-426.90	388.52	66.89	1337.73	0.290
Concio 1	303	I[803]	ST RARA Mobili	Characteristic	-631.90	575.09	66.89	1337.73	0.430
Concio 1	303	J[804]	ST RARA Mobili	Characteristic	-374.50	340.83	66.89	1337.73	0.255
Concio 2	304	I[804]	ST RARA Mobili	Characteristic	-374.50	333.22	66.89	1003.30	0.332
Concio 2	304	J[805]	ST RARA Mobili	Characteristic	455.52	405.30	66.89	1003.30	0.404
Concio 2	305	I[805]	ST RARA Mobili	Characteristic	-466.12	414.73	66.89	1003.30	0.413
Concio 2	305	J[806]	ST RARA Mobili	Characteristic	607.57	540.59	66.89	1003.30	0.539
Concio 2	306	I[806]	ST RARA Mobili	Characteristic	368.17	327.58	66.89	1003.30	0.327
Concio 2	306	J[807]	ST RARA Mobili	Characteristic	607.17	540.23	66.89	1003.30	0.538
Concio 3	307	I[807]	ST RARA Mobili	Characteristic	607.17	546.69	66.89	1337.73	0.409
Concio 3	307	J[808]	ST RARA Mobili	Characteristic	773.32	696.29	66.89	1337.73	0.520
Concio 3	308	I[808]	ST RARA Mobili	Characteristic	569.65	512.90	66.89	1337.73	0.383
Concio 3	308	J[809]	ST RARA Mobili	Characteristic	978.38	880.92	66.89	1337.73	0.659
Concio 3	309	I[809]	ST RARA Mobili	Characteristic	825.11	742.92	66.89	1337.73	0.555
Concio 3	309	J[810]	ST RARA Mobili	Characteristic	863.43	777.42	66.89	1337.73	0.581
Concio 4 H=var	310	I[810]	ST RARA Mobili	Characteristic	857.31	755.60	66.89	1337.73	0.565
Concio 4 H=var	310	J[811]	ST RARA Mobili	Characteristic	1088.06	663.03	66.89	1337.73	0.496
Concio 4 H=var	311	I[811]	ST RARA Mobili	Characteristic	1003.78	611.67	66.89	1337.73	0.457
Concio 4 H=var	311	J[812]	ST RARA Mobili	Characteristic	1239.23	575.52	66.89	1337.73	0.430
Concio 4 H=200	312	I[812]	ST RARA Mobili	Characteristic	1238.43	575.15	66.89	1337.73	0.430
Concio 4 H=200	312	J[813]	ST RARA Mobili	Characteristic	1283.47	596.06	66.89	1337.73	0.446
Concio 4 H=200	313	I[813]	ST RARA Mobili	Characteristic	-1214.78	564.16	66.89	1337.73	0.422
Concio 4 H=200	313	J[814]	ST RARA Mobili	Characteristic	-1169.79	543.27	66.89	1337.73	0.406
Concio 4 H=var	314	I[814]	ST RARA Mobili	Characteristic	-1170.47	543.58	66.89	1337.73	0.406
Concio 4 H=var	314	J[815]	ST RARA Mobili	Characteristic	-935.03	569.78	66.89	1337.73	0.426
Concio 4 H=var	315	I[815]	ST RARA Mobili	Characteristic	-1020.91	622.11	66.89	1337.73	0.465
Concio 4 H=var	315	J[816]	ST RARA Mobili	Characteristic	-790.86	697.03	66.89	1337.73	0.521
Concio 5	316	I[816]	ST RARA Mobili	Characteristic	-797.10	717.70	66.89	1337.73	0.537
Concio 5	316	J[817]	ST RARA Mobili	Characteristic	-758.90	683.30	66.89	1337.73	0.511
Concio 5	317	I[817]	ST RARA Mobili	Characteristic	-917.31	825.93	66.89	1337.73	0.617
Concio 5	317	J[818]	ST RARA Mobili	Characteristic	-494.25	445.01	66.89	1337.73	0.333
Concio 5	318	I[818]	ST RARA Mobili	Characteristic	-706.41	636.04	66.89	1337.73	0.475
Concio 5	318	J[819]	ST RARA Mobili	Characteristic	-515.86	464.48	66.89	1337.73	0.347
Concio 6	319	I[819]	ST RARA Mobili	Characteristic	-515.86	455.21	66.89	1003.30	0.454
Concio 6	319	J[820]	ST RARA Mobili	Characteristic	382.93	337.90	66.89	1003.30	0.337
Concio 6	320	I[820]	ST RARA Mobili	Characteristic	-536.58	473.49	66.89	1003.30	0.472
Concio 6	320	J[821]	ST RARA Mobili	Characteristic	-319.79	282.18	66.89	1003.30	0.281
Concio 6	321	I[821]	ST RARA Mobili	Characteristic	-319.79	282.18	66.89	1003.30	0.281
Concio 6	321	J[822]	ST RARA Mobili	Characteristic	536.44	473.37	66.89	1003.30	0.472
Concio 6	322	I[822]	ST RARA Mobili	Characteristic	-383.09	338.04	66.89	1003.30	0.337



Concio 6	322	J[823]	ST RARA Mobili	Characteristic	515.74	455.10	66.89	1003.30	0.454
Concio 5	323	I[823]	ST RARA Mobili	Characteristic	515.74	464.36	66.89	1337.73	0.347
Concio 5	323	J[824]	ST RARA Mobili	Characteristic	706.30	635.94	66.89	1337.73	0.475
Concio 5	324	I[824]	ST RARA Mobili	Characteristic	494.14	444.92	66.89	1337.73	0.333
Concio 5	324	J[825]	ST RARA Mobili	Characteristic	917.12	825.76	66.89	1337.73	0.617
Concio 5	325	I[825]	ST RARA Mobili	Characteristic	758.71	683.13	66.89	1337.73	0.511
Concio 5	325	J[826]	ST RARA Mobili	Characteristic	796.91	717.53	66.89	1337.73	0.536
Concio 4 H=var	326	I[826]	ST RARA Mobili	Characteristic	790.66	696.85	66.89	1337.73	0.521
Concio 4 H=var	326	J[827]	ST RARA Mobili	Characteristic	1020.77	622.02	66.89	1337.73	0.465
Concio 4 H=var	327	I[827]	ST RARA Mobili	Characteristic	934.87	569.68	66.89	1337.73	0.426
Concio 4 H=var	327	J[828]	ST RARA Mobili	Characteristic	1170.29	543.50	66.89	1337.73	0.406
Concio 4 H=200	328	I[828]	ST RARA Mobili	Characteristic	1169.61	543.19	66.89	1337.73	0.406
Concio 4 H=200	328	J[829]	ST RARA Mobili	Characteristic	1214.61	564.08	66.89	1337.73	0.422
Concio 4 H=200	329	I[829]	ST RARA Mobili	Characteristic	-1283.65	596.15	66.89	1337.73	0.446
Concio 4 H=200	329	J[830]	ST RARA Mobili	Characteristic	-1238.62	575.24	66.89	1337.73	0.430
Concio 4 H=var	330	I[830]	ST RARA Mobili	Characteristic	-1239.42	575.60	66.89	1337.73	0.430
Concio 4 H=var	330	J[831]	ST RARA Mobili	Characteristic	-1003.95	611.78	66.89	1337.73	0.457
Concio 4 H=var	331	I[831]	ST RARA Mobili	Characteristic	-1088.23	663.13	66.89	1337.73	0.496
Concio 4 H=var	331	J[832]	ST RARA Mobili	Characteristic	-857.52	755.78	66.89	1337.73	0.565
Concio 3	332	I[832]	ST RARA Mobili	Characteristic	-863.64	777.61	66.89	1337.73	0.581
Concio 3	332	J[833]	ST RARA Mobili	Characteristic	-825.32	743.10	66.89	1337.73	0.555
Concio 3	333	I[833]	ST RARA Mobili	Characteristic	-978.59	881.11	66.89	1337.73	0.659
Concio 3	333	J[834]	ST RARA Mobili	Characteristic	-569.77	513.01	66.89	1337.73	0.383
Concio 3	334	I[834]	ST RARA Mobili	Characteristic	-773.45	696.41	66.89	1337.73	0.521
Concio 3	334	J[835]	ST RARA Mobili	Characteristic	-607.33	546.83	66.89	1337.73	0.409
Concio 2	335	I[835]	ST RARA Mobili	Characteristic	-607.33	540.38	66.89	1003.30	0.539
Concio 2	335	J[836]	ST RARA Mobili	Characteristic	-368.33	327.72	66.89	1003.30	0.327
Concio 2	336	I[836]	ST RARA Mobili	Characteristic	-607.77	540.77	66.89	1003.30	0.539
Concio 2	336	J[837]	ST RARA Mobili	Characteristic	465.90	414.54	66.89	1003.30	0.413
Concio 2	337	I[837]	ST RARA Mobili	Characteristic	-455.71	405.47	66.89	1003.30	0.404
Concio 2	337	J[838]	ST RARA Mobili	Characteristic	374.33	333.06	66.89	1003.30	0.332
Concio 1	338	I[838]	ST RARA Mobili	Characteristic	374.33	340.68	66.89	1337.73	0.255
Concio 1	338	J[839]	ST RARA Mobili	Characteristic	631.69	574.90	66.89	1337.73	0.430
Concio 1	339	I[839]	ST RARA Mobili	Characteristic	426.74	388.37	66.89	1337.73	0.290
Concio 1	339	J[840]	ST RARA Mobili	Characteristic	912.33	830.31	66.89	1337.73	0.621
Concio 1	340	I[840]	ST RARA Mobili	Characteristic	-333.00	303.06	66.89	1337.73	0.227
Concio 1	340	J[841]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	401	I[842]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	401	J[843]	ST RARA Mobili	Characteristic	12.85	11.65	66.89	1337.73	0.009
Concio 1	402	I[843]	ST RARA Mobili	Characteristic	-473.71	429.42	66.89	1337.73	0.321
Concio 1	402	J[844]	ST RARA Mobili	Characteristic	-393.41	356.63	66.89	1337.73	0.267
Concio 1	403	I[844]	ST RARA Mobili	Characteristic	-259.09	234.87	66.89	1337.73	0.176
Concio 1	403	J[845]	ST RARA Mobili	Characteristic	-210.91	191.20	66.89	1337.73	0.143
Concio 2	404	I[845]	ST RARA Mobili	Characteristic	-210.91	186.86	66.89	1003.30	0.186
Concio 2	404	J[846]	ST RARA Mobili	Characteristic	-178.79	158.41	66.89	1003.30	0.158
Concio 2	405	I[846]	ST RARA Mobili	Characteristic	129.60	114.82	66.89	1003.30	0.114
Concio 2	405	J[847]	ST RARA Mobili	Characteristic	204.25	180.96	66.89	1003.30	0.180
Concio 2	406	I[847]	ST RARA Mobili	Characteristic	326.82	289.55	66.89	1003.30	0.289
Concio 2	406	J[848]	ST RARA Mobili	Characteristic	375.00	332.24	66.89	1003.30	0.331
Concio 3	407	I[848]	ST RARA Mobili	Characteristic	375.00	336.03	66.89	1337.73	0.251
Concio 3	407	J[849]	ST RARA Mobili	Characteristic	407.12	364.81	66.89	1337.73	0.273
Concio 3	408	I[849]	ST RARA Mobili	Characteristic	522.71	468.40	66.89	1337.73	0.350
Concio 3	408	J[850]	ST RARA Mobili	Characteristic	603.01	540.35	66.89	1337.73	0.404
Concio 3	409	I[850]	ST RARA Mobili	Characteristic	683.25	612.25	66.89	1337.73	0.458
Concio 3	409	J[851]	ST RARA Mobili	Characteristic	691.28	619.45	66.89	1337.73	0.463
Concio 4 H=var	410	I[851]	ST RARA Mobili	Characteristic	695.63	608.56	66.89	1337.73	0.455
Concio 4 H=var	410	J[852]	ST RARA Mobili	Characteristic	739.30	445.92	66.89	1337.73	0.333
Concio 4 H=var	411	I[852]	ST RARA Mobili	Characteristic	802.65	484.13	66.89	1337.73	0.362
Concio 4 H=var	411	J[853]	ST RARA Mobili	Characteristic	846.31	388.27	66.89	1337.73	0.290

Concio 4 H=200	412	I[853]	ST RARA Mobili	Characteristic	844.02	387.22	66.89	1337.73	0.289
Concio 4 H=200	412	J[854]	ST RARA Mobili	Characteristic	852.75	391.23	66.89	1337.73	0.292
Concio 4 H=200	413	I[854]	ST RARA Mobili	Characteristic	-785.66	360.45	66.89	1337.73	0.269
Concio 4 H=200	413	J[855]	ST RARA Mobili	Characteristic	-776.93	356.44	66.89	1337.73	0.266
Concio 4 H=var	414	I[855]	ST RARA Mobili	Characteristic	-780.88	358.25	66.89	1337.73	0.268
Concio 4 H=var	414	J[856]	ST RARA Mobili	Characteristic	-737.22	444.66	66.89	1337.73	0.332
Concio 4 H=var	415	I[856]	ST RARA Mobili	Characteristic	-672.16	405.42	66.89	1337.73	0.303
Concio 4 H=var	415	J[857]	ST RARA Mobili	Characteristic	-628.48	549.81	66.89	1337.73	0.411
Concio 5	416	I[857]	ST RARA Mobili	Characteristic	-621.95	557.32	66.89	1337.73	0.417
Concio 5	416	J[858]	ST RARA Mobili	Characteristic	-613.92	550.12	66.89	1337.73	0.411
Concio 5	417	I[858]	ST RARA Mobili	Characteristic	-533.53	478.09	66.89	1337.73	0.357
Concio 5	417	J[859]	ST RARA Mobili	Characteristic	-450.02	403.25	66.89	1337.73	0.301
Concio 5	418	I[859]	ST RARA Mobili	Characteristic	-334.81	300.02	66.89	1337.73	0.224
Concio 5	418	J[860]	ST RARA Mobili	Characteristic	-297.87	266.92	66.89	1337.73	0.200
Concio 6	419	I[860]	ST RARA Mobili	Characteristic	-297.87	261.82	66.89	1003.30	0.261
Concio 6	419	J[861]	ST RARA Mobili	Characteristic	-251.30	220.88	66.89	1003.30	0.220
Concio 6	420	I[861]	ST RARA Mobili	Characteristic	-129.60	113.91	66.89	1003.30	0.114
Concio 6	420	J[862]	ST RARA Mobili	Characteristic	-87.84	77.21	66.89	1003.30	0.077
Concio 6	421	I[862]	ST RARA Mobili	Characteristic	-87.84	77.21	66.89	1003.30	0.077
Concio 6	421	J[863]	ST RARA Mobili	Characteristic	129.59	113.90	66.89	1003.30	0.114
Concio 6	422	I[863]	ST RARA Mobili	Characteristic	251.31	220.89	66.89	1003.30	0.220
Concio 6	422	J[864]	ST RARA Mobili	Characteristic	297.89	261.83	66.89	1003.30	0.261
Concio 5	423	I[864]	ST RARA Mobili	Characteristic	297.89	266.93	66.89	1337.73	0.200
Concio 5	423	J[865]	ST RARA Mobili	Characteristic	334.82	300.03	66.89	1337.73	0.224
Concio 5	424	I[865]	ST RARA Mobili	Characteristic	450.04	403.27	66.89	1337.73	0.301
Concio 5	424	J[866]	ST RARA Mobili	Characteristic	533.55	478.11	66.89	1337.73	0.357
Concio 5	425	I[866]	ST RARA Mobili	Characteristic	613.91	550.12	66.89	1337.73	0.411
Concio 5	425	J[867]	ST RARA Mobili	Characteristic	621.94	557.31	66.89	1337.73	0.417
Concio 4 H=var	426	I[867]	ST RARA Mobili	Characteristic	628.47	549.80	66.89	1337.73	0.411
Concio 4 H=var	426	J[868]	ST RARA Mobili	Characteristic	672.14	405.41	66.89	1337.73	0.303
Concio 4 H=var	427	I[868]	ST RARA Mobili	Characteristic	737.15	444.62	66.89	1337.73	0.332
Concio 4 H=var	427	J[869]	ST RARA Mobili	Characteristic	780.81	358.22	66.89	1337.73	0.268
Concio 4 H=200	428	I[869]	ST RARA Mobili	Characteristic	776.87	356.41	66.89	1337.73	0.266
Concio 4 H=200	428	J[870]	ST RARA Mobili	Characteristic	785.60	360.42	66.89	1337.73	0.269
Concio 4 H=200	429	I[870]	ST RARA Mobili	Characteristic	-852.85	391.27	66.89	1337.73	0.292
Concio 4 H=200	429	J[871]	ST RARA Mobili	Characteristic	-844.12	387.27	66.89	1337.73	0.289
Concio 4 H=var	430	I[871]	ST RARA Mobili	Characteristic	-846.41	388.32	66.89	1337.73	0.290
Concio 4 H=var	430	J[872]	ST RARA Mobili	Characteristic	-802.75	484.19	66.89	1337.73	0.362
Concio 4 H=var	431	I[872]	ST RARA Mobili	Characteristic	-739.35	445.94	66.89	1337.73	0.333
Concio 4 H=var	431	J[873]	ST RARA Mobili	Characteristic	-695.67	608.59	66.89	1337.73	0.455
Concio 3	432	I[873]	ST RARA Mobili	Characteristic	-691.31	619.48	66.89	1337.73	0.463
Concio 3	432	J[874]	ST RARA Mobili	Characteristic	-683.28	612.28	66.89	1337.73	0.458
Concio 3	433	I[874]	ST RARA Mobili	Characteristic	-603.02	540.36	66.89	1337.73	0.404
Concio 3	433	J[875]	ST RARA Mobili	Characteristic	-522.72	468.40	66.89	1337.73	0.350
Concio 3	434	I[875]	ST RARA Mobili	Characteristic	-407.12	364.82	66.89	1337.73	0.273
Concio 3	434	J[876]	ST RARA Mobili	Characteristic	-375.00	336.03	66.89	1337.73	0.251
Concio 2	435	I[876]	ST RARA Mobili	Characteristic	-375.00	332.24	66.89	1003.30	0.331
Concio 2	435	J[877]	ST RARA Mobili	Characteristic	-326.82	289.56	66.89	1003.30	0.289
Concio 2	436	I[877]	ST RARA Mobili	Characteristic	-204.24	180.95	66.89	1003.30	0.180
Concio 2	436	J[878]	ST RARA Mobili	Characteristic	-129.58	114.81	66.89	1003.30	0.114
Concio 2	437	I[878]	ST RARA Mobili	Characteristic	178.79	158.40	66.89	1003.30	0.158
Concio 2	437	J[879]	ST RARA Mobili	Characteristic	210.91	186.86	66.89	1003.30	0.186
Concio 1	438	I[879]	ST RARA Mobili	Characteristic	210.91	191.19	66.89	1337.73	0.143
Concio 1	438	J[880]	ST RARA Mobili	Characteristic	259.09	234.87	66.89	1337.73	0.176
Concio 1	439	I[880]	ST RARA Mobili	Characteristic	393.42	356.64	66.89	1337.73	0.267
Concio 1	439	J[881]	ST RARA Mobili	Characteristic	473.72	429.43	66.89	1337.73	0.321
Concio 1	440	I[881]	ST RARA Mobili	Characteristic	-12.85	11.65	66.89	1337.73	0.009
Concio 1	440	J[882]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000

### 9.3 STATO LIMITE ULTIMO DI FATICA – RESISTENZA AL TAGLIO LONGITUDINALE

Si effettuano le verifiche allo stato limite ultimo per fatica a “danneggiamento accettabile” dei connettori trave-soletta.

L’impalcato si considera caricato secondo il modello di carico a fatica 3, applicato sulla corsia lenta (ved. 3.4.2).

Il coefficiente parziale di sicurezza per le verifiche dei connettori è pari a:

$$\gamma_{Mf,s} = 1 \quad \text{EN 1994-1-1:2005; § 6.8.2}$$

I coefficienti di equivalenza assumono i seguenti valori:

$$\lambda_{v1} = 1.55: \quad \text{EN 1994-2:2005, § 6.8.6.2(4)}$$

$$\lambda_{v2} = \frac{Q_{M1}}{Q_0} \left( \frac{N_{obs}}{N_0} \right)^{1/8} = 1.10: \quad \text{EN 1993-2, § 9.5.2}$$

$Q_{M1} = 440 \text{ kN}$ : massa complessiva a pieno carico autotreno o autoarticolato a 5 assi

$Q_0 = 480 \text{ kN}$

$N_{obs} = 2 \times 10^6$  : flusso annuo di veicoli pesanti sulla corsia lenta per strade ed autostrade caratterizzate da intenso traffico pesante.

$N_0 = 2 \times 10^6$

$$\lambda_{v3} = \left( \frac{t_{ld}}{100} \right)^{1/8} = 0.871$$

$t_{ld} = 50$  anni: vita di progetto del ponte

$$\lambda_{v4} = 1: \text{fattore per traffico pesante sulle altre corsie}$$

$$\lambda_v = \lambda_{v1} \cdot \lambda_{v2} \cdot \lambda_{v3} \cdot \lambda_{v4} = 1.484$$

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem property: nome delle caratteristiche geometriche dell’elemento

Elem: numero dell’elemento

Position: nodo iniziale (I) o finale (J) dell’elemento

Lcom: combinazione di carico più gravosa

Type: sollecitazione (massima o minima)

lamda\_v: coefficienti di danno equivalente

Delta\_Tau: ampiezza delle tensioni tangenziali per il carico da fatica

Delta\_Tau\_E,2 = Lamda\_v x Delta\_Tau: ampiezza costante delle tensioni tangenziali relative a  $2 \times 10^6$  cicli di carico all’anno

Delta\_Tau\_c: tensione tangenziale limite (resistenza a fatica connettori a piolo)

$$\text{Verification Ratio} = \gamma_{Mf,s} \frac{\text{Delta\_Tau\_E\_2}}{\text{Delta\_Tau\_c}} \quad \text{verificato se } \leq 1$$

Elem property	Elem number	Position	Lcom	Type	Lamda_v	Delta_Tau (kN/m <sup>2</sup> )	Delta_Tau_E_2 (kN/m <sup>2</sup> )	Delta_Tau_c (kN/m <sup>2</sup> )	Verif. Ratio
Concio 1	101	I[719]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	101	J[720]	ST FATICA DANN.ACC.	FX-MAX	1.484	1223.433	1815.927	90000.000	0.020
Concio 1	102	I[720]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23641.749	35091.181	90000.000	0.390
Concio 1	102	J[721]	ST FATICA DANN.ACC.	FZ-MIN	1.484	15995.296	23741.637	90000.000	0.264
Concio 1	103	I[721]	ST FATICA DANN.ACC.	MZ-MIN	1.484	12003.246	17816.283	90000.000	0.198
Concio 1	103	J[722]	ST FATICA DANN.ACC.	FZ-MIN	1.484	7352.018	10912.517	90000.000	0.121
Concio 2	104	I[722]	ST FATICA DANN.ACC.	FZ-MIN	1.484	9580.593	14220.366	90000.000	0.158
Concio 2	104	J[723]	ST FATICA DANN.ACC.	MY-MIN	1.484	9014.593	13380.259	90000.000	0.149

Concio 2	105	I[723]	ST FATICA DANN.ACC.	MY-MIN	1.484	7593.876	11271.504	90000.000	0.125
Concio 2	105	J[724]	ST FATICA DANN.ACC.	MZ-MIN	1.484	17840.275	26480.119	90000.000	0.294
Concio 2	106	I[724]	ST FATICA DANN.ACC.	FX-MIN	1.484	21887.336	32487.125	90000.000	0.361
Concio 2	106	J[725]	ST FATICA DANN.ACC.	FX-MIN	1.484	27865.903	41361.045	90000.000	0.460
Concio 3	107	I[725]	ST FATICA DANN.ACC.	FX-MIN	1.484	21137.950	31374.821	90000.000	0.349
Concio 3	107	J[726]	ST FATICA DANN.ACC.	MZ-MIN	1.484	24728.088	36703.622	90000.000	0.408
Concio 3	108	I[726]	ST FATICA DANN.ACC.	FZ-MAX	1.484	25527.312	37889.901	90000.000	0.421
Concio 3	108	J[727]	ST FATICA DANN.ACC.	MZ-MIN	1.484	34471.590	51165.792	90000.000	0.569
Concio 3	109	I[727]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32929.565	48876.982	90000.000	0.543
Concio 3	109	J[728]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33685.415	49998.882	90000.000	0.556
Concio 4 H=var	110	I[728]	ST FATICA DANN.ACC.	MX-MIN	1.484	33235.370	49330.884	90000.000	0.548
Concio 4 H=var	110	J[729]	ST FATICA DANN.ACC.	MZ-MIN	1.484	27576.661	40931.727	90000.000	0.455
Concio 4 H=var	111	I[729]	ST FATICA DANN.ACC.	FZ-MAX	1.484	25414.012	37721.731	90000.000	0.419
Concio 4 H=var	111	J[730]	ST FATICA DANN.ACC.	FZ-MAX	1.484	21474.381	31874.181	90000.000	0.354
Concio 4 H=200	112	I[730]	ST FATICA DANN.ACC.	FZ-MAX	1.484	21298.190	31612.663	90000.000	0.351
Concio 4 H=200	112	J[731]	ST FATICA DANN.ACC.	MZ-MIN	1.484	23300.617	34584.843	90000.000	0.384
Concio 4 H=200	113	I[731]	ST FATICA DANN.ACC.	FY-MIN	1.484	20283.947	30107.234	90000.000	0.335
Concio 4 H=200	113	J[732]	ST FATICA DANN.ACC.	FY-MIN	1.484	19855.420	29471.176	90000.000	0.328
Concio 4 H=var	114	I[732]	ST FATICA DANN.ACC.	FY-MIN	1.484	20151.650	29910.867	90000.000	0.332
Concio 4 H=var	114	J[733]	ST FATICA DANN.ACC.	FY-MIN	1.484	23675.021	35140.566	90000.000	0.391
Concio 4 H=var	115	I[733]	ST FATICA DANN.ACC.	MZ-MIN	1.484	23691.723	35165.357	90000.000	0.391
Concio 4 H=var	115	J[734]	ST FATICA DANN.ACC.	FZ-MIN	1.484	30203.731	44831.057	90000.000	0.498
Concio 5	116	I[734]	ST FATICA DANN.ACC.	FZ-MIN	1.484	30346.803	45043.417	90000.000	0.501
Concio 5	116	J[735]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29590.953	43921.517	90000.000	0.488
Concio 5	117	I[735]	ST FATICA DANN.ACC.	MZ-MIN	1.484	29414.074	43658.979	90000.000	0.485
Concio 5	117	J[736]	ST FATICA DANN.ACC.	FZ-MIN	1.484	21524.068	31947.931	90000.000	0.355
Concio 5	118	I[736]	ST FATICA DANN.ACC.	MZ-MIN	1.484	19177.843	28465.456	90000.000	0.316
Concio 5	118	J[737]	ST FATICA DANN.ACC.	FZ-MIN	1.484	15667.666	23255.339	90000.000	0.258
Concio 6	119	I[737]	ST FATICA DANN.ACC.	FZ-MIN	1.484	20490.975	30414.523	90000.000	0.338
Concio 6	119	J[738]	ST FATICA DANN.ACC.	FZ-MIN	1.484	14757.447	21904.312	90000.000	0.243
Concio 6	120	I[738]	ST FATICA DANN.ACC.	FZ-MIN	1.484	11195.995	16618.089	90000.000	0.185
Concio 6	120	J[739]	ST FATICA DANN.ACC.	MZ-MIN	1.484	6067.430	9005.817	90000.000	0.100
Concio 6	121	I[739]	ST FATICA DANN.ACC.	MZ-MIN	1.484	6067.430	9005.817	90000.000	0.100
Concio 6	121	J[740]	ST FATICA DANN.ACC.	MZ-MIN	1.484	10975.725	16291.144	90000.000	0.181
Concio 6	122	I[740]	ST FATICA DANN.ACC.	FX-MIN	1.484	12911.094	19163.791	90000.000	0.213
Concio 6	122	J[741]	ST FATICA DANN.ACC.	MZ-MIN	1.484	19530.055	28988.240	90000.000	0.322
Concio 5	123	I[741]	ST FATICA DANN.ACC.	MZ-MIN	1.484	14932.933	22164.784	90000.000	0.246
Concio 5	123	J[742]	ST FATICA DANN.ACC.	MZ-MIN	1.484	18902.183	28056.297	90000.000	0.312
Concio 5	124	I[742]	ST FATICA DANN.ACC.	FZ-MAX	1.484	18994.303	28193.029	90000.000	0.313
Concio 5	124	J[743]	ST FATICA DANN.ACC.	MZ-MIN	1.484	28746.784	42668.528	90000.000	0.474
Concio 5	125	I[743]	ST FATICA DANN.ACC.	FZ-MAX	1.484	26564.080	39428.765	90000.000	0.438
Concio 5	125	J[744]	ST FATICA DANN.ACC.	FZ-MAX	1.484	27319.930	40550.664	90000.000	0.451
Concio 4 H=var	126	I[744]	ST FATICA DANN.ACC.	FZ-MAX	1.484	27172.980	40332.548	90000.000	0.448
Concio 4 H=var	126	J[745]	ST FATICA DANN.ACC.	MZ-MIN	1.484	23394.279	34723.864	90000.000	0.386
Concio 4 H=var	127	I[745]	ST FATICA DANN.ACC.	FZ-MAX	1.484	21335.961	31668.726	90000.000	0.352
Concio 4 H=var	127	J[746]	ST FATICA DANN.ACC.	MZ-MIN	1.484	19978.841	29654.369	90000.000	0.330
Concio 4 H=200	128	I[746]	ST FATICA DANN.ACC.	MZ-MIN	1.484	19721.266	29272.052	90000.000	0.325
Concio 4 H=200	128	J[747]	ST FATICA DANN.ACC.	MZ-MIN	1.484	20145.974	29902.442	90000.000	0.332
Concio 4 H=200	129	I[747]	ST FATICA DANN.ACC.	FY-MIN	1.484	23444.180	34797.931	90000.000	0.387
Concio 4 H=200	129	J[748]	ST FATICA DANN.ACC.	FY-MIN	1.484	23015.652	34161.873	90000.000	0.380
Concio 4 H=var	130	I[748]	ST FATICA DANN.ACC.	FY-MIN	1.484	23256.677	34519.623	90000.000	0.384
Concio 4 H=var	130	J[749]	ST FATICA DANN.ACC.	FY-MIN	1.484	27757.192	41199.687	90000.000	0.458
Concio 4 H=var	131	I[749]	ST FATICA DANN.ACC.	MZ-MIN	1.484	27850.458	41338.120	90000.000	0.459
Concio 4 H=var	131	J[750]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36217.330	53756.975	90000.000	0.597
Concio 3	132	I[750]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36635.430	54377.555	90000.000	0.604
Concio 3	132	J[751]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35879.580	53255.655	90000.000	0.592
Concio 3	133	I[751]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35555.504	52774.633	90000.000	0.586
Concio 3	133	J[752]	ST FATICA DANN.ACC.	FZ-MIN	1.484	27997.004	41555.636	90000.000	0.462
Concio 3	134	I[752]	ST FATICA DANN.ACC.	MZ-MIN	1.484	25226.646	37443.625	90000.000	0.416



Concio 3	134	J[753]	ST FATICA DANN.ACC.	FZ-MIN	1.484	22148.097	32874.170	90000.000	0.365
Concio 2	135	I[753]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29197.567	43337.619	90000.000	0.482
Concio 2	135	J[754]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23219.000	34463.699	90000.000	0.383
Concio 2	136	I[754]	ST FATICA DANN.ACC.	MZ-MIN	1.484	18310.420	27177.950	90000.000	0.302
Concio 2	136	J[755]	ST FATICA DANN.ACC.	FZ-MIN	1.484	8320.009	12349.296	90000.000	0.137
Concio 2	137	I[755]	ST FATICA DANN.ACC.	FZ-MIN	1.484	9003.574	13363.903	90000.000	0.149
Concio 2	137	J[756]	ST FATICA DANN.ACC.	FX-MIN	1.484	8296.340	12314.164	90000.000	0.137
Concio 1	138	I[756]	ST FATICA DANN.ACC.	FX-MIN	1.484	6366.500	9449.724	90000.000	0.105
Concio 1	138	J[757]	ST FATICA DANN.ACC.	MZ-MIN	1.484	11646.365	17286.568	90000.000	0.192
Concio 1	139	I[757]	ST FATICA DANN.ACC.	FX-MIN	1.484	15023.899	22299.804	90000.000	0.248
Concio 1	139	J[758]	ST FATICA DANN.ACC.	FX-MIN	1.484	22670.353	33649.349	90000.000	0.374
Concio 1	140	I[758]	ST FATICA DANN.ACC.	FX-MAX	1.484	1223.433	1815.927	90000.000	0.020
Concio 1	140	J[759]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	201	I[760]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	201	J[761]	ST FATICA DANN.ACC.	FZ-MAX	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	202	I[761]	ST FATICA DANN.ACC.	FZ-MIN	1.484	39351.234	58408.593	90000.000	0.649
Concio 1	202	J[762]	ST FATICA DANN.ACC.	FY-MIN	1.484	13151.053	19519.959	90000.000	0.217
Concio 1	203	I[762]	ST FATICA DANN.ACC.	MZ-MIN	1.484	29166.089	43290.896	90000.000	0.481
Concio 1	203	J[763]	ST FATICA DANN.ACC.	FZ-MAX	1.484	13831.137	20529.400	90000.000	0.228
Concio 2	204	I[763]	ST FATICA DANN.ACC.	FZ-MAX	1.484	18029.294	26760.678	90000.000	0.297
Concio 2	204	J[764]	ST FATICA DANN.ACC.	FZ-MAX	1.484	29756.920	44167.860	90000.000	0.491
Concio 2	205	I[764]	ST FATICA DANN.ACC.	MZ-MIN	1.484	32109.085	47659.152	90000.000	0.530
Concio 2	205	J[765]	ST FATICA DANN.ACC.	MX-MIN	1.484	44071.828	65415.317	90000.000	0.727
Concio 2	206	I[765]	ST FATICA DANN.ACC.	MX-MIN	1.484	29566.993	43885.954	90000.000	0.488
Concio 2	206	J[766]	ST FATICA DANN.ACC.	MX-MIN	1.484	44842.848	66559.733	90000.000	0.740
Concio 3	207	I[766]	ST FATICA DANN.ACC.	MX-MIN	1.484	34033.931	50516.180	90000.000	0.561
Concio 3	207	J[767]	ST FATICA DANN.ACC.	MX-MIN	1.484	43060.062	63913.564	90000.000	0.710
Concio 3	208	I[767]	ST FATICA DANN.ACC.	FZ-MAX	1.484	31070.569	46117.695	90000.000	0.512
Concio 3	208	J[768]	ST FATICA DANN.ACC.	MX-MIN	1.484	53983.086	80126.486	90000.000	0.890
Concio 3	209	I[768]	ST FATICA DANN.ACC.	FZ-MAX	1.484	42459.160	63021.652	90000.000	0.700
Concio 3	209	J[769]	ST FATICA DANN.ACC.	FZ-MAX	1.484	44251.903	65682.599	90000.000	0.730
Concio 4 H=var	210	I[769]	ST FATICA DANN.ACC.	FZ-MAX	1.484	43485.303	64544.745	90000.000	0.717
Concio 4 H=var	210	J[770]	ST FATICA DANN.ACC.	MX-MIN	1.484	40243.200	59732.528	90000.000	0.664
Concio 4 H=var	211	I[770]	ST FATICA DANN.ACC.	FZ-MAX	1.484	34414.929	51081.691	90000.000	0.568
Concio 4 H=var	211	J[771]	ST FATICA DANN.ACC.	MX-MIN	1.484	34290.415	50896.876	90000.000	0.566
Concio 4 H=200	212	I[771]	ST FATICA DANN.ACC.	MX-MIN	1.484	34236.301	50816.554	90000.000	0.565
Concio 4 H=200	212	J[772]	ST FATICA DANN.ACC.	MX-MIN	1.484	35586.137	52820.102	90000.000	0.587
Concio 4 H=200	213	I[772]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29551.278	43862.628	90000.000	0.487
Concio 4 H=200	213	J[773]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28031.106	41606.254	90000.000	0.462
Concio 4 H=var	214	I[773]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28338.722	42062.845	90000.000	0.467
Concio 4 H=var	214	J[774]	ST FATICA DANN.ACC.	FX-MIN	1.484	28304.198	42011.602	90000.000	0.467
Concio 4 H=var	215	I[774]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32293.951	47933.547	90000.000	0.533
Concio 4 H=var	215	J[775]	ST FATICA DANN.ACC.	FX-MIN	1.484	38491.944	57133.159	90000.000	0.635
Concio 5	216	I[775]	ST FATICA DANN.ACC.	FX-MIN	1.484	39459.321	58569.024	90000.000	0.651
Concio 5	216	J[776]	ST FATICA DANN.ACC.	FX-MIN	1.484	38994.097	57878.498	90000.000	0.643
Concio 5	217	I[776]	ST FATICA DANN.ACC.	FZ-MIN	1.484	42394.931	62926.318	90000.000	0.699
Concio 5	217	J[777]	ST FATICA DANN.ACC.	FX-MIN	1.484	27283.070	40495.952	90000.000	0.450
Concio 5	218	I[777]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31015.714	46036.275	90000.000	0.512
Concio 5	218	J[778]	ST FATICA DANN.ACC.	FZ-MIN	1.484	19279.167	28615.850	90000.000	0.318
Concio 6	219	I[778]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25192.660	37393.181	90000.000	0.416
Concio 6	219	J[779]	ST FATICA DANN.ACC.	FZ-MAX	1.484	21850.673	32432.707	90000.000	0.360
Concio 6	220	I[779]	ST FATICA DANN.ACC.	MZ-MIN	1.484	35592.979	52830.257	90000.000	0.587
Concio 6	220	J[780]	ST FATICA DANN.ACC.	MX-MIN	1.484	21441.033	31824.683	90000.000	0.354
Concio 6	221	I[780]	ST FATICA DANN.ACC.	MX-MIN	1.484	21441.033	31824.683	90000.000	0.354
Concio 6	221	J[781]	ST FATICA DANN.ACC.	MX-MIN	1.484	36235.117	53783.375	90000.000	0.598
Concio 6	222	I[781]	ST FATICA DANN.ACC.	MX-MIN	1.484	20619.012	30604.567	90000.000	0.340
Concio 6	222	J[782]	ST FATICA DANN.ACC.	MX-MIN	1.484	35349.994	52469.596	90000.000	0.583
Concio 5	223	I[782]	ST FATICA DANN.ACC.	MX-MIN	1.484	27052.261	40153.366	90000.000	0.446
Concio 5	223	J[783]	ST FATICA DANN.ACC.	MX-MIN	1.484	37206.148	55224.665	90000.000	0.614

Concio 5	224	I[783]	ST FATICA DANN.ACC.	FZ-MAX	1.484	24857.011	36894.981	90000.000	0.410
Concio 5	224	J[784]	ST FATICA DANN.ACC.	MX-MIN	1.484	48378.155	71807.150	90000.000	0.798
Concio 5	225	I[784]	ST FATICA DANN.ACC.	FZ-MAX	1.484	36464.198	54123.398	90000.000	0.601
Concio 5	225	J[785]	ST FATICA DANN.ACC.	FZ-MAX	1.484	38248.603	56771.969	90000.000	0.631
Concio 4 H=var	226	I[785]	ST FATICA DANN.ACC.	FZ-MAX	1.484	37668.760	55911.315	90000.000	0.621
Concio 4 H=var	226	J[786]	ST FATICA DANN.ACC.	MX-MIN	1.484	36214.890	53753.352	90000.000	0.597
Concio 4 H=var	227	I[786]	ST FATICA DANN.ACC.	FZ-MAX	1.484	30265.626	44922.926	90000.000	0.499
Concio 4 H=var	227	J[787]	ST FATICA DANN.ACC.	MX-MIN	1.484	31125.665	46199.474	90000.000	0.513
Concio 4 H=200	228	I[787]	ST FATICA DANN.ACC.	MX-MIN	1.484	31047.039	46082.769	90000.000	0.512
Concio 4 H=200	228	J[788]	ST FATICA DANN.ACC.	MX-MIN	1.484	32394.926	48083.423	90000.000	0.534
Concio 4 H=200	229	I[788]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32751.709	48612.992	90000.000	0.540
Concio 4 H=200	229	J[789]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31229.194	46353.140	90000.000	0.515
Concio 4 H=var	230	I[789]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31507.263	46765.875	90000.000	0.520
Concio 4 H=var	230	J[790]	ST FATICA DANN.ACC.	FX-MIN	1.484	32524.497	48275.744	90000.000	0.536
Concio 4 H=var	231	I[790]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36336.027	53933.154	90000.000	0.599
Concio 4 H=var	231	J[791]	ST FATICA DANN.ACC.	FX-MIN	1.484	44315.277	65776.665	90000.000	0.731
Concio 3	232	I[791]	ST FATICA DANN.ACC.	FX-MIN	1.484	45588.671	67666.750	90000.000	0.752
Concio 3	232	J[792]	ST FATICA DANN.ACC.	FX-MIN	1.484	45123.447	66976.224	90000.000	0.744
Concio 3	233	I[792]	ST FATICA DANN.ACC.	FZ-MIN	1.484	48002.208	71249.135	90000.000	0.792
Concio 3	233	J[793]	ST FATICA DANN.ACC.	FX-MIN	1.484	33333.445	49476.455	90000.000	0.550
Concio 3	234	I[793]	ST FATICA DANN.ACC.	MZ-MIN	1.484	40304.523	59823.549	90000.000	0.665
Concio 3	234	J[794]	ST FATICA DANN.ACC.	FZ-MIN	1.484	26567.552	39433.917	90000.000	0.438
Concio 2	235	I[794]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35005.203	51957.828	90000.000	0.577
Concio 2	235	J[795]	ST FATICA DANN.ACC.	FY-MIN	1.484	26254.230	38968.857	90000.000	0.433
Concio 2	236	I[795]	ST FATICA DANN.ACC.	MZ-MIN	1.484	51450.478	76367.363	90000.000	0.849
Concio 2	236	J[796]	ST FATICA DANN.ACC.	MX-MIN	1.484	27024.038	40111.475	90000.000	0.446
Concio 2	237	I[796]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23401.280	34734.256	90000.000	0.386
Concio 2	237	J[797]	ST FATICA DANN.ACC.	MX-MIN	1.484	21467.170	31863.477	90000.000	0.354
Concio 1	238	I[797]	ST FATICA DANN.ACC.	MX-MIN	1.484	16468.496	24444.002	90000.000	0.272
Concio 1	238	J[798]	ST FATICA DANN.ACC.	MX-MIN	1.484	29948.881	44452.786	90000.000	0.494
Concio 1	239	I[798]	ST FATICA DANN.ACC.	FZ-MAX	1.484	16723.785	24822.925	90000.000	0.276
Concio 1	239	J[799]	ST FATICA DANN.ACC.	MX-MIN	1.484	43067.667	63924.851	90000.000	0.710
Concio 1	240	I[799]	ST FATICA DANN.ACC.	FZ-MIN	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	240	J[800]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	301	I[801]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	301	J[802]	ST FATICA DANN.ACC.	FZ-MAX	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	302	I[802]	ST FATICA DANN.ACC.	FZ-MIN	1.484	39351.234	58408.593	90000.000	0.649
Concio 1	302	J[803]	ST FATICA DANN.ACC.	MX-MIN	1.484	14199.759	21076.541	90000.000	0.234
Concio 1	303	I[803]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23923.935	35510.026	90000.000	0.395
Concio 1	303	J[804]	ST FATICA DANN.ACC.	FZ-MAX	1.484	13831.137	20529.400	90000.000	0.228
Concio 2	304	I[804]	ST FATICA DANN.ACC.	FZ-MAX	1.484	18029.294	26760.678	90000.000	0.297
Concio 2	304	J[805]	ST FATICA DANN.ACC.	FZ-MAX	1.484	29756.918	44167.857	90000.000	0.491
Concio 2	305	I[805]	ST FATICA DANN.ACC.	MZ-MAX	1.484	20551.386	30504.191	90000.000	0.339
Concio 2	305	J[806]	ST FATICA DANN.ACC.	FZ-MAX	1.484	41624.076	61782.146	90000.000	0.687
Concio 2	306	I[806]	ST FATICA DANN.ACC.	FZ-MAX	1.484	28213.919	41877.602	90000.000	0.465
Concio 2	306	J[807]	ST FATICA DANN.ACC.	FZ-MAX	1.484	42447.454	63004.277	90000.000	0.700
Concio 3	307	I[807]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32215.923	47817.731	90000.000	0.531
Concio 3	307	J[808]	ST FATICA DANN.ACC.	FZ-MAX	1.484	40419.622	59994.389	90000.000	0.667
Concio 3	308	I[808]	ST FATICA DANN.ACC.	FZ-MAX	1.484	31070.570	46117.696	90000.000	0.512
Concio 3	308	J[809]	ST FATICA DANN.ACC.	FZ-MAX	1.484	50452.246	74885.699	90000.000	0.832
Concio 3	309	I[809]	ST FATICA DANN.ACC.	FZ-MAX	1.484	42459.160	63021.652	90000.000	0.700
Concio 3	309	J[810]	ST FATICA DANN.ACC.	FZ-MAX	1.484	44251.903	65682.599	90000.000	0.730
Concio 4 H=var	310	I[810]	ST FATICA DANN.ACC.	FZ-MAX	1.484	43485.303	64544.745	90000.000	0.717
Concio 4 H=var	310	J[811]	ST FATICA DANN.ACC.	FZ-MAX	1.484	37757.164	56042.532	90000.000	0.623
Concio 4 H=var	311	I[811]	ST FATICA DANN.ACC.	FZ-MAX	1.484	34414.929	51081.691	90000.000	0.568
Concio 4 H=var	311	J[812]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32212.283	47812.329	90000.000	0.531
Concio 4 H=200	312	I[812]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32020.766	47528.062	90000.000	0.528
Concio 4 H=200	312	J[813]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33238.429	49335.425	90000.000	0.548
Concio 4 H=200	313	I[813]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29551.278	43862.628	90000.000	0.487



Concio 4 H=200	313	J[814]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28031.106	41606.254	90000.000	0.462
Concio 4 H=var	314	I[814]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28338.720	42062.843	90000.000	0.467
Concio 4 H=var	314	J[815]	ST FATICA DANN.ACC.	FX-MIN	1.484	28304.198	42011.602	90000.000	0.467
Concio 4 H=var	315	I[815]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32293.950	47933.545	90000.000	0.533
Concio 4 H=var	315	J[816]	ST FATICA DANN.ACC.	FX-MIN	1.484	38491.946	57133.161	90000.000	0.635
Concio 5	316	I[816]	ST FATICA DANN.ACC.	FX-MIN	1.484	39459.321	58569.024	90000.000	0.651
Concio 5	316	J[817]	ST FATICA DANN.ACC.	FX-MIN	1.484	38994.097	57878.498	90000.000	0.643
Concio 5	317	I[817]	ST FATICA DANN.ACC.	FZ-MIN	1.484	42394.931	62926.318	90000.000	0.699
Concio 5	317	J[818]	ST FATICA DANN.ACC.	FX-MIN	1.484	27283.070	40495.952	90000.000	0.450
Concio 5	318	I[818]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31015.714	46036.275	90000.000	0.512
Concio 5	318	J[819]	ST FATICA DANN.ACC.	FZ-MIN	1.484	19279.166	28615.849	90000.000	0.318
Concio 6	319	I[819]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25192.659	37393.179	90000.000	0.416
Concio 6	319	J[820]	ST FATICA DANN.ACC.	MX-MIN	1.484	25100.263	37256.037	90000.000	0.414
Concio 6	320	I[820]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28095.279	41701.505	90000.000	0.463
Concio 6	320	J[821]	ST FATICA DANN.ACC.	FZ-MAX	1.484	19918.222	29564.392	90000.000	0.329
Concio 6	321	I[821]	ST FATICA DANN.ACC.	FZ-MAX	1.484	19918.222	29564.392	90000.000	0.329
Concio 6	321	J[822]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33506.623	49733.502	90000.000	0.553
Concio 6	322	I[822]	ST FATICA DANN.ACC.	FZ-MAX	1.484	19804.561	29395.686	90000.000	0.327
Concio 6	322	J[823]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32937.617	48888.933	90000.000	0.543
Concio 5	323	I[823]	ST FATICA DANN.ACC.	FZ-MAX	1.484	25206.144	37413.195	90000.000	0.416
Concio 5	323	J[824]	ST FATICA DANN.ACC.	FZ-MAX	1.484	34485.362	51186.234	90000.000	0.569
Concio 5	324	I[824]	ST FATICA DANN.ACC.	FZ-MAX	1.484	24857.011	36894.981	90000.000	0.410
Concio 5	324	J[825]	ST FATICA DANN.ACC.	FZ-MAX	1.484	44790.403	66481.889	90000.000	0.739
Concio 5	325	I[825]	ST FATICA DANN.ACC.	FZ-MAX	1.484	36464.198	54123.398	90000.000	0.601
Concio 5	325	J[826]	ST FATICA DANN.ACC.	FZ-MAX	1.484	38248.603	56771.969	90000.000	0.631
Concio 4 H=var	326	I[826]	ST FATICA DANN.ACC.	FZ-MAX	1.484	37668.756	55911.309	90000.000	0.621
Concio 4 H=var	326	J[827]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33714.518	50042.078	90000.000	0.556
Concio 4 H=var	327	I[827]	ST FATICA DANN.ACC.	FZ-MAX	1.484	30265.626	44922.926	90000.000	0.499
Concio 4 H=var	327	J[828]	ST FATICA DANN.ACC.	FZ-MAX	1.484	29043.602	43109.091	90000.000	0.479
Concio 4 H=200	328	I[828]	ST FATICA DANN.ACC.	FZ-MAX	1.484	28822.240	42780.525	90000.000	0.475
Concio 4 H=200	328	J[829]	ST FATICA DANN.ACC.	FZ-MAX	1.484	30038.013	44585.084	90000.000	0.495
Concio 4 H=200	329	I[829]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32751.711	48612.995	90000.000	0.540
Concio 4 H=200	329	J[830]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31229.194	46353.140	90000.000	0.515
Concio 4 H=var	330	I[830]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31507.263	46765.875	90000.000	0.520
Concio 4 H=var	330	J[831]	ST FATICA DANN.ACC.	FX-MIN	1.484	32524.497	48275.744	90000.000	0.536
Concio 4 H=var	331	I[831]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36336.027	53933.154	90000.000	0.599
Concio 4 H=var	331	J[832]	ST FATICA DANN.ACC.	FX-MIN	1.484	44315.277	65776.665	90000.000	0.731
Concio 3	332	I[832]	ST FATICA DANN.ACC.	FX-MIN	1.484	45588.670	67666.748	90000.000	0.752
Concio 3	332	J[833]	ST FATICA DANN.ACC.	FX-MIN	1.484	45123.446	66976.221	90000.000	0.744
Concio 3	333	I[833]	ST FATICA DANN.ACC.	FZ-MIN	1.484	48002.208	71249.135	90000.000	0.792
Concio 3	333	J[834]	ST FATICA DANN.ACC.	FX-MIN	1.484	33333.443	49476.453	90000.000	0.550
Concio 3	334	I[834]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36910.673	54786.095	90000.000	0.609
Concio 3	334	J[835]	ST FATICA DANN.ACC.	FZ-MIN	1.484	26567.552	39433.917	90000.000	0.438
Concio 2	335	I[835]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35005.203	51957.828	90000.000	0.577
Concio 2	335	J[836]	ST FATICA DANN.ACC.	FY-MAX	1.484	22268.587	33053.012	90000.000	0.367
Concio 2	336	I[836]	ST FATICA DANN.ACC.	MZ-MAX	1.484	39288.762	58315.865	90000.000	0.648
Concio 2	336	J[837]	ST FATICA DANN.ACC.	FZ-MAX	1.484	24430.989	36262.641	90000.000	0.403
Concio 2	337	I[837]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23401.280	34734.256	90000.000	0.386
Concio 2	337	J[838]	ST FATICA DANN.ACC.	FZ-MAX	1.484	19470.440	28899.755	90000.000	0.321
Concio 1	338	I[838]	ST FATICA DANN.ACC.	FZ-MAX	1.484	14936.709	22170.388	90000.000	0.246
Concio 1	338	J[839]	ST FATICA DANN.ACC.	FZ-MAX	1.484	27363.958	40616.014	90000.000	0.451
Concio 1	339	I[839]	ST FATICA DANN.ACC.	FZ-MAX	1.484	16723.785	24822.925	90000.000	0.276
Concio 1	339	J[840]	ST FATICA DANN.ACC.	FZ-MAX	1.484	39727.601	58967.230	90000.000	0.655
Concio 1	340	I[840]	ST FATICA DANN.ACC.	FZ-MIN	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	340	J[841]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	401	I[842]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	401	J[843]	ST FATICA DANN.ACC.	FX-MAX	1.484	1223.433	1815.927	90000.000	0.020
Concio 1	402	I[843]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23641.749	35091.181	90000.000	0.390
Concio 1	402	J[844]	ST FATICA DANN.ACC.	FZ-MIN	1.484	15995.296	23741.637	90000.000	0.264

Concio 1	403	I[844]	ST FATICA DANN.ACC.	FZ-MIN	1.484	11939.890	17722.244	90000.000	0.197
Concio 1	403	J[845]	ST FATICA DANN.ACC.	FZ-MIN	1.484	7352.018	10912.517	90000.000	0.121
Concio 2	404	I[845]	ST FATICA DANN.ACC.	FZ-MIN	1.484	9580.593	14220.366	90000.000	0.158
Concio 2	404	J[846]	ST FATICA DANN.ACC.	MY-MIN	1.484	9014.593	13380.259	90000.000	0.149
Concio 2	405	I[846]	ST FATICA DANN.ACC.	FY-MIN	1.484	7990.619	11860.385	90000.000	0.132
Concio 2	405	J[847]	ST FATICA DANN.ACC.	FY-MIN	1.484	17954.898	26650.252	90000.000	0.296
Concio 2	406	I[847]	ST FATICA DANN.ACC.	MZ-MIN	1.484	22980.185	34109.229	90000.000	0.379
Concio 2	406	J[848]	ST FATICA DANN.ACC.	FY-MIN	1.484	28916.701	42920.733	90000.000	0.477
Concio 3	407	I[848]	ST FATICA DANN.ACC.	FY-MIN	1.484	21935.043	32557.937	90000.000	0.362
Concio 3	407	J[849]	ST FATICA DANN.ACC.	FY-MIN	1.484	24958.443	37045.536	90000.000	0.412
Concio 3	408	I[849]	ST FATICA DANN.ACC.	FY-MIN	1.484	27853.819	41343.108	90000.000	0.459
Concio 3	408	J[850]	ST FATICA DANN.ACC.	FY-MIN	1.484	35412.319	52562.105	90000.000	0.584
Concio 3	409	I[850]	ST FATICA DANN.ACC.	FY-MIN	1.484	35231.017	52293.000	90000.000	0.581
Concio 3	409	J[851]	ST FATICA DANN.ACC.	FY-MIN	1.484	35986.867	53414.900	90000.000	0.594
Concio 4 H=var	410	I[851]	ST FATICA DANN.ACC.	FY-MIN	1.484	35411.251	52560.520	90000.000	0.584
Concio 4 H=var	410	J[852]	ST FATICA DANN.ACC.	FY-MIN	1.484	27233.884	40422.946	90000.000	0.449
Concio 4 H=var	411	I[852]	ST FATICA DANN.ACC.	FY-MIN	1.484	27424.030	40705.178	90000.000	0.452
Concio 4 H=var	411	J[853]	ST FATICA DANN.ACC.	FY-MIN	1.484	23003.263	34143.484	90000.000	0.379
Concio 4 H=200	412	I[853]	ST FATICA DANN.ACC.	FY-MIN	1.484	22805.826	33850.430	90000.000	0.376
Concio 4 H=200	412	J[854]	ST FATICA DANN.ACC.	FY-MIN	1.484	23234.353	34486.488	90000.000	0.383
Concio 4 H=200	413	I[854]	ST FATICA DANN.ACC.	FZ-MIN	1.484	20261.879	30074.479	90000.000	0.334
Concio 4 H=200	413	J[855]	ST FATICA DANN.ACC.	FZ-MIN	1.484	19833.352	29438.421	90000.000	0.327
Concio 4 H=var	414	I[855]	ST FATICA DANN.ACC.	FZ-MIN	1.484	20130.288	29879.159	90000.000	0.332
Concio 4 H=var	414	J[856]	ST FATICA DANN.ACC.	MZ-MIN	1.484	23654.026	35109.403	90000.000	0.390
Concio 4 H=var	415	I[856]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23643.509	35093.793	90000.000	0.390
Concio 4 H=var	415	J[857]	ST FATICA DANN.ACC.	FZ-MIN	1.484	30203.731	44831.057	90000.000	0.498
Concio 5	416	I[857]	ST FATICA DANN.ACC.	FZ-MIN	1.484	30346.803	45043.417	90000.000	0.501
Concio 5	416	J[858]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29590.953	43921.517	90000.000	0.488
Concio 5	417	I[858]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29384.910	43615.690	90000.000	0.485
Concio 5	417	J[859]	ST FATICA DANN.ACC.	FZ-MIN	1.484	21524.070	31947.934	90000.000	0.355
Concio 5	418	I[859]	ST FATICA DANN.ACC.	FZ-MIN	1.484	19144.576	28416.077	90000.000	0.316
Concio 5	418	J[860]	ST FATICA DANN.ACC.	FZ-MIN	1.484	15667.666	23255.339	90000.000	0.258
Concio 6	419	I[860]	ST FATICA DANN.ACC.	FZ-MIN	1.484	20490.975	30414.523	90000.000	0.338
Concio 6	419	J[861]	ST FATICA DANN.ACC.	FZ-MIN	1.484	14757.447	21904.312	90000.000	0.243
Concio 6	420	I[861]	ST FATICA DANN.ACC.	FZ-MIN	1.484	11195.995	16618.089	90000.000	0.185
Concio 6	420	J[862]	ST FATICA DANN.ACC.	FZ-MIN	1.484	6055.590	8988.244	90000.000	0.100
Concio 6	421	I[862]	ST FATICA DANN.ACC.	FZ-MIN	1.484	6055.590	8988.244	90000.000	0.100
Concio 6	421	J[863]	ST FATICA DANN.ACC.	FY-MIN	1.484	10950.415	16253.577	90000.000	0.181
Concio 6	422	I[863]	ST FATICA DANN.ACC.	FY-MIN	1.484	14555.477	21604.531	90000.000	0.240
Concio 6	422	J[864]	ST FATICA DANN.ACC.	FY-MIN	1.484	20289.006	30114.742	90000.000	0.335
Concio 5	423	I[864]	ST FATICA DANN.ACC.	FY-MIN	1.484	15513.237	23026.122	90000.000	0.256
Concio 5	423	J[865]	ST FATICA DANN.ACC.	FY-MIN	1.484	18990.147	28186.861	90000.000	0.313
Concio 5	424	I[865]	ST FATICA DANN.ACC.	FY-MIN	1.484	21367.663	31715.781	90000.000	0.352
Concio 5	424	J[866]	ST FATICA DANN.ACC.	FY-MIN	1.484	29228.503	43383.537	90000.000	0.482
Concio 5	425	I[866]	ST FATICA DANN.ACC.	FY-MIN	1.484	28845.648	42815.270	90000.000	0.476
Concio 5	425	J[867]	ST FATICA DANN.ACC.	FY-MIN	1.484	29601.498	43937.169	90000.000	0.488
Concio 4 H=var	426	I[867]	ST FATICA DANN.ACC.	FY-MIN	1.484	29372.726	43597.606	90000.000	0.484
Concio 4 H=var	426	J[868]	ST FATICA DANN.ACC.	FY-MIN	1.484	23070.564	34243.378	90000.000	0.381
Concio 4 H=var	427	I[868]	ST FATICA DANN.ACC.	FY-MIN	1.484	23348.691	34656.198	90000.000	0.385
Concio 4 H=var	427	J[869]	ST FATICA DANN.ACC.	FY-MIN	1.484	19903.434	29542.442	90000.000	0.328
Concio 4 H=200	428	I[869]	ST FATICA DANN.ACC.	FY-MIN	1.484	19655.131	29173.890	90000.000	0.324
Concio 4 H=200	428	J[870]	ST FATICA DANN.ACC.	FY-MIN	1.484	20083.659	29809.948	90000.000	0.331
Concio 4 H=200	429	I[870]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23393.002	34721.968	90000.000	0.386
Concio 4 H=200	429	J[871]	ST FATICA DANN.ACC.	FZ-MIN	1.484	22964.475	34085.910	90000.000	0.379
Concio 4 H=var	430	I[871]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23217.425	34461.363	90000.000	0.383
Concio 4 H=var	430	J[872]	ST FATICA DANN.ACC.	MZ-MIN	1.484	27737.970	41171.155	90000.000	0.458
Concio 4 H=var	431	I[872]	ST FATICA DANN.ACC.	FZ-MIN	1.484	27789.645	41247.856	90000.000	0.458
Concio 4 H=var	431	J[873]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36217.332	53756.977	90000.000	0.597
Concio 3	432	I[873]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36635.430	54377.555	90000.000	0.604

Concio 3	432	J[874]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35879.580	53255.655	90000.000	0.592
Concio 3	433	I[874]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35555.504	52774.633	90000.000	0.586
Concio 3	433	J[875]	ST FATICA DANN.ACC.	FZ-MIN	1.484	27997.004	41555.636	90000.000	0.462
Concio 3	434	I[875]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25171.497	37361.769	90000.000	0.415
Concio 3	434	J[876]	ST FATICA DANN.ACC.	FZ-MIN	1.484	22148.097	32874.170	90000.000	0.365
Concio 2	435	I[876]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29197.567	43337.619	90000.000	0.482
Concio 2	435	J[877]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23219.000	34463.699	90000.000	0.383
Concio 2	436	I[877]	ST FATICA DANN.ACC.	FZ-MIN	1.484	18284.288	27139.163	90000.000	0.302
Concio 2	436	J[878]	ST FATICA DANN.ACC.	FZ-MIN	1.484	8320.009	12349.296	90000.000	0.137
Concio 2	437	I[878]	ST FATICA DANN.ACC.	FZ-MIN	1.484	9003.574	13363.903	90000.000	0.149
Concio 2	437	J[879]	ST FATICA DANN.ACC.	FY-MIN	1.484	9345.564	13871.515	90000.000	0.154
Concio 1	438	I[879]	ST FATICA DANN.ACC.	FY-MIN	1.484	7171.660	10644.814	90000.000	0.118
Concio 1	438	J[880]	ST FATICA DANN.ACC.	FY-MIN	1.484	11759.532	17454.541	90000.000	0.194
Concio 1	439	I[880]	ST FATICA DANN.ACC.	FY-MIN	1.484	15742.800	23366.860	90000.000	0.260
Concio 1	439	J[881]	ST FATICA DANN.ACC.	FY-MIN	1.484	23389.254	34716.405	90000.000	0.386
Concio 1	440	I[881]	ST FATICA DANN.ACC.	FX-MAX	1.484	1223.433	1815.927	90000.000	0.020
Concio 1	440	J[882]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000

## 10 VERIFICHE DI RESISTENZA DEI PROFILATI IN ACCIAIO ALLO S.L.U.

Si effettuano le verifiche di resistenza dei diaframmi e dei controventi allo stato limite ultimo; seguono gli schemi grafici e i tabulati di calcolo degli elementi maggiormente sollecitati.

### 10.1 DIAFRAMMI SU SPALLE E INTEREDI – TRASVERSI INFERIORI

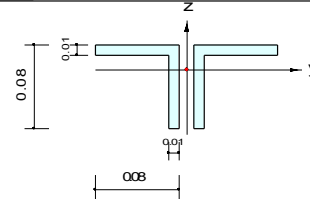
#### MIDAS/Civil

#### Steel Checking Result

	<b>Company</b>	<b>Project Title</b>	Viadotto Agrò
	<b>Author</b>	<b>File Name</b>	D:\...\Cavalc Coste V_1.mob
	Renato Vaira		

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 1110  
 Material : S355W (No:1)  
                   (Fy = 355000, Es = 210000000)  
 Section Name : HINF\_CORR 2L80x10d15 (No:18)  
                   (Built-up Section).  
 Member Length : 1.60000



#### 2. Member Forces

Axial Force            Fxx = 542.434 (LCB: 19+, POS:I)  
 Bending Moments        My = 1.22630, Mz = 1.00226  
 End Moments            Myi = 1.22630, Myj = 1.67445 (for Lb)  
                               Myi = 1.22630, Myj = 1.67445 (for Ly)  
                               Mzi = 1.00226, Mzj = 0.25446 (for Lz)  
 Shear Forces           Fyy = 0.78557 (LCB: 19+, POS:I)  
                               Fzz = -3.0613 (LCB: 19-, POS:J)

Depth	0.08000	Web Thick	0.01000
Flg Width	0.08000	Flg Thick	0.01000
BTB Spacing	0.01500		
Area	0.00300	Asz	0.00133
Cyb	0.00159	Czb	0.00320
Iyy	0.00000	Izz	0.00000
Ybar	0.08750	Zbar	0.05633
Wely	0.00003	Welz	0.00005
ry	0.02436	rz	0.03855

#### 3. Design Parameters

Unbraced Lengths            Ly = 1.60000, Lz = 1.60000, Lb = 1.60000  
 Effective Length Factors      Ky = 0.65, Kz = 1.30  
 Equivalent Uniform Moment Factors    Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

Axial Resistance  
 $N_{Ed}/N_{t,Rd} = 542.43/1014.29 = 0.535 < 1.000$  ..... O.K

Bending Resistance  
 $M_{Edy}/M_{Rdy} = 1.2263/19.2503 = 0.064 < 1.000$  ..... O.K  
 $M_{Edz}/M_{Rdz} = 1.0023/31.6119 = 0.032 < 1.000$  ..... O.K

Combined Resistance  
 $R_{NRd} = \text{MAX}[ M_{Edy}/M_{Ny\_Rd}, M_{Edz}/M_{Nz\_Rd} ]$   
 $R_{max1} = (M_{Edy}/M_{Ny\_Rd})^{\text{Alpha}} + (M_{Edz}/M_{Nz\_Rd})^{\text{Beta}}$   
 $R_{00m} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{Ny\_Rd} + M_{Edz}/M_{Nz\_Rd}$   
 $R_{max} = \text{MAX}[ R_{NRd}, R_{max1}, (R_{00m} + R_{bend}) ] = 0.630 < 1.000$  ..... O.K

Shear Resistance  
 $V_{Edy}/V_{y,Rd} = 0.003 < 1.000$  ..... O.K  
 $V_{Edz}/V_{z,Rd} = 0.010 < 1.000$  ..... O.K





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- 
- 23+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 23- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 24+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 24- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 25+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 25- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 28+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 28- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 29+ 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 29- 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30+ 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
          +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30- 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
          +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 31+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 31- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 32+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          +Mobili SLE INV( 1.000)
  - 32- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          +Mobili SLE INV( 1.000)
  - 33+ 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
          +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 33- 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
          +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 34+ 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 34- 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 35+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 35- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
-



-----  
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 =====

\*. PROJECT : Viadotto Agrò  
 \*. MEMBER NO = 1110, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19+, MATERIAL NO = 1, SECTION NO = 18  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = HINF\_CORR 2L80x10d15  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.015  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.00000e-003, Avy = 1.60000e-003, Avz = 1.60000e-003  
 Ybar = 8.75000e-002, Zbar = 5.63333e-002, Qyb = 1.58672e-003, Qzb = 3.20000e-003  
 Wely = 3.15917e-005, Welz = 5.36429e-005, Wply = 5.69375e-005, Wplz = 9.35000e-005  
 Iyy = 1.77967e-006, Izz = 4.69375e-006, Iyz = 0.00000e+000  
 iy = 2.43562e-002, iz = 3.95548e-002  
 J = 1.00000e-007, Cwp = 4.24931e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 1.60000e+000, Lz = 1.60000e+000, Lu = 1.60000e+000  
 Ky = 6.50000e-001, Kz = 1.30000e+000

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (I) POINT :  
 Axial Force Fxx = 5.42434e+002  
 Shear Forces Fyy = 7.85567e-001, Fzz = 1.62825e+000  
 Bending Moments My = 1.22630e+000, Mz = 1.00226e+000  
 End Moments Myi = 1.22630e+000, Myj = 1.67445e+000 (for Lb)  
                   Myi = 1.22630e+000, Myj = 1.67445e+000 (for Ly)  
                   Mzi = 1.00226e+000, Mzj = 2.54462e-001 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of tension flanges(double angle).  
 - Not Checking the Section Classification.

( ). Determine classification of tension flanges(double angle).  
 - Not Checking the Section Classification.

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

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- ( ). Determine classification of tension elements(Double angle).
- Not Checking the Section Classification.

[[[\*]]] APPLIED FACTORS.

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).
- [ Eurocode3:05 Annex A. Table A.1, A.2 ]
- Cmy,0 = 0.841
- Cmz,0 = 0.759
- Cmy (Default or User Defined Value) = 1.000
- Cmz (Default or User Defined Value) = 1.000
- CmLT (Default or User Defined Value) = 1.000

- ( ). Partial Factors (Gamma\_Mi).
- [ Eurocode3:05 6.1 ]
- Gamma\_M0 = 1.05
- Gamma\_M1 = 1.10
- Gamma\_M2 = 1.25

[[[\*]]] CHECK AXIAL RESISTANCE.

- ( ). Check slenderness ratio of axial tension member (l/i).
- [ Eurocode3:05 6.3.1 ]
- l/i = 65.7 < 300.0 ----> O.K.

- ( ). Calculate parameters for combined resistance.
- Lambda1 = Pi \* SQRT(Es/fy) = 76.409
- Lambda\_bz = (KLz/iz) / Lambda1 = 0.529

- ( ). Calculate axial tensile resistance (Nt\_Rd).
- [ Eurocode3:05 6.2.3 ]
- Nt\_Rd = fy \* Area / Gamma\_M0 = 1014.29 kN.

- ( ). Check ratio of axial resistance (N\_Ed/Nt\_Rd).
- |       |         |
|-------|---------|
| N_Ed  | 542.43  |
| -     | -----   |
| Nt_Rd | 1014.29 |
- = 0.535 < 1.000 ----> O.K.

[[[\*]]] CHECK SHEAR RESISTANCE.

- ( ). Calculate shear area.
- [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]
- Avy = 2\*B\*tf = 0.0016 m^2.
- Avz = 2\*h\*tw = 0.0016 m^2.

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( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 - Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 =     312.32 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19+, POS = I )  
 - Applied shear force : V\_Edy =     0.79 kN.  
   V\_Edy                0.79  
 - ----- = ----- = 0.003 < 1.000 ----> O.K.  
   Vpl\_Rdy            312.32

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 - Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 =     312.32 kN.

( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 - HTR < 72\*e/Eta ----> No need to check!

( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19-, POS = J )  
 - Applied shear force : V\_Edz =     3.06 kN.  
   V\_Edz                3.06  
 - ----- = ----- = 0.010 < 1.000 ----> O.K.  
   Vpl\_Rdz            312.32

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 - Wply = 5.6938e-005 m^3.  
 - Mc\_Rdy = Wply \* fy / Gamma\_M0 =     19.25 kN-m.

( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  
   M\_Edy                1.23  
 - ----- = ----- = 0.064 < 1.000 ----> O.K.  
   Mc\_Rdy             19.25

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 - Wplz = 9.3500e-005 m^3.  
 - Mc\_Rdz = Wplz \* fy / Gamma\_M0 =     31.61 kN-m.

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( ). Check ratio of moment resistance ( $M_{Edz}/M_{c\_Rdz}$ ).

$$\frac{M_{Edz}}{M_{c\_Rdz}} = \frac{1.00}{31.61} = 0.032 < 1.000 \text{ ---> O.K.}$$

[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.

( ). Calculate Major reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]  
 - In case of  $V_{Edz} / V_{pl\_Rdz} < 0.5$   
 -  $M_{y\_Rd} = M_{c\_Rdy} = 19.25 \text{ kN-m.}$

( ). Calculate Minor reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]  
 - In case of  $V_{Edy} / V_{pl\_Rdy} < 0.5$   
 -  $M_{z\_Rd} = M_{c\_Rdz} = 31.61 \text{ kN-m.}$

( ). Check general interaction ratio.

[ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2

$$R_{max1} = \frac{N_{Ed}}{N_{Rd}} + \frac{M_{Edy}}{M_{y\_Rd}} + \frac{M_{Edz}}{M_{z\_Rd}}$$

= 0.630 < 1.000 ---> O.K.

( ). Check interaction ratio of bending and axial force member.

[ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2

-  $n = N_{Ed} / N_{pl\_Rd} = 0.535$   
 -  $a = \text{MIN}[(\text{Area}-2b*tf)/\text{Area}, 0.5] = 0.500$   
 -  $\text{Alpha} = 2.000$   
 -  $\text{Beta} = \text{MAX}[5*n, 1.0] = 2.674$   
 -  $M_{ny\_Rd} = \text{MIN}[M_{ply\_Rd}*(1-n)/(1-0.5*a), M_{ply\_Rd}] = 11.94 \text{ kN-m.}$   
 -  $R_{maxy} = M_{Edy} / M_{ny\_Rd} = 0.103 < 1.000 \text{ ---> O.K.}$

- In case of  $n > a$   
 -  $M_{nz\_Rd} = M_{plz\_Rd} * [1 - ((n-a)/(1-a))^2] = 31.46 \text{ kN-m.}$   
 -  $R_{maxz} = M_{Edz} / M_{nz\_Rd} = 0.032 < 1.000 \text{ ---> O.K.}$

$$R_{max2} = \left[ \frac{M_{Edy}}{M_{ny\_Rd}} \right]^{\text{Alpha}} + \left[ \frac{M_{Edz}}{M_{nz\_Rd}} \right]^{\text{Beta}}$$

= 0.011 < 1.000 ---> O.K.

-  $R_{max} = \text{MAX}[R_{max1}, R_{max2}] = 0.630 < 1.000 \text{ ---> O.K.}$

## 10.2 DIAFRAMMI SU SPALLE E INTEREDI – TRASVERSI SUPERIORI

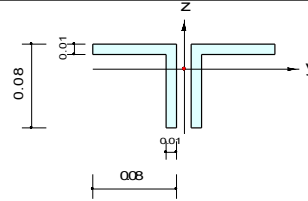
**MIDAS/Civil**

### Steel Checking Result

<b>Company</b>		<b>Project Title</b>	Viadotto Agrò
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...\Cavalc Coste V_1.mcb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 1439  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : HSUP\_CORR 2L80x10d15 (No:17)  
 (Built-up Section).  
 Member Length : 3.20000



#### 2. Member Forces

Axial Force Fxx = -208.47 (LCB: 22-, POS:I)  
 Bending Moments My = -0.6696, Mz = -0.0626  
 End Moments Myi = -0.6696, Myj = -0.6646 (for Lb)  
 Myi = -0.6696, Myj = -0.6646 (for Ly)  
 Mzi = -0.0626, Mzj = -0.0626 (for Lz)  
 Shear Forces Fyy = 0.05561 (LCB: 19+, POS:I)  
 Fzz = 0.93623 (LCB: 19+, POS:J)

Depth	0.08000	Web Thick	0.01000
Flg Width	0.08000	Flg Thick	0.01000
BTB Spacing	0.01500		
Area	0.00300	Asz	0.00133
Cyb	0.00159	Czb	0.00320
Iyy	0.00000	Izz	0.00000
Ybar	0.08750	Zbar	0.05633
Wely	0.00003	Welz	0.00005
ry	0.02436	rz	0.03955

#### 3. Design Parameters

Unbraced Lengths Ly = 3.20000, Lz = 3.20000, Lb = 3.20000  
 Effective Length Factors Ky = 0.65, Kz = 0.65  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c\_Rd}, N_{b\_Rd}] = 208.466/508.054 = 0.410 < 1.000 \dots\dots\dots \text{O.K}$$

Bending Resistance

$$M_{Edy}/M_{Rdy} = 0.6696/19.2503 = 0.035 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 0.0626/31.6119 = 0.002 < 1.000 \dots\dots\dots \text{O.K}$$

Combined Resistance

$$R_{NRd} = \text{MAX}[M_{Edy}/M_{ny\_Rd}, M_{Edz}/M_{nz\_Rd}]$$

$$R_{max1} = (M_{Edy}/M_{ny\_Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz\_Rd})^{\text{Beta}}$$

$$R_{\text{bend}} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{\text{bend}} = M_{Edy}/M_{y\_Rd} + M_{Edz}/M_{z\_Rd}$$

$$R_{c\_LT1} = N_{Ed}/(X_{iy} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT1} = (k_{yy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (k_{yz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{c\_LT2} = N_{Ed}/(X_{iz} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT2} = (K_{zy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (K_{zz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{max} = \text{MAX}[R_{NRd}, R_{max1}, (R_{\text{bend}} + R_{\text{bend}}), \text{MAX}(R_{c\_LT1} + R_{b\_LT1}, R_{c\_LT2} + R_{b\_LT2})] = 0.469 < 1.000 \dots\dots \text{O.K}$$

Shear Resistance

$$V_{Edy}/V_{y\_Rd} = 0.000 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z\_Rd} = 0.003 < 1.000 \dots\dots\dots \text{O.K}$$





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 =====

- 23+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 23- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 24+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 24- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 25+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 25- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 28+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 28- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 29+ 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 29- 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30+ 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
          +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30- 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
          +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 31+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 31- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 32+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          +Mobili SLE INV( 1.000)
  - 32- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          +Mobili SLE INV( 1.000)
  - 33+ 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
          +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 33- 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
          +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 34+ 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 34- 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 35+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 35- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
-

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- \*. PROJECT : Viadotto Agrò
- \*. MEMBER NO = 1439, ELEMENT TYPE = Beam
- \*. LOADCOMB NO = 22-, MATERIAL NO = 1, SECTION NO = 17
- \*. UNIT SYSTEM : kN, m
  
- \*. SECTION PROPERTIES : Designation = HSUP\_CORR 2L80x10d15  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.015  
 Web Thick = 0.010, Flg Thick = 0.010
  
- Area = 3.00000e-003, Avy = 1.60000e-003, Avz = 1.60000e-003  
 Ybar = 8.75000e-002, Zbar = 5.63333e-002, Qyb = 1.58672e-003, Qzb = 3.20000e-003  
 Wely = 3.15917e-005, Welz = 5.36429e-005, Wply = 5.69375e-005, Wplz = 9.35000e-005  
 Iyy = 1.77967e-006, Izz = 4.69375e-006, Iyz = 0.00000e+000  
 iy = 2.43562e-002, iz = 3.95548e-002  
 J = 1.00000e-007, Cwp = 4.24931e-011
  
- \*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.20000e+000, Lz = 3.20000e+000, Lu = 3.20000e+000  
 Ky = 6.50000e-001, Kz = 6.50000e-001
  
- \*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W
  
- \*. FORCES AND MOMENTS AT (I) POINT :  
 Axial Force Fxx = -2.08466e+002  
 Shear Forces Fyy = -3.81125e-002, Fzz = -8.14832e-001  
 Bending Moments My = -6.69568e-001, Mz = -6.26441e-002  
 End Moments Myi = -6.69568e-001, Myj = -6.64598e-001 (for Lb)  
                   Myi = -6.69568e-001, Myj = -6.64598e-001 (for Ly)  
                   Mzi = -6.26441e-002, Mzj = -6.26241e-002 (for Lz)
  
- \*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

[[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).

- ( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- . e =  $\sqrt{235/f_y}$  = 0.81
- . b/t = BTR = 7.00
- . sigma1 = 61752.308 KPa.
- . sigma2 = 60684.605 KPa.
- . BTR < 9\*e ( Class 1 : Plastic ).

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 =====

=====  
 [[[\*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  
 =====

- ( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
  - b/t = BTR = 7.00
  - sigma1 = 60484.411 KPa.
  - sigma2 = 59416.708 KPa.
  - BTR < 9\*e ( Class 1 : Plastic ).

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

- ( ). Determine classification of compression element(Double angles).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
  - d/t = HTR = 8.00
  - sigma1 = 90916.658 KPa.
  - sigma2 = 64580.392 KPa.
  - HTR < 10\*e ( Class 2 : Compact ).

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).  
 [ Eurocode3:05 Annex A. Table A.1, A.2 ]
- Cmy,0 = 0.755
  - Cmz,0 = 0.907
  - Cmy (Default or User Defined Value) = 1.000
  - Cmz (Default or User Defined Value) = 1.000
  - CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).  
 [ Eurocode3:05 6.1 ]
- Gamma\_M0 = 1.05
  - Gamma\_M1 = 1.10
  - Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial compression member (Kl/i).  
 [ Eurocode3:05 6.3.1 ]
- Kl/i = 85.4 < 200.0 ----> O.K.

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( ). Calculate axial compressive resistance (Nc\_Rd).

[ Eurocode3:05 6.1, 6.2.4 ]

- Nc\_Rd = fy \* Area / Gamma\_M0 = 1014.29 kN.

( ). Check ratio of axial resistance (N\_Ed/Nc\_Rd).

$\frac{N_{Ed}}{N_{cRd}} = \frac{208.47}{1014.29} = 0.206 < 1.000 \rightarrow \text{O.K.}$

( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).

[ Eurocode3:05 6.3.1.1, 6.3.1.2 ]

- Beta\_A = Aeff / Area = 1.000  
 - Lambda1 = Pi \* SQRT(Es/fy) = 76.409  
 - Lambda\_by = {(KLy/iy)/Lambda1} \* SQRT(Beta\_A) = 1.118  
 - Ncry = Pi^2 \* Es \* Ryy / KLy^2 = 852.57 kN.  
 - Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.  
 - Alphay = 0.340  
 - Phiy = 0.5 \* [ 1 + Alphay\*(Lambda\_by-0.2) + Lambda\_by^2 ] = 1.281  
 - Xiy = MIN [ 1 / [Phiy + SQRT(Phiy^2 - Lambda\_by^2)], 1.0 ] = 0.525  
 - Nb\_Rdy = Xiy\*Beta\_A\*Area\*fy / Gamma\_M1 = 508.05 kN.

- Lambda\_bz = {(KLz/iz)/Lambda1} \* SQRT(Beta\_A) = 0.688  
 - Ncrz = Pi^2 \* Es \* Rzz / KLz^2 = 2248.60 kN.  
 - Lambda\_bz > 0.2 and N\_Ed/Ncrz > 0.04 --> Need to check.  
 - Alphaz = 0.340  
 - Phiz = 0.5 \* [ 1 + Alphaz\*(Lambda\_bz-0.2) + Lambda\_bz^2 ] = 0.820  
 - Xiz = MIN [ 1 / [Phiz + SQRT(Phiz^2 - Lambda\_bz^2)], 1.0 ] = 0.790  
 - Nb\_Rdz = Xiz\*Beta\_A\*Area\*fy / Gamma\_M1 = 765.18 kN.

( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).

- Nb\_Rd = MIN[ Nb\_Rdy, Nb\_Rdz ] = 508.05 kN.  
 $\frac{N_{Ed}}{Nb_{Rd}} = \frac{208.47}{508.05} = 0.410 < 1.000 \rightarrow \text{O.K.}$   
 Nb\_Rd = 508.05

[[[\*]]] CHECK SHEAR RESISTANCE.

( ). Calculate shear area.

[ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]

- Avy = 2\*B\*tf = 0.0016 m^2.  
 - Avz = 2\*h\*tw = 0.0016 m^2.

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).

[ Eurocode3:05 6.1, 6.2.6 ]

- Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 312.32 kN.



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( ). Check ratio of shear resistance ( $V_{Edy}/V_{pl\_Rdy}$ ).

( LCB = 19+, POS = J )

- Applied shear force :  $V_{Edy} = 0.06$  kN.

$V_{Edy} = 0.06$

-  $\frac{V_{Edy}}{V_{pl\_Rdy}} = \frac{0.06}{312.32} = 1.781e-004 < 1.000 \rightarrow$  O.K.

$V_{pl\_Rdy} = 312.32$

( ). Calculate plastic shear resistance in local-z direction ( $V_{pl\_Rdz}$ ).

[ Eurocode3:05 6.1, 6.2.6 ]

-  $V_{pl\_Rdz} = [ Avz \cdot fy / \sqrt{3} ] / \Gamma_{M0} = 312.32$  kN.

( ). Shear Buckling Check.

[ Eurocode3:05 6.2.6 ]

-  $HTR < 72 \cdot e / \eta \rightarrow$  No need to check!

( ). Check ratio of shear resistance ( $V_{Edz}/V_{pl\_Rdz}$ ).

( LCB = 19+, POS = J )

- Applied shear force :  $V_{Edz} = 0.94$  kN.

$V_{Edz} = 0.94$

-  $\frac{V_{Edz}}{V_{pl\_Rdz}} = \frac{0.94}{312.32} = 0.003 < 1.000 \rightarrow$  O.K.

$V_{pl\_Rdz} = 312.32$

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.

( ). Calculate plastic resistance moment about major axis.

[ Eurocode3:05 6.1, 6.2.5 ]

-  $W_{ply} = 5.6938e-005$  m<sup>3</sup>.

-  $M_{c\_Rdy} = W_{ply} \cdot fy / \Gamma_{M0} = 19.25$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edy}/M_{c\_Rdy}$ ).

$M_{Edy} = 0.67$

-  $\frac{M_{Edy}}{M_{c\_Rdy}} = \frac{0.67}{19.25} = 0.035 < 1.000 \rightarrow$  O.K.

$M_{c\_Rdy} = 19.25$

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.

( ). Calculate plastic resistance moment about minor axis.

[ Eurocode3:05 6.1, 6.2.5 ]

-  $W_{plz} = 9.3500e-005$  m<sup>3</sup>.

-  $M_{c\_Rdz} = W_{plz} \cdot fy / \Gamma_{M0} = 31.61$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edz}/M_{c\_Rdz}$ ).

$M_{Edz} = 0.06$

-  $\frac{M_{Edz}}{M_{c\_Rdz}} = \frac{0.06}{31.61} = 0.002 < 1.000 \rightarrow$  O.K.

$M_{c\_Rdz} = 31.61$

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[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.

( ). Calculate Major reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]

- In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$

-  $M_{y,Rd} = M_{c,Rdy} = 19.25$  kN-m.

( ). Calculate Minor reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]

- In case of  $V_{Edy} / V_{pl,Rdy} < 0.5$

-  $M_{z,Rd} = M_{c,Rdz} = 31.61$  kN-m.

( ). Check general interaction ratio.

[ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2

$$R_{max1} = \frac{N_{Ed}}{N_{Rd}} + \frac{M_{Edy}}{M_{y,Rd}} + \frac{M_{Edz}}{M_{z,Rd}}$$

= 0.242 < 1.000 ---> O.K.

( ). Check interaction ratio of bending and axial force member.

[ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2

-  $n = N_{Ed} / N_{pl,Rd} = 0.206$

-  $a = \text{MIN}[(\text{Area}-2b*tf)/\text{Area}, 0.5] = 0.500$

-  $\text{Alpha} = 2.000$

-  $\text{Beta} = \text{MAX}[5*n, 1.0] = 1.028$

-  $M_{ny,Rd} = \text{MIN}[M_{ply,Rd}*(1-n)/(1-0.5*a), M_{ply,Rd}] = 19.25$  kN-m.

-  $R_{maxy} = M_{Edy} / M_{ny,Rd} = 0.035 < 1.000$  ---> O.K.

- In case of  $n < a$

-  $M_{nz,Rd} = M_{plz,Rd} = 31.61$  kN-m.

-  $R_{maxz} = M_{Edz} / M_{nz,Rd} = 0.002 < 1.000$  ---> O.K.

$$R_{max2} = \left[ \frac{|M_{Edy}|^{\text{Alpha}}}{M_{ny,Rd}^{\text{Alpha}}} + \frac{|M_{Edz}|^{\text{Beta}}}{M_{nz,Rd}^{\text{Beta}}} \right]$$

= 0.003 < 1.000 ---> O.K.

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( ). Check interaction ratio of bending and axial compression member.

[ Eurocode3:05 6.3.1, 6.2.9.3 (6.61, 6.62), Annex A ]

- N\_Ed = -208.47 kN.
- M\_Edy = -0.67 kN-m.
- M\_Edz = -0.06 kN-m.
- kyy = 1.216
- kyz = 0.598
- kzy = 0.804
- kzz = 1.212
- Xiy = 0.525
- Xiz = 0.790
- XiLT = 0.772

- N\_Rk = A\*fy = 1065.00 kN.
- My\_Rk = Wply\*fy = 20.21 kN-m.
- Mz\_Rk = Wplz\*fy = 33.19 kN-m.
- N\_Ed\*eNy = 0.0 (Not Slender)
- N\_Ed\*eNz = 0.0 (Not Slender)

$$\begin{aligned}
 \text{- Rmax\_LT1} &= \frac{N_{Ed}}{Xiy * N_{Rk} / \Gamma_{M1}} + k_{yy} * \frac{M_{Edy} + N_{Ed} * e_{Ny}}{XiLT * My_{Rk} / \Gamma_{M1}} + k_{yz} * \frac{M_{Edz} + N_{Ed} * e_{Nz}}{Mz_{Rk} / \Gamma_{M1}} \\
 &= 0.469 < 1.000 \text{ ---> O.K.}
 \end{aligned}$$

$$\begin{aligned}
 \text{- Rmax\_LT2} &= \frac{N_{Ed}}{Xiz * N_{Rk} / \Gamma_{M1}} + k_{zy} * \frac{M_{Edy} + N_{Ed} * e_{Ny}}{XiLT * My_{Rk} / \Gamma_{M1}} + k_{zz} * \frac{M_{Edz} + N_{Ed} * e_{Nz}}{Mz_{Rk} / \Gamma_{M1}} \\
 &= 0.313 < 1.000 \text{ ---> O.K.}
 \end{aligned}$$

$$\text{- Rmax} = \text{MAX}[ \text{MAX}(R_{max1}, R_{max2}), \text{MAX}(R_{max\_LT1}, R_{max\_LT2}) ] = 0.469 < 1.000 \text{ ---> O.K.}$$

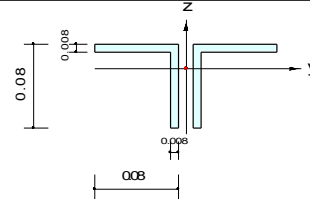
### 10.3 DIAFRAMMI SU SPALLE E INTERMEDI – DIAGONALI

#### MIDAS/Civil Steel Checking Result

<b>Company</b>		<b>Project Title</b>	Viadotto Agrò
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...\Cavalc Coste V_1.mdb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 1422  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : DIAG\_CORR 2L80x8d15 (No:64)  
 (Built-up Section).  
 Member Length : 1.70880



#### 2. Member Forces

Axial Force Fxx = -149.23 (LCB: 19-, POS:I)  
 Bending Moments My = -2.0028, Mz = -0.4744  
 End Moments Myi = -2.0028, Myj = -0.7243 (for Lb)  
 Mzi = -2.0028, Mzj = -0.7243 (for Ly)  
 Mzi = -0.4744, Mzj = -0.2855 (for Lz)  
 Shear Forces Fyy = -0.1107 (LCB: 19-, POS:I)  
 Fzz = -2.0966 (LCB: 19-, POS:J)

Depth	0.08000	Web Thick	0.00800
Flg Width	0.08000	Flg Thick	0.00800
BTB Spacing	0.01500		
Area	0.00243	Asz	0.00107
Cyb	0.00163	Qzb	0.00320
Iyy	0.00000	Izz	0.00000
Ybar	0.08750	Zbar	0.05705
Wely	0.00003	Welz	0.00004
ry	0.02462	rz	0.03916

#### 3. Design Parameters

Unbraced Lengths Ly = 1.70880, Lz = 1.70880, Lb = 1.70880  
 Effective Length Factors Ky = 0.65, Kz = 1.30  
 Equivalent Uniform Moment Factors Cmy = 1.00, Crnz = 1.00, CmLT = 1.00

#### 4. Checking Results

##### Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c,Rd}, N_{b,Rd}] = 149.229/595.881 = 0.250 < 1.000 \dots\dots\dots \text{O.K}$$

##### Bending Resistance

$$M_{Edy}/M_{Rdy} = 2.00283/8.34129 = 0.240 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 0.4744/11.6551 = 0.041 < 1.000 \dots\dots\dots \text{O.K}$$

##### Combined Resistance

$$\begin{aligned} R_{oom} &= N_{Ed}/(A_{eff} \cdot f_y / \gamma_{M0}), R_{bend} = (M_{Edy} + N_{Ed} \cdot e_{Ny})/M_{y,Rd} + (M_{Edz} + N_{Ed} \cdot e_{Nz})/M_{z,Rd} \\ R_{c\_LT1} &= N_{Ed}/(X_{iy} \cdot A_{eff} \cdot f_y / \gamma_{M1}) \\ R_{b\_LT1} &= k_{yy} \cdot (M_{Edy} + N_{Ed} \cdot e_{Ny}) / (X_{i\_LT} \cdot W_{effy} \cdot f_y / \gamma_{M1}) + k_{yz} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (W_{effz} \cdot f_y / \gamma_{M1}) \\ R_{c\_LT2} &= N_{Ed}/(X_{iz} \cdot A_{eff} \cdot f_y / \gamma_{M1}) \\ R_{b\_LT2} &= k_{zy} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (X_{i\_LT} \cdot W_{effy} \cdot f_y / \gamma_{M1}) + k_{zz} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (W_{effz} \cdot f_y / \gamma_{M1}) \\ R_{max} &= \text{MAX}[R_{oom} + R_{bend}, \text{MAX}(R_{c\_LT1} + R_{b\_LT1}, R_{c\_LT2} + R_{b\_LT2})] = 0.563 < 1.000 \dots\dots \text{O.K} \end{aligned}$$

##### Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.000 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.008 < 1.000 \dots\dots\dots \text{O.K}$$





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- 
- 23+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 23- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 24+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 24- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 25+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 25- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 28+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 28- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 29+ 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 29- 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30+ 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30- 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 31+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 31- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 32+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
  - 32- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
  - 33+ 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 33- 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 34+ 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 34- 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 35+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 35- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
-

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\*. PROJECT : Viadotto Agrò  
 \*. MEMBER NO = 1422, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19-, MATERIAL NO = 1, SECTION NO = 64  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = DIAG\_CORR 2L80x8d15  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.015  
 Web Thick = 0.008, Flg Thick = 0.008

Area = 2.43200e-003, Avy = 1.28000e-003, Avz = 1.28000e-003  
 Ybar = 8.75000e-002, Zbar = 5.70526e-002, Qyb = 1.62750e-003, Qzb = 3.20000e-003  
 Wely = 2.58462e-005, Welz = 4.26190e-005, Wply = 4.65664e-005, Wplz = 7.40480e-005  
 Iyy = 1.47460e-006, Izz = 3.72916e-006, Iyz = 0.00000e+000  
 iy = 2.46238e-002, iz = 3.91583e-002  
 J = 5.18827e-008, Cwp = 2.31033e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 1.70880e+000, Lz = 1.70880e+000, Lu = 1.70880e+000  
 Ky = 6.50000e-001, Kz = 1.30000e+000

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (I) POINT :  
 Axial Force Fxx = -1.49229e+002  
 Shear Forces Fyy = -1.10743e-001, Fzz = -1.63158e+000  
 Bending Moments My = -2.00283e+000, Mz = -4.74407e-001  
 End Moments Myi = -2.00283e+000, Myj = -7.24258e-001 (for Lb)  
                   Myi = -2.00283e+000, Myj = -7.24258e-001 (for Ly)  
                   Mzi = -4.74407e-001, Mzj = -2.85479e-001 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

[[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).

( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -. e = SQRT( 235/fy ) = 0.81  
 -. b/t = BTR = 9.00  
 -. sigma1 = 41324.319 KPa.  
 -. sigma2 = 31147.091 KPa.  
 -. BTR\_L = (H+B) / (2\*tf) = 10.00  
 -. BTR > 15\*e or BTR\_L > 11.5\*e ( Class 4 : Slender ).

-----  
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 =====

=====  
 [[[\*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  
 =====

- ( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
- b/t = BTR = 9.00
- sigma1 = 29238.861 KPa.
- sigma2 = 19061.633 KPa.
- BTR\_L = (H+B) / (2\*tf) = 10.00
- BTR > 15\*e or BTR\_L > 11.5\*e ( Class 4 : Slender ).

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

- ( ). Determine classification of compression element(Double angles).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
- d/t = HTR = 10.00
- sigma1 = 140822.580 KPa.
- sigma2 = 43030.591 KPa.
- HTR\_L = (h+B) / (2\*tw) = 10.00
- HTR > 15\*e or HTR\_L > 11.5\*e ( Class 4 : Slender ).

=====  
 [[[\*]]] CALCULATE EFFECTIVE AREA.  
 =====

- ( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- In case of Psi = 1.0
- k\_sigma = 0.4300
  
- ( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- RatT = 9.0000
- Lambda\_p = RatT / [ 28.4\*Eps\*SQRT(k\_sigma) ] = 0.5940
- Rho = 1.0
- sigma\_max = MAX( sigma1, sigma2 ) = 61360.598 KPa.
- sigma\_min = MIN( sigma1, sigma2 ) = 61360.598 KPa.
- r = 0.000 m.
- bc = 0.080 m.
- beff = Rho\*bc + r = 0.080 m.
- Aeff = beff \* tf = 6.400e-004 m^2.
- yeff = beff/2 = 0.040 m.
  
- ( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]

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- In case of  $\Psi = 1.0$
- $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $RatT = 9.0000$
- $\Lambda_{p} = RatT / [ 28.4 * \epsilon * \sqrt{k_{\sigma}} ] = 0.5940$
- $\rho = 1.0$
- $\sigma_{max} = MAX(\sigma_1, \sigma_2) = 61360.598 \text{ KPa.}$
- $\sigma_{min} = MIN(\sigma_1, \sigma_2) = 61360.598 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $bc = 0.080 \text{ m.}$
- $b_{eff} = \rho * bc + r = 0.080 \text{ m.}$
- $A_{eff} = b_{eff} * t_f = 6.400e-004 \text{ m}^2.$
- $y_{eff} = b_{eff} / 2 = 0.040 \text{ m.}$

( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]

- In case of  $\Psi = 1.0$
- $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $RatT = 10.0000$
- $\Lambda_{p} = RatT / [ 28.4 * \epsilon * \sqrt{k_{\sigma}} ] = 0.6600$
- $\rho = 1.0$
- $\sigma_{max} = MAX(\sigma_1, \sigma_2) = 61360.598 \text{ KPa.}$
- $\sigma_{min} = MIN(\sigma_1, \sigma_2) = 61360.598 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $A_r = 0.000 \text{ m}^2.$
- $dc = 0.072 \text{ m.}$
- $d_{eff1} = 2 * (\rho * dc) / [ 5 - \sigma_{min} / \sigma_{max} ] + r = 0.036 \text{ m.}$
- $A_{eff1} = d_{eff1} * t_w + 2 * A_r = 2.880e-004 \text{ m}^2.$
- $z_{eff1} = d_{eff1} / 2 + t_f = 0.018 \text{ m.}$
- $d_{eff2} = (\rho * dc) - d_{eff1} + r = 0.036 \text{ m.}$
- $A_{eff2} = d_{eff2} * t_w + 2 * A_r = 2.880e-004 \text{ m}^2.$
- $z_{eff2} = (h + 2 * r) - d_{eff2} / 2 + t_f = 0.054 \text{ m.}$

( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]

- In case of  $\Psi = 1.0$
- $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $RatT = 10.0000$
- $\Lambda_{p} = RatT / [ 28.4 * \epsilon * \sqrt{k_{\sigma}} ] = 0.6600$
- $\rho = 1.0$

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- sigma\_max = MAX( sigma1, sigma2 ) = 61360.598 KPa.
- sigma\_min = MIN( sigma1, sigma2 ) = 61360.598 KPa.
- r = 0.000 m.
- Ar = 0.000 m<sup>2</sup>.
- dc = 0.072 m.
- deff1 = 2\*(Rho\*dc) / [ 5 - sigma\_min/sigma\_max ] + r = 0.036 m.
- Aeff1 = deff1 \* tw + 2\*Ar = 2.880e-004 m<sup>2</sup>.
- zeff1 = deff1/2 + tf = 0.018 m.
- deff2 = (Rho\*dc) - deff1 + r = 0.036 m.
- Aeff2 = deff2 \* tw + 2\*Ar = 2.880e-004 m<sup>2</sup>.
- zeff2 = (h+2\*r) - deff2/2 + tf = 0.054 m.

[[[\*]]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MAJOR AXIS.

( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- Rho = 1.0 (Only tensile stresses).
  - sigma\_max = MAX( sigma1, sigma2 ) = -31167.622 KPa.
  - sigma\_min = MIN( sigma1, sigma2 ) = -31167.622 KPa.
  - r = 0.000 m.
  - bc = 0.080 m.
  - beff = Rho\*bc + r = 0.080 m.
  - Aeff = beff \* tf = 6.400e-004 m<sup>2</sup>.
  - yeff = beff/2 = 0.040 m.

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- Rho = 1.0 (Only tensile stresses).
  - sigma\_max = MAX( sigma1, sigma2 ) = -31167.622 KPa.
  - sigma\_min = MIN( sigma1, sigma2 ) = -31167.622 KPa.
  - r = 0.000 m.
  - bc = 0.080 m.
  - beff = Rho\*bc + r = 0.080 m.
  - Aeff = beff \* tf = 6.400e-004 m<sup>2</sup>.
  - yeff = beff/2 = 0.040 m.

( ). Calculate buckling factor of outstand compression element.

- [ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- k\_sigma = 0.57 - 0.21\*Psi + 0.07\*Psi<sup>2</sup> = 0.6298

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- RatT = 10.0000
  - Lambda\_p = RatT / [ 28.4\*Eps\*SQRT(k\_sigma) ] = 0.5453
  - Rho = 1.0



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- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 77490.143 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -20301.846 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $Ar = 0.000 \text{ m}^2.$
- $dc = (h \cdot \sigma_{max}) / (\sigma_{max} - \sigma_{min}) = 0.057 \text{ m.}$
- $deff1 = 0.4 \cdot \rho \cdot dc + r = 0.023 \text{ m.}$
- $A_{eff1} = deff1 \cdot t_w + 2 \cdot Ar = 1.826e-004 \text{ m}^2.$
- $z_{eff1} = deff1/2 + t_f = 0.011 \text{ m.}$
- $deff2 = 0.6 \cdot \rho \cdot dc + (h - dc) + r = 0.049 \text{ m.}$
- $A_{eff2} = deff2 \cdot t_w + 2 \cdot Ar = 3.934e-004 \text{ m}^2.$
- $z_{eff2} = (h + 2 \cdot r) - deff2/2 + t_f = 0.047 \text{ m.}$

- ( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]  
 -  $k_{\sigma} = 0.57 - 0.21 \cdot \Psi + 0.07 \cdot \Psi^2 = 0.6298$

- ( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]  
 -  $RatT = 10.0000$   
 -  $\lambda_p = RatT / [ 28.4 \cdot \epsilon \cdot \sqrt{k_{\sigma}} ] = 0.5453$   
 -  $\rho = 1.0$   
 -  $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 77490.143 \text{ KPa.}$   
 -  $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -20301.846 \text{ KPa.}$   
 -  $r = 0.000 \text{ m.}$   
 -  $Ar = 0.000 \text{ m}^2.$   
 -  $dc = (h \cdot \sigma_{max}) / (\sigma_{max} - \sigma_{min}) = 0.057 \text{ m.}$   
 -  $deff1 = 0.4 \cdot \rho \cdot dc + r = 0.023 \text{ m.}$   
 -  $A_{eff1} = deff1 \cdot t_w + 2 \cdot Ar = 1.826e-004 \text{ m}^2.$   
 -  $z_{eff1} = deff1/2 + t_f = 0.011 \text{ m.}$   
 -  $deff2 = 0.6 \cdot \rho \cdot dc + (h - dc) + r = 0.049 \text{ m.}$   
 -  $A_{eff2} = deff2 \cdot t_w + 2 \cdot Ar = 3.934e-004 \text{ m}^2.$   
 -  $z_{eff2} = (h + 2 \cdot r) - deff2/2 + t_f = 0.047 \text{ m.}$

[[[\*]]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MINOR AXIS.

- ( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]  
 -  $k_{\sigma} = 0.57 - 0.21 \cdot \Psi + 0.07 \cdot \Psi^2 = 0.5525$
- ( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]  
 -  $RatT = 9.0000$   
 -  $\lambda_p = RatT / [ 28.4 \cdot \epsilon \cdot \sqrt{k_{\sigma}} ] = 0.5240$   
 -  $\rho = 1.0$

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- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 11131.343 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = 954.115 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $bc = 0.080 \text{ m.}$
- $b_{eff} = \rho \cdot bc + r = 0.080 \text{ m.}$
- $A_{eff} = b_{eff} \cdot t_f = 6.400e-004 \text{ m}^2.$
- $y_{eff} = b_{eff}/2 = 0.040 \text{ m.}$

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $\rho = 1.0$  (Only tensile stresses).
- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = -954.115 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -11131.343 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $bc = 0.080 \text{ m.}$
- $b_{eff} = \rho \cdot bc + r = 0.080 \text{ m.}$
- $A_{eff} = b_{eff} \cdot t_f = 6.400e-004 \text{ m}^2.$
- $y_{eff} = b_{eff}/2 = 0.040 \text{ m.}$

( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]

- In case of  $\psi = 1.0$
- $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $RatT = 10.0000$
- $\lambda_p = RatT / [ 28.4 \cdot \epsilon \cdot \sqrt{k_{\sigma}} ] = 0.6600$
- $\rho = 1.0$
- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 1971.838 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = 1971.838 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $Ar = 0.000 \text{ m}^2.$
- $dc = 0.072 \text{ m.}$
- $d_{eff1} = 2 \cdot (\rho \cdot dc) / [ 5 - \sigma_{min}/\sigma_{max} ] + r = 0.036 \text{ m.}$
- $A_{eff1} = d_{eff1} \cdot t_w + 2 \cdot Ar = 2.880e-004 \text{ m}^2.$
- $z_{eff1} = d_{eff1}/2 + t_f = 0.018 \text{ m.}$
- $d_{eff2} = (\rho \cdot dc) - d_{eff1} + r = 0.036 \text{ m.}$
- $A_{eff2} = d_{eff2} \cdot t_w + 2 \cdot Ar = 2.880e-004 \text{ m}^2.$
- $z_{eff2} = (h+2 \cdot r) - d_{eff2}/2 + t_f = 0.054 \text{ m.}$

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $\rho = 1.0$  (Only tensile stresses).
- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = -1971.838 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -1971.838 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $Ar = 0.000 \text{ m}^2.$
- $dc = 0.072 \text{ m.}$
- $d_{eff} = dc + r = 0.072 \text{ m.}$
- $A_{eff} = d_{eff} \cdot t_w + 4 \cdot Ar = 5.760e-004 \text{ m}^2.$
- $z_{eff} = (h+2 \cdot r) - d_{eff}/2 = 0.036 \text{ m.}$

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=====  
 [[[\*]]] EFFECTIVE SECTION PPOPERTIES.  
 =====

- ( ). Calculated effective cross-section properties of Class4 cross-section.
- Aeff = 0.0024 m<sup>2</sup>. (for calculating axial resistance)
  - Aeffy = 0.0024 m<sup>2</sup>.
  - Weffy = 2.5846e-005 m<sup>3</sup>.
  - Aeffz = 0.0024 m<sup>2</sup>.
  - Weffz = 3.6115e-005 m<sup>3</sup>.
  - eNy = 1.4211e-017 m.
  - eNz = 0.0000 m.

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).  
 [ Eurocode3:05 Annex A. Table A.1, A.2 ]
- Cmy,0 = 0.940
  - Cmz,0 = 0.926
  - Cmy (Default or User Defined Value) = 1.000
  - Cmz (Default or User Defined Value) = 1.000
  - CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).  
 [ Eurocode3:05 6.1 ]
- Gamma\_M0 = 1.05
  - Gamma\_M1 = 1.10
  - Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial compression member (Kl/i).  
 [ Eurocode3:05 6.3.1 ]
- Kl/i = 56.7 < 200.0 ----> O.K.
- ( ). Calculate axial compressive resistance (Nc\_Rd).  
 [ Eurocode3:05 6.1, 6.2.4 ]
- Nc\_Rd = fy \* Aeff / Gamma\_M0 = 822.25 kN.
- ( ). Check ratio of axial resistance (N\_Ed/Nc\_Rd).
- N\_Ed            149.23
- ----- = ----- = 0.181 < 1.000 ----> O.K.
- Nc\_Rd            822.25

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( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).

[ Eurocode3:05 6.3.1.1, 6.3.1.2 ]

- Beta\_A = Aeff / Area = 1.000
- Lambda1 = Pi \* SQRT(Es/fy) = 76.409
- Lambda\_by = {(KLy/iy)/Lambda1} \* SQRT(Beta\_A) = 0.590
- Ncry = Pi^2\*Es\*Ryy / KLy^2 = 2477.32 kN.
- Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.
- Alphay = 0.340
- Phiy = 0.5 \* [ 1 + Alphay\*(Lambda\_by-0.2) + Lambda\_by^2 ] = 0.741
- Xiy = MIN [ 1 / [Phiy + SQRT(Phiy^2 - Lambda\_by^2)], 1.0 ] = 0.842
- Nb\_Rdy = Xiy\*Beta\_A\*Area\*fy / Gamma\_M1 = 660.77 kN.

- Lambda\_bz = {(KLz/iz)/Lambda1} \* SQRT(Beta\_A) = 0.742
- Ncrz = Pi^2\*Es\*Rzz / KLz^2 = 1566.25 kN.
- Lambda\_bz > 0.2 and N\_Ed/Ncrz > 0.04 --> Need to check.
- Alphaz = 0.340
- Phiz = 0.5 \* [ 1 + Alphaz\*(Lambda\_bz-0.2) + Lambda\_bz^2 ] = 0.868
- Xiz = MIN [ 1 / [Phiz + SQRT(Phiz^2 - Lambda\_bz^2)], 1.0 ] = 0.759
- Nb\_Rdz = Xiz\*Beta\_A\*Area\*fy / Gamma\_M1 = 595.88 kN.

( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).

- Nb\_Rd = MIN[ Nb\_Rdy, Nb\_Rdz ] = 595.88 kN.
- N\_Ed        149.23
- ----- = ----- = 0.250 < 1.000 ---> O.K.
- Nb\_Rd        595.88

[[[\*]]] CHECK SHEAR RESISTANCE.

( ). Calculate shear area.

[ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]

- Avy = 2\*B\*tf = 0.0013 m^2.
- Avz = 2\*h\*tw = 0.0013 m^2.

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).

[ Eurocode3:05 6.1, 6.2.6 ]

- Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 249.86 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).

( LCB = 19-, POS = 1 )

- Applied shear force : V\_Edy = 0.11 kN.
- V\_Edy        0.11
- ----- = ----- = 4.432e-004 < 1.000 ---> O.K.
- Vpl\_Rdy      249.86

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).

[ Eurocode3:05 6.1, 6.2.6 ]

- Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 249.86 kN.

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( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 - HTR <  $72 \cdot e / \eta$  ---> No need to check!

( ). Check ratio of shear resistance ( $V_{Edz} / V_{pl,Rdz}$ ).  
 ( LCB = 19-, POS = J )  
 - Applied shear force :  $V_{Edz} = 2.10$  kN.  
 $V_{Edz} \quad 2.10$   
 -  $\frac{\quad}{V_{pl,Rdz}} = \frac{\quad}{249.86} = 0.008 < 1.000$  ---> O.K.

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.

( ). Calculate local buckling resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -  $W_{effy} = 2.5846e-005$  m<sup>3</sup>.  
 -  $M_{c,Rdy} = W_{effy} \cdot f_y / \gamma_{M1} = 8.34$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edy} / M_{c,Rdy}$ ).  
 $M_{Edy} \quad 2.00$   
 -  $\frac{\quad}{M_{c,Rdy}} = \frac{\quad}{8.34} = 0.240 < 1.000$  ---> O.K.

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.

( ). Calculate local buckling resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -  $W_{effz} = 3.6115e-005$  m<sup>3</sup>.  
 -  $M_{c,Rdz} = W_{effz} \cdot f_y / \gamma_{M1} = 11.66$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edz} / M_{c,Rdz}$ ).  
 $M_{Edz} \quad 0.47$   
 -  $\frac{\quad}{M_{c,Rdz}} = \frac{\quad}{11.66} = 0.041 < 1.000$  ---> O.K.

[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.

( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 - In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$   
 -  $M_{y,Rd} = M_{c,Rdy} = 8.34$  kN-m.

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( ). Calculate Minor reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]

- In case of  $V_{Edy} / V_{pl,Rdy} < 0.5$

-  $M_{z,Rd} = M_{c,Rdz} = 11.66 \text{ kN-m.}$

( ). Check interaction ratio of bending and axial force.

[ Eurocode3:05 6.2.9.3 (6.44) ] - Class4

$$R_{max1} = \frac{N_{Ed}}{A_{eff} \cdot f_y / \gamma_{M0}} + \frac{M_{Edy} + N_{Ed} \cdot e_{Nz}}{M_{y,Rd}} + \frac{M_{Edz} + N_{Ed} \cdot e_{Ny}}{M_{z,Rd}}$$

= 0.462 < 1.000 ---> O.K.

( ). Check interaction ratio of bending and axial compression member.

[ Eurocode3:05 6.3.1, 6.2.9.3 (6.61, 6.62), Annex A ]

-  $N_{Ed} = -149.23 \text{ kN.}$

-  $M_{Edy} = -2.00 \text{ kN-m.}$

-  $M_{Edz} = -0.47 \text{ kN-m.}$

-  $k_{yy} = 1.053$

-  $k_{yz} = 1.094$

-  $k_{zy} = 1.038$

-  $k_{zz} = 1.078$

-  $\chi_{iy} = 0.842$

-  $\chi_{iz} = 0.759$

-  $\chi_{iLT} = 0.927$

-  $A_{eff} = 0.0024 \text{ m}^2.$

-  $W_{effy} = 2.5846e-005 \text{ m}^3.$

-  $W_{effz} = 3.6115e-005 \text{ m}^3.$

-  $e_{Ny} = 1.4211e-017 \text{ m.}$

-  $e_{Nz} = 0.0000 \text{ m.}$

-  $N_{Rk} = A_{eff} \cdot f_y = 863.36 \text{ kN.}$

-  $M_{y,Rk} = W_{effy} \cdot f_y = 9.18 \text{ kN-m.}$

-  $M_{z,Rk} = W_{effz} \cdot f_y = 12.82 \text{ kN-m.}$

-  $N_{Ed} \cdot e_{Ny} = 2.12e-015 \text{ kN-m.}$

-  $N_{Ed} \cdot e_{Nz} = 0.00 \text{ kN-m.}$

$$R_{max\_LT1} = \frac{N_{Ed}}{\chi_{iy} \cdot N_{Rk} / \gamma_{M1}} + k_{yy} \cdot \frac{M_{Edy} + N_{Ed} \cdot e_{Ny}}{\chi_{iLT} \cdot M_{y,Rk} / \gamma_{M1}} + k_{yz} \cdot \frac{M_{Edz} + N_{Ed} \cdot e_{Nz}}{M_{z,Rk} / \gamma_{M1}}$$

= 0.543 < 1.000 ---> O.K.

$$R_{max\_LT2} = \frac{N_{Ed}}{\chi_{iz} \cdot N_{Rk} / \gamma_{M1}} + k_{zy} \cdot \frac{M_{Edy} + N_{Ed} \cdot e_{Ny}}{\chi_{iLT} \cdot M_{y,Rk} / \gamma_{M1}} + k_{zz} \cdot \frac{M_{Edz} + N_{Ed} \cdot e_{Nz}}{M_{z,Rk} / \gamma_{M1}}$$

= 0.563 < 1.000 ---> O.K.

-  $R_{max} = \text{MAX}[ R_{max1}, \text{MAX}(R_{max\_LT1}, R_{max\_LT2}) ] = 0.563 < 1.000 \text{ ---> O.K.}$



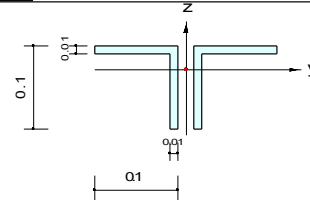
## 10.4 DIAFRAMMI SU PILE – TRASVERSI INFERIORI

### MIDAS/Civil Steel Checking Result

<b>Company</b>		<b>Project Title</b>	Viadotto Agrò
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...\Cavalc Coste V_1.mcb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 2001  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : HINF\_PILE 2L100x10d20 (No:65)  
 (Built-up Section).  
 Member Length : 3.20000



#### 2. Member Forces

Axial Force Fxx = 363.171 (LCB: 19+, POS:J)  
 Bending Moments My = 1.50851, Mz = 1.01624  
 End Moments Myi = 0.87476, Myj = 1.50851 (for Lb)  
 Myi = 0.87476, Myj = 1.50851 (for Ly)  
 Mzi = 0.94496, Mzj = 1.01624 (for Lz)  
 Shear Forces Fyy = -0.6312 (LCB: 19-, POS:I)  
 Fzz = -1.5601 (LCB: 19-, POS:J)

Depth	0.10000	Web Thick	0.01000
Flg Width	0.10000	Flg Thick	0.01000
BTB Spacing	0.02000		
Area	0.00380	Asz	0.00167
Cyb	0.00254	Czb	0.00500
Iyy	0.00000	Izz	0.00001
Ybar	0.11000	Zbar	0.07132
Wely	0.00005	Welz	0.00008
ry	0.03078	rz	0.04944

#### 3. Design Parameters

Unbraced Lengths Ly = 3.20000, Lz = 3.20000, Lb = 3.20000  
 Effective Length Factors Ky = 0.65, Kz = 0.65  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

Axial Resistance  
 $N_{Ed}/N_{t,Rd} = 363.17/1284.76 = 0.283 < 1.000$  ..... O.K

Bending Resistance  
 $M_{Edy}/M_{Rdy} = 1.5085/30.7498 = 0.049 < 1.000$  ..... O.K  
 $M_{Edz}/M_{Rdz} = 1.0162/49.7000 = 0.020 < 1.000$  ..... O.K

Combined Resistance  
 $R_{NRd} = \text{MAX}[ M_{Edy}/M_{ny,Rd}, M_{Edz}/M_{nz,Rd} ]$   
 $R_{max1} = (M_{Edy}/M_{ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz,Rd})^{\text{Beta}}$   
 $R_{com} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$   
 $R_{max} = \text{MAX}[ R_{NRd}, R_{max1}, (R_{com} + R_{bend}) ] = 0.352 < 1.000$  ..... O.K

Shear Resistance  
 $V_{Edy}/V_{y,Rd} = 0.002 < 1.000$  ..... O.K  
 $V_{Edz}/V_{z,Rd} = 0.004 < 1.000$  ..... O.K



-----  
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 =====

- 23+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 23- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 24+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 24- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 25+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 25- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 28+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 28- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 29+ 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 29- 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
          +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30+ 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
          +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30- 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
          +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 31+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 31- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
          +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 32+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          +Mobili SLE INV( 1.000)
  - 32- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
          +Mobili SLE INV( 1.000)
  - 33+ 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
          +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 33- 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
          +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 34+ 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 34- 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 35+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 35- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
-

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                      Version 8.3.5  
 =====

\*. PROJECT : Viadotto Agrò  
 \*. MEMBER NO = 2001, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19+, MATERIAL NO = 1, SECTION NO = 65  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = HINF\_PILE 2L100x10d20  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.100, Flg Width = 0.100, BTB Spacing = 0.020  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.80000e-003, Avy = 2.00000e-003, Avz = 2.00000e-003  
 Ybar = 1.10000e-001, Zbar = 7.13158e-002, Qyb = 2.54297e-003, Qzb = 5.00000e-003  
 Wely = 5.04809e-005, Welz = 8.44242e-005, Wply = 9.09500e-005, Wplz = 1.47000e-004  
 Iyy = 3.60009e-006, Izz = 9.28667e-006, Iyz = 0.00000e+000  
 iy = 3.07797e-002, iz = 4.94354e-002  
 J = 1.26667e-007, Cwp = 8.81319e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.20000e+000, Lz = 3.20000e+000, Lu = 3.20000e+000  
 Ky = 6.50000e-001, Kz = 6.50000e-001

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = 3.63171e+002  
 Shear Forces Fyy = 5.96221e-001, Fzz = -3.79864e-001  
 Bending Moments My = 1.50851e+000, Mz = 1.01624e+000  
 End Moments Myi = 8.74756e-001, Myj = 1.50851e+000 (for Lb)  
                   Myi = 8.74756e-001, Myj = 1.50851e+000 (for Ly)  
                   Mzi = 9.44956e-001, Mzj = 1.01624e+000 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

- ( ). Determine classification of tension flanges(double angle).
  - Not Checking the Section Classification.
  
- ( ). Determine classification of tension flanges(double angle).
  - Not Checking the Section Classification.

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

-----  
 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                      Version 8.3.5  
 =====

- ( ). Determine classification of tension elements(Double angle).
- Not Checking the Section Classification.

=====

[[[\*]]] APPLIED FACTORS.

=====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).
- [ Eurocode3:05 Annex A. Table A.1, A.2 ]
- Cmy,0 = 0.789
- Cmz,0 = 0.918
- Cmy (Default or User Defined Value) = 1.000
- Cmz (Default or User Defined Value) = 1.000
- CmLT (Default or User Defined Value) = 1.000

- ( ). Partial Factors (Gamma\_Mi).
- [ Eurocode3:05 6.1 ]
- Gamma\_M0 = 1.05
- Gamma\_M1 = 1.10
- Gamma\_M2 = 1.25

=====

[[[\*]]] CHECK AXIAL RESISTANCE.

=====

- ( ). Check slenderness ratio of axial tension member (l/i).
- [ Eurocode3:05 6.3.1 ]
- l/i = 104.0 < 300.0 ---> O.K.

- ( ). Calculate parameters for combined resistance.
- Lambda1 = Pi \* SQRT(Es/fy) = 76.409
- Lambda\_bz = (KLz/iz) / Lambda1 = 0.847

- ( ). Calculate axial tensile resistance (Nt\_Rd).
- [ Eurocode3:05 6.2.3 ]
- Nt\_Rd = fy \* Area / Gamma\_M0 = 1284.76 kN.

- ( ). Check ratio of axial resistance (N\_Ed/Nt\_Rd).
- |       |                           |
|-------|---------------------------|
| N_Ed  | 363.17                    |
| -     | -----                     |
|       | = 0.283 < 1.000 ---> O.K. |
| Nt_Rd | 1284.76                   |

=====

[[[\*]]] CHECK SHEAR RESISTANCE.

=====

- ( ). Calculate shear area.
- [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]
- Avy = 2\*B\*tf = 0.0020 m^2.
- Avz = 2\*h\*tw = 0.0020 m^2.

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( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 - Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 =     390.40 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19-, POS = J )  
 - Applied shear force : V\_Edy =     0.63 kN.  
   V\_Edy                0.63  
 - ----- = ----- = 0.002 < 1.000 ----> O.K.  
   Vpl\_Rdy            390.40

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 - Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 =     390.40 kN.

( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 - HTR < 72\*e/Eta ----> No need to check!

( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19-, POS = J )  
 - Applied shear force : V\_Edz =     1.56 kN.  
   V\_Edz                1.56  
 - ----- = ----- = 0.004 < 1.000 ----> O.K.  
   Vpl\_Rdz            390.40

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.

( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 - Wply = 9.0950e-005 m^3.  
 - Mc\_Rdy = Wply \* fy / Gamma\_M0 =     30.75 kN-m.

( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  
   M\_Edy                1.51  
 - ----- = ----- = 0.049 < 1.000 ----> O.K.  
   Mc\_Rdy             30.75

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.

( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 - Wplz = 0.0001 m^3.  
 - Mc\_Rdz = Wplz \* fy / Gamma\_M0 =     49.70 kN-m.



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( ). Check ratio of moment resistance ( $M_{Edz}/M_{c\_Rdz}$ ).

$$\frac{M_{Edz}}{M_{c\_Rdz}} = \frac{1.02}{49.70} = 0.020 < 1.000 \text{ ---> O.K.}$$

[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.

( ). Calculate Major reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]  
 - In case of  $V_{Edz} / V_{pl\_Rdz} < 0.5$   
 -  $M_{y\_Rd} = M_{c\_Rdy} = 30.75 \text{ kN-m.}$

( ). Calculate Minor reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]  
 - In case of  $V_{Edy} / V_{pl\_Rdy} < 0.5$   
 -  $M_{z\_Rd} = M_{c\_Rdz} = 49.70 \text{ kN-m.}$

( ). Check general interaction ratio.

[ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2

$$R_{max1} = \frac{N_{Ed}}{N_{Rd}} + \frac{M_{Edy}}{M_{y\_Rd}} + \frac{M_{Edz}}{M_{z\_Rd}}$$

$$= 0.352 < 1.000 \text{ ---> O.K.}$$

( ). Check interaction ratio of bending and axial force member.

[ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2

$$n = \frac{N_{Ed}}{N_{pl\_Rd}} = 0.283$$

$$a = \text{MIN}[(\text{Area}-2b*tf)/\text{Area}, 0.5] = 0.500$$

$$\text{Alpha} = 2.000$$

$$\text{Beta} = \text{MAX}[5*n, 1.0] = 1.413$$

$$M_{ny\_Rd} = \text{MIN}[M_{ply\_Rd}*(1-n)/(1-0.5*a), M_{ply\_Rd}] = 29.41 \text{ kN-m.}$$

$$R_{maxy} = \frac{M_{Edy}}{M_{ny\_Rd}} = 0.051 < 1.000 \text{ ---> O.K.}$$

- In case of  $n < a$   
 -  $M_{nz\_Rd} = M_{plz\_Rd} = 49.70 \text{ kN-m.}$   
 -  $R_{maxz} = \frac{M_{Edz}}{M_{nz\_Rd}} = 0.020 < 1.000 \text{ ---> O.K.}$

$$R_{max2} = \left[ \frac{|M_{Edy}|^{\text{Alpha}}}{|M_{ny\_Rd}|} + \frac{|M_{Edz}|^{\text{Beta}}}{|M_{nz\_Rd}|} \right]$$

$$= 0.007 < 1.000 \text{ ---> O.K.}$$

$$R_{max} = \text{MAX}[R_{max1}, R_{max2}] = 0.352 < 1.000 \text{ ---> O.K.}$$

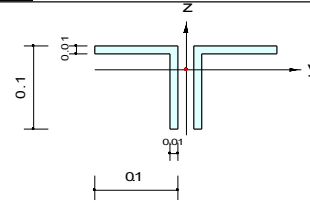
## 10.5 DIAFRAMMI SU PILE – TRASVERSI SUPERIORI

### MIDAS/Civil Steel Checking Result

<b>Company</b>		<b>Project Title</b>	Viadotto Agrò
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...Cavalc Coste V_1.mcb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 2045  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : HSUP\_PILE 2L100x10d20 (No:19)  
 (Built-up Section).  
 Member Length : 3.20000



#### 2. Member Forces

Axial Force Fxx = -135.95 (LCB: 22-, POS:J)  
 Bending Moments My = -1.3638, Mz = -0.1462  
 End Moments Myi = -0.8778, Mj = -1.3638 (for Lb)  
 Myi = -0.8778, Mj = -1.3638 (for Ly)  
 Mzi = -0.0734, Mzj = -0.1462 (for Lz)  
 Shear Forces Fyy = 0.10930 (LCB: 19+, POS:I)  
 Fzz = 1.56013 (LCB: 19+, POS:J)

Depth	0.10000	Web Thick	0.01000
Flg Width	0.10000	Flg Thick	0.01000
BTB Spacing	0.02000		
Area	0.00380	Asz	0.00167
Qyb	0.00254	Qzb	0.00500
Iyy	0.00000	Izz	0.00001
Ybar	0.11000	Zbar	0.07132
Wely	0.00005	Welz	0.00008
ry	0.03078	rz	0.04944

#### 3. Design Parameters

Unbraced Lengths Ly = 3.20000, Lz = 3.20000, Lb = 3.20000  
 Effective Length Factors Ky = 0.65, Kz = 0.65  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

##### Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c,Rd}, N_{b,Rd}] = 135.953/823.126 = 0.165 < 1.000 \dots\dots\dots \text{O.K}$$

##### Bending Resistance

$$M_{Edy}/M_{Rdy} = 1.3638/16.2916 = 0.084 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 0.1462/22.8706 = 0.006 < 1.000 \dots\dots\dots \text{O.K}$$

##### Combined Resistance

$$\begin{aligned} R_{om} &= N_{Ed}/(A_{eff}f_y/\Gamma_{M0}), R_{bend} = (M_{Edy}+N_{Ed}eNy)/M_{y,Rd} + (M_{Edz}+N_{Ed}eNz)/M_{z,Rd} \\ R_{c\_LT1} &= N_{Ed}/(X_{iy}A_{eff}f_y/\Gamma_{M1}) \\ R_{b\_LT1} &= k_{y^*}(M_{Edy}+N_{Ed}eNy)/(X_{i\_LT}W_{effy}f_y/\Gamma_{M1}) + k_{yz}*(M_{Edz}+N_{Ed}eNz)/(W_{effz}f_y/\Gamma_{M1}) \\ R_{c\_LT2} &= N_{Ed}/(X_{iz}A_{eff}f_y/\Gamma_{M1}) \\ R_{b\_LT2} &= k_{z^*}(M_{Edz}+N_{Ed}eNz)/(X_{i\_LT}W_{effy}f_y/\Gamma_{M1}) + k_{zz}*(M_{Edz}+N_{Ed}eNz)/(W_{effz}f_y/\Gamma_{M1}) \\ R_{max} &= \text{MAX}[R_{om}+R_{bend}, \text{MAX}(R_{c\_LT1}+R_{b\_LT1}, R_{c\_LT2}+R_{b\_LT2})] = 0.274 < 1.000 \dots\dots \text{O.K} \end{aligned}$$

##### Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.000 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.004 < 1.000 \dots\dots\dots \text{O.K}$$



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- 
- 23+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 23- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 24+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 24- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 25+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 25- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 28+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 28- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 29+ 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 29- 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30+ 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30- 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 31+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 31- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 32+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
  - 32- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
  - 33+ 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 33- 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 34+ 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 34- 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 35+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 35- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
-

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\*. PROJECT : Viadotto Agrò  
 \*. MEMBER NO = 2045, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 22-, MATERIAL NO = 1, SECTION NO = 19  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = HSUP\_PILE 2L100x10d20  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.100, Flg Width = 0.100, BTB Spacing = 0.020  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.80000e-003, Avy = 2.00000e-003, Avz = 2.00000e-003  
 Ybar = 1.10000e-001, Zbar = 7.13158e-002, Qyb = 2.54297e-003, Qzb = 5.00000e-003  
 Wely = 5.04809e-005, Welz = 8.44242e-005, Wply = 9.09500e-005, Wplz = 1.47000e-004  
 Iyy = 3.60009e-006, Izz = 9.28667e-006, Iyz = 0.00000e+000  
 iy = 3.07797e-002, iz = 4.94354e-002  
 J = 1.26667e-007, Cwp = 8.81319e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.20000e+000, Lz = 3.20000e+000, Lu = 3.20000e+000  
 Ky = 6.50000e-001, Kz = 6.50000e-001

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = -1.35953e+002  
 Shear Forces Fyy = -4.81978e-002, Fzz = 4.13173e-001  
 Bending Moments My = -1.36383e+000, Mz = -1.46225e-001  
 End Moments Myi = -8.77770e-001, Myj = -1.36383e+000 (for Lb)  
                   Myi = -8.77770e-001, Myj = -1.36383e+000 (for Ly)  
                   Mzi = -7.33964e-002, Mzj = -1.46225e-001 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

[[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).

( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -. e = SQRT( 235/fy ) = 0.81  
 -. b/t = BTR = 9.00  
 -. sigma1 = 26642.724 KPa.  
 -. sigma2 = 25068.152 KPa.  
 -. BTR\_L = (H+B) / (2\*tf) = 10.00  
 -. BTR > 15\*e or BTR\_L > 11.5\*e ( Class 4 : Slender ).

-----  
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 =====

=====  
 [[[\*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  
 =====

- ( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
- b/t = BTR = 9.00
- sigma1 = 24753.237 KPa.
- sigma2 = 23178.665 KPa.
- BTR\_L = (H+B) / (2\*tf) = 10.00
- BTR > 15\*e or BTR\_L > 11.5\*e ( Class 4 : Slender ).

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

- ( ). Determine classification of compression element(Double angles).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
- d/t = HTR = 10.00
- sigma1 = 63108.783 KPa.
- sigma2 = 29013.926 KPa.
- HTR\_L = (h+B) / (2\*tw) = 10.00
- HTR > 15\*e or HTR\_L > 11.5\*e ( Class 4 : Slender ).

=====  
 [[[\*]]] CALCULATE EFFECTIVE AREA.  
 =====

- ( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- In case of Psi = 1.0
- k\_sigma = 0.4300
  
- ( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- RatT = 9.0000
- Lambda\_p = RatT / [ 28.4\*Eps\*SQRT(k\_sigma) ] = 0.5940
- Rho = 1.0
- sigma\_max = MAX( sigma1, sigma2 ) = 35777.184 KPa.
- sigma\_min = MIN( sigma1, sigma2 ) = 35777.184 KPa.
- r = 0.000 m.
- bc = 0.100 m.
- beff = Rho\*bc + r = 0.100 m.
- Aeff = beff \* tf = 0.001 m^2.
- yeff = beff/2 = 0.050 m.
  
- ( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]



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- In case of  $\Psi = 1.0$
- $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $RatT = 9.0000$
- $\Lambda_p = RatT / [ 28.4 * \epsilon * \sqrt{k_{\sigma}} ] = 0.5940$
- $\rho = 1.0$
- $\sigma_{max} = MAX(\sigma_1, \sigma_2) = 35777.184 \text{ KPa.}$
- $\sigma_{min} = MIN(\sigma_1, \sigma_2) = 35777.184 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $bc = 0.100 \text{ m.}$
- $b_{eff} = \rho * bc + r = 0.100 \text{ m.}$
- $A_{eff} = b_{eff} * t_f = 0.001 \text{ m}^2.$
- $y_{eff} = b_{eff} / 2 = 0.050 \text{ m.}$

( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]

- In case of  $\Psi = 1.0$
- $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $RatT = 10.0000$
- $\Lambda_p = RatT / [ 28.4 * \epsilon * \sqrt{k_{\sigma}} ] = 0.6600$
- $\rho = 1.0$
- $\sigma_{max} = MAX(\sigma_1, \sigma_2) = 35777.184 \text{ KPa.}$
- $\sigma_{min} = MIN(\sigma_1, \sigma_2) = 35777.184 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $A_r = 0.000 \text{ m}^2.$
- $dc = 0.090 \text{ m.}$
- $d_{eff1} = 2 * (\rho * dc) / [ 5 - \sigma_{min} / \sigma_{max} ] + r = 0.045 \text{ m.}$
- $A_{eff1} = d_{eff1} * t_w + 2 * A_r = 4.500e-004 \text{ m}^2.$
- $z_{eff1} = d_{eff1} / 2 + t_f = 0.022 \text{ m.}$
- $d_{eff2} = (\rho * dc) - d_{eff1} + r = 0.045 \text{ m.}$
- $A_{eff2} = d_{eff2} * t_w + 2 * A_r = 4.500e-004 \text{ m}^2.$
- $z_{eff2} = (h + 2 * r) - d_{eff2} / 2 + t_f = 0.068 \text{ m.}$

( ). Calculate buckling factor of outstand compression element.  
 [ Eurocode3 Part 1-5 4.4, Table 4.2 ]

- In case of  $\Psi = 1.0$
- $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).  
 [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]

- $RatT = 10.0000$
- $\Lambda_p = RatT / [ 28.4 * \epsilon * \sqrt{k_{\sigma}} ] = 0.6600$
- $\rho = 1.0$

-----  
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 =====

- sigma\_max = MAX( sigma1, sigma2 ) = 35777.184 KPa.
- sigma\_min = MIN( sigma1, sigma2 ) = 35777.184 KPa.
- r = 0.000 m.
- Ar = 0.000 m<sup>2</sup>.
- dc = 0.090 m.
- deff1 = 2\*(Rho\*dc) / [ 5 - sigma\_min/sigma\_max ] + r = 0.045 m.
- Aeff1 = deff1 \* tw + 2\*Ar = 4.500e-004 m<sup>2</sup>.
- zeff1 = deff1/2 + tf = 0.022 m.
- deff2 = (Rho\*dc) - deff1 + r = 0.045 m.
- Aeff2 = deff2 \* tw + 2\*Ar = 4.500e-004 m<sup>2</sup>.
- zeff2 = (h+2\*r) - deff2/2 + tf = 0.068 m.

=====  
 [[[\*]]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MAJOR AXIS.  
 =====

( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- Rho = 1.0 (Only tensile stresses).
  - sigma\_max = MAX( sigma1, sigma2 ) = -10866.489 KPa.
  - sigma\_min = MIN( sigma1, sigma2 ) = -10866.489 KPa.
  - r = 0.000 m.
  - bc = 0.100 m.
  - beff = Rho\*bc + r = 0.100 m.
  - Aeff = beff \* tf = 0.001 m<sup>2</sup>.
  - yeff = beff/2 = 0.050 m.

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- Rho = 1.0 (Only tensile stresses).
  - sigma\_max = MAX( sigma1, sigma2 ) = -10866.489 KPa.
  - sigma\_min = MIN( sigma1, sigma2 ) = -10866.489 KPa.
  - r = 0.000 m.
  - bc = 0.100 m.
  - beff = Rho\*bc + r = 0.100 m.
  - Aeff = beff \* tf = 0.001 m<sup>2</sup>.
  - yeff = beff/2 = 0.050 m.

( ). Calculate buckling factor of outstand compression element.

- [ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- k\_sigma = 0.57 - 0.21\*Psi + 0.07\*Psi<sup>2</sup> = 0.6298

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- RatT = 10.0000
  - Lambda\_p = RatT / [ 28.4\*Eps\*SQRT(k\_sigma) ] = 0.5453
  - Rho = 1.0

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- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 27016.685 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -7078.172 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $A_r = 0.000 \text{ m}^2.$
- $dc = (h \cdot \sigma_{max}) / (\sigma_{max} - \sigma_{min}) = 0.071 \text{ m.}$
- $deff1 = 0.4 \cdot \rho \cdot dc + r = 0.029 \text{ m.}$
- $A_{eff1} = deff1 \cdot t_w + 2 \cdot A_r = 2.853e-004 \text{ m}^2.$
- $z_{eff1} = deff1 / 2 + t_f = 0.014 \text{ m.}$
- $deff2 = 0.6 \cdot \rho \cdot dc + (h - dc) + r = 0.061 \text{ m.}$
- $A_{eff2} = deff2 \cdot t_w + 2 \cdot A_r = 6.147e-004 \text{ m}^2.$
- $z_{eff2} = (h + 2 \cdot r) - deff2 / 2 + t_f = 0.059 \text{ m.}$

( ). Calculate buckling factor of outstand compression element.

- [ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- $k_{\sigma} = 0.57 - 0.21 \cdot \psi + 0.07 \cdot \psi^2 = 0.6298$

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- $RatT = 10.0000$
- $\lambda_p = RatT / [ 28.4 \cdot \epsilon \cdot \sqrt{k_{\sigma}} ] = 0.5453$
- $\rho = 1.0$
- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 27016.685 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -7078.172 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $A_r = 0.000 \text{ m}^2.$
- $dc = (h \cdot \sigma_{max}) / (\sigma_{max} - \sigma_{min}) = 0.071 \text{ m.}$
- $deff1 = 0.4 \cdot \rho \cdot dc + r = 0.029 \text{ m.}$
- $A_{eff1} = deff1 \cdot t_w + 2 \cdot A_r = 2.853e-004 \text{ m}^2.$
- $z_{eff1} = deff1 / 2 + t_f = 0.014 \text{ m.}$
- $deff2 = 0.6 \cdot \rho \cdot dc + (h - dc) + r = 0.061 \text{ m.}$
- $A_{eff2} = deff2 \cdot t_w + 2 \cdot A_r = 6.147e-004 \text{ m}^2.$
- $z_{eff2} = (h + 2 \cdot r) - deff2 / 2 + t_f = 0.059 \text{ m.}$

[[[\*]]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MINOR AXIS.

( ). Calculate buckling factor of outstand compression element.

- [ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- $k_{\sigma} = 0.57 - 0.21 \cdot \psi + 0.07 \cdot \psi^2 = 0.5515$

( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- $RatT = 9.0000$
- $\lambda_p = RatT / [ 28.4 \cdot \epsilon \cdot \sqrt{k_{\sigma}} ] = 0.5245$
- $\rho = 1.0$

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- $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 1732.029 \text{ KPa.}$
- $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = 157.457 \text{ KPa.}$
- $r = 0.000 \text{ m.}$
- $bc = 0.100 \text{ m.}$
- $b_{eff} = \rho \cdot bc + r = 0.100 \text{ m.}$
- $A_{eff} = b_{eff} \cdot t_f = 0.001 \text{ m}^2.$
- $y_{eff} = b_{eff}/2 = 0.050 \text{ m.}$

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- $\rho = 1.0$  (Only tensile stresses).
  - $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = -157.457 \text{ KPa.}$
  - $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -1732.029 \text{ KPa.}$
  - $r = 0.000 \text{ m.}$
  - $bc = 0.100 \text{ m.}$
  - $b_{eff} = \rho \cdot bc + r = 0.100 \text{ m.}$
  - $A_{eff} = b_{eff} \cdot t_f = 0.001 \text{ m}^2.$
  - $y_{eff} = b_{eff}/2 = 0.050 \text{ m.}$

( ). Calculate buckling factor of outstand compression element.

- [ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- In case of  $\psi = 1.0$
  - $k_{\sigma} = 0.4300$

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- $RatT = 10.0000$
  - $\lambda_p = RatT / [ 28.4 \cdot \epsilon \cdot \sqrt{k_{\sigma}} ] = 0.6600$
  - $\rho = 1.0$
  - $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = 314.914 \text{ KPa.}$
  - $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = 314.914 \text{ KPa.}$
  - $r = 0.000 \text{ m.}$
  - $A_r = 0.000 \text{ m}^2.$
  - $dc = 0.090 \text{ m.}$
  - $d_{eff1} = 2 \cdot (\rho \cdot dc) / [ 5 - \sigma_{min}/\sigma_{max} ] + r = 0.045 \text{ m.}$
  - $A_{eff1} = d_{eff1} \cdot t_w + 2 \cdot A_r = 4.500e-004 \text{ m}^2.$
  - $z_{eff1} = d_{eff1}/2 + t_f = 0.022 \text{ m.}$
  - $d_{eff2} = (\rho \cdot dc) - d_{eff1} + r = 0.045 \text{ m.}$
  - $A_{eff2} = d_{eff2} \cdot t_w + 2 \cdot A_r = 4.500e-004 \text{ m}^2.$
  - $z_{eff2} = (h+2 \cdot r) - d_{eff2}/2 + t_f = 0.068 \text{ m.}$

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).

- [ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- $\rho = 1.0$  (Only tensile stresses).
  - $\sigma_{max} = \text{MAX}(\sigma_1, \sigma_2) = -314.914 \text{ KPa.}$
  - $\sigma_{min} = \text{MIN}(\sigma_1, \sigma_2) = -314.914 \text{ KPa.}$
  - $r = 0.000 \text{ m.}$
  - $A_r = 0.000 \text{ m}^2.$
  - $dc = 0.090 \text{ m.}$
  - $d_{eff} = dc + r = 0.090 \text{ m.}$
  - $A_{eff} = d_{eff} \cdot t_w + 4 \cdot A_r = 9.000e-004 \text{ m}^2.$
  - $z_{eff} = (h+2 \cdot r) - d_{eff}/2 = 0.045 \text{ m.}$

-----  
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 =====

=====  
 [[[\*]]] EFFECTIVE SECTION PPOPERTIES.  
 =====

- ( ). Calculated effective cross-section properties of Class4 cross-section.
- Aeff = 0.0038 m<sup>2</sup>. (for calculating axial resistance)
  - Aeffy = 0.0038 m<sup>2</sup>.
  - Weffy = 5.0481e-005 m<sup>3</sup>.
  - Aeffz = 0.0038 m<sup>2</sup>.
  - Weffz = 7.0867e-005 m<sup>3</sup>.
  - eNy = 0.0000 m.
  - eNz = 0.0000 m.

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).  
 [ Eurocode3:05 Annex A. Table A.1, A.2 ]
- Cmy,0 = 0.921
  - Cmz,0 = 0.969
  - Cmy (Default or User Defined Value) = 1.000
  - Cmz (Default or User Defined Value) = 1.000
  - CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).  
 [ Eurocode3:05 6.1 ]
- Gamma\_M0 = 1.05
  - Gamma\_M1 = 1.10
  - Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial compression member (Kl/i).  
 [ Eurocode3:05 6.3.1 ]
- Kl/i = 67.6 < 200.0 ---> O.K.
- ( ). Calculate axial compressive resistance (Nc\_Rd).  
 [ Eurocode3:05 6.1, 6.2.4 ]
- Nc\_Rd = fy \* Aeff / Gamma\_M0 = 1284.76 kN.
- ( ). Check ratio of axial resistance (N\_Ed/Nc\_Rd).
- N\_Ed            135.95
- ----- = ----- = 0.106 < 1.000 ---> O.K.
- Nc\_Rd           1284.76

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( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).

[ Eurocode3:05 6.3.1.1, 6.3.1.2 ]

- Beta\_A = Aeff / Area = 1.000
- Lambda1 = Pi \* SQRT(Es/fy) = 76.409
- Lambda\_by = {(KLy/iy)/Lambda1} \* SQRT(Beta\_A) = 0.884
- Ncry = Pi^2\*Es\*Ryy / KLy^2 = 1724.67 kN.
- Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.
- Alphay = 0.340
- Phiy = 0.5 \* [ 1 + Alphay\*(Lambda\_by-0.2) + Lambda\_by^2 ] = 1.007
- Xiy = MIN [ 1 / [Phiy + SQRT(Phiy^2 - Lambda\_by^2)], 1.0 ] = 0.671
- Nb\_Rdy = Xiy\*Beta\_A\*Area\*fy / Gamma\_M1 = 823.13 kN.
  
- Lambda\_bz = {(KLz/iz)/Lambda1} \* SQRT(Beta\_A) = 0.551
- Ncrz = Pi^2\*Es\*Rzz / KLz^2 = 4448.90 kN.
- Lambda\_bz < 0.2 or N\_Ed/Ncrz < 0.04 --> No need to check.

( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).

- Nb\_Rd = MIN[ Nb\_Rdy, Nb\_Rdz ] = 823.13 kN.
- N\_Ed            135.95
- ----- = ----- = 0.165 < 1.000 ---> O.K.
- Nb\_Rd            823.13

[[[\*]]] CHECK SHEAR RESISTANCE.

( ). Calculate shear area.

[ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]

- Avy = 2\*B\*tf = 0.0020 m^2.
- Avz = 2\*h\*tw = 0.0020 m^2.

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).

[ Eurocode3:05 6.1, 6.2.6 ]

- Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 390.40 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).

( LCB = 19+, POS = J )

- Applied shear force : V\_Edy = 0.11 kN.
- V\_Edy            0.11
- ----- = ----- = 2.800e-004 < 1.000 ---> O.K.
- Vpl\_Rdy        390.40

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).

[ Eurocode3:05 6.1, 6.2.6 ]

- Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 390.40 kN.

( ). Shear Buckling Check.

[ Eurocode3:05 6.2.6 ]

- HTR < 72\*e/Eta ---> No need to check!



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( ). Check ratio of shear resistance ( $V_{Edz}/V_{pl,Rdz}$ ).  
 ( LCB = 19+, POS = J )  
 - Applied shear force :  $V_{Edz} = 1.56$  kN.  
 $V_{Edz} \quad 1.56$   
 -  $\frac{\quad}{V_{pl,Rdz} \quad 390.40} = 0.004 < 1.000 \rightarrow$  O.K.

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.

( ). Calculate local buckling resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -  $W_{effy} = 5.0481e-005$  m<sup>3</sup>.  
 -  $M_{c,Rdy} = W_{effy} * f_y / \Gamma_{M1} = 16.29$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edy}/M_{c,Rdy}$ ).  
 $M_{Edy} \quad 1.36$   
 -  $\frac{\quad}{M_{c,Rdy} \quad 16.29} = 0.084 < 1.000 \rightarrow$  O.K.

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.

( ). Calculate local buckling resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -  $W_{effz} = 7.0867e-005$  m<sup>3</sup>.  
 -  $M_{c,Rdz} = W_{effz} * f_y / \Gamma_{M1} = 22.87$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edz}/M_{c,Rdz}$ ).  
 $M_{Edz} \quad 0.15$   
 -  $\frac{\quad}{M_{c,Rdz} \quad 22.87} = 0.006 < 1.000 \rightarrow$  O.K.

[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.

( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 - In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$   
 -  $M_{y,Rd} = M_{c,Rdy} = 16.29$  kN-m.

( ). Calculate Minor reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 - In case of  $V_{Edy} / V_{pl,Rdy} < 0.5$   
 -  $M_{z,Rd} = M_{c,Rdz} = 22.87$  kN-m.

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( ). Check interaction ratio of bending and axial force.

[ Eurocode3:05 6.2.9.3 (6.44) ] - Class4

$$\begin{aligned}
 & N_{Ed} \quad M_{Edy} + N_{Ed} \cdot e_{Nz} \quad M_{Edz} + N_{Ed} \cdot e_{Ny} \\
 - \text{Rmax1} &= \frac{\quad}{A_{eff} \cdot f_y / \Gamma_{M0}} + \frac{\quad}{M_{y,Rd}} + \frac{\quad}{M_{z,Rd}} \\
 &= 0.196 < 1.000 \text{ ---> O.K.}
 \end{aligned}$$

( ). Check interaction ratio of bending and axial compression member.

[ Eurocode3:05 6.3.1, 6.2.9.3 (6.61, 6.62), Annex A ]

- $N_{Ed} = -135.95 \text{ kN.}$
- $M_{Edy} = -1.36 \text{ kN-m.}$
- $M_{Edz} = -0.15 \text{ kN-m.}$
- $k_{yy} = 1.056$
- $k_{yz} = 1.003$
- $k_{zy} = 1.081$
- $k_{zz} = 1.027$
- $X_{iy} = 0.671$
- $X_{iz} = 0.861$
- $X_{iLT} = 0.863$
- $A_{eff} = 0.0038 \text{ m}^2.$
- $W_{effy} = 5.0481e-005 \text{ m}^3.$
- $W_{effz} = 7.0867e-005 \text{ m}^3.$
- $e_{Ny} = 0.0000 \text{ m.}$
- $e_{Nz} = 0.0000 \text{ m.}$
- $N_{Rk} = A_{eff} \cdot f_y = 1349.00 \text{ kN.}$
- $M_{y,Rk} = W_{effy} \cdot f_y = 17.92 \text{ kN-m.}$
- $M_{z,Rk} = W_{effz} \cdot f_y = 25.16 \text{ kN-m.}$
- $N_{Ed} \cdot e_{Ny} = 0.00 \text{ kN-m.}$
- $N_{Ed} \cdot e_{Nz} = 0.00 \text{ kN-m.}$
- $R_{max\_LT1} = \frac{N_{Ed}}{X_{iy} \cdot N_{Rk} / \Gamma_{M1}} + k_{yy} \cdot \frac{M_{Edy} + N_{Ed} \cdot e_{Ny}}{X_{iLT} \cdot M_{y,Rk} / \Gamma_{M1}} + k_{yz} \cdot \frac{M_{Edz} + N_{Ed} \cdot e_{Nz}}{M_{z,Rk} / \Gamma_{M1}}$   
 $= 0.274 < 1.000 \text{ ---> O.K.}$
- $R_{max\_LT2} = \frac{N_{Ed}}{X_{iz} \cdot N_{Rk} / \Gamma_{M1}} + k_{zy} \cdot \frac{M_{Edy} + N_{Ed} \cdot e_{Ny}}{X_{iLT} \cdot M_{y,Rk} / \Gamma_{M1}} + k_{zz} \cdot \frac{M_{Edz} + N_{Ed} \cdot e_{Nz}}{M_{z,Rk} / \Gamma_{M1}}$   
 $= 0.240 < 1.000 \text{ ---> O.K.}$
- $R_{max} = \text{MAX}[ R_{max1}, \text{MAX}(R_{max\_LT1}, R_{max\_LT2}) ] = 0.274 < 1.000 \text{ ---> O.K.}$

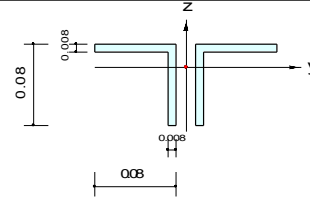
## 10.6 DIAFRAMMI SU PILE – DIAGONALI

### MIDAS/Civil Steel Checking Result

<b>Company</b>		<b>Project Title</b>	Viadotto Agrò
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...\Cavalc Coste V_1.mcb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 2034  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : DIAG\_PILE 2L80x8d20 (No:20)  
 (Built-up Section).  
 Member Length : 1.78220



#### 2. Member Forces

Axial Force Fxx = 267.493 (LCB: 19+, POS:J)  
 Bending Moments My = 0.50268, Mz = 0.11710  
 End Moments Myi = 0.07638, Myj = 0.50268 (for Lb)  
 Myi = 0.07638, Myj = 0.50268 (for Ly)  
 Mzi = 0.18668, Mzj = 0.11710 (for Lz)  
 Shear Forces Fyy = -0.0630 (LCB: 19-, POS:I)  
 Fzz = -0.5865 (LCB: 19-, POS:J)

Depth	0.08000	Web Thick	0.00800
Flg Width	0.08000	Flg Thick	0.00800
BTB Spacing	0.02000		
Area	0.00243	Asz	0.00107
Cyb	0.00163	Czb	0.00320
Iyy	0.00000	Izz	0.00000
Ybar	0.09000	Zbar	0.05705
Wely	0.00003	Welz	0.00005
ry	0.02462	rz	0.04113

#### 3. Design Parameters

Unbraced Lengths Ly = 1.78220, Lz = 1.78220, Lb = 1.78220  
 Effective Length Factors Ky = 0.65, Kz = 1.30  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

Axial Resistance  
 $N_{Ed}/N_{t,Rd} = 267.493/822.248 = 0.325 < 1.000$  ..... O.K

Bending Resistance  
 $M_{Edy}/M_{Rdy} = 0.5027/15.7439 = 0.032 < 1.000$  ..... O.K  
 $M_{Edz}/M_{Rdz} = 0.1171/27.0909 = 0.004 < 1.000$  ..... O.K

Combined Resistance  
 $R_{NRd} = \text{MAX}[ M_{Edy}/M_{ny,Rd}, M_{Edz}/M_{nz,Rd} ]$   
 $R_{max1} = (M_{Edy}/M_{ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz,Rd})^{\text{Beta}}$   
 $R_{com} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$   
 $R_{max} = \text{MAX}[ R_{NRd}, R_{max1}, (R_{com} + R_{bend}) ] = 0.362 < 1.000$  ..... O.K

Shear Resistance  
 $V_{Edy}/V_{y,Rd} = 0.000 < 1.000$  ..... O.K  
 $V_{Edz}/V_{z,Rd} = 0.002 < 1.000$  ..... O.K



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- 
- 23+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 23- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
  - 24+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 24- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
  - 25+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 25- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
  - 28+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 28- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
  - 29+ 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 29- 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30+ 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 30- 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
  - 31+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 31- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
  - 32+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
  - 32- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
  - 33+ 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 33- 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 34+ 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 34- 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
  - 35+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
  - 35- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
-

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\*. PROJECT : Viadotto Agrò  
 \*. MEMBER NO = 2034, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19+, MATERIAL NO = 1, SECTION NO = 20  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = DIAG\_PILE 2L80x8d20  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.020  
 Web Thick = 0.008, Flg Thick = 0.008

Area = 2.43200e-003, Avy = 1.28000e-003, Avz = 1.28000e-003  
 Ybar = 9.00000e-002, Zbar = 5.70526e-002, Qyb = 1.62750e-003, Qzb = 3.20000e-003  
 Wely = 2.58462e-005, Welz = 4.57178e-005, Wply = 4.65664e-005, Wplz = 8.01280e-005  
 Iyy = 1.47460e-006, Izz = 4.11460e-006, Iyz = 0.00000e+000  
 iy = 2.46238e-002, iz = 4.11322e-002  
 J = 5.18827e-008, Cwp = 2.31033e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 1.78220e+000, Lz = 1.78220e+000, Lu = 1.78220e+000  
 Ky = 6.50000e-001, Kz = 1.30000e+000

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = 2.67493e+002  
 Shear Forces Fyy = 3.90550e-002, Fzz = -1.90850e-001  
 Bending Moments My = 5.02682e-001, Mz = 1.17105e-001  
 End Moments Myi = 7.63805e-002, Myj = 5.02682e-001 (for Lb)  
                   Myi = 7.63805e-002, Myj = 5.02682e-001 (for Ly)  
                   Mzi = 1.86676e-001, Mzj = 1.17105e-001 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of tension flanges(double angle).  
 - Not Checking the Section Classification.

( ). Determine classification of tension flanges(double angle).  
 - Not Checking the Section Classification.

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====



-----  
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 =====

- ( ). Determine classification of tension elements(Double angle).
- Not Checking the Section Classification.

=====

[[[\*]]] APPLIED FACTORS.

=====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).
- [ Eurocode3:05 Annex A. Table A.1, A.2 ]
- Cmy,0 = 0.883
- Cmz,0 = 0.940
- Cmy (Default or User Defined Value) = 1.000
- Cmz (Default or User Defined Value) = 1.000
- CmLT (Default or User Defined Value) = 1.000

- ( ). Partial Factors (Gamma\_Mi).
- [ Eurocode3:05 6.1 ]
- Gamma\_M0 = 1.05
- Gamma\_M1 = 1.10
- Gamma\_M2 = 1.25

=====

[[[\*]]] CHECK AXIAL RESISTANCE.

=====

- ( ). Check slenderness ratio of axial tension member (l/i).
- [ Eurocode3:05 6.3.1 ]
- l/i = 72.4 < 300.0 ----> O.K.

- ( ). Calculate parameters for combined resistance.
- Lambda1 = Pi \* SQRT(Es/fy) = 76.409
- Lambda\_bz = (KLz/iz) / Lambda1 = 0.567

- ( ). Calculate axial tensile resistance (Nt\_Rd).
- [ Eurocode3:05 6.2.3 ]
- Nt\_Rd = fy \* Area / Gamma\_M0 = 822.25 kN.

- ( ). Check ratio of axial resistance (N\_Ed/Nt\_Rd).
- |       |                            |
|-------|----------------------------|
| N_Ed  | 267.49                     |
| -     | -----                      |
|       | = 0.325 < 1.000 ----> O.K. |
| Nt_Rd | 822.25                     |

=====

[[[\*]]] CHECK SHEAR RESISTANCE.

=====

- ( ). Calculate shear area.
- [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]
- Avy = 2\*B\*tf = 0.0013 m^2.
- Avz = 2\*h\*tw = 0.0013 m^2.

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( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 - Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 =      249.86 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19-, POS = J )  
 - Applied shear force : V\_Edy =      0.06 kN.  
   V\_Edy                  0.06  
 - ----- = ----- =2.520e-004 < 1.000 ---> O.K.  
   Vpl\_Rdy                249.86

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 - Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 =      249.86 kN.

( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 - HTR < 72\*e/Eta ---> No need to check!

( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19-, POS = J )  
 - Applied shear force : V\_Edz =      0.59 kN.  
   V\_Edz                  0.59  
 - ----- = ----- = 0.002 < 1.000 ---> O.K.  
   Vpl\_Rdz                249.86

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.

( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 - Wply = 4.6566e-005 m^3.  
 - Mc\_Rdy = Wply \* fy / Gamma\_M0 =      15.74 kN-m.

( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  
   M\_Edy                  0.50  
 - ----- = ----- = 0.032 < 1.000 ---> O.K.  
   Mc\_Rdy                15.74

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.

( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 - Wplz = 8.0128e-005 m^3.  
 - Mc\_Rdz = Wplz \* fy / Gamma\_M0 =      27.09 kN-m.

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( ). Check ratio of moment resistance ( $M_{Edz}/M_{c\_Rdz}$ ).

$$\frac{M_{Edz}}{M_{c\_Rdz}} = \frac{0.12}{27.09} = 0.004 < 1.000 \text{ ---> O.K.}$$

[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.

( ). Calculate Major reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]

- In case of  $V_{Edz} / V_{pl\_Rdz} < 0.5$

-  $M_{y\_Rd} = M_{c\_Rdy} = 15.74 \text{ kN-m.}$

( ). Calculate Minor reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]

- In case of  $V_{Edy} / V_{pl\_Rdy} < 0.5$

-  $M_{z\_Rd} = M_{c\_Rdz} = 27.09 \text{ kN-m.}$

( ). Check general interaction ratio.

[ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2

$$R_{max1} = \frac{N_{Ed}}{N_{Rd}} + \frac{M_{Edy}}{M_{y\_Rd}} + \frac{M_{Edz}}{M_{z\_Rd}} = 0.362 < 1.000 \text{ ---> O.K.}$$

( ). Check interaction ratio of bending and axial force member.

[ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2

-  $n = N_{Ed} / N_{pl\_Rd} = 0.325$

-  $a = \text{MIN}[(\text{Area}-2b*tf)/\text{Area}, 0.5] = 0.500$

-  $\text{Alpha} = 2.000$

-  $\text{Beta} = \text{MAX}[5*n, 1.0] = 1.627$

-  $M_{ny\_Rd} = \text{MIN}[M_{ply\_Rd}*(1-n)/(1-0.5*a), M_{ply\_Rd}] = 14.16 \text{ kN-m.}$

-  $R_{maxy} = M_{Edy} / M_{ny\_Rd} = 0.035 < 1.000 \text{ ---> O.K.}$

- In case of  $n < a$

-  $M_{nz\_Rd} = M_{plz\_Rd} = 27.09 \text{ kN-m.}$

-  $R_{maxz} = M_{Edz} / M_{nz\_Rd} = 0.004 < 1.000 \text{ ---> O.K.}$

$$R_{max2} = \left[ \frac{|M_{Edy}|^{\text{Alpha}}}{|M_{ny\_Rd}|} + \frac{|M_{Edz}|^{\text{Beta}}}{|M_{nz\_Rd}|} \right] = 0.001 < 1.000 \text{ ---> O.K.}$$

-  $R_{max} = \text{MAX}[R_{max1}, R_{max2}] = 0.362 < 1.000 \text{ ---> O.K.}$

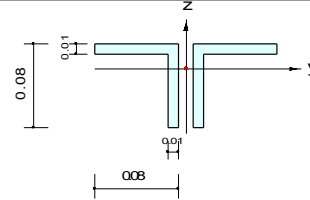
## 10.7 CONTROVENTI

### MIDAS/Civil Steel Checking Result

<b>Company</b>		<b>Project Title</b>	Viadotto Agrò
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...Cavalc Coste V_1.mdb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 3148  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : CONTROV 2L80x10d15 (No:15)  
 (Built-up Section).  
 Member Length : 3.05287



#### 2. Member Forces

Axial Force Fxx = -255.94 (LCB: 19-, POS:J)  
 Bending Moments My = -0.2859, Mz = -2.5854  
 End Moments Myi = -0.0070, Myj = -0.2859 (for Lb)  
 Myi = -0.0070, Myj = -0.2859 (for Ly)  
 Mzi = -0.0494, Mzj = -2.5854 (for Lz)  
 Shear Forces Fyy = 1.57644 (LCB: 19+, POS:J)  
 Fzz = 0.19112 (LCB: 19+, POS:J)

Depth	0.08000	Web Thick	0.01000
Flg Width	0.08000	Flg Thick	0.01000
BTB Spacing	0.01500		
Area	0.00300	Asz	0.00133
Cyb	0.00159	Czb	0.00320
Iyy	0.00000	Izz	0.00000
Ybar	0.08750	Zbar	0.05633
Wely	0.00003	Welz	0.00005
ry	0.02436	rz	0.03955

#### 3. Design Parameters

Unbraced Lengths Ly = 3.05287, Lz = 3.05287, Lb = 3.05287  
 Effective Length Factors Ky = 0.65, Kz = 1.30  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

##### Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c,Rd}, N_{b,Rd}] = 255.941/407.225 = 0.628 < 1.000 \dots\dots\dots \text{O.K}$$

##### Bending Resistance

$$M_{Edy}/M_{Rdy} = 0.2859/19.2503 = 0.015 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 2.5854/31.6119 = 0.082 < 1.000 \dots\dots\dots \text{O.K}$$

##### Combined Resistance

$$R_{NRd} = \text{MAX}[M_{Edy}/M_{ny,Rd}, M_{Edz}/M_{nz,Rd}]$$

$$R_{max1} = (M_{Edy}/M_{ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz,Rd})^{\text{Beta}}$$

$$R_{oom} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$$

$$R_{c\_LT1} = N_{Ed}/(X_{iy} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT1} = (k_{yy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (k_{yz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{c\_LT2} = N_{Ed}/(X_{iz} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT2} = (K_{zy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (K_{zz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{max} = \text{MAX}[R_{NRd}, R_{max1}, (R_{oom} + R_{bend}), \text{MAX}(R_{c\_LT1} + R_{b\_LT1}, R_{c\_LT2} + R_{b\_LT2})] = 0.767 < 1.000 \dots\dots \text{O.K}$$

##### Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.005 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.001 < 1.000 \dots\dots\dots \text{O.K}$$



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- 
- 23+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
- 23- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
- 24+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
- 24- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
- 25+ 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
- 25- 1    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
- 28+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
- 28- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV( 1.000)
- 29+ 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
- 29- 2    Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV( 1.000)  
           +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
- 30+ 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
- 30- 2    Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)  
           +Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
- 31+ 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
- 31- 2    Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV( 0.600)  
           +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV( 1.000)
- 32+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
- 32- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
           +Mobili SLE INV( 1.000)
- 33+ 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
- 33- 2    Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV( 0.200)  
           +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
- 34+ 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
- 34- 2    Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV( 0.600)
- 35+ 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
- 35- 2    Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
-



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\*. PROJECT : Viadotto Agrò  
 \*. MEMBER NO = 3148, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19-, MATERIAL NO = 1, SECTION NO = 15  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = CONTROV 2L80x10d15  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.015  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.00000e-003, Avy = 1.60000e-003, Avz = 1.60000e-003  
 Ybar = 8.75000e-002, Zbar = 5.63333e-002, Qyb = 1.58672e-003, Qzb = 3.20000e-003  
 Wely = 3.15917e-005, Welz = 5.36429e-005, Wply = 5.69375e-005, Wplz = 9.35000e-005  
 Iyy = 1.77967e-006, Izz = 4.69375e-006, Iyz = 0.00000e+000  
 iy = 2.43562e-002, iz = 3.95548e-002  
 J = 1.00000e-007, Cwp = 4.24931e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.05287e+000, Lz = 3.05287e+000, Lu = 3.05287e+000  
 Ky = 6.50000e-001, Kz = 1.30000e+000

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = -2.55941e+002  
 Shear Forces Fyy = 1.12537e+000, Fzz = -9.49347e-003  
 Bending Moments My = -2.85899e-001, Mz = -2.58537e+000  
 End Moments Myi = -7.00124e-003, Myj = -2.85899e-001 (for Lb)  
                   Myi = -7.00124e-003, Myj = -2.85899e-001 (for Ly)  
                   Mzi = -4.93978e-002, Mzj = -2.58537e+000 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

[[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).

( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -.  $e = \sqrt{235/f_y} = 0.81$   
 -.  $b/t = BTR = 7.00$   
 -.  $\sigma_1 = 129707.724 \text{ KPa}$ .  
 -.  $\sigma_2 = 85642.750 \text{ KPa}$ .  
 -.  $BTR < 9 * e$  ( Class 1 : Plastic ).

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=====  
 [[[\*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  
 =====

- ( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
  - b/t = BTR = 7.00
  - sigma1 = 77380.568 KPa.
  - sigma2 = 33315.594 KPa.
  - BTR < 9\*e ( Class 1 : Plastic ).

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

- ( ). Determine classification of compression element(Double angles).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
- e = SQRT( 235/fy ) = 0.81
  - d/t = HTR = 8.00
  - sigma1 = 104002.678 KPa.
  - sigma2 = 92757.348 KPa.
  - HTR < 10\*e ( Class 2 : Compact ).

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).  
 [ Eurocode3:05 Annex A. Table A.1, A.2 ]
- Cmy,0 = 0.727
  - Cmz,0 = 0.586
  - Cmy (Default or User Defined Value) = 1.000
  - Cmz (Default or User Defined Value) = 1.000
  - CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).  
 [ Eurocode3:05 6.1 ]
- Gamma\_M0 = 1.05
  - Gamma\_M1 = 1.10
  - Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial compression member (Kl/i).  
 [ Eurocode3:05 6.3.1 ]
- Kl/i = 100.3 < 200.0 ---> O.K.

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( ). Calculate axial compressive resistance (Nc\_Rd).

[ Eurocode3:05 6.1, 6.2.4 ]

- Nc\_Rd = fy \* Area / Gamma\_M0 = 1014.29 kN.

( ). Check ratio of axial resistance (N\_Ed/Nc\_Rd).

N\_Ed            255.94  
 - ----- = ----- = 0.252 < 1.000 ----> O.K.  
 Nc\_Rd            1014.29

( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).

[ Eurocode3:05 6.3.1.1, 6.3.1.2 ]

- Beta\_A = Aeff / Area = 1.000  
 - Lambda1 = Pi \* SQRT(Es/fy) = 76.409  
 - Lambda\_by = {(KLy/iy)/Lambda1} \* SQRT(Beta\_A) = 1.066  
 - Ncry = Pi^2\*Es\*Ryy / KLy^2 = 936.73 kN.  
 - Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.  
 - Alphay = 0.340  
 - Phiy = 0.5 \* [ 1 + Alphay\*(Lambda\_by-0.2) + Lambda\_by^2 ] = 1.216  
 - Xiy = MIN [ 1 / [Phiy + SQRT(Phiy^2 - Lambda\_by^2)], 1.0 ] = 0.556  
 - Nb\_Rdy = Xiy\*Beta\_A\*Area\*fy / Gamma\_M1 = 537.95 kN.

- Lambda\_bz = {(KLz/iz)/Lambda1} \* SQRT(Beta\_A) = 1.313  
 - Ncrz = Pi^2\*Es\*Rzz / KLz^2 = 617.64 kN.  
 - Lambda\_bz > 0.2 and N\_Ed/Ncrz > 0.04 --> Need to check.  
 - Alphaz = 0.340  
 - Phiz = 0.5 \* [ 1 + Alphaz\*(Lambda\_bz-0.2) + Lambda\_bz^2 ] = 1.551  
 - Xiz = MIN [ 1 / [Phiz + SQRT(Phiz^2 - Lambda\_bz^2)], 1.0 ] = 0.421  
 - Nb\_Rdz = Xiz\*Beta\_A\*Area\*fy / Gamma\_M1 = 407.23 kN.

( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).

- Nb\_Rd = MIN[ Nb\_Rdy, Nb\_Rdz ] = 407.23 kN.  
 N\_Ed            255.94  
 - ----- = ----- = 0.628 < 1.000 ----> O.K.  
 Nb\_Rd            407.23

[[[\*]]] CHECK SHEAR RESISTANCE.

( ). Calculate shear area.

[ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]

- Avy = 2\*B\*tf = 0.0016 m^2.  
 - Avz = 2\*h\*tw = 0.0016 m^2.

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).

[ Eurocode3:05 6.1, 6.2.6 ]

- Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 312.32 kN.

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( ). Check ratio of shear resistance ( $V_{Edy}/V_{pl\_Rdy}$ ).

( LCB = 19+, POS = J )

- Applied shear force :  $V_{Edy} = 1.58$  kN.

$V_{Edy} \quad 1.58$

-  $\frac{V_{Edy}}{V_{pl\_Rdy}} = 0.005 < 1.000 \rightarrow$  O.K.

$V_{pl\_Rdy} \quad 312.32$

( ). Calculate plastic shear resistance in local-z direction ( $V_{pl\_Rdz}$ ).

[ Eurocode3:05 6.1, 6.2.6 ]

-  $V_{pl\_Rdz} = [ Avz \cdot fy / \sqrt{3} ] / \Gamma_{M0} = 312.32$  kN.

( ). Shear Buckling Check.

[ Eurocode3:05 6.2.6 ]

-  $HTR < 72 \cdot e / \eta \rightarrow$  No need to check!

( ). Check ratio of shear resistance ( $V_{Edz}/V_{pl\_Rdz}$ ).

( LCB = 19+, POS = J )

- Applied shear force :  $V_{Edz} = 0.19$  kN.

$V_{Edz} \quad 0.19$

-  $\frac{V_{Edz}}{V_{pl\_Rdz}} = 6.120e-004 < 1.000 \rightarrow$  O.K.

$V_{pl\_Rdz} \quad 312.32$

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.

( ). Calculate plastic resistance moment about major axis.

[ Eurocode3:05 6.1, 6.2.5 ]

-  $W_{ply} = 5.6938e-005$  m<sup>3</sup>.

-  $M_{c\_Rdy} = W_{ply} \cdot fy / \Gamma_{M0} = 19.25$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edy}/M_{c\_Rdy}$ ).

$M_{Edy} \quad 0.29$

-  $\frac{M_{Edy}}{M_{c\_Rdy}} = 0.015 < 1.000 \rightarrow$  O.K.

$M_{c\_Rdy} \quad 19.25$

[[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.

( ). Calculate plastic resistance moment about minor axis.

[ Eurocode3:05 6.1, 6.2.5 ]

-  $W_{plz} = 9.3500e-005$  m<sup>3</sup>.

-  $M_{c\_Rdz} = W_{plz} \cdot fy / \Gamma_{M0} = 31.61$  kN-m.

( ). Check ratio of moment resistance ( $M_{Edz}/M_{c\_Rdz}$ ).

$M_{Edz} \quad 2.59$

-  $\frac{M_{Edz}}{M_{c\_Rdz}} = 0.082 < 1.000 \rightarrow$  O.K.

$M_{c\_Rdz} \quad 31.61$

MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                      Version 8.3.5

[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.

( ). Calculate Major reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]

- In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$

-  $M_{y,Rd} = M_{c,Rdy} = 19.25$  kN-m.

( ). Calculate Minor reduced design resistance of bending and shear.

[ Eurocode3:05 6.2.8 (6.30) ]

- In case of  $V_{Edy} / V_{pl,Rdy} < 0.5$

-  $M_{z,Rd} = M_{c,Rdz} = 31.61$  kN-m.

( ). Check general interaction ratio.

[ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2

$$R_{max1} = \frac{N_{Ed}}{N_{Rd}} + \frac{M_{Edy}}{M_{y,Rd}} + \frac{M_{Edz}}{M_{z,Rd}}$$

= 0.349 < 1.000 ---> O.K.

( ). Check interaction ratio of bending and axial force member.

[ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2

-  $n = N_{Ed} / N_{pl,Rd} = 0.252$

-  $a = \text{MIN}[(\text{Area} - 2b \cdot t_f) / \text{Area}, 0.5] = 0.500$

-  $\text{Alpha} = 2.000$

-  $\text{Beta} = \text{MAX}[5 \cdot n, 1.0] = 1.262$

-  $M_{ny,Rd} = \text{MIN}[M_{ply,Rd} \cdot (1-n) / (1-0.5 \cdot a), M_{ply,Rd}] = 19.19$  kN-m.

-  $R_{maxy} = M_{Edy} / M_{ny,Rd} = 0.015 < 1.000$  ---> O.K.

- In case of  $n < a$

-  $M_{nz,Rd} = M_{plz,Rd} = 31.61$  kN-m.

-  $R_{maxz} = M_{Edz} / M_{nz,Rd} = 0.082 < 1.000$  ---> O.K.

$$R_{max2} = \left[ \frac{|M_{Edy}|^{\text{Alpha}}}{M_{ny,Rd}^{\text{Alpha}}} + \frac{|M_{Edz}|^{\text{Beta}}}{M_{nz,Rd}^{\text{Beta}}} \right]$$

= 0.043 < 1.000 ---> O.K.

MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                      Version 8.3.5

( ). Check interaction ratio of bending and axial compression member.

[ Eurocode3:05 6.3.1, 6.2.9.3 (6.61, 6.62), Annex A ]

- N\_Ed = -255.94 kN.

- M\_Edy = -0.29 kN-m.

- M\_Edz = -2.59 kN-m.

- kyy = 1.410

- kyz = 1.043

- kzy = 0.694

- kzz = 1.453

- Xiy = 0.556

- Xiz = 0.421

- XiLT = 0.780

- N\_Rk = A\*fy = 1065.00 kN.

- My\_Rk = Wply\*fy = 20.21 kN-m.

- Mz\_Rk = Wplz\*fy = 33.19 kN-m.

- N\_Ed\*eNy = 0.0 (Not Slender)

- N\_Ed\*eNz = 0.0 (Not Slender)

$$- R_{max\_LT1} = \frac{N_{Ed}}{X_{iy} * N_{Rk} / \Gamma_{M1}} + k_{yy} * \frac{M_{Edy} + N_{Ed} * e_{Ny}}{X_{iLT} * M_{y\_Rk} / \Gamma_{M1}} + k_{yz} * \frac{M_{Edz} + N_{Ed} * e_{Nz}}{M_{z\_Rk} / \Gamma_{M1}}$$

= 0.593 < 1.000 ---> O.K.

$$- R_{max\_LT2} = \frac{N_{Ed}}{X_{iz} * N_{Rk} / \Gamma_{M1}} + k_{zy} * \frac{M_{Edy} + N_{Ed} * e_{Ny}}{X_{iLT} * M_{y\_Rk} / \Gamma_{M1}} + k_{zz} * \frac{M_{Edz} + N_{Ed} * e_{Nz}}{M_{z\_Rk} / \Gamma_{M1}}$$

= 0.767 < 1.000 ---> O.K.

- Rmax = MAX[ MAX(Rmax1, Rmax2), MAX(Rmax\_LT1, Rmax\_LT2) ] = 0.767 < 1.000 ---> O.K.

## 11 VERIFICA DEI COLLEGAMENTI SALDATI

### 11.1 TRAVI PRINCIPALI - GIUNTI TRA I CONCI

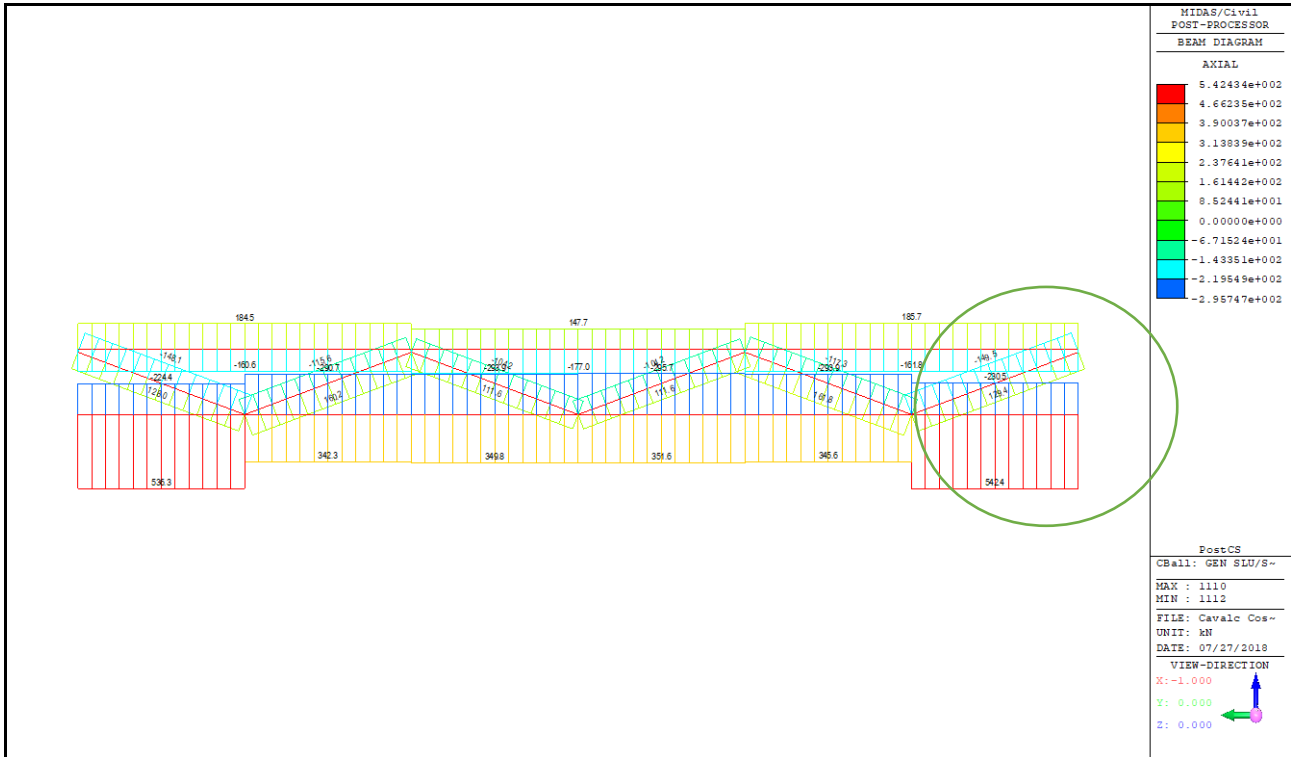
I vari concetti sono uniti con saldature di testa a piena penetrazione; supponendo che il materiale di saldatura abbia caratteristiche resistenti maggiori delle travi si omettono ulteriori verifiche.

### 11.2 TRAVI PRINCIPALI - SALDATURE PIASTRE DIAFRAMMI - VERIFICA SLU

I piatti su cui sono imbullonati i profilati dei diaframmi sono saldati alle travi con doppi cordoni di 8 mm di lato; si verifica il piatto collegato al diaframma più sollecitato.

Le sollecitazioni massime a cui sono sottoposte le saldature sono ricercate, a favore della sicurezza, nella combinazione involuppo di tutte le SLU statiche e SLV sismiche, componendo le azioni assiali di trazione dei traversi inferiori, superiori e diagonali.





**COMBINAZIONE INVILUPPO SLU/SLV - MASSIMI SFORZI ASSIALI DIAFRAMMA INTERMEDIO (SPALLA SX)**

Componendo le azioni e trascurando la componente verticale dei diagonali (di compressione), rispetto alla base della piastra si ha:

$$F_x = 185.7 + 129.4 \times \cos 22.1 + 542.4 = 848.0 \text{ kN}$$

$$M_z = -185.7 \times 0.86 - 119.9 \times 0.81 - 542.4 \times 0.16 = -343.61 \text{ kNm}$$

I cordoni di saldatura hanno lato  $2 \times 8 = 16 \text{ mm}$  (altezza totale di gola  $11.3 \text{ mm}$ ) e lunghezza, dedotte le lunette di scarico:

- alle ali della trave:  $240 - 30 = 210 \text{ mm}$
- all' anima della trave:  $930 - 2 \times 30 = 870 \text{ mm}$

La tensione ideale di resistenza vale:  $\sigma_{w,Rd} = f_{tk} / (\beta_w \gamma_{M2}) = 510 / (0.9 \times 1.25) = 453.33 \text{ N/mm}^2$

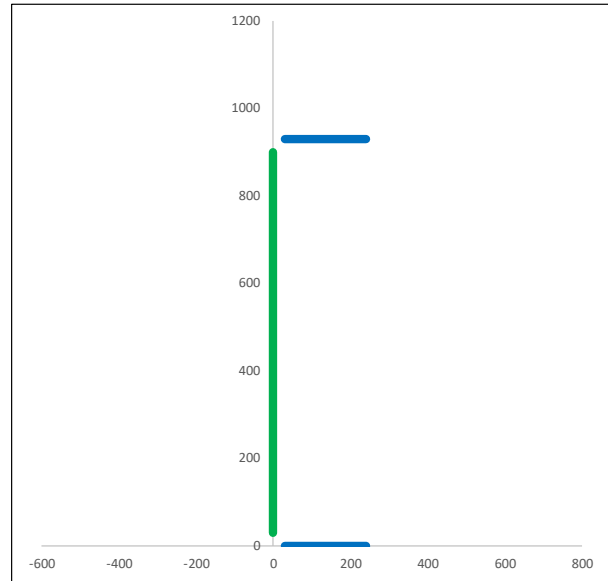
Verifica: la tensione ideale risulta nei 3 cordoni:

- inferiore:  $\sigma_{w,Ed} = 138.25 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- superiore:  $\sigma_{w,Ed} = 63.03 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- anima:  $\sigma_{w,Ed} = 100.70 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )

Segue tabulato foglio di calcolo.

**VERIFICA DI RESISTENZA SALDATURE A CORDONE D'ANGOLO**  
 NTC 2008, § 4.2.8.2.4, FORMULA (4.2.75)

Coefficiente di sicurezza saldature	$\gamma_{M2}$	1.25	
Spessore nominale massimo:	t	15	mm
Classe Acciaio:		S355 W	
Tensione di snervamento:	$f_{yk}$	355	N/mm <sup>2</sup>
Tensione di rottura:	$f_{tk}$	510	N/mm <sup>2</sup>
Coefficiente:	$\beta_w$	0.9	
Resistenza (tensione ideale):	$\sigma_{w,Rd} = f_{tk} / (\beta \cdot \gamma_{M2})$	453.33	N/mm <sup>2</sup>
Sollecitazione (tensione ideale):	$\sigma_{w,Ed} = [ \sigma_{\perp}^2 + 3 ( \tau_{\perp}^2 + \tau_{\parallel}^2 ) ]^{0.5}$		
Punto di applicazione sollecitazioni (vuoto=baricentro)	X=	0	mm
	Y=	0	mm
Sollecitazioni: (terna destrorsa)	FX =	848000	N
	FY =	0	N
	FZ =	0	N
	MX =	0	N mm
	MY =	0	N mm
	MZ =	-34361000	N mm



Cordoni paralleli asse X

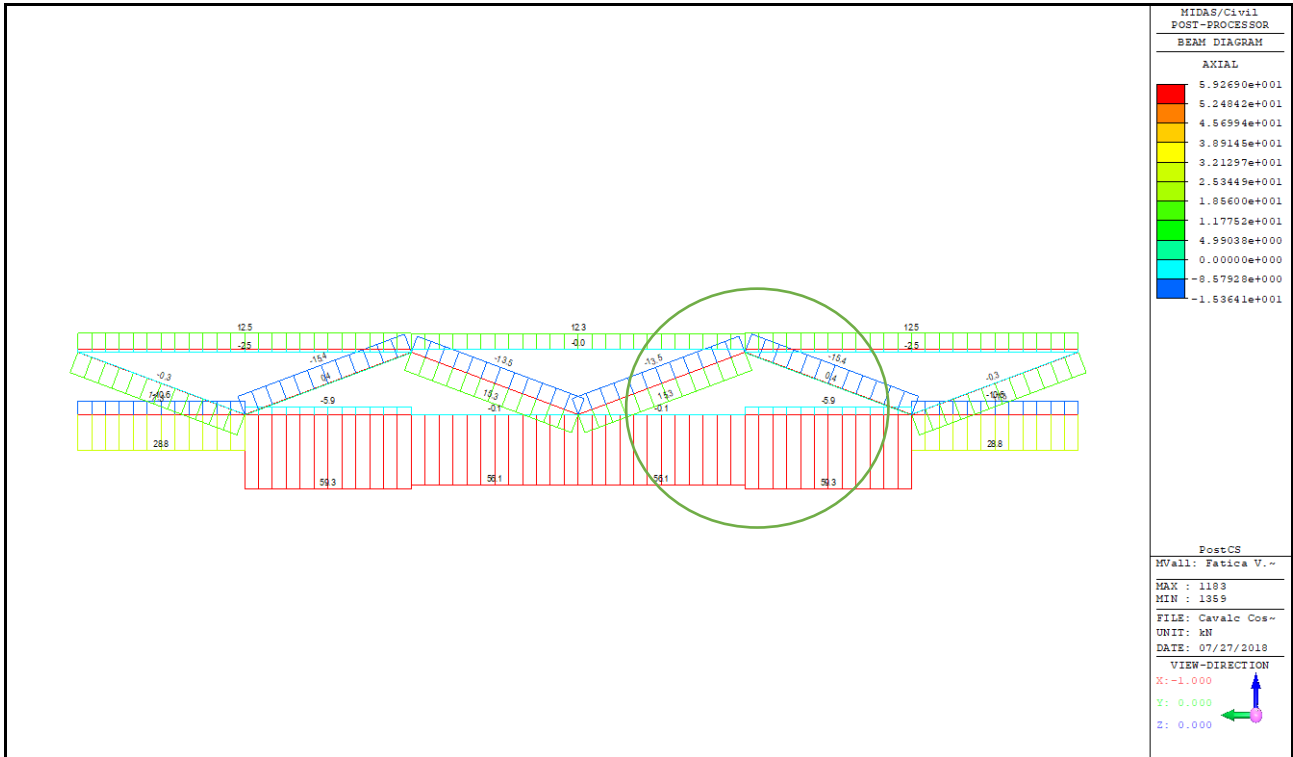
N°	Y	Xini	Xfin	bx	ax	$\sigma_{w,Ed}$	Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$
1	0	30	240	16	11.31	138.251	0.305
2	930	30	240	16	11.31	63.025	0.139
3						0.00	
4						0.00	
5						0.00	
6						0.00	
7						0.00	
8						0.00	
9						0.00	
10						0.00	

Cordoni paralleli asse Y

N°	X	Yini	Yfin	by	ay	$\sigma_{w,Ed}$	Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$
1	0	30	900	16	11.31	100.701	0.222
2						0.00	
3						0.00	
4						0.00	
5						0.00	
6						0.00	
7						0.00	
8						0.00	
9						0.00	
10						0.00	

### 11.3 TRAVI PRINCIPALI - SALDATURE PIASTRE DIAFRAMMI - VERIFICA A FATICA

Le sollecitazioni massime a cui sono sottoposte le saldature sono ricercate nella condizione di carico mobile "Fatica V.ILL.", componendo le azioni assiali di trazione dei traversi inferiori, superiori e diagonali.



**CONDIZIONE DI CARICO FATICA V.I.LL. - MASSIMI SFORZI ASSIALI DIAFRAMMA INTERMEDIO**

Componendo le azioni e trascurando la componente verticale dei diagonali (di compressione), rispetto alla base della piastra si ha:

$$F_x = 12.5 + 0.4 \times \cos 22.1 + 59.3 = 72.17 \text{ kN}$$

$$M_z = -12.5 \times 0.86 - 0.37 \times 0.81 - 59.3 \times 0.16 = -20.538 \text{ kNm}$$

I cordoni di saldatura hanno lato  $2 \times 8 = 16 \text{ mm}$  (altezza di gola  $11.3 \text{ mm}$ ) e lunghezza, dedotte le lunette di scarico:

- alle ali della trave:  $240 - 30 = 210 \text{ mm}$
- all' anima della trave:  $930 - 2 \times 30 = 870 \text{ mm}$

Caratteristiche di resistenza:

- classe del dettaglio:  $\Delta\sigma_c = 80 \text{ N/mm}^2$  (Tabella C.4.2.XVI, part. 7)
- limite di fatica ad ampiezza costante:  $\Delta\sigma_D = 0.737 \times 80 = 58.96 \text{ N/mm}^2$
- coefficiente di sicurezza a fatica:  $\gamma_{Mf,s} = 1.35$

La tensione ideale di resistenza per lo stato limite di fatica vale:  $\sigma_{w,Rd} = \Delta\sigma_D / \gamma_{Mf,s} = 43.67 \text{ N/mm}^2$

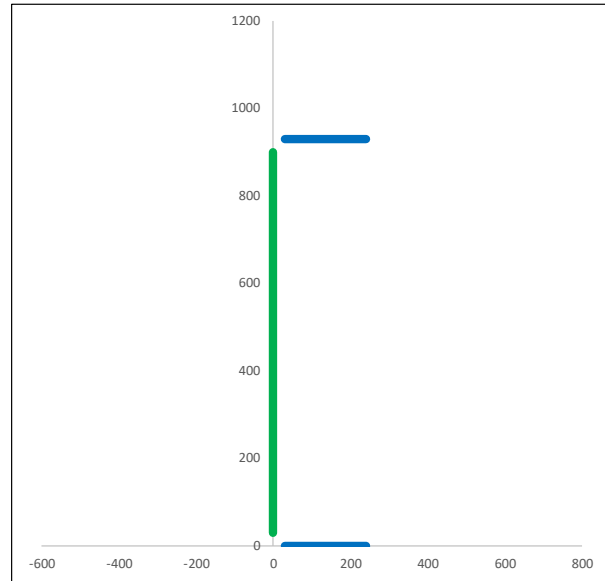
Verifica: la tensione ideale risulta nei 3 cordoni:

- inferiore:  $\sigma_{w,Ed} = 18.22 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- superiore:  $\sigma_{w,Ed} = 1.09 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- anima:  $\sigma_{w,Ed} = 8.61 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )

Segue tabulato foglio di calcolo.

**VERIFICA DI RESISTENZA SALDATURE A CORDONE D'ANGOLO**  
 NTC 2008, § 4.2.8.2.4, FORMULA (4.2.75)

Coefficiente di sicurezza saldature	$\gamma_{M2}$	1.25	
Spessore nominale massimo:	t	15	mm
Classe Acciaio:		S355 W	
Tensione di snervamento:	$f_{yk}$	355	N/mm <sup>2</sup>
Tensione di rottura:	$f_{tk}$	510	N/mm <sup>2</sup>
Coefficiente:	$\beta_w$	0.9	
Resistenza (tensione ideale):	$\sigma_{w,Rd} = f_{tk} / (\beta \cdot \gamma_{M2})$	453.33	N/mm <sup>2</sup>
Sollecitazione (tensione ideale):	$\sigma_{w,Ed} = [ \sigma_{\perp}^2 + 3 ( \tau_{\perp}^2 + \tau_{\parallel}^2 ) ]^{0.5}$		
Punto di applicazione sollecitazioni (vuoto=baricentro)	X=	0 mm	
	Y=	0 mm	
Sollecitazioni: (terna destrorsa)	FX =	72170 N	
	FY =	0 N	
	FZ =	0 N	
	MX =	0 N mm	
	MY =	0 N mm	
	MZ =	-2053800 N mm	



**Cordoni paralleli asse X**

N°	Y	Xini	Xfin	bx	ax	$\sigma_{w,Ed}$	Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$
1	0	30	240	16	11.31	18.223	0.040
2	930	30	240	16	11.31	1.093	0.002
3						0.00	
4						0.00	
5						0.00	
6						0.00	
7						0.00	
8						0.00	
9						0.00	
10						0.00	

**Cordoni paralleli asse Y**

N°	X	Yini	Yfin	by	ay	$\sigma_{w,Ed}$	Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$
1	0	30	900	16	11.31	8.613	0.019
2						0.00	
3						0.00	
4						0.00	
5						0.00	
6						0.00	
7						0.00	
8						0.00	
9						0.00	
10						0.00	

## 12 VERIFICA DEI COLLEGAMENTI BULLONATI

Si verificano a taglio e rifollamento le unioni bullonate dei trasversi e dei controventi.

Si utilizzano bulloni di classe 10.9; si suppone che il piano di taglio interessi la parte filettata del bullone e si considera quindi l'area resistente efficace.

Seguono i dati generali, comuni a tutti i collegamenti.

Qualità acciaio:

Tipo	Classe	Spessore $t \leq 40$ mm		Spessore $40 \text{ mm} < t \leq 80$ mm	
		$f_{yk}$ [N/mm <sup>2</sup> ]	$f_{tk}$ [N/mm <sup>2</sup> ]	$f_{yk}$ [N/mm <sup>2</sup> ]	$f_{tk}$ [N/mm <sup>2</sup> ]
Profilati	S355 W	355	510	335	490
Lamiere	S355 W	355	510	335	490

Coefficienti parziali di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):

Resistenza dei bulloni e delle sezioni tese, indebolite dai fori:  $\gamma_{M2} = 1.25$   
 Resistenza allo scorrimento allo SLU:  $\gamma_{M3} = 1.25$   
 Resistenza allo scorrimento allo SLE:  $\gamma_{M3} = 1.10$   
 Precarico bulloni ad alta resistenza (classe 8.8 o 10.9):  $\gamma_{M7} = 1.10$

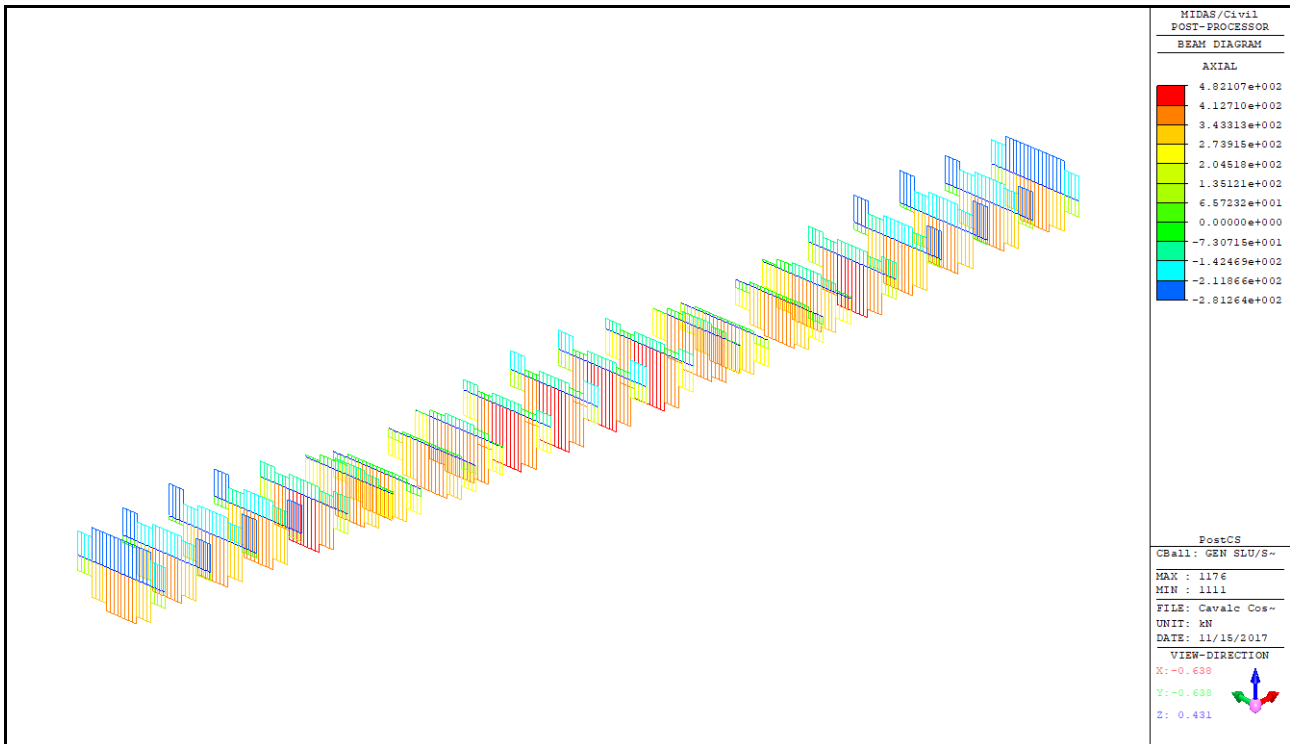
Dati meccanici e geometrici dei bulloni classe 10.9.

Resistenza di snervamento:  $f_{yb} = 900 \text{ N/mm}^2$   
 Resistenza di rottura:  $f_{tb} = 1000 \text{ N/mm}^2$   
 Coefficiente di attrito:  $\mu = 0.30$   
 Tipo di serraggio: Controllato

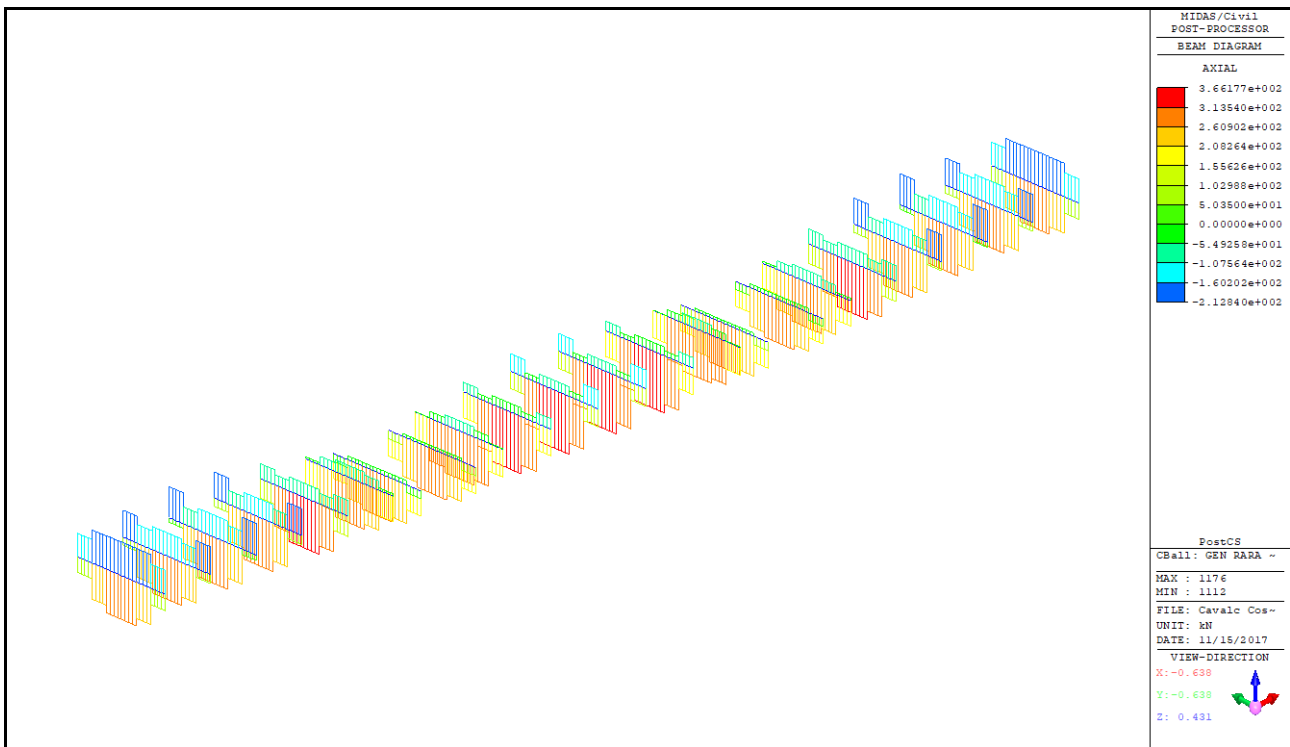
Tutti i nodi bullonati sono verificati a rifollamento e ad attrito allo SLE; si ammette (ipotesi di progetto) che possano non essere verificati ad attrito allo SLU; corrispondono quindi alla Categoria B o C (EN 1993-1-8; § 3.4).

<b>Diametro nominale</b>	<b>d [mm]</b>	<b>M16</b>	<b>M20</b>	<b>M24</b>	<b>M27</b>
Diametro fori (normali)	$d_0$ [mm]	17	21	25.5	28.5
Area nominale	$A$ [mm <sup>2</sup> ]	201	314	452	573
Area resistente (gambo filettato)	$A_{res}$ [mm <sup>2</sup> ]	157	245	353	459
Resistenza a taglio SLU	$F_{v,Rd}$ [kN]	62.7	97.9	141	183.8
Forza di precarico	$F_{p,Cd}$ [kN]	109.7	171.3	246.7	321.6
Resistenza a scorrimento SLU	$F_{s,Rd}$ [kN]	26.3	41.1	59.2	77.2
Resistenza a scorrimento SLE	$F_{s,Rd,es}$ [kN]	29.9	46.7	67.3	87.7

## 12.1 DIAFRAMMI SU SPALLE E INTEREDI - TRASVERSI INFERIORI



TRASVERSI INFERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLU/SLV



TRASVERSI INFERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLE



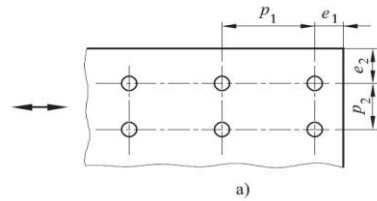
**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI IN CAMPATA - CORRENTI INFERIORI (2 L 80x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  $\gamma_{ME} =$   
 Resistenza a scorrimento allo SLU  $\gamma_{MS} =$   
 Resistenza a scorrimento allo SLE  $\gamma_{MS} =$   
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)  $\gamma_{MT} =$

$\gamma_{ME} = 1.25$   
 $\gamma_{MS} = 1.25$   
 $\gamma_{MS} = 1.10$   
 $\gamma_{MT} = 1.10$

Dati geometrici bulloni: Diametro nominale:  $d =$   
 Tipo fori: accoppiamento **normale**  $d_0 =$   
 Area nominale gambo non filettato:  $A =$   
 Area resistente gambo filettato:  $A_{res} =$   
 Classe bulloni: **10.9**  $f_{yb} =$   
 $f_{tb} =$

$d = 27$  mm  
 $d_0 = 28.5$  mm  
 $A = 573$  mm<sup>2</sup>  
 $A_{res} = 459$  mm<sup>2</sup>  
 $f_{yb} = 900.00$  N/mm<sup>2</sup>  
 $f_{tb} = 1000.00$  N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte:  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} =$   
 Resistenza a scorrimento: coefficiente di attrito:  $\mu =$   
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} =$   
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} =$   
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} =$

**filettata**  
 $F_{v,Rd} = 183.76$  kN  
 $\mu = 0.30$  (tutte le superfici)  
 $F_{p,Cd} = 321.58$  kN  
 $F_{s,Rd} = 77.18$  kN  
 $F_{s,Rd,es} = 87.70$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm  $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$   $f_{tk}$   $f_{yk}$   $f_{tk}$   
 [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W** 355 510 335 490  
 Qualità acciaio lamiera / coprigiunti: **S 355 W** 355 510 335 490

UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **3**  
 Numero superfici di taglio: **2**  
 Interassi:  $p_1 =$  **70** mm  
 (per fila singola porre:  $p_2 = 0$ )  $p_2 =$  **0** mm

Limiti massimi e minimi  
 Min. Max.  
 62.7 175 mm  
 0 0 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2**  
 Spessore flangia bullonata: **15** mm  
 Area sezione singolo profilato: **1510** mm<sup>2</sup>  
 Distanze bulloni dal bordo:  $e_1 =$  **50** mm  
 $e_2 =$  **40** mm

Limiti massimi e minimi  
 Min. Max.  
 34.2 125 mm  
 34.2 125 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici lamiera / coprigiunti: Numero lamiera: **1**  
 Spessore: **15** mm  
 Distanze bulloni dal bordo:  $e_1 =$  **50** mm  
 $e_2 =$  **40** mm

Limiti massimi e minimi  
 Min. Max.  
 34.2 125 mm  
 34.2 125 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} =$  **482.10** kN  
 S.L.E.  $N_{Ed,es} =$  **366.20** kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} =$  80.35 kN

Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{80.35}{183.76} = 0.437 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} =$  80.35 kN

Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{80.35}{77.18} = 1.041 > 1$ : NON Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} =$  61.03 kN

Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{61.03}{87.70} = 0.696 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} =$  80.35 kN  
 coefficienti:  $k_1 = 2.230$   
 $\alpha_b = 0.569$   
 Resistenza al rifollamento:  $F_{b,Rd} =$  209.5 kN

Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{80.35}{209.55} = 0.383 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} =$  160.70 kN  
 coefficienti:  $k_1 = 2.230$   
 $\alpha_b = 0.569$   
 Resistenza al rifollamento:  $F_{b,Rd} =$  209.5 kN

Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{160.70}{209.55} = 0.767 \leq 1$ : Verificato

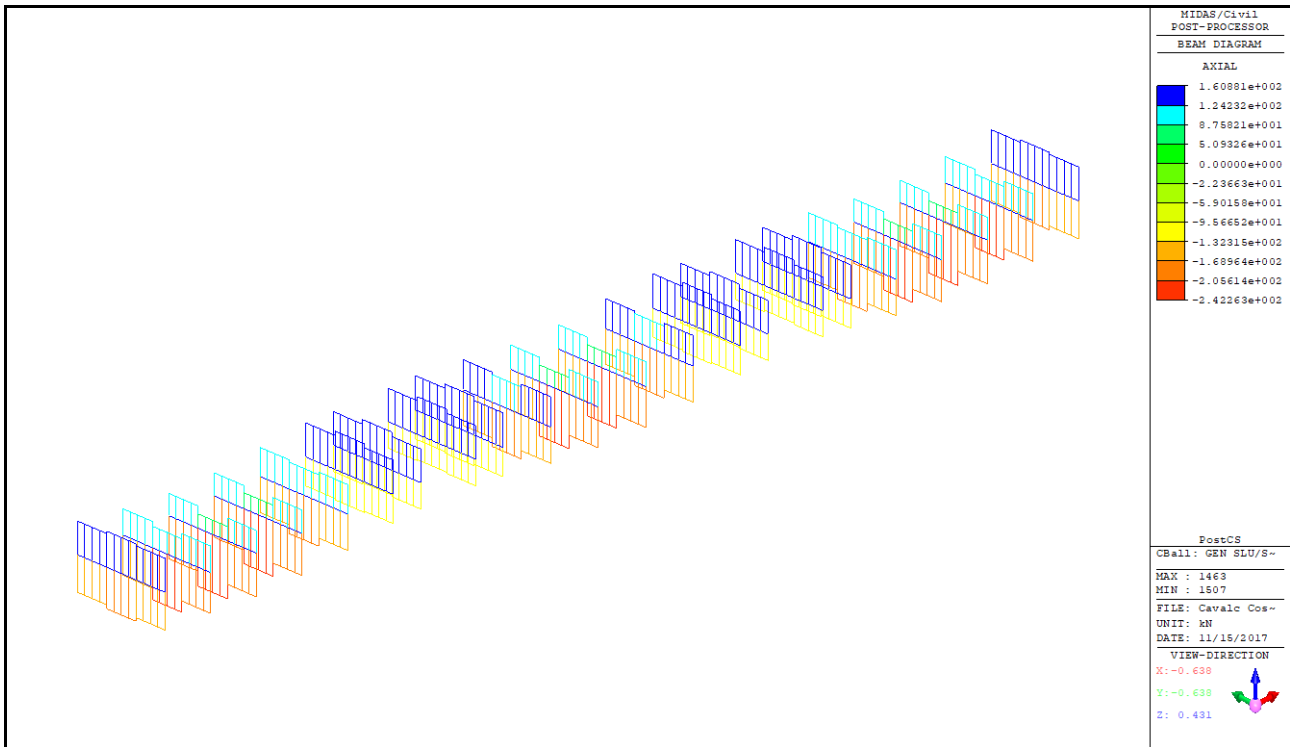
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} =$  2165 mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} =$  794.99 kN

Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{482.10}{794.99} = 0.606 \leq 1$ : Verificato

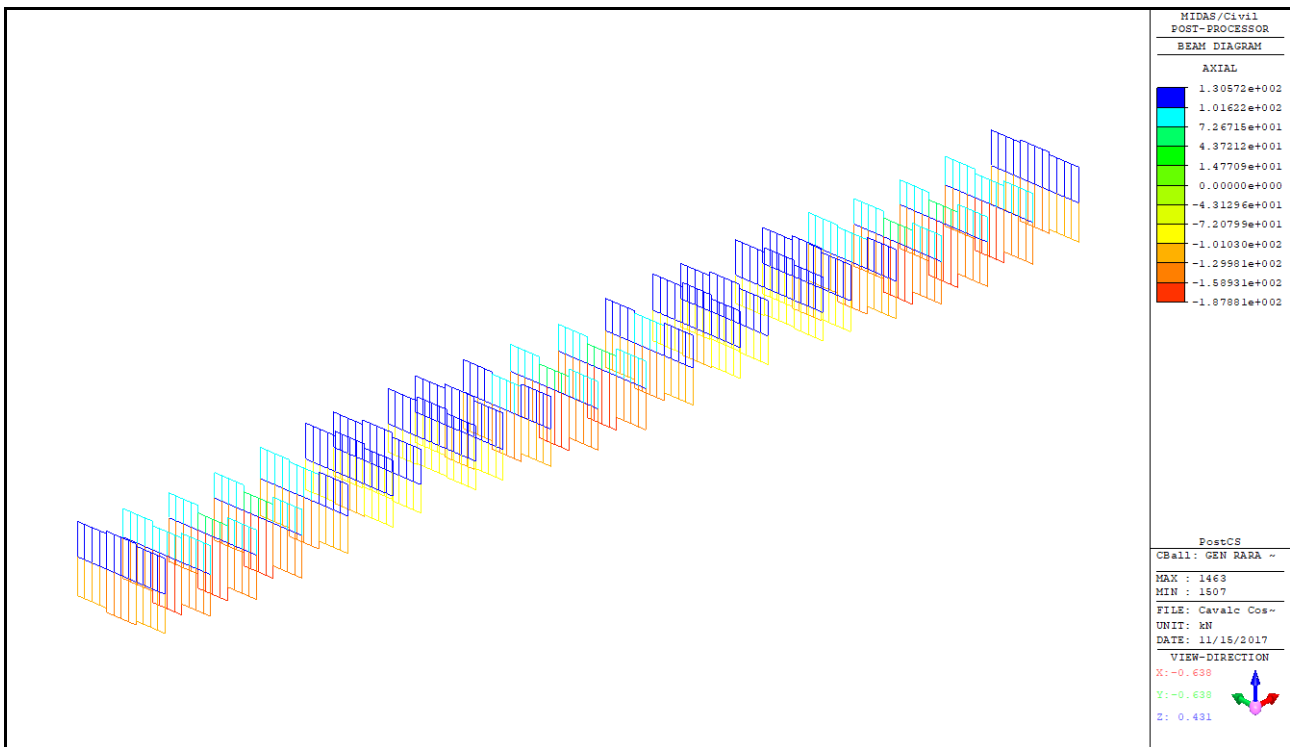
**Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4):**

**Categoria B: connessioni ad attrito allo S.L.E.**

## 12.2 DIAFRAMMI SU SPALLE E INTEREDI - TRASVERSI SUPERIORI



TRASVERSI SUPERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLU

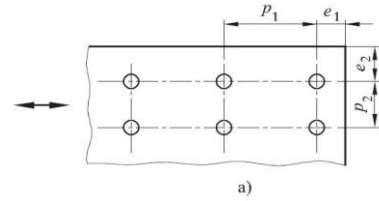


TRASVERSI SUPERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI IN CAMPATA - CORRENTI SUPERIORI (2 L 80x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  $\gamma_{ME} = 1.25$   
 Resistenza a scorrimento allo SLU  $\gamma_{MS} = 1.25$   
 Resistenza a scorrimento allo SLE  $\gamma_{MS} = 1.10$   
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)  $\gamma_{MT} = 1.10$

Dati geometrici bulloni: Diametro nominale:  $d = 20$  mm  
 Tipo fori: accoppiamento **normale**  $d_0 = 21.0$  mm  
 Area nominale gambo non filettato:  $A = 314$  mm<sup>2</sup>  
 Area resistente gambo filettato:  $A_{res} = 245$  mm<sup>2</sup>  
 Classe bulloni: **10.9**  $f_{yb} = 900.00$  N/mm<sup>2</sup>  
 $f_{tb} = 1000.00$  N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte: **filettata**  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 97.92$  kN  
 Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} = 171.36$  kN  
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 41.13$  kN  
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 46.73$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm  $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$   $f_{tk}$   $f_{yk}$   $f_{tk}$   
 [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 Qualità acciaio lamiera / coprigiunti: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **3** Limiti massimi e minimi  
 Numero superfici di taglio: **2** Min. Max.  
 Interassi:  $p_1 = 60$  mm 46.2 140 mm Entro limiti: Verificato  
 (per fila singola porre:  $p_2 = 0$ )  $p_2 = 0$  mm 0 0 mm Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2** Limiti massimi e minimi  
 Spessore flangia bullonata: **10** mm Min. Max.  
 Area sezione singolo profilato: **1510** mm<sup>2</sup> 25.2 125 mm Entro limiti: Verificato  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 25.2 125 mm Entro limiti: Verificato  
 $e_2 = 40$  mm

Dati geometrici lamiera / coprigiunti: Numero lamiera: **1** Limiti massimi e minimi  
 Spessore: **15** mm Min. Max.  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 25.2 125 mm Entro limiti: Verificato  
 $e_2 = 40$  mm 25.2 125 mm Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = -242.30$  kN  
 S.L.E.  $N_{Ed,es} = -187.90$  kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 40.38$  kN Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{40.38}{97.92} = 0.412 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 40.38$  kN Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{40.38}{41.13} = 0.982 \leq 1$ : Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 31.32$  kN Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{31.32}{46.73} = 0.670 \leq 1$ : Verificato

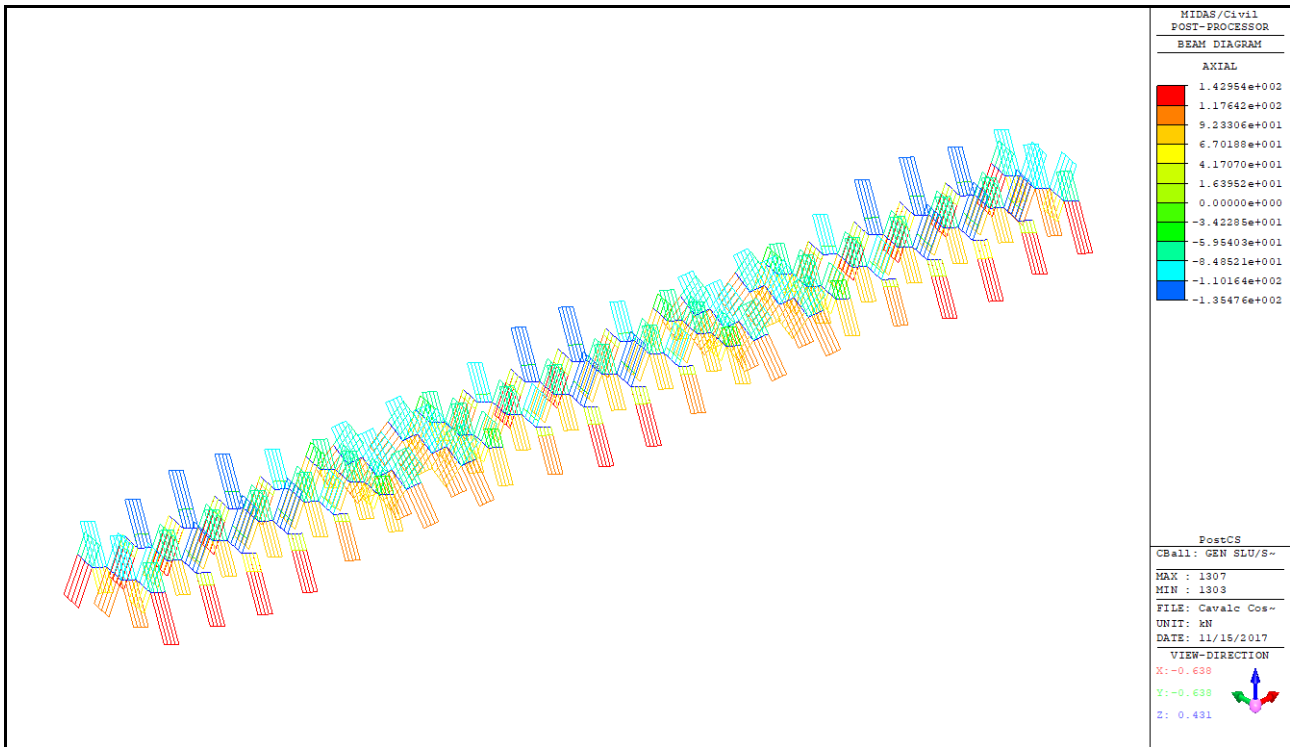
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 40.38$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.635$   
 Resistenza al rifollamento:  $F_{b,Rd} = 129.5$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{40.38}{129.5} = 0.312 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} = 80.77$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.635$   
 Resistenza al rifollamento:  $F_{b,Rd} = 194.3$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{80.77}{194.29} = 0.416 \leq 1$ : Verificato

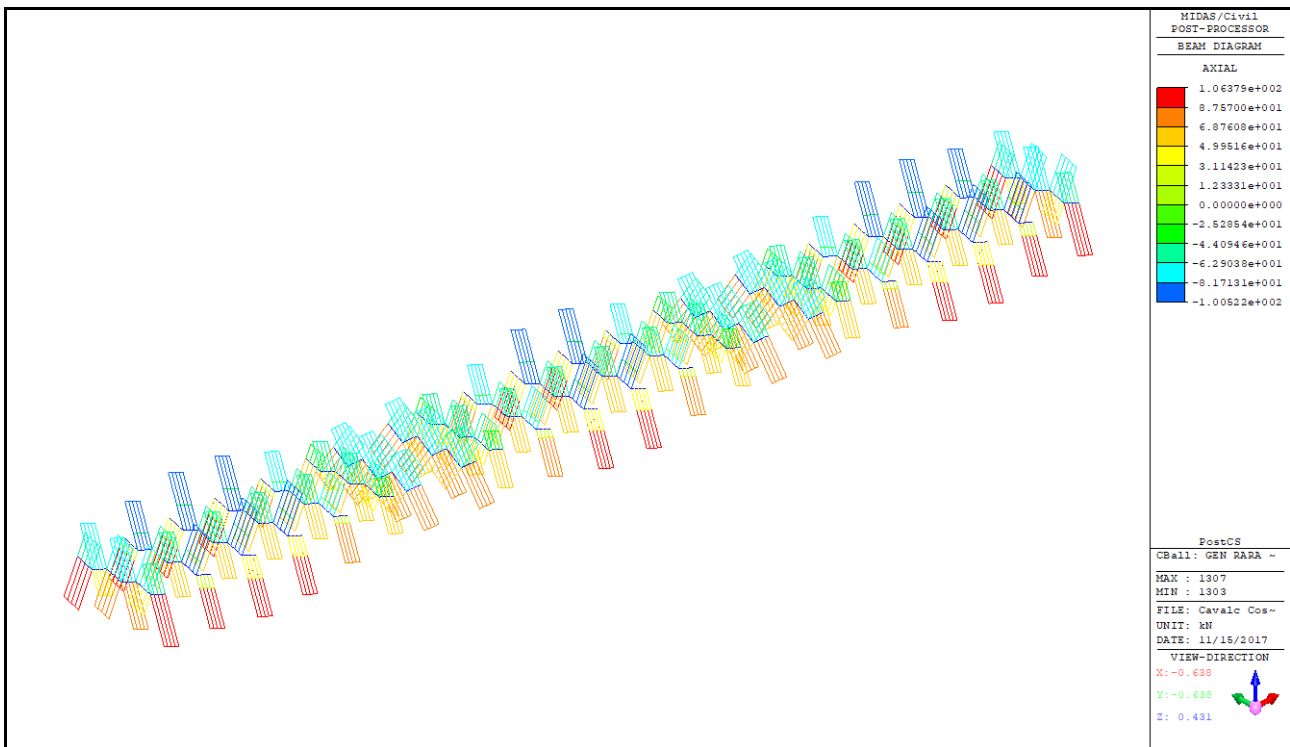
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 2600$  mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} = 954.72$  kN Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{0.00}{954.72} = 0.000$  N.A.

**Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4):** **Categoria C: connessioni ad attrito allo S.L.U.**

### 12.3 DIAFRAMMI SU SPALLE E INTERMEDI - DIAGONALI



DIAGONALI CORRENTI – SFORZO ASSIALE – INVILUPPO SLU

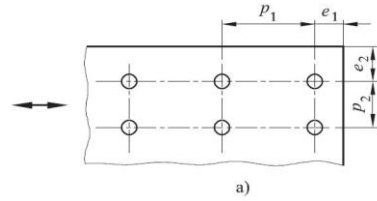


DIAGONALI CORRENTI – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI IN CAMPATA - DIAGONALI (2 L 80x8)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  $\gamma_{M2} = 1.25$   
 Resistenza a scorrimento allo SLU  $\gamma_{M3} = 1.25$   
 Resistenza a scorrimento allo SLE  $\gamma_{M3} = 1.10$   
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)  $\gamma_{M7} = 1.10$

Dati geometrici bulloni: Diametro nominale:  $d = 20$  mm  
 Tipo fori: accoppiamento **normale**  $d_0 = 21.0$  mm  
 Area nominale gambo non filettato:  $A = 314$  mm<sup>2</sup>  
 Area resistente gambo filettato:  $A_{res} = 245$  mm<sup>2</sup>  
 Classe bulloni: **10.9**  $f_{yb} = 900.00$  N/mm<sup>2</sup>  
 $f_{tb} = 1000.00$  N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte: **filettata**  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 97.92$  kN  
 Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} = 171.36$  kN  
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 41.13$  kN  
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 46.73$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm  $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$   $f_{tk}$   $f_{yk}$   $f_{tk}$   
 [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 Qualità acciaio lamiera / coprigiunti: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **2** Limiti massimi e minimi  
 Numero superfici di taglio: **2** Min. Max.  
 Interassi:  $p_1 = 60$  mm 46.2 112 mm Entro limiti: Verificato  
 (per fila singola porre:  $p_2 = 0$ )  $p_2 = 0$  mm 0 0 mm Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2** Limiti massimi e minimi  
 Spessore flangia bullonata: **8** mm Min. Max.  
 Area sezione singolo profilato: **1230** mm<sup>2</sup> 25.2 125 mm Entro limiti: Verificato  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 25.2 125 mm Entro limiti: Verificato  
 $e_2 = 40$  mm

Dati geometrici lamiera / coprigiunti: Numero lamiera: **1** Limiti massimi e minimi  
 Spessore: **15** mm Min. Max.  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 25.2 125 mm Entro limiti: Verificato  
 $e_2 = 40$  mm 25.2 125 mm Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = 143.00$  kN  
 S.L.E.  $N_{Ed,es} = 106.40$  kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 35.75$  kN Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{35.75}{97.92} = 0.365 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 35.75$  kN Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{35.75}{41.13} = 0.869 \leq 1$ : Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 26.60$  kN Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{26.60}{46.73} = 0.569 \leq 1$ : Verificato

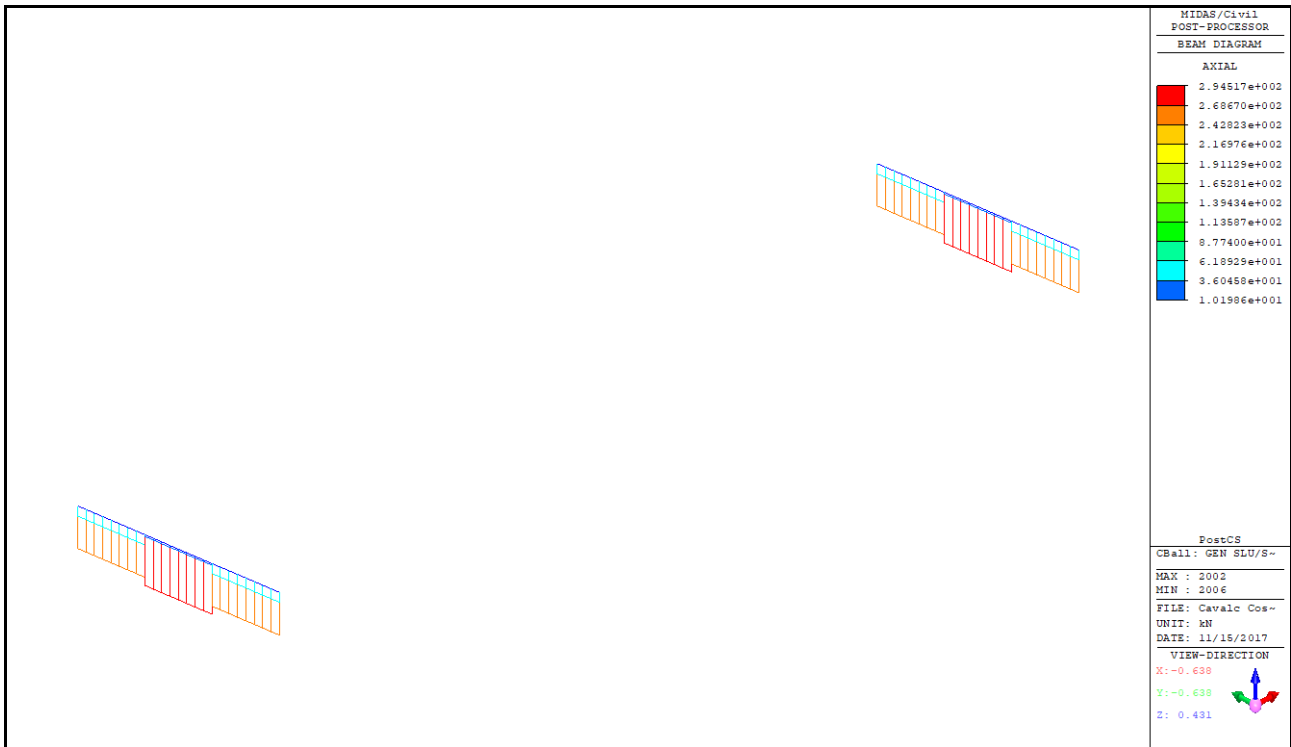
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 35.75$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.635$   
 Resistenza al rifollamento:  $F_{b,Rd} = 103.6$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{35.75}{103.6} = 0.345 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} = 71.50$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.635$   
 Resistenza al rifollamento:  $F_{b,Rd} = 194.3$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{71.50}{194.29} = 0.368 \leq 1$ : Verificato

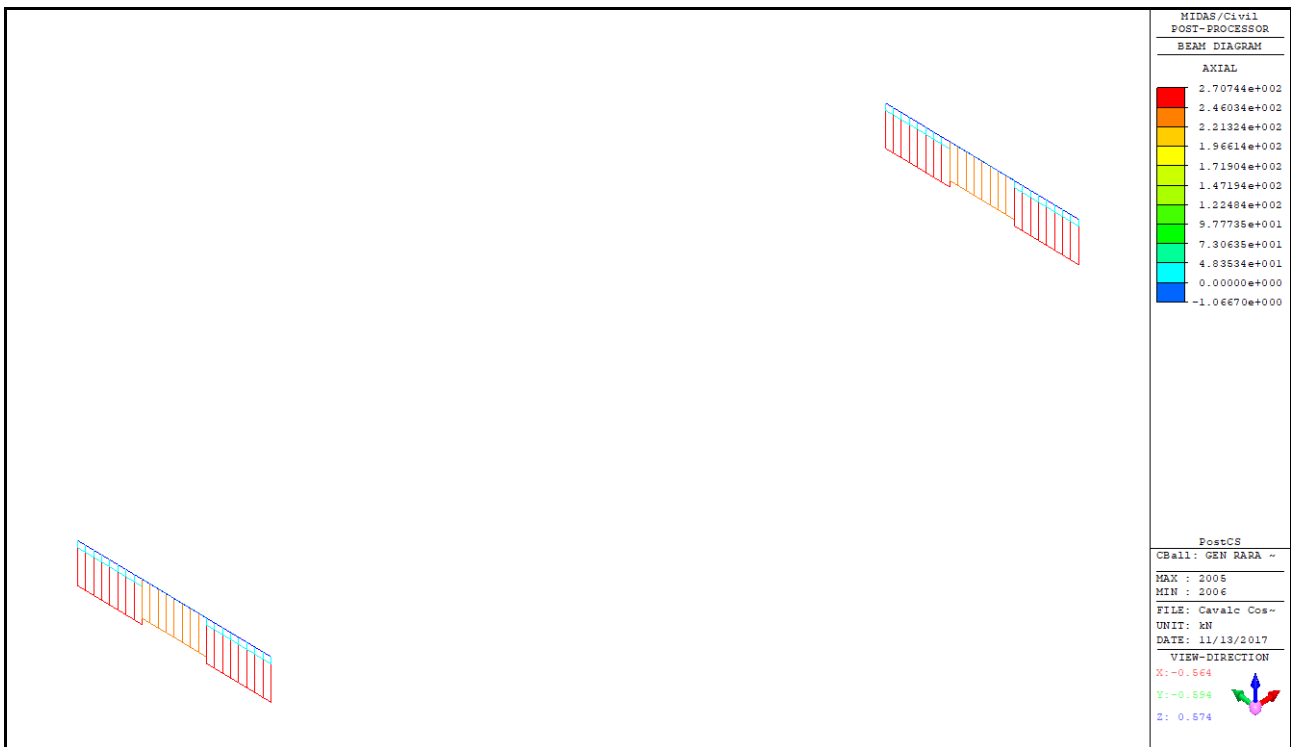
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 2124$  mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} = 779.93$  kN Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{143.00}{779.93} = 0.183 \leq 1$ : Verificato

**Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4):** **Categoria C: connessioni ad attrito allo S.L.U.**

## 12.4 DIAFRAMMI SU PILE – TRASVERSI INFERIORI



TRASVERSI INFERIORI PILE – SFORZO ASSIALE – INVILUPPO SLU



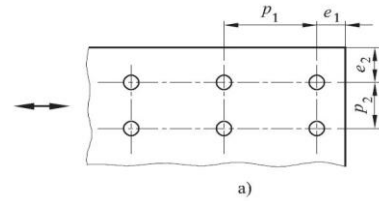
TRASVERSI INFERIORI PILE – SFORZO ASSIALE – INVILUPPO SLE



**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI SU PILA - CORRENTI INFERIORI (2 L 100x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  
 Resistenza a scorrimento allo SLU  
 Resistenza a scorrimento allo SLE  
 Pre-carico dei bulloni ad alta resistenza (classe 8.8 o 10.9)

$\gamma_{M2} = 1.25$   
 $\gamma_{M3} = 1.25$   
 $\gamma_{M8} = 1.10$   
 $\gamma_{M7} = 1.10$



Dati geometrici bulloni: Diametro nominale:  $d = 24$  mm  
 Tipo fori: accoppiamento **normale**  $d_0 = 25.5$  mm  
 Area nominale gambo non filettato:  $A = 452$  mm<sup>2</sup>  
 Area resistente gambo filettato:  $A_{res} = 353$  mm<sup>2</sup>  
 Classe bulloni: **10.9**  $f_{yb} = 900.00$  N/mm<sup>2</sup>  
 $f_{tb} = 1000.00$  N/mm<sup>2</sup>

Resistenza a taglio: il piano di taglio interessa la parte: **filettata**  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 141.00$  kN  
 Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di pre-carico:  $F_{p,Cd} = 246.75$  kN  
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 59.22$  kN  
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 67.30$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm       $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$        $f_{tk}$        $f_{yk}$        $f_{tk}$   
 [N/mm<sup>2</sup>]      [N/mm<sup>2</sup>]      [N/mm<sup>2</sup>]      [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W**      355      510      335      490      UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 Qualità acciaio lamiere / coprigiunti: **S 355 W**      355      510      335      490      UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **3**      Limiti massimi e minimi  
 Numero superfici di taglio: **2**      Min.      Max.  
 Interassi:  $p_1 = 60$  mm      56.1      140 mm      Entro limiti: Verificato  
 (per fila singola porre:  $p_2 = 0$ )       $p_2 = 0$  mm      0      0 mm      Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2**      Limiti massimi e minimi  
 Spessore flangia bullonata: **10** mm      Min.      Max.  
 Area sezione singolo profilato: **1920** mm<sup>2</sup>      30.6      125 mm      Entro limiti: Verificato  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm      30.6      125 mm      Entro limiti: Verificato  
 $e_2 = 50$  mm

Dati geometrici lamiere / coprigiunti: Numero lamiera: **1**      Limiti massimi e minimi  
 Spessore: **20** mm      Min.      Max.  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm      30.6      160 mm      Entro limiti: Verificato  
 $e_2 = 50$  mm      30.6      160 mm      Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = 294.50$  kN  
 S.L.E.  $N_{Ed,es} = 222.10$  kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 49.08$  kN      Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{49.08}{141.00} = 0.348 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 49.08$  kN      Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{49.08}{59.22} = 0.829 \leq 1$ : Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 37.02$  kN      Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{37.02}{67.30} = 0.550 \leq 1$ : Verificato

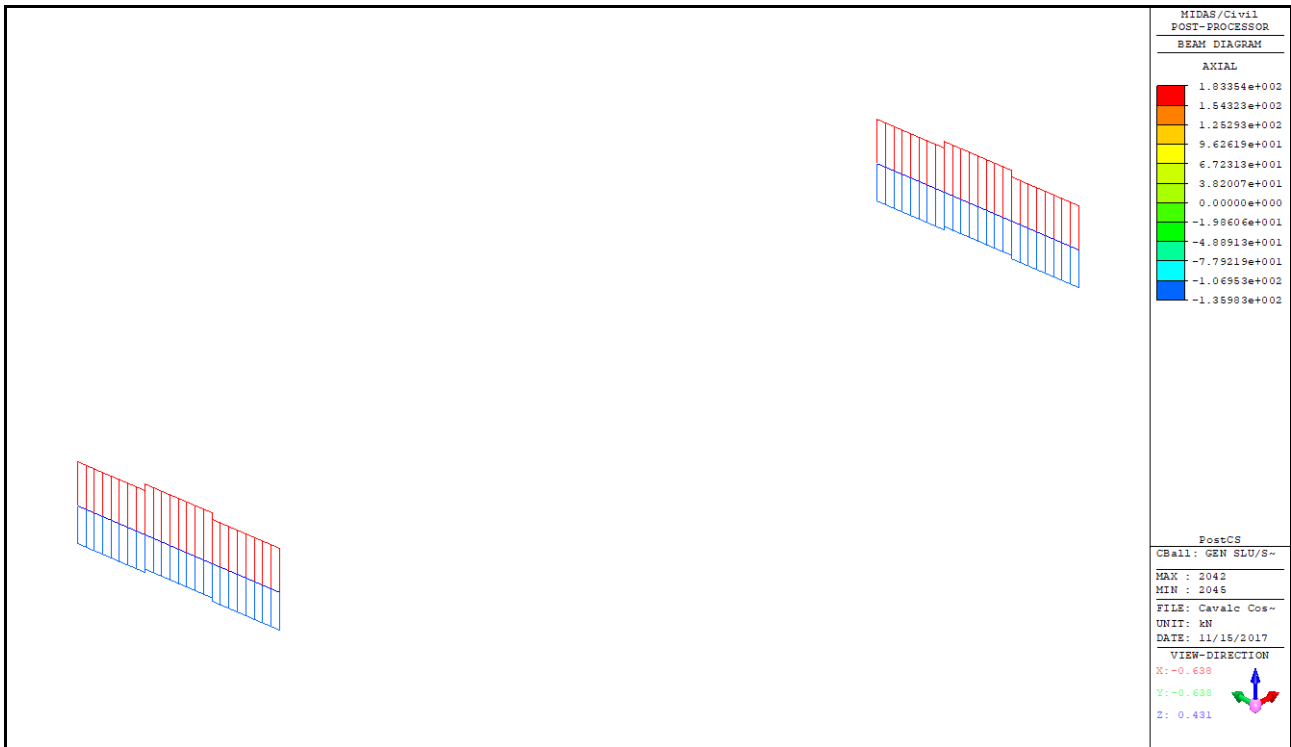
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 49.08$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 128.00$  kN      Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{49.08}{128.00} = 0.383 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiere:  $F_{b,Ed} = 98.17$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 256.00$  kN      Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{98.17}{256.00} = 0.383 \leq 1$ : Verificato

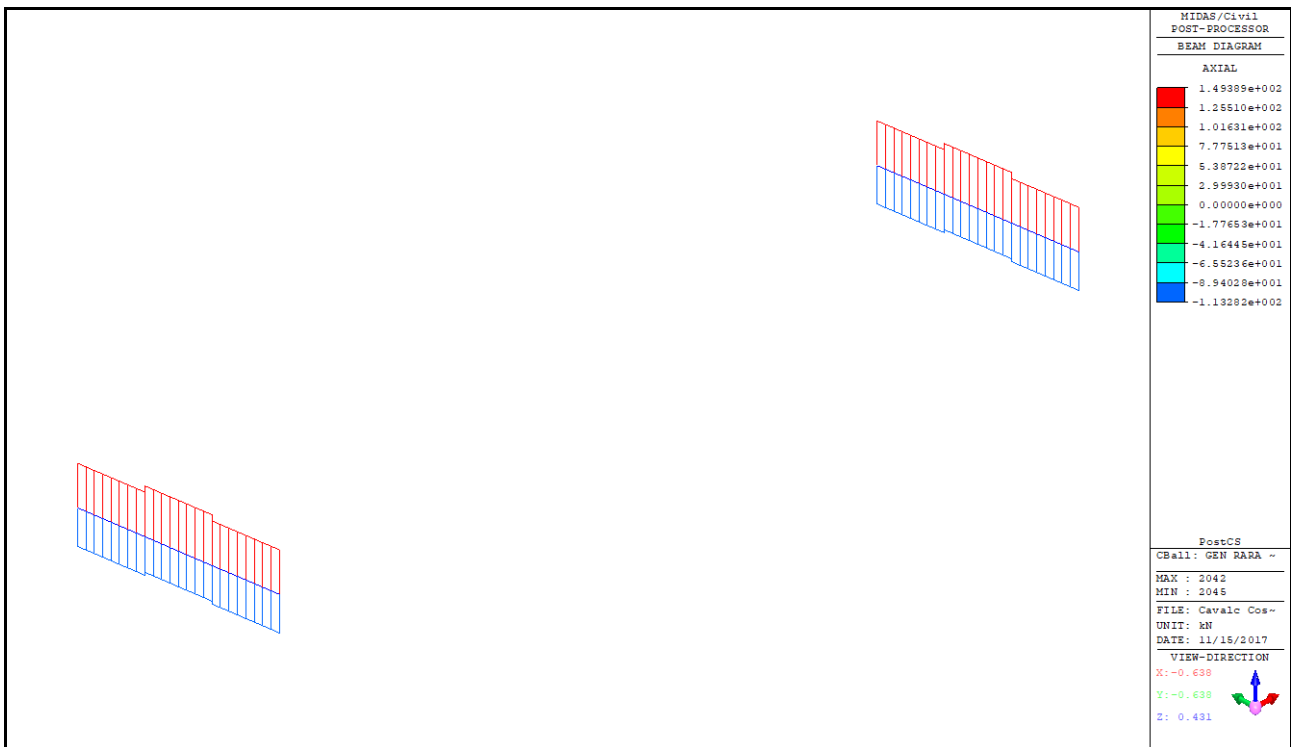
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 3330$  mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} = 1222.78$  kN      Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{294.50}{1222.78} = 0.241 \leq 1$ : Verificato

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4): **Categoria C: connessioni ad attrito allo S.L.U.**

## 12.5 DIAFRAMMI SU PILE – TRASVERSI SUPERIORI



TRASVERSI SUPERIORI PILE – SFORZO ASSIALE – INVILUPPO SLU

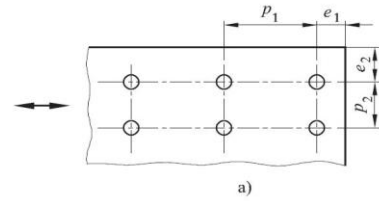


TRASVERSI SUPERIORI PILE – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI SU PILA - CORRENTI SUPERIORI (2 L 100x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  $\gamma_{M2} = 1.25$   
 Resistenza a scorrimento allo SLU  $\gamma_{M3} = 1.25$   
 Resistenza a scorrimento allo SLE  $\gamma_{M3} = 1.10$   
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)  $\gamma_{M7} = 1.10$

Dati geometrici bulloni: Diametro nominale:  $d = 20$  mm  
 Tipo fori: accoppiamento di precisione  $d_0 = 20.3$  mm  
 Area nominale gambo non filettato:  $A = 314$  mm<sup>2</sup>  
 Area resistente gambo filettato:  $A_{res} = 245$  mm<sup>2</sup>  
 Classe bulloni: 10.9  $f_{yb} = 900.00$  N/mm<sup>2</sup>  
 $f_{tb} = 1000.00$  N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte: filettata  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 97.92$  kN  
 Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 (solo classi 8.8 o 10.9) tipo di serraggio: controllato  
 forza di precarico:  $F_{p,Cd} = 171.36$  kN  
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 41.13$  kN  
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 46.73$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm  $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$  [N/mm<sup>2</sup>]  $f_{tk}$  [N/mm<sup>2</sup>]  $f_{yk}$  [N/mm<sup>2</sup>]  $f_{tk}$  [N/mm<sup>2</sup>]

Qualità acciaio profilati: S 355 W  $f_{yk} = 355$   $f_{tk} = 510$   $f_{yk} = 335$   $f_{tk} = 490$  UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 Qualità acciaio lamiera / coprigiunti: S 355 W  $f_{yk} = 355$   $f_{tk} = 510$   $f_{yk} = 335$   $f_{tk} = 490$  UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: 1  
 Numero bulloni: 3 Limiti massimi e minimi  
 Numero superfici di taglio: 2 Min. Max.  
 Interassi:  $p_1 = 60$  mm 44.66 140 mm Entro limiti: Verificato  
 (per fila singola porre:  $p_2 = 0$ )  $p_2 = 0$  mm 0 0 mm Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: 2 Limiti massimi e minimi  
 Spessore flangia bullonata: 10 mm Min. Max.  
 Area sezione singolo profilato: 1920 mm<sup>2</sup> 24.36 125 mm Entro limiti: Verificato  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 24.36 125 mm Entro limiti: Verificato  
 $e_2 = 50$  mm

Dati geometrici lamiera / coprigiunti: Numero lamiera: 1 Limiti massimi e minimi  
 Spessore: 20 mm Min. Max.  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 24.36 160 mm Entro limiti: Verificato  
 $e_2 = 50$  mm 24.36 160 mm Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = 183.30$  kN  
 S.L.E.  $N_{Ed,es} = 149.40$  kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 30.55$  kN Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{30.55}{97.92} = 0.312 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 30.55$  kN Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{30.55}{41.13} = 0.743 \leq 1$ : Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 24.90$  kN Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{24.90}{46.73} = 0.533 \leq 1$ : Verificato

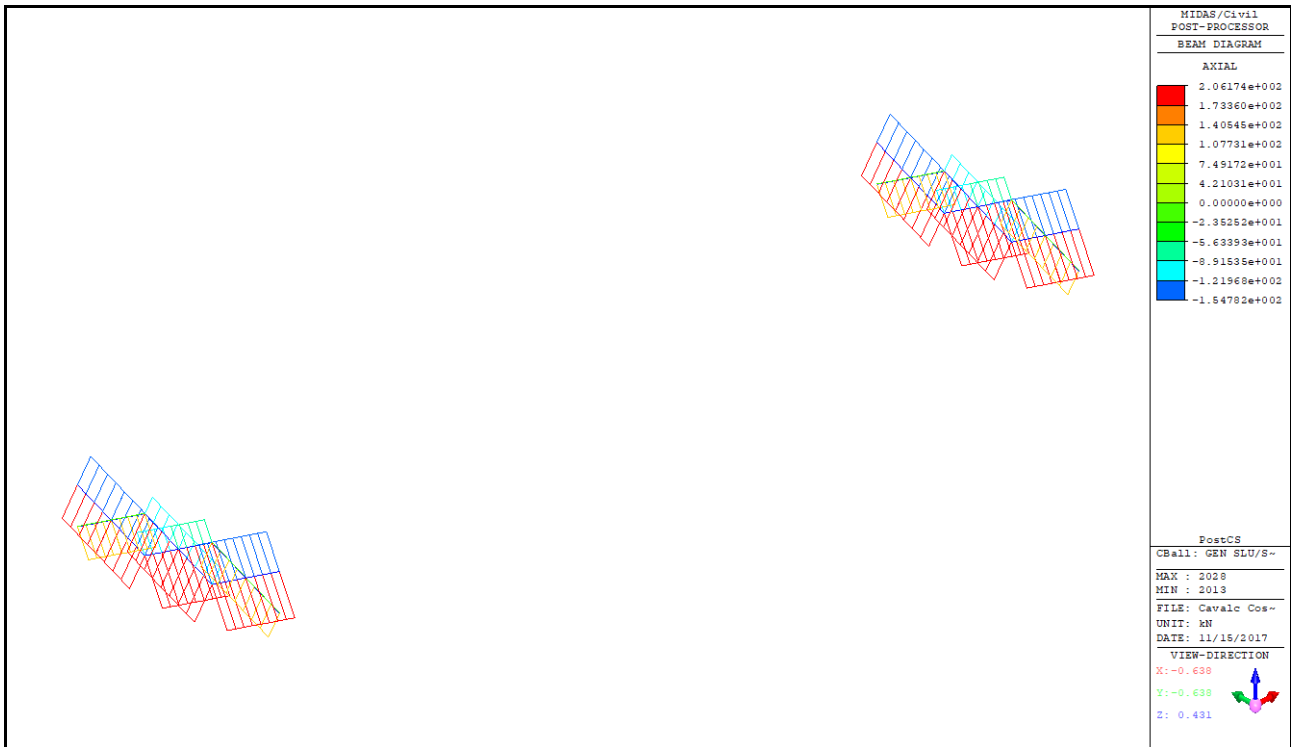
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 30.55$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{30.55}{133.99} = 0.228 \leq 1$ : Verificato  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.657$   
 Resistenza al rifollamento:  $F_{b,Rd} = 134.0$  kN

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} = 61.10$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{61.10}{267.98} = 0.228 \leq 1$ : Verificato  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.657$   
 Resistenza al rifollamento:  $F_{b,Rd} = 268.0$  kN

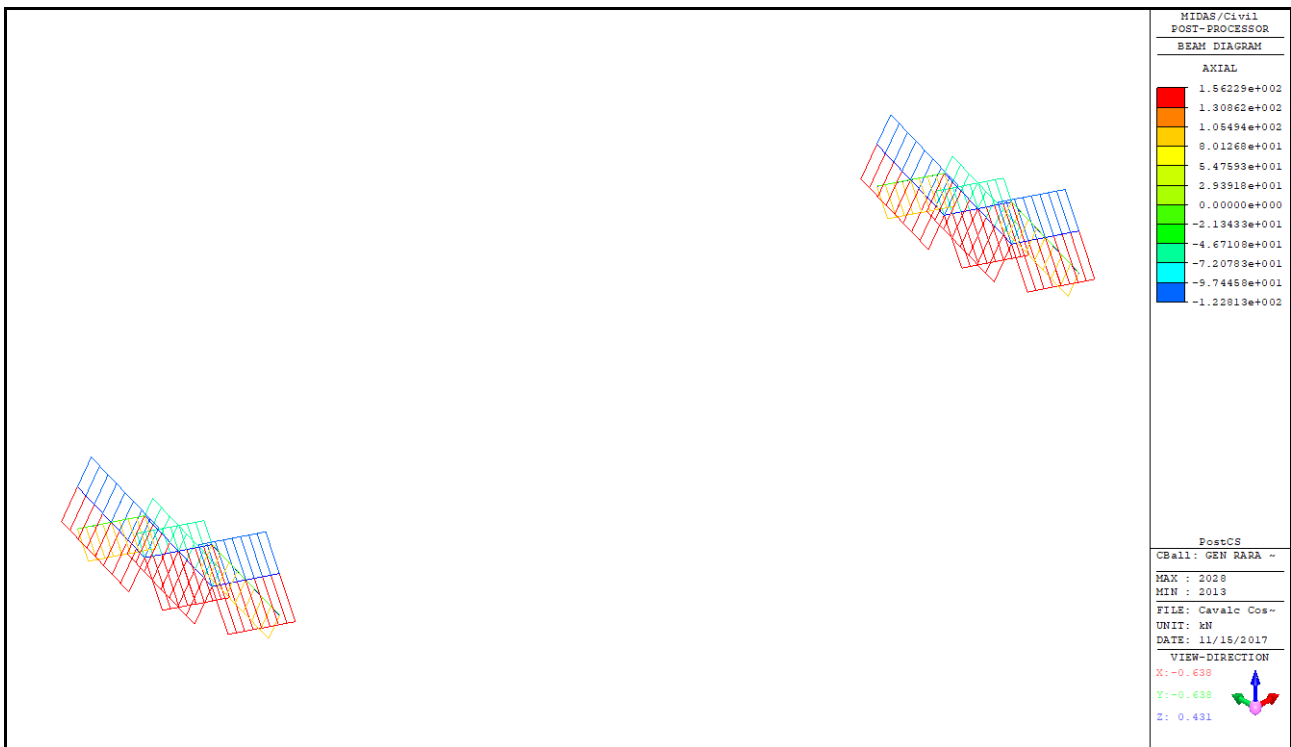
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 3434$  mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} = 1260.96$  kN Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{183.30}{1260.96} = 0.145 \leq 1$ : Verificato

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4): **Categoria C: connessioni ad attrito allo S.L.U.**

## 12.6 DIAFRAMMI SU PILE – DIAGONALI



DIAGONALI TRASVERSI SU PILE – SFORZO ASSIALE – INVILUPPO SLU

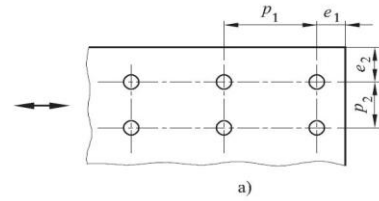


DIAGONALI TRASVERSI SU PILE – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI SU PILA - DIAGONALI (2 L 80x8)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  $\gamma_{M2} = 1.25$   
 Resistenza a scorrimento allo SLU  $\gamma_{M3} = 1.25$   
 Resistenza a scorrimento allo SLE  $\gamma_{M3} = 1.10$   
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)  $\gamma_{M7} = 1.10$

Dati geometrici bulloni: Diametro nominale:  $d = 24$  mm  
 Tipo fori: accoppiamento **normale**  $d_0 = 25.5$  mm  
 Area nominale gambo non filettato:  $A = 452$  mm<sup>2</sup>  
 Area resistente gambo filettato:  $A_{res} = 353$  mm<sup>2</sup>  
 Classe bulloni: **10.9**  $f_{yb} = 900.00$  N/mm<sup>2</sup>  
 $f_{tb} = 1000.00$  N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte: **filettata**  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 141.00$  kN  
 Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} = 246.75$  kN  
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 59.22$  kN  
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 67.30$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm  $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$   $f_{tk}$   $f_{yk}$   $f_{tk}$   
 [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 Qualità acciaio lamiera / coprigiunti: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **2** Limiti massimi e minimi  
 Numero superfici di taglio: **2** Min. Max.  
 Interassi:  $p_1 = 60$  mm 56.1 112 mm Entro limiti: Verificato  
 (per fila singola porre:  $p_2 = 0$ )  $p_2 = 0$  mm 0 0 mm Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2** Limiti massimi e minimi  
 Spessore flangia bullonata: **8** mm Min. Max.  
 Area sezione singolo profilato: **1230** mm<sup>2</sup> 30.6 125 mm Entro limiti: Verificato  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 30.6 125 mm Entro limiti: Verificato  
 $e_2 = 40$  mm

Dati geometrici lamiera / coprigiunti: Numero lamiera: **1** Limiti massimi e minimi  
 Spessore: **20** mm Min. Max.  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm 30.6 160 mm Entro limiti: Verificato  
 $e_2 = 40$  mm 30.6 160 mm Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = 206.20$  kN  
 S.L.E.  $N_{Ed,es} = 156.20$  kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 51.55$  kN Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{51.55}{141.00} = 0.366 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 51.55$  kN Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{51.55}{59.22} = 0.870 \leq 1$ : Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 39.05$  kN Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{39.05}{67.30} = 0.580 \leq 1$ : Verificato

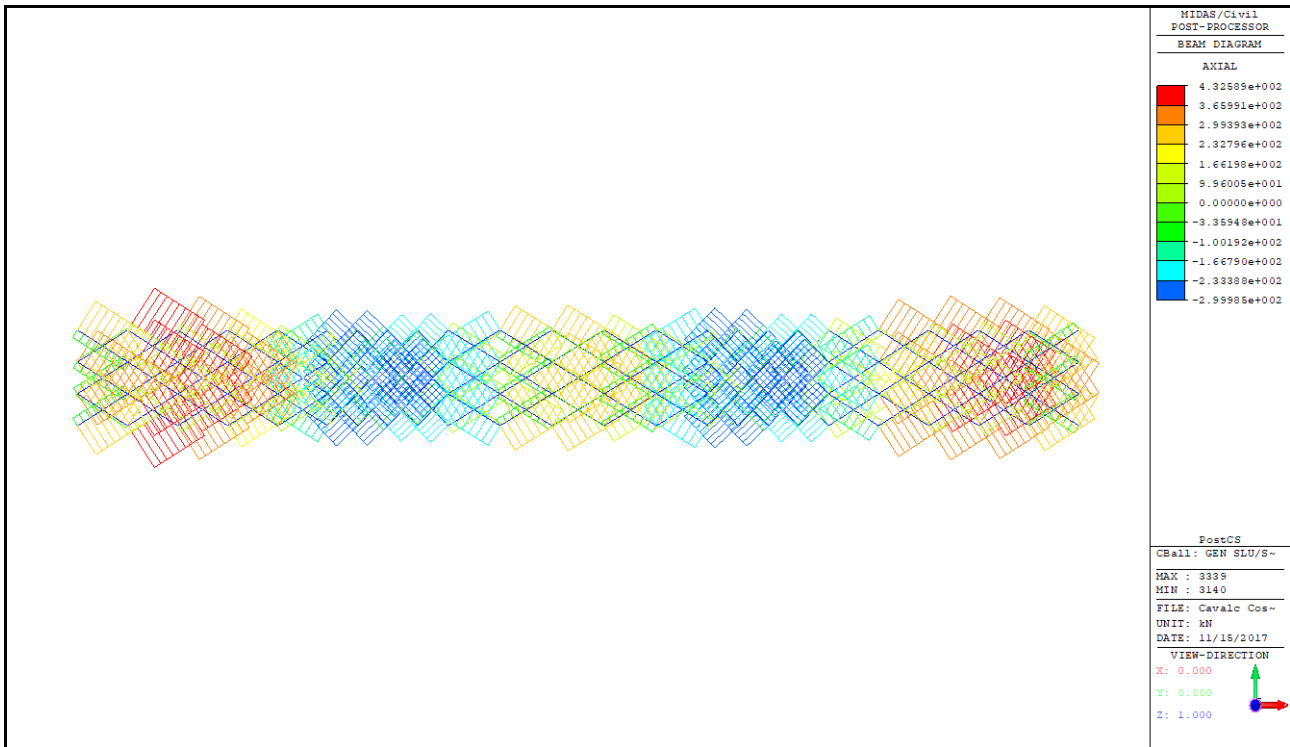
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 51.55$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 102.4$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{51.55}{102.40} = 0.503 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} = 103.10$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 256.0$  kN Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{103.10}{256.00} = 0.403 \leq 1$ : Verificato

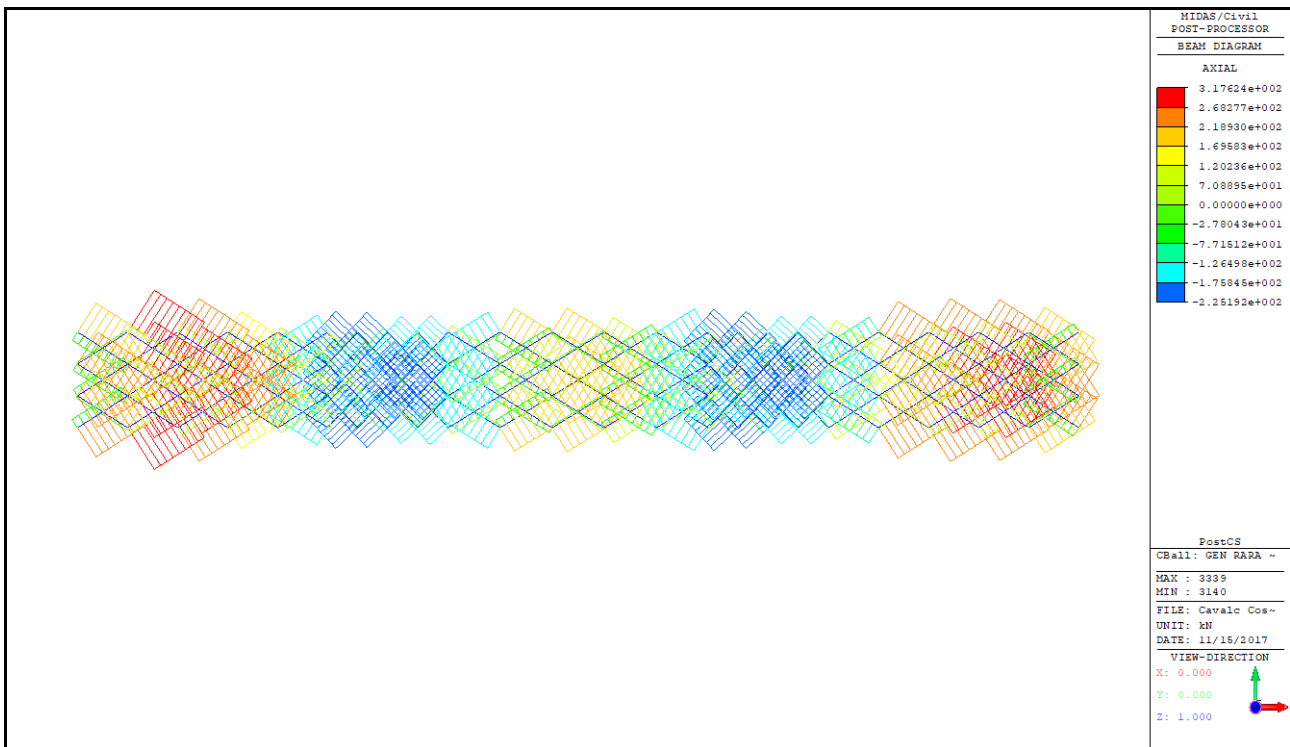
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 2052$  mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} = 753.49$  kN Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{206.20}{753.49} = 0.274 \leq 1$ : Verificato

**Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4):** **Categoria C: connessioni ad attrito allo S.L.U.**

## 12.7 CONTROVENTI



CONTROVENTI – SFORZO ASSIALE – INVILUPPO SLU



CONTROVENTI – SFORZO ASSIALE – INVILUPPO SLE



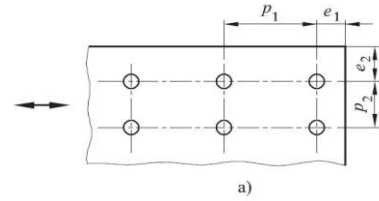
**VERIFICA CONNESSIONE BULLONATA A TAGLIO: CONTROVENTI (2 L 80x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  $\gamma_{M2} =$   
 Resistenza a scorrimento allo SLU  $\gamma_{M3} =$   
 Resistenza a scorrimento allo SLE  $\gamma_{M3} =$   
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)  $\gamma_{M7} =$

1.25  
 1.25  
 1.10  
 1.10

Dati geometrici bulloni: Diametro nominale:  $d =$   
 Tipo fori: accoppiamento **normale**  $d_0 =$   
 Area nominale gambo non filettato:  $A =$   
 Area resistente gambo filettato:  $A_{res} =$   
 Classe bulloni: **10.9**  $f_{yb} =$   
 $f_{tb} =$

27 mm  
 28.5 mm  
 573 mm<sup>2</sup>  
 459 mm<sup>2</sup>  
 900.00 N/mm<sup>2</sup>  
 1000.00 N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte:  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} =$   
 Resistenza a scorrimento: coefficiente di attrito:  $\mu =$   
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} =$   
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} =$   
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} =$

**filettata**  
 183.76 kN  
 0.30 (tutte le superfici)  
 321.58 kN  
 77.18 kN  
 87.70 kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm  $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$   $f_{tk}$   $f_{yk}$   $f_{tk}$   
 [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W** 355 510 335 490  
 Qualità acciaio lamiera / coprigiunti: **S 355 W** 355 510 335 490

UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **3** Limiti massimi e minimi  
 Numero superfici di taglio: **2** Min. Max.  
 Interassi:  $p_1 =$  **70** mm 62.7 140 mm  
 (per fila singola porre:  $p_2 = 0$ )  $p_2 =$  **0** mm 0 0 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2** Limiti massimi e minimi  
 Spessore flangia bullonata: **10** mm Min. Max.  
 Area sezione singolo profilato: **1510** mm<sup>2</sup> 34.2 125 mm  
 Distanze bulloni dal bordo:  $e_1 =$  **40** mm 34.2 125 mm  
 $e_2 =$  **40** mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici lamiera / coprigiunti: Numero lamiera: **1** Limiti massimi e minimi  
 Spessore: **15** mm Min. Max.  
 Distanze bulloni dal bordo:  $e_1 =$  **40** mm 34.2 125 mm  
 $e_2 =$  **40** mm 34.2 125 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} =$  **432.60** kN  
 S.L.E.  $N_{Ed,es} =$  **317.60** kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} =$  72.10 kN  
 Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{72.10}{183.76} =$  **0.392**  $\leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} =$  72.10 kN  
 Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{72.10}{77.18} =$  **0.934**  $\leq 1$ : Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} =$  52.93 kN  
 Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{52.93}{87.70} =$  **0.604**  $\leq 1$ : Verificato

S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} =$  72.10 kN  
 coefficienti:  $k_1 =$  2.230  
 $\alpha_b =$  0.468  
 Resistenza al rifollamento:  $F_{b,Rd} =$  114.9 kN  
 Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{72.10}{114.92} =$  **0.627**  $\leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} =$  144.20 kN  
 coefficienti:  $k_1 =$  2.230  
 $\alpha_b =$  0.468  
 Resistenza al rifollamento:  $F_{b,Rd} =$  172.4 kN  
 Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{144.20}{172.38} =$  **0.837**  $\leq 1$ : Verificato

S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} =$  2450 mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} =$  899.64 kN  
 Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{432.60}{899.64} =$  **0.481**  $\leq 1$ : Verificato

**Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4):**

**Categoria C: connessioni ad attrito allo S.L.U.**

## 13 VERIFICA DEGLI ISOLATORI SISMICI

Si effettuano le verifiche allo stato limite ultimo degli isolatori elastomerici effettuate secondo EN 1337-3 per le fasi statiche e secondo OPCM 3274 (allegato 10.A) per le fasi sismiche.

### CAVALCAVIA STRADA VICINALE DELLE COSTE

#### VERIFICA APPOGGI ELASTOMERICI ARMATI

##### Dati di progetto:

	<b>SPALLE</b>		<b>PILE</b>
Deformazione massima sismica di progetto SLC (d/h ≤ 2)	Dsp= 152.0 mm		Dsp= 156.0 mm
Deformazione massima statica di progetto SLU/SLD (d/h ≤ 1)	Dp= 76.0 mm		Dp= 78.0 mm

##### Risultati da modello di calcolo

Carico verticale massimo statico SLU	V(max)= -1811.9 kN		V(max)= -4463.7 kN
Carico verticale minimo statico SLU	V(min)= -453.7 kN		V(min)= -1785.6 kN
Carico orizzontale massimo statico SLU	H(max)= 54.0 kN		H(max)= 116.8 kN
Rotazione massima statica SLU	R (max)= 2.092E-02 rad		R (max)= 4.837E-03 rad

Carico verticale massimo sismico SLC	Vs(max)= -551.4 kN		Vs(max)= -2143.6 kN
Carico verticale minimo sismico SLC	Vs(min)= -360.7 kN		Vs(min)= -1280.9 kN
Carico orizzontale massimo sismico SLC	Hs(max)= 102.0 kN		Hs(max)= 218.2 kN
Rotazione massima sismica SLC	Rs (max)= 1.271E-02 rad		Rs (max)= 2.363E-03 rad

Carico verticale massimo statico SLE (caratt.)	Ve(max)= -1415.5 kN		V(max)= -3412.6 kN
Carico verticale minimo statico SLE (caratt.)	Ve(min)= -364.9 kN		V(min)= -1426.3 kN
Carico orizzontale massimo statico SLE (caratt.)	He(max)= 40.5 kN		H(max)= 80.6 kN
Rotazione massima statica SLE (caratt.)	Re(max)= 1.472E-02 rad		R (max)= 4.065E-03 rad

##### Caratteristiche di progetto isolatore

	<b>SPALLE</b>		<b>PILE</b>
Tensione caratteristica di snervamento lamiera (≥ 235 N/mm <sup>2</sup> )	fyk= 275 N/mm <sup>2</sup>		fyk= 275 N/mm <sup>2</sup>
Modulo di elasticità volumetrica elastomero	Eb= 2000 N/mm <sup>2</sup>		Eb= 2000 N/mm <sup>2</sup>
Modulo di elasticità tangenziale dinamico elastomero (γ = D/te = 1)	Gdin= 1.4 N/mm <sup>2</sup>		Gdin= 1.4 N/mm <sup>2</sup>
Spostamento massimo di progetto tra le due facce dell'isolatore allo SLC	de= 152 mm		de= 156 mm
Diametro elastomero	Dg= 300 mm		Dg= 450 mm
Numero di strati di elastomero	ng= 19		ng= 13
Spessore singolo strato di elastomero (5 mm < tg < 25 mm)	tg= 4 mm		tg= 6 mm
Spessore singola lamiera di acciaio interna (≥ 2 mm)	ts= 2 mm		ts= 3 mm
Spessore lamiera di acciaio esterna (≥ 20 mm)	tse= 20 mm		tse= 20 mm
Lato piastra di ancoraggio	Z= 350 mm		Z= 500 mm
Spessore piastra di ancoraggio	ta= 25 mm		ta= 25 mm
Ricoprimento laterale lamiera di acciaio (> 4 mm)	rs= 10 mm		rs= 10 mm
Diametro lamiera di acciaio interne (Dg-D' > 2x4 = 8 mm)	D'= 280 mm		D'= 430 mm
Spessore totale elastomero	te= 76 mm		te= 78 mm
Spessore complessivo isolatore (escluse piastre ancoraggio)	h= 152 mm		h= 154 mm
Spessore complessivo isolatore (incluse piastre ancoraggio)	H= 202 mm		H= 204 mm
Tensione di trazione massima (min(1;2*Gdin))	Sigma_t= -1.00 N/mm <sup>2</sup>		Sigma_t= -1.00 N/mm <sup>2</sup>
Fattore di forma primario	S1= 17.50		S1= 17.92
	Fi(de)= 2.0790 rad		Fi(de)= 2.4336 rad
Modulo di compressibilità assiale	Ec= 947.51 N/mm <sup>2</sup>		Ec= 963.83 N/mm <sup>2</sup>

##### Rigidità orizzontale equivalente (γ = D/te = 1)

<b>Rigidità orizzontale equivalente</b>	<b>Ke= 1302 N/mm</b>		<b>Ke= 2855 N/mm</b>
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<b>Rigidità verticale</b>	<b>Kv= 767676 N/mm</b>		<b>Kv= 1794462 N/mm</b>
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**Verifiche statiche secondo EN 1337-3:2005**

Carico verticale massimo statico SLU	V(max)= 1811900 N	V(max)= 4463730 N
Carico orizzontale massimo statico SLU	H(max)= 53960 N	H(max)= 116762 N
Carico orizzontale statico a $v = D/te = 1$ :	H(y=1)= 98960 N	H(y=1)= 222660 N
Spostamento statico totale	D= 36.56 mm	D= 35.72 mm
Rotazione massima statica SLU	R (max)= 2.092E-02 rad	R (max)= 4.837E-03 rad
	Fi(de)= 2.8973 rad	Fi(de)= 2.9827 rad
Area ridotta efficace dell'isolatore	Ar= 52046 mmq	Ar= 130559 mmq

1. Deformazione di taglio massima dovuta alla compressione:	Gamma_c= 2.1314	Gamma_c= 2.0446
dovuta allo spostamento statico tot (< 1)	Gamma_s= 0.4810	Gamma_s= 0.4579
dovuta alle rotazioni	Gamma_alfa= 2.3231	Gamma_alfa= 0.7849
totale: (< 7)	Gamma_t= 4.9355	Gamma_t= 3.2874

2. Spessore minimo lamiere di acciaio ( $\geq 2$ mm)	ts(min)= 2.00 mm	ts(min)= 2.00 mm
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3. Rotazione limite:		
spostamento verticale massimo:	Dz= 2.161 mm	Dz= 2.266 mm
spostamento dovuto alla rotazione massima	D_alfa= 1.953 mm	D_alfa= 0.693 mm
Verifica: (Dz - D_alfa) > 0	Dz - D_alfa= 0.208 mm	Dz - D_alfa= 1.572 mm

4. Verifica di instabilità: Pmax < Plim		
pressione limite	Plim= 60.175 N/mmq	Plim= 92.187 N/mmq
pressione massima: (< Plim)	Pmax= 34.814 N/mmq	Pmax= 34.189 N/mmq

**Pressione sulle sottostrutture**

Diametro efficace di ripartizione a 60° (< Z)	De= 350 mm	De= 500 mm
Pressione massima SLU/SLC	P'max= 18.833 N/mmq	P'max= 22.734 N/mmq
Pressione minima SLU/SLC	P'min= 3.749 N/mmq	P'min= 6.524 N/mmq

**Verifiche sismiche secondo OPCM 3274 e s.m. (Allegato 10.A)**

Sforzo normale massimo sull'isolatore allo SLC	V= 551420 N	V= 2143590 N
Carico orizzontale massimo sismico SLC	Hs(max)= 102036 N	Hs(max)= 218159 N
Carico orizzontale sismico a $v = D/te = 1$ :	H(y=1)= 98960 N	H(y=1)= 222660 N
Spostamento sismico totale	Ds= 78.36 mm (< Dsp)	Ds= 76.20 mm (< Dsp)
Rotazione massima sismica SLC	Rs (max)= 1.271E-02 rad	Rs (max)= 2.363E-03 rad
	Fi(de)= 2.61305 rad	Fi(de)= 2.80128 rad
Area ridotta efficace dell'isolatore	Ar= 41332 mmq	Ar= 114060 mmq

Deformazioni di taglio dell'elastomero:		
dovuta alla compressione:	Gamma_c= 0.8168	Gamma_c= 1.1239
dovuta allo spostamento sismico totale	Gamma_s= 1.0311	Gamma_s= 0.9769
dovuta alle rotazioni	Gamma_alfa= 1.4111	Gamma_alfa= 0.3834
Deformazione di taglio totale di progetto dell'elastomero:	Gamma_t= 3.2590	Gamma_t= 2.4842
Carico assiale critico	Vcr= 3997237 N	Vcr= 16505870 N

1. Tensione degli inserti in acciaio		
Tensione massimal amiere (< 235 N/mmq)	Sigma_s= 69.374 N/mmq	Sigma_s= 97.726 N/mmq
2. Deformazioni di taglio massime (< 7)	Gamma_s= 1.0311	Gamma_s= 0.9769
	Gamma_t= 3.2590	Gamma_t= 2.4842
3. Instabilità dell'isolatore (Vcr/2.5)	Vmax= 1598895 N	Vmax= 6602348 N

Seguono i tabulati contenenti le reazioni massime e minime per ogni appoggio.

**REAZIONI APPOGGI (ISOLATORI SISMICI ELASTOMERICI)**

**STATI LIMITE ULTIMI - STATICI**

Tipo isolatore	Struttura Appoggio	No.	Load	Node	Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
1	Spalla 1	185	GEN SLU Mobili(max)	77	-473.83	32.02	5.36	32.47	1.302	24.93	2.09E-02
1	Spalla 1	185	GEN SLU Vento(max)	77	-471.35	49.54	5.31	49.82	1.302	38.26	1.92E-02
1	Spalla 1	185	GEN SLU Frenamento(max)	77	-507.79	1.88	38.76	38.81	1.302	29.80	1.91E-02
1	Spalla 1	185	GEN SLU Termico(max)	77	-453.66	30.78	10.98	32.68	1.302	25.10	1.94E-02
1	Spalla 1	185	GEN SLU Mobili(min)	77	-1299.17	-32.68	-22.58	39.72	1.302	30.51	1.52E-02
1	Spalla 1	185	GEN SLU Vento(min)	77	-1081.84	-50.26	-19.47	53.90	1.302	41.39	1.51E-02
1	Spalla 1	185	GEN SLU Frenamento(min)	77	-1045.40	-2.60	-52.92	52.98	1.302	40.69	1.51E-02
1	Spalla 1	185	GEN SLU Termico(min)	77	-1099.53	-31.50	-25.14	40.30	1.302	30.95	1.48E-02
1	Spalla 1	186	GEN SLU Mobili(max)	79	-558.21	32.74	4.62	33.06	1.302	25.39	2.08E-02
1	Spalla 1	186	GEN SLU Vento(max)	79	-551.88	50.12	4.97	50.37	1.302	38.68	1.92E-02
1	Spalla 1	186	GEN SLU Frenamento(max)	79	-556.07	2.30	38.68	38.75	1.302	29.76	1.91E-02
1	Spalla 1	186	GEN SLU Termico(max)	79	-518.05	31.04	10.86	32.88	1.302	25.26	1.94E-02

1	Spalla 1	186	GEN SLU Mobili(min)	79	-1811.90	-32.29	-21.54	38.82	1.302	29.81	1.58E-02
1	Spalla 1	186	GEN SLU Vento(min)	79	-1436.09	-49.81	-18.72	53.21	1.302	40.87	1.55E-02
1	Spalla 1	186	GEN SLU Frenamento(min)	79	-1431.90	-1.99	-52.44	52.48	1.302	40.30	1.56E-02
1	Spalla 1	186	GEN SLU Termico(min)	79	-1469.92	-30.73	-24.61	39.37	1.302	30.24	1.53E-02
1	Spalla 1	187	GEN SLU Mobili(max)	81	-559.07	32.29	4.62	32.62	1.302	25.05	2.08E-02
1	Spalla 1	187	GEN SLU Vento(max)	81	-553.30	49.83	4.97	50.08	1.302	38.46	1.92E-02
1	Spalla 1	187	GEN SLU Frenamento(max)	81	-556.07	1.99	38.68	38.73	1.302	29.74	1.92E-02
1	Spalla 1	187	GEN SLU Termico(max)	81	-518.91	30.74	10.86	32.60	1.302	25.04	1.94E-02
1	Spalla 1	187	GEN SLU Mobili(min)	81	-1811.05	-32.75	-21.55	39.20	1.302	30.11	1.58E-02
1	Spalla 1	187	GEN SLU Vento(min)	81	-1434.67	-50.13	-18.73	53.51	1.302	41.10	1.55E-02
1	Spalla 1	187	GEN SLU Frenamento(min)	81	-1431.90	-2.30	-52.44	52.49	1.302	40.31	1.55E-02
1	Spalla 1	187	GEN SLU Termico(min)	81	-1469.06	-31.05	-24.62	39.63	1.302	30.43	1.53E-02
1	Spalla 1	188	GEN SLU Mobili(max)	83	-479.70	32.73	5.33	33.16	1.302	25.47	2.09E-02
1	Spalla 1	188	GEN SLU Vento(max)	83	-481.14	50.34	5.27	50.62	1.302	38.87	1.92E-02
1	Spalla 1	188	GEN SLU Frenamento(max)	83	-507.79	2.60	38.76	38.85	1.302	29.83	1.91E-02
1	Spalla 1	188	GEN SLU Termico(max)	83	-459.53	31.55	10.96	33.40	1.302	25.65	1.94E-02
1	Spalla 1	188	GEN SLU Mobili(min)	83	-1293.29	-32.07	-22.56	39.21	1.302	30.11	1.52E-02
1	Spalla 1	188	GEN SLU Vento(min)	83	-1072.05	-49.62	-19.43	53.29	1.302	40.92	1.51E-02
1	Spalla 1	188	GEN SLU Frenamento(min)	83	-1045.40	-1.88	-52.92	52.95	1.302	40.67	1.51E-02
1	Spalla 1	188	GEN SLU Termico(min)	83	-1093.66	-30.83	-25.11	39.76	1.302	30.54	1.49E-02
1	Spalla 2	190	GEN SLU Mobili(max)	78	-473.90	32.02	22.58	39.18	1.302	30.09	1.52E-02
1	Spalla 2	190	GEN SLU Vento(max)	78	-471.38	49.54	19.47	53.23	1.302	40.88	1.51E-02
1	Spalla 2	190	GEN SLU Frenamento(max)	78	-507.84	1.88	52.92	52.95	1.302	40.67	1.51E-02
1	Spalla 2	190	GEN SLU Termico(max)	78	-453.70	30.78	25.14	39.74	1.302	30.52	1.49E-02
1	Spalla 2	190	GEN SLU Mobili(min)	78	-1299.15	-32.68	-5.36	33.12	1.302	25.43	2.09E-02
1	Spalla 2	190	GEN SLU Vento(min)	78	-1081.86	-50.26	-5.31	50.54	1.302	38.81	1.92E-02
1	Spalla 2	190	GEN SLU Frenamento(min)	78	-1045.39	-2.60	-38.76	38.85	1.302	29.83	1.91E-02
1	Spalla 2	190	GEN SLU Termico(min)	78	-1099.54	-31.50	-10.98	33.36	1.302	25.62	1.94E-02
1	Spalla 2	191	GEN SLU Mobili(max)	80	-558.21	32.74	21.54	39.19	1.302	30.10	1.58E-02
1	Spalla 2	191	GEN SLU Vento(max)	80	-551.93	50.12	18.73	53.51	1.302	41.09	1.55E-02
1	Spalla 2	191	GEN SLU Frenamento(max)	80	-556.11	2.30	52.44	52.49	1.302	40.31	1.55E-02
1	Spalla 2	191	GEN SLU Termico(max)	80	-518.10	31.04	24.62	39.62	1.302	30.43	1.53E-02
1	Spalla 2	191	GEN SLU Mobili(min)	80	-1811.61	-32.29	-4.62	32.62	1.302	25.05	2.08E-02
1	Spalla 2	191	GEN SLU Vento(min)	80	-1435.92	-49.81	-4.97	50.06	1.302	38.44	1.92E-02
1	Spalla 2	191	GEN SLU Frenamento(min)	80	-1431.74	-1.99	-38.68	38.73	1.302	29.74	1.92E-02
1	Spalla 2	191	GEN SLU Termico(min)	80	-1469.75	-30.73	-10.86	32.59	1.302	25.03	1.94E-02
1	Spalla 2	192	GEN SLU Mobili(max)	82	-559.06	32.29	21.55	38.82	1.302	29.81	1.58E-02
1	Spalla 2	192	GEN SLU Vento(max)	82	-553.35	49.82	18.73	53.22	1.302	40.88	1.55E-02
1	Spalla 2	192	GEN SLU Frenamento(max)	82	-556.11	1.99	52.44	52.48	1.302	40.30	1.56E-02
1	Spalla 2	192	GEN SLU Termico(max)	82	-518.95	30.74	24.62	39.38	1.302	30.25	1.53E-02
1	Spalla 2	192	GEN SLU Mobili(min)	82	-1810.76	-32.74	-4.62	33.06	1.302	25.39	2.08E-02
1	Spalla 2	192	GEN SLU Vento(min)	82	-1434.50	-50.13	-4.97	50.38	1.302	38.69	1.92E-02
1	Spalla 2	192	GEN SLU Frenamento(min)	82	-1431.74	-2.30	-38.68	38.75	1.302	29.76	1.91E-02
1	Spalla 2	192	GEN SLU Termico(min)	82	-1468.90	-31.05	-10.86	32.89	1.302	25.26	1.94E-02
1	Spalla 2	193	GEN SLU Mobili(max)	84	-479.77	32.73	22.56	39.75	1.302	30.53	1.52E-02
1	Spalla 2	193	GEN SLU Vento(max)	84	-481.16	50.34	19.43	53.96	1.302	41.44	1.51E-02
1	Spalla 2	193	GEN SLU Frenamento(max)	84	-507.84	2.60	52.92	52.98	1.302	40.69	1.51E-02
1	Spalla 2	193	GEN SLU Termico(max)	84	-459.57	31.55	25.12	40.33	1.302	30.97	1.49E-02
1	Spalla 2	193	GEN SLU Mobili(min)	84	-1293.28	-32.07	-5.34	32.51	1.302	24.97	2.09E-02
1	Spalla 2	193	GEN SLU Vento(min)	84	-1072.07	-49.62	-5.27	49.90	1.302	38.32	1.92E-02
1	Spalla 2	193	GEN SLU Frenamento(min)	84	-1045.39	-1.88	-38.76	38.81	1.302	29.80	1.91E-02
1	Spalla 2	193	GEN SLU Termico(min)	84	-1093.67	-30.83	-10.96	32.72	1.302	25.13	1.94E-02
2	Pila 1	195	GEN SLU Mobili(max)	21	-1837.85	72.35	29.88	78.28	2.855	27.42	8.69E-04
2	Pila 1	195	GEN SLU Vento(max)	21	-1860.77	113.18	28.50	116.71	2.855	40.89	1.29E-03
2	Pila 1	195	GEN SLU Frenamento(max)	21	-1982.79	6.22	101.40	101.59	2.855	35.59	1.27E-03
2	Pila 1	195	GEN SLU Termico(max)	21	-1785.60	71.62	33.35	79.00	2.855	27.68	1.24E-03
2	Pila 1	195	GEN SLU Mobili(min)	21	-3643.35	-66.93	-4.21	67.06	2.855	23.49	4.84E-03
2	Pila 1	195	GEN SLU Vento(min)	21	-3144.38	-108.32	-1.03	108.32	2.855	37.95	4.35E-03
2	Pila 1	195	GEN SLU Frenamento(min)	21	-3022.36	-1.36	-73.93	73.94	2.855	25.90	4.32E-03
2	Pila 1	195	GEN SLU Termico(min)	21	-3219.55	-66.76	-5.87	67.02	2.855	23.48	4.45E-03
2	Pila 1	196	GEN SLU Mobili(max)	23	-2408.35	68.89	28.94	74.72	2.855	26.18	1.09E-03
2	Pila 1	196	GEN SLU Vento(max)	23	-2402.06	110.19	27.59	113.59	2.855	39.79	1.42E-03
2	Pila 1	196	GEN SLU Frenamento(max)	23	-2416.77	3.04	100.70	100.75	2.855	35.29	1.39E-03
2	Pila 1	196	GEN SLU Termico(max)	23	-2283.95	67.68	32.51	75.08	2.855	26.30	1.35E-03
2	Pila 1	196	GEN SLU Mobili(min)	23	-4460.62	-67.25	-2.65	67.30	2.855	23.58	4.57E-03
2	Pila 1	196	GEN SLU Vento(min)	23	-3741.51	-108.71	0.12	108.71	2.855	38.08	4.11E-03
2	Pila 1	196	GEN SLU Frenamento(min)	23	-3726.80	-1.57	-72.99	73.01	2.855	25.57	4.11E-03
2	Pila 1	196	GEN SLU Termico(min)	23	-3859.62	-66.20	-4.80	66.37	2.855	23.25	4.19E-03
2	Pila 1	197	GEN SLU Mobili(max)	885	-2405.24	67.26	28.94	73.22	2.855	25.65	1.24E-03
2	Pila 1	197	GEN SLU Vento(max)	885	-2396.87	108.74	27.59	112.19	2.855	39.30	1.51E-03
2	Pila 1	197	GEN SLU Frenamento(max)	885	-2416.77	1.57	100.70	100.71	2.855	35.28	1.45E-03
2	Pila 1	197	GEN SLU Termico(max)	885	-2280.84	66.21	32.51	73.76	2.855	25.84	1.44E-03
2	Pila 1	197	GEN SLU Mobili(min)	885	-4463.73	-68.90	-2.65	68.95	2.855	24.15	4.53E-03
2	Pila 1	197	GEN SLU Vento(min)	885	-3746.70	-110.21	0.12	110.21	2.855	38.61	4.08E-03
2	Pila 1	197	GEN SLU Frenamento(min)	885	-3726.80	-3.04	-72.99	73.05	2.855	25.59	4.09E-03
2	Pila 1	197	GEN SLU Termico(min)	885	-3862.73	-67.69	-4.80	67.86	2.855	23.77	4.16E-03
2	Pila 1	198	GEN SLU Mobili(max)	27	-1845.68	66.96	29.88	73.32	2.855	25.69	1.43E-03

2	Pila 1	198	GEN SLU Vento(max)	27	-1873.82	108.37	28.50	112.05	2.855	39.25	1.65E-03
2	Pila 1	198	GEN SLU Frenamento(max)	27	-1982.79	1.36	101.40	101.41	2.855	35.52	1.56E-03
2	Pila 1	198	GEN SLU Termico(max)	27	-1793.43	66.79	33.35	74.65	2.855	26.15	1.69E-03
2	Pila 1	198	GEN SLU Mobili(min)	27	-3635.52	-72.38	-4.21	72.50	2.855	25.40	4.70E-03
2	Pila 1	198	GEN SLU Vento(min)	27	-3131.33	-113.23	-1.03	113.23	2.855	39.67	4.22E-03
2	Pila 1	198	GEN SLU Frenamento(min)	27	-3022.36	-6.22	-73.93	74.19	2.855	25.99	4.23E-03
2	Pila 1	198	GEN SLU Termico(min)	27	-3211.72	-71.65	-5.87	71.89	2.855	25.18	4.30E-03
2	Pila 2	200	GEN SLU Mobili(max)	22	-1837.83	72.35	4.21	72.47	2.855	25.39	4.70E-03
2	Pila 2	200	GEN SLU Vento(max)	22	-1860.74	113.18	1.02	113.18	2.855	39.65	4.22E-03
2	Pila 2	200	GEN SLU Frenamento(max)	22	-1982.84	6.22	73.93	74.19	2.855	25.99	4.23E-03
2	Pila 2	200	GEN SLU Termico(max)	22	-1785.58	71.62	5.87	71.86	2.855	25.17	4.30E-03
2	Pila 2	200	GEN SLU Mobili(min)	22	-3643.38	-66.93	-29.88	73.30	2.855	25.68	1.44E-03
2	Pila 2	200	GEN SLU Vento(min)	22	-3144.43	-108.32	-28.50	112.01	2.855	39.24	1.65E-03
2	Pila 2	200	GEN SLU Frenamento(min)	22	-3022.33	-1.36	-101.41	101.42	2.855	35.53	1.56E-03
2	Pila 2	200	GEN SLU Termico(min)	22	-3219.58	-66.76	-33.35	74.63	2.855	26.14	1.69E-03
2	Pila 2	201	GEN SLU Mobili(max)	24	-2408.36	68.89	2.65	68.94	2.855	24.15	4.54E-03
2	Pila 2	201	GEN SLU Vento(max)	24	-2402.07	110.19	-0.12	110.19	2.855	38.60	4.08E-03
2	Pila 2	201	GEN SLU Frenamento(max)	24	-2416.75	3.04	72.99	73.05	2.855	25.59	4.09E-03
2	Pila 2	201	GEN SLU Termico(max)	24	-2283.96	67.68	4.80	67.85	2.855	23.77	4.16E-03
2	Pila 2	201	GEN SLU Mobili(min)	24	-4460.60	-67.25	-28.94	73.21	2.855	25.65	1.24E-03
2	Pila 2	201	GEN SLU Vento(min)	24	-3741.48	-108.71	-27.59	112.16	2.855	39.29	1.51E-03
2	Pila 2	201	GEN SLU Frenamento(min)	24	-3726.80	-1.57	-100.70	100.71	2.855	35.28	1.45E-03
2	Pila 2	201	GEN SLU Termico(min)	24	-3859.59	-66.20	-32.51	73.75	2.855	25.84	1.45E-03
2	Pila 2	202	GEN SLU Mobili(max)	886	-2405.24	67.26	2.65	67.31	2.855	23.58	4.57E-03
2	Pila 2	202	GEN SLU Vento(max)	886	-2396.86	108.74	-0.12	108.74	2.855	38.09	4.11E-03
2	Pila 2	202	GEN SLU Frenamento(max)	886	-2416.75	1.57	72.99	73.01	2.855	25.57	4.11E-03
2	Pila 2	202	GEN SLU Termico(max)	886	-2280.84	66.21	4.80	66.38	2.855	23.25	4.19E-03
2	Pila 2	202	GEN SLU Mobili(min)	886	-4463.73	-68.90	-28.94	74.73	2.855	26.18	1.09E-03
2	Pila 2	202	GEN SLU Vento(min)	886	-3746.70	-110.21	-27.59	113.61	2.855	39.80	1.42E-03
2	Pila 2	202	GEN SLU Frenamento(min)	886	-3726.80	-3.04	-100.70	100.75	2.855	35.29	1.39E-03
2	Pila 2	202	GEN SLU Termico(min)	886	-3862.72	-67.69	-32.51	75.09	2.855	26.31	1.35E-03
2	Pila 2	203	GEN SLU Mobili(max)	28	-1845.66	66.96	4.21	67.09	2.855	23.50	4.84E-03
2	Pila 2	203	GEN SLU Vento(max)	28	-1873.78	108.37	1.02	108.37	2.855	37.96	4.34E-03
2	Pila 2	203	GEN SLU Frenamento(max)	28	-1982.84	1.36	73.93	73.94	2.855	25.90	4.32E-03
2	Pila 2	203	GEN SLU Termico(max)	28	-1793.41	66.79	5.87	67.05	2.855	23.49	4.45E-03
2	Pila 2	203	GEN SLU Mobili(min)	28	-3635.56	-72.38	-29.88	78.31	2.855	27.43	8.67E-04
2	Pila 2	203	GEN SLU Vento(min)	28	-3131.38	-113.23	-28.50	116.76	2.855	40.90	1.29E-03
2	Pila 2	203	GEN SLU Frenamento(min)	28	-3022.33	-6.22	-101.41	101.60	2.855	35.59	1.27E-03
2	Pila 2	203	GEN SLU Termico(min)	28	-3211.75	-71.65	-33.35	79.03	2.855	27.69	1.24E-03

		Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
<b>SFORZI MASSIMI:</b>	Spalle	-453.66	50.34	52.92	53.96	1.302	41.44	2.09E-02
	Pile	-1785.58	113.18	101.40	116.76	2.855	40.90	4.84E-03
<b>SFORZI MINIMI:</b>	Spalle	-1811.90	-50.26	-52.92				
	Pile	-4463.73	-113.23	-101.41				

**STATI LIMITE ULTIMI - SISMICI (SLC)**

Tipo isolatore	Struttura Appoggio	No.	Load	Node	Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
1	Spalla 1	185	GEN SLC Long(max)	77	-386.72	25.99	91.73	95.34	1.302	73.22	1.24E-02
1	Spalla 1	185	GEN SLC Trasv(max)	77	-360.73	86.54	29.99	91.59	1.302	70.34	1.24E-02
1	Spalla 1	185	GEN SLC Vert(max)	77	-368.06	25.99	30.03	39.71	1.302	30.50	1.25E-02
1	Spalla 1	185	GEN SLC Long(min)	77	-525.43	-26.59	-98.51	102.04	1.302	78.36	1.16E-02
1	Spalla 1	185	GEN SLC Trasv(min)	77	-551.42	-87.15	-36.77	94.59	1.302	72.64	1.16E-02
1	Spalla 1	185	GEN SLC Vert(min)	77	-544.09	-26.60	-36.82	45.42	1.302	34.88	1.14E-02
1	Spalla 1	186	GEN SLC Long(max)	79	-381.04	25.97	91.77	95.37	1.302	73.25	1.25E-02
1	Spalla 1	186	GEN SLC Trasv(max)	79	-387.23	86.63	29.78	91.61	1.302	70.35	1.25E-02
1	Spalla 1	186	GEN SLC Vert(max)	79	-364.94	25.97	30.06	39.72	1.302	30.51	1.27E-02
1	Spalla 1	186	GEN SLC Long(min)	79	-503.25	-26.14	-98.50	101.91	1.302	78.27	1.18E-02
1	Spalla 1	186	GEN SLC Trasv(min)	79	-497.05	-86.80	-36.51	94.17	1.302	72.32	1.18E-02
1	Spalla 1	186	GEN SLC Vert(min)	79	-519.35	-26.14	-36.79	45.13	1.302	34.66	1.17E-02
1	Spalla 1	187	GEN SLC Long(max)	81	-381.04	26.14	91.77	95.42	1.302	73.28	1.25E-02
1	Spalla 1	187	GEN SLC Trasv(max)	81	-387.23	86.80	29.78	91.77	1.302	70.48	1.25E-02
1	Spalla 1	187	GEN SLC Vert(max)	81	-364.94	26.14	30.06	39.84	1.302	30.59	1.27E-02
1	Spalla 1	187	GEN SLC Long(min)	81	-503.25	-25.97	-98.50	101.87	1.302	78.23	1.18E-02
1	Spalla 1	187	GEN SLC Trasv(min)	81	-497.05	-86.63	-36.51	94.01	1.302	72.20	1.18E-02
1	Spalla 1	187	GEN SLC Vert(min)	81	-519.34	-25.97	-36.79	45.03	1.302	34.58	1.17E-02
1	Spalla 1	188	GEN SLC Long(max)	83	-386.73	26.59	91.73	95.51	1.302	73.35	1.24E-02
1	Spalla 1	188	GEN SLC Trasv(max)	83	-360.74	87.15	29.99	92.17	1.302	70.78	1.24E-02
1	Spalla 1	188	GEN SLC Vert(max)	83	-368.07	26.60	30.03	40.12	1.302	30.81	1.25E-02
1	Spalla 1	188	GEN SLC Long(min)	83	-525.43	-25.99	-98.51	101.88	1.302	78.24	1.16E-02



1	Spalla 1	188	GEN SLC Trasv(min)	83	-551.42	-86.54	-36.77	94.03	1.302	72.21	1.16E-02
1	Spalla 1	188	GEN SLC Vert(min)	83	-544.08	-25.99	-36.82	45.07	1.302	34.61	1.14E-02
1	Spalla 2	190	GEN SLC Long(max)	78	-386.73	25.99	98.51	101.88	1.302	78.24	1.16E-02
1	Spalla 2	190	GEN SLC Trasv(max)	78	-360.74	86.54	36.77	94.03	1.302	72.21	1.16E-02
1	Spalla 2	190	GEN SLC Vert(max)	78	-368.07	25.99	36.82	45.07	1.302	34.61	1.14E-02
1	Spalla 2	190	GEN SLC Long(min)	78	-525.43	-26.59	-91.73	95.51	1.302	73.35	1.24E-02
1	Spalla 2	190	GEN SLC Trasv(min)	78	-551.42	-87.15	-29.99	92.17	1.302	70.78	1.24E-02
1	Spalla 2	190	GEN SLC Vert(min)	78	-544.08	-26.60	-30.03	40.12	1.302	30.81	1.25E-02
1	Spalla 2	191	GEN SLC Long(max)	80	-381.04	25.97	98.50	101.87	1.302	78.23	1.18E-02
1	Spalla 2	191	GEN SLC Trasv(max)	80	-387.23	86.63	36.51	94.01	1.302	72.20	1.18E-02
1	Spalla 2	191	GEN SLC Vert(max)	80	-364.94	25.97	36.79	45.03	1.302	34.58	1.17E-02
1	Spalla 2	191	GEN SLC Long(min)	80	-503.25	-26.14	-91.77	95.42	1.302	73.28	1.25E-02
1	Spalla 2	191	GEN SLC Trasv(min)	80	-497.05	-86.80	-29.78	91.77	1.302	70.48	1.25E-02
1	Spalla 2	191	GEN SLC Vert(min)	80	-519.34	-26.14	-30.06	39.84	1.302	30.59	1.27E-02
1	Spalla 2	192	GEN SLC Long(max)	82	-381.04	26.14	98.50	101.91	1.302	78.27	1.18E-02
1	Spalla 2	192	GEN SLC Trasv(max)	82	-387.23	86.80	36.51	94.17	1.302	72.32	1.18E-02
1	Spalla 2	192	GEN SLC Vert(max)	82	-364.94	26.14	36.79	45.13	1.302	34.66	1.17E-02
1	Spalla 2	192	GEN SLC Long(min)	82	-503.25	-25.97	-91.77	95.37	1.302	73.25	1.25E-02
1	Spalla 2	192	GEN SLC Trasv(min)	82	-497.05	-86.63	-29.78	91.61	1.302	70.35	1.25E-02
1	Spalla 2	192	GEN SLC Vert(min)	82	-519.35	-25.97	-30.06	39.72	1.302	30.51	1.27E-02
1	Spalla 2	193	GEN SLC Long(max)	84	-386.72	26.59	98.51	102.04	1.302	78.36	1.16E-02
1	Spalla 2	193	GEN SLC Trasv(max)	84	-360.73	87.15	36.77	94.59	1.302	72.64	1.16E-02
1	Spalla 2	193	GEN SLC Vert(max)	84	-368.06	26.60	36.82	45.42	1.302	34.88	1.14E-02
1	Spalla 2	193	GEN SLC Long(min)	84	-525.43	-25.99	-91.73	95.34	1.302	73.22	1.24E-02
1	Spalla 2	193	GEN SLC Trasv(min)	84	-551.42	-86.54	-29.99	91.59	1.302	70.34	1.24E-02
1	Spalla 2	193	GEN SLC Vert(min)	84	-544.09	-25.99	-30.03	39.71	1.302	30.50	1.25E-02
2	Pila 1	195	GEN SLC Long(max)	21	-1404.85	60.99	209.46	218.16	2.855	76.42	1.93E-03
2	Pila 1	195	GEN SLC Trasv(max)	21	-1280.92	196.50	75.49	210.50	2.855	73.74	1.98E-03
2	Pila 1	195	GEN SLC Vert(max)	21	-1339.52	61.03	75.30	96.93	2.855	33.95	1.98E-03
2	Pila 1	195	GEN SLC Long(min)	21	-1838.87	-57.78	-185.28	194.08	2.855	67.99	2.36E-03
2	Pila 1	195	GEN SLC Trasv(min)	21	-1962.80	-193.30	-51.31	199.99	2.855	70.06	2.36E-03
2	Pila 1	195	GEN SLC Vert(min)	21	-1904.20	-57.83	-51.12	77.19	2.855	27.04	2.31E-03
2	Pila 1	196	GEN SLC Long(max)	23	-1721.99	59.04	209.31	217.48	2.855	76.18	1.91E-03
2	Pila 1	196	GEN SLC Trasv(max)	23	-1711.16	194.78	75.06	208.74	2.855	73.12	1.98E-03
2	Pila 1	196	GEN SLC Vert(max)	23	-1648.71	59.05	75.13	95.56	2.855	33.48	1.96E-03
2	Pila 1	196	GEN SLC Long(min)	23	-2070.31	-58.05	-185.23	194.11	2.855	68.00	2.28E-03
2	Pila 1	196	GEN SLC Trasv(min)	23	-2081.14	-193.79	-50.99	200.39	2.855	70.20	2.23E-03
2	Pila 1	196	GEN SLC Vert(min)	23	-2143.59	-58.06	-51.06	77.32	2.855	27.09	2.22E-03
2	Pila 1	197	GEN SLC Long(max)	885	-1721.99	58.05	209.31	217.21	2.855	76.09	1.92E-03
2	Pila 1	197	GEN SLC Trasv(max)	885	-1711.16	193.79	75.06	207.82	2.855	72.80	1.99E-03
2	Pila 1	197	GEN SLC Vert(max)	885	-1648.71	58.06	75.13	94.95	2.855	33.26	1.97E-03
2	Pila 1	197	GEN SLC Long(min)	885	-2070.31	-59.04	-185.23	194.41	2.855	68.10	2.27E-03
2	Pila 1	197	GEN SLC Trasv(min)	885	-2081.14	-194.78	-50.99	201.34	2.855	70.53	2.22E-03
2	Pila 1	197	GEN SLC Vert(min)	885	-2143.58	-59.05	-51.06	78.06	2.855	27.35	2.22E-03
2	Pila 1	198	GEN SLC Long(max)	27	-1404.86	57.78	209.46	217.28	2.855	76.12	2.00E-03
2	Pila 1	198	GEN SLC Trasv(max)	27	-1280.93	193.30	75.49	207.52	2.855	72.70	2.10E-03
2	Pila 1	198	GEN SLC Vert(max)	27	-1339.53	57.83	75.30	94.94	2.855	33.26	2.06E-03
2	Pila 1	198	GEN SLC Long(min)	27	-1838.86	-60.99	-185.28	195.06	2.855	68.33	2.30E-03
2	Pila 1	198	GEN SLC Trasv(min)	27	-1962.80	-196.50	-51.31	203.09	2.855	71.14	2.26E-03
2	Pila 1	198	GEN SLC Vert(min)	27	-1904.19	-61.03	-51.12	79.61	2.855	27.89	2.25E-03
2	Pila 2	200	GEN SLC Long(max)	22	-1404.86	60.99	185.28	195.06	2.855	68.33	2.30E-03
2	Pila 2	200	GEN SLC Trasv(max)	22	-1280.92	196.50	51.31	203.09	2.855	71.14	2.26E-03
2	Pila 2	200	GEN SLC Vert(max)	22	-1339.52	61.03	51.12	79.61	2.855	27.89	2.25E-03
2	Pila 2	200	GEN SLC Long(min)	22	-1838.86	-57.78	-209.46	217.28	2.855	76.12	2.00E-03
2	Pila 2	200	GEN SLC Trasv(min)	22	-1962.80	-193.30	-75.49	207.52	2.855	72.70	2.10E-03
2	Pila 2	200	GEN SLC Vert(min)	22	-1904.20	-57.83	-75.30	94.94	2.855	33.26	2.06E-03
2	Pila 2	201	GEN SLC Long(max)	24	-1721.99	59.04	185.23	194.41	2.855	68.10	2.27E-03
2	Pila 2	201	GEN SLC Trasv(max)	24	-1711.15	194.78	50.99	201.34	2.855	70.53	2.22E-03
2	Pila 2	201	GEN SLC Vert(max)	24	-1648.71	59.05	51.06	78.06	2.855	27.35	2.22E-03
2	Pila 2	201	GEN SLC Long(min)	24	-2070.31	-58.05	-209.31	217.21	2.855	76.09	1.92E-03
2	Pila 2	201	GEN SLC Trasv(min)	24	-2081.14	-193.79	-75.06	207.82	2.855	72.80	1.99E-03
2	Pila 2	201	GEN SLC Vert(min)	24	-2143.59	-58.06	-75.13	94.95	2.855	33.26	1.97E-03
2	Pila 2	202	GEN SLC Long(max)	886	-1721.99	58.05	185.23	194.11	2.855	68.00	2.28E-03
2	Pila 2	202	GEN SLC Trasv(max)	886	-1711.16	193.79	50.99	200.39	2.855	70.20	2.23E-03
2	Pila 2	202	GEN SLC Vert(max)	886	-1648.72	58.06	51.06	77.32	2.855	27.09	2.22E-03
2	Pila 2	202	GEN SLC Long(min)	886	-2070.31	-59.04	-209.31	217.48	2.855	76.18	1.91E-03
2	Pila 2	202	GEN SLC Trasv(min)	886	-2081.13	-194.78	-75.06	208.74	2.855	73.12	1.98E-03
2	Pila 2	202	GEN SLC Vert(min)	886	-2143.58	-59.05	-75.13	95.56	2.855	33.48	1.96E-03
2	Pila 2	203	GEN SLC Long(max)	28	-1404.85	57.78	185.28	194.08	2.855	67.99	2.36E-03
2	Pila 2	203	GEN SLC Trasv(max)	28	-1280.93	193.30	51.31	199.99	2.855	70.06	2.36E-03
2	Pila 2	203	GEN SLC Vert(max)	28	-1339.53	57.83	51.12	77.19	2.855	27.04	2.31E-03
2	Pila 2	203	GEN SLC Long(min)	28	-1838.87	-60.99	-209.46	218.16	2.855	76.42	1.93E-03
2	Pila 2	203	GEN SLC Trasv(min)	28	-1962.79	-196.50	-75.49	210.50	2.855	73.74	1.98E-03
2	Pila 2	203	GEN SLC Vert(min)	28	-1904.20	-61.03	-75.30	96.93	2.855	33.95	1.98E-03

**Axial (kN)**    **Shear-y (kN)**    **Shear-z (kN)**    **Shear-tot (kN)**    **Kh eq. (kN/mm)**    **Deform. (mm)**    **Rotazione (rad)**



<b>SFORZI MASSIMI:</b>	<b>Spalle</b>	<b>-360.73</b>	<b>87.15</b>	<b>98.51</b>	<b>102.04</b>	<b>1.302</b>	<b>78.36</b>	<b>1.27E-02</b>
	<b>Pile</b>	<b>-1280.92</b>	<b>196.50</b>	<b>209.46</b>	<b>218.16</b>	<b>2.855</b>	<b>76.42</b>	<b>2.36E-03</b>

<b>SFORZI MINIMI:</b>	<b>Spalle</b>	<b>-551.42</b>	<b>-87.15</b>	<b>-98.51</b>				
	<b>Pile</b>	<b>-2143.59</b>	<b>-196.50</b>	<b>-209.46</b>				

**STATI LIMITE ULTIMI - SISMICI (SLV)**

Tipo isolatore	Struttura Appoggio	No.	Load	Node	Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
1	Spalla 1	185	GEN SLV Long(max)	77	-395.15	20.23	71.99	74.78	1.302	57.43	1.23E-02
1	Spalla 1	185	GEN SLV Trasv(max)	77	-374.92	67.37	23.92	71.49	1.302	54.90	1.24E-02
1	Spalla 1	185	GEN SLV Vert(max)	77	-382.56	20.24	23.93	31.34	1.302	24.07	1.24E-02
1	Spalla 1	185	GEN SLV Long(min)	77	-517.00	-20.84	-78.77	81.48	1.302	62.58	1.16E-02
1	Spalla 1	185	GEN SLV Trasv(min)	77	-537.23	-67.98	-30.71	74.59	1.302	57.29	1.16E-02
1	Spalla 1	185	GEN SLV Vert(min)	77	-529.60	-20.84	-30.71	37.11	1.302	28.50	1.15E-02
1	Spalla 1	186	GEN SLV Long(max)	79	-386.78	20.21	72.06	74.84	1.302	57.48	1.25E-02
1	Spalla 1	186	GEN SLV Trasv(max)	79	-391.61	67.43	23.79	71.50	1.302	54.91	1.25E-02
1	Spalla 1	186	GEN SLV Vert(max)	79	-376.09	20.21	23.98	31.36	1.302	24.08	1.26E-02
1	Spalla 1	186	GEN SLV Long(min)	79	-497.50	-20.38	-78.79	81.38	1.302	62.50	1.19E-02
1	Spalla 1	186	GEN SLV Trasv(min)	79	-492.67	-67.60	-30.52	74.17	1.302	56.96	1.19E-02
1	Spalla 1	186	GEN SLV Vert(min)	79	-508.20	-20.38	-30.71	36.86	1.302	28.31	1.17E-02
1	Spalla 1	187	GEN SLV Long(max)	81	-386.78	20.38	72.06	74.89	1.302	57.51	1.25E-02
1	Spalla 1	187	GEN SLV Trasv(max)	81	-391.61	67.60	23.79	71.66	1.302	55.04	1.25E-02
1	Spalla 1	187	GEN SLV Vert(max)	81	-376.09	20.38	23.98	31.47	1.302	24.17	1.26E-02
1	Spalla 1	187	GEN SLV Long(min)	81	-497.50	-20.21	-78.79	81.34	1.302	62.47	1.19E-02
1	Spalla 1	187	GEN SLV Trasv(min)	81	-492.67	-67.43	-30.52	74.02	1.302	56.84	1.19E-02
1	Spalla 1	187	GEN SLV Vert(min)	81	-508.19	-20.21	-30.71	36.76	1.302	28.23	1.17E-02
1	Spalla 1	188	GEN SLV Long(max)	83	-395.15	20.84	71.99	74.95	1.302	57.56	1.23E-02
1	Spalla 1	188	GEN SLV Trasv(max)	83	-374.92	67.98	23.92	72.07	1.302	55.35	1.23E-02
1	Spalla 1	188	GEN SLV Vert(max)	83	-382.57	20.84	23.93	31.73	1.302	24.37	1.24E-02
1	Spalla 1	188	GEN SLV Long(min)	83	-517.00	-20.23	-78.77	81.33	1.302	62.46	1.16E-02
1	Spalla 1	188	GEN SLV Trasv(min)	83	-537.23	-67.37	-30.71	74.04	1.302	56.86	1.16E-02
1	Spalla 1	188	GEN SLV Vert(min)	83	-529.59	-20.24	-30.71	36.78	1.302	28.25	1.15E-02
1	Spalla 2	190	GEN SLV Long(max)	78	-395.15	20.23	78.77	81.33	1.302	62.46	1.16E-02
1	Spalla 2	190	GEN SLV Trasv(max)	78	-374.92	67.37	30.71	74.04	1.302	56.86	1.16E-02
1	Spalla 2	190	GEN SLV Vert(max)	78	-382.56	20.24	30.71	36.78	1.302	28.25	1.15E-02
1	Spalla 2	190	GEN SLV Long(min)	78	-517.00	-20.84	-71.99	74.95	1.302	57.56	1.23E-02
1	Spalla 2	190	GEN SLV Trasv(min)	78	-537.23	-67.98	-23.92	72.07	1.302	55.35	1.23E-02
1	Spalla 2	190	GEN SLV Vert(min)	78	-529.59	-20.84	-23.93	31.73	1.302	24.37	1.24E-02
1	Spalla 2	191	GEN SLV Long(max)	80	-386.78	20.21	78.79	81.34	1.302	62.47	1.19E-02
1	Spalla 2	191	GEN SLV Trasv(max)	80	-391.61	67.43	30.52	74.02	1.302	56.84	1.19E-02
1	Spalla 2	191	GEN SLV Vert(max)	80	-376.09	20.21	30.71	36.76	1.302	28.23	1.17E-02
1	Spalla 2	191	GEN SLV Long(min)	80	-497.50	-20.38	-72.06	74.89	1.302	57.51	1.25E-02
1	Spalla 2	191	GEN SLV Trasv(min)	80	-492.67	-67.60	-23.79	71.66	1.302	55.04	1.25E-02
1	Spalla 2	191	GEN SLV Vert(min)	80	-508.20	-20.38	-23.98	31.47	1.302	24.17	1.26E-02
1	Spalla 2	192	GEN SLV Long(max)	82	-386.78	20.38	78.79	81.38	1.302	62.50	1.19E-02
1	Spalla 2	192	GEN SLV Trasv(max)	82	-391.61	67.60	30.52	74.17	1.302	56.96	1.19E-02
1	Spalla 2	192	GEN SLV Vert(max)	82	-376.09	20.38	30.71	36.86	1.302	28.31	1.17E-02
1	Spalla 2	192	GEN SLV Long(min)	82	-497.50	-20.21	-72.06	74.84	1.302	57.48	1.25E-02
1	Spalla 2	192	GEN SLV Trasv(min)	82	-492.67	-67.43	-23.79	71.50	1.302	54.91	1.25E-02
1	Spalla 2	192	GEN SLV Vert(min)	82	-508.20	-20.21	-23.98	31.36	1.302	24.08	1.26E-02
1	Spalla 2	193	GEN SLV Long(max)	84	-395.15	20.84	78.77	81.48	1.302	62.58	1.16E-02
1	Spalla 2	193	GEN SLV Trasv(max)	84	-374.92	67.98	30.71	74.59	1.302	57.29	1.16E-02
1	Spalla 2	193	GEN SLV Vert(max)	84	-382.56	20.84	30.71	37.11	1.302	28.50	1.15E-02
1	Spalla 2	193	GEN SLV Long(min)	84	-517.00	-20.23	-71.99	74.78	1.302	57.43	1.23E-02
1	Spalla 2	193	GEN SLV Trasv(min)	84	-537.23	-67.37	-23.92	71.49	1.302	54.90	1.24E-02
1	Spalla 2	193	GEN SLV Vert(min)	84	-529.60	-20.24	-23.93	31.34	1.302	24.07	1.24E-02
2	Pila 1	195	GEN SLV Long(max)	21	-1426.45	48.11	166.88	173.68	2.855	60.84	1.95E-03
2	Pila 1	195	GEN SLV Trasv(max)	21	-1329.98	153.61	62.59	165.87	2.855	58.11	1.99E-03
2	Pila 1	195	GEN SLV Vert(max)	21	-1380.80	48.14	62.42	78.83	2.855	27.61	1.99E-03
2	Pila 1	195	GEN SLV Long(min)	21	-1817.27	-44.91	-142.70	149.60	2.855	52.41	2.34E-03
2	Pila 1	195	GEN SLV Trasv(min)	21	-1913.74	-150.41	-38.41	155.24	2.855	54.38	2.33E-03
2	Pila 1	195	GEN SLV Vert(min)	21	-1862.92	-44.94	-38.24	59.01	2.855	20.67	2.30E-03
2	Pila 1	196	GEN SLV Long(max)	23	-1734.35	46.15	166.75	173.02	2.855	60.61	1.93E-03
2	Pila 1	196	GEN SLV Trasv(max)	23	-1725.93	151.82	62.24	164.08	2.855	57.48	1.98E-03
2	Pila 1	196	GEN SLV Vert(max)	23	-1683.19	46.16	62.28	77.52	2.855	27.16	1.97E-03
2	Pila 1	196	GEN SLV Long(min)	23	-2057.95	-45.16	-142.67	149.65	2.855	52.42	2.25E-03
2	Pila 1	196	GEN SLV Trasv(min)	23	-2066.37	-150.83	-38.17	155.58	2.855	54.50	2.22E-03
2	Pila 1	196	GEN SLV Vert(min)	23	-2109.11	-45.17	-38.21	59.16	2.855	20.73	2.21E-03
2	Pila 1	197	GEN SLV Long(max)	885	-1734.35	45.16	166.75	172.76	2.855	60.52	1.94E-03
2	Pila 1	197	GEN SLV Trasv(max)	885	-1725.93	150.83	62.24	163.17	2.855	57.16	2.00E-03
2	Pila 1	197	GEN SLV Vert(max)	885	-1683.19	45.17	62.28	76.94	2.855	26.95	1.98E-03
2	Pila 1	197	GEN SLV Long(min)	885	-2057.95	-46.15	-142.67	149.95	2.855	52.53	2.25E-03
2	Pila 1	197	GEN SLV Trasv(min)	885	-2066.37	-151.82	-38.17	156.54	2.855	54.84	2.20E-03
2	Pila 1	197	GEN SLV Vert(min)	885	-2109.11	-46.16	-38.21	59.92	2.855	20.99	2.20E-03
2	Pila 1	198	GEN SLV Long(max)	27	-1426.45	44.91	166.88	172.82	2.855	60.54	2.02E-03

2	Pila 1	198	GEN SLV Trasv(max)	27	-1329.98	150.41	62.59	162.91	2.855	57.07	2.10E-03
2	Pila 1	198	GEN SLV Vert(max)	27	-1380.80	44.94	62.42	76.91	2.855	26.94	2.06E-03
2	Pila 1	198	GEN SLV Long(min)	27	-1817.27	-48.11	-142.70	150.59	2.855	52.75	2.28E-03
2	Pila 1	198	GEN SLV Trasv(min)	27	-1913.74	-153.61	-38.41	158.34	2.855	55.47	2.24E-03
2	Pila 1	198	GEN SLV Vert(min)	27	-1862.92	-48.14	-38.24	61.48	2.855	21.54	2.23E-03
2	Pila 2	200	GEN SLV Long(max)	22	-1426.45	48.11	142.70	150.59	2.855	52.75	2.28E-03
2	Pila 2	200	GEN SLV Trasv(max)	22	-1329.98	153.61	38.41	158.34	2.855	55.47	2.24E-03
2	Pila 2	200	GEN SLV Vert(max)	22	-1380.80	48.14	38.24	61.48	2.855	21.54	2.23E-03
2	Pila 2	200	GEN SLV Long(min)	22	-1817.27	-44.91	-166.88	172.82	2.855	60.54	2.02E-03
2	Pila 2	200	GEN SLV Trasv(min)	22	-1913.74	-150.41	-62.59	162.91	2.855	57.07	2.10E-03
2	Pila 2	200	GEN SLV Vert(min)	22	-1862.92	-44.94	-62.42	76.91	2.855	26.94	2.06E-03
2	Pila 2	201	GEN SLV Long(max)	24	-1734.35	46.15	142.67	149.95	2.855	52.53	2.25E-03
2	Pila 2	201	GEN SLV Trasv(max)	24	-1725.93	151.82	38.17	156.54	2.855	54.84	2.20E-03
2	Pila 2	201	GEN SLV Vert(max)	24	-1683.19	46.16	38.21	59.92	2.855	20.99	2.20E-03
2	Pila 2	201	GEN SLV Long(min)	24	-2057.95	-45.16	-166.75	172.76	2.855	60.52	1.94E-03
2	Pila 2	201	GEN SLV Trasv(min)	24	-2066.37	-150.83	-62.24	163.17	2.855	57.16	2.00E-03
2	Pila 2	201	GEN SLV Vert(min)	24	-2109.11	-45.17	-62.28	76.94	2.855	26.95	1.98E-03
2	Pila 2	202	GEN SLV Long(max)	886	-1734.35	45.16	142.67	149.65	2.855	52.42	2.25E-03
2	Pila 2	202	GEN SLV Trasv(max)	886	-1725.93	150.83	38.17	155.58	2.855	54.50	2.22E-03
2	Pila 2	202	GEN SLV Vert(max)	886	-1683.19	45.17	38.21	59.16	2.855	20.73	2.21E-03
2	Pila 2	202	GEN SLV Long(min)	886	-2057.95	-46.15	-166.75	173.02	2.855	60.61	1.93E-03
2	Pila 2	202	GEN SLV Trasv(min)	886	-2066.36	-151.82	-62.24	164.08	2.855	57.48	1.98E-03
2	Pila 2	202	GEN SLV Vert(min)	886	-2109.11	-46.16	-62.28	77.52	2.855	27.16	1.97E-03
2	Pila 2	203	GEN SLV Long(max)	28	-1426.45	44.91	142.70	149.60	2.855	52.41	2.34E-03
2	Pila 2	203	GEN SLV Trasv(max)	28	-1329.98	150.41	38.41	155.24	2.855	54.38	2.33E-03
2	Pila 2	203	GEN SLV Vert(max)	28	-1380.80	44.94	38.24	59.01	2.855	20.67	2.30E-03
2	Pila 2	203	GEN SLV Long(min)	28	-1817.27	-48.11	-166.88	173.68	2.855	60.84	1.95E-03
2	Pila 2	203	GEN SLV Trasv(min)	28	-1913.74	-153.61	-62.59	165.87	2.855	58.11	1.99E-03
2	Pila 2	203	GEN SLV Vert(min)	28	-1862.92	-48.14	-62.42	78.83	2.855	27.61	1.99E-03

		Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
<b>SFORZI MASSIMI:</b>	Spalle	-374.92	67.98	78.79	81.48	1.302	62.58	1.26E-02
	Pile	-1329.98	153.61	166.88	173.68	2.855	60.84	2.34E-03

<b>SFORZI MINIMI:</b>	Spalle	-537.23	-67.98	-78.79				
	Pile	-2109.11	-153.61	-166.88				

**STATI LIMITE DI ESERCIZIO - COMBINAZIONI CARATTERISTICHE (RARE)**

Tipo isolatore	Struttura Appoggio	No.	Load	Node	Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
1	Spalla 1	185	GEN RARA Mobili(max)	77	-326.22	21.66	5.47	22.34	1.302	17.16	1.57E-02
1	Spalla 1	185	GEN RARA Vento(max)	77	-325.56	33.22	5.42	33.66	1.302	25.85	1.45E-02
1	Spalla 1	185	GEN RARA Frenam(max)	77	-349.61	1.45	30.23	30.26	1.302	23.24	1.44E-02
1	Spalla 1	185	GEN RARA Termico(max)	77	-308.17	20.77	10.17	23.13	1.302	17.76	1.46E-02
1	Spalla 1	185	GEN RARA Mobili(min)	77	-943.38	-22.12	-16.81	27.78	1.302	21.34	1.14E-02
1	Spalla 1	185	GEN RARA Vento(min)	77	-781.22	-33.73	-14.49	36.71	1.302	28.19	1.13E-02
1	Spalla 1	185	GEN RARA Frenam(min)	77	-757.17	-1.95	-39.30	39.35	1.302	30.22	1.14E-02
1	Spalla 1	185	GEN RARA Termico(min)	77	-798.61	-21.27	-19.24	28.68	1.302	22.03	1.12E-02
1	Spalla 1	186	GEN RARA Mobili(max)	79	-392.40	22.14	4.94	22.68	1.302	17.42	1.56E-02
1	Spalla 1	186	GEN RARA Vento(max)	79	-387.96	33.60	5.19	34.00	1.302	26.11	1.44E-02
1	Spalla 1	186	GEN RARA Frenam(max)	79	-390.45	1.71	30.17	30.22	1.302	23.21	1.44E-02
1	Spalla 1	186	GEN RARA Termico(max)	79	-359.21	20.89	10.11	23.21	1.302	17.82	1.46E-02
1	Spalla 1	186	GEN RARA Mobili(min)	79	-1330.65	-21.79	-16.08	27.08	1.302	20.80	1.19E-02
1	Spalla 1	186	GEN RARA Vento(min)	79	-1052.02	-33.36	-13.99	36.17	1.302	27.78	1.17E-02
1	Spalla 1	186	GEN RARA Frenam(min)	79	-1049.53	-1.48	-38.97	39.00	1.302	29.95	1.17E-02
1	Spalla 1	186	GEN RARA Termico(min)	79	-1080.76	-20.65	-18.91	28.00	1.302	21.50	1.15E-02
1	Spalla 1	187	GEN RARA Mobili(max)	81	-392.97	21.80	4.94	22.35	1.302	17.17	1.56E-02
1	Spalla 1	187	GEN RARA Vento(max)	81	-388.91	33.37	5.19	33.77	1.302	25.94	1.45E-02
1	Spalla 1	187	GEN RARA Frenam(max)	81	-390.45	1.48	30.17	30.21	1.302	23.20	1.44E-02
1	Spalla 1	187	GEN RARA Termico(max)	81	-359.79	20.65	10.11	22.99	1.302	17.66	1.46E-02
1	Spalla 1	187	GEN RARA Mobili(min)	81	-1330.07	-22.14	-16.08	27.36	1.302	21.01	1.18E-02
1	Spalla 1	187	GEN RARA Vento(min)	81	-1051.07	-33.60	-13.99	36.40	1.302	27.95	1.17E-02
1	Spalla 1	187	GEN RARA Frenam(min)	81	-1049.53	-1.71	-38.97	39.01	1.302	29.96	1.17E-02
1	Spalla 1	187	GEN RARA Termico(min)	81	-1080.19	-20.89	-18.91	28.18	1.302	21.64	1.15E-02
1	Spalla 1	188	GEN RARA Mobili(max)	83	-330.13	22.15	5.46	22.81	1.302	17.52	1.57E-02
1	Spalla 1	188	GEN RARA Vento(max)	83	-332.09	33.78	5.40	34.21	1.302	26.27	1.44E-02
1	Spalla 1	188	GEN RARA Frenam(max)	83	-349.61	1.95	30.23	30.29	1.302	23.26	1.44E-02
1	Spalla 1	188	GEN RARA Termico(max)	83	-312.08	21.30	10.16	23.60	1.302	18.12	1.46E-02
1	Spalla 1	188	GEN RARA Mobili(min)	83	-939.47	-21.69	-16.79	27.43	1.302	21.07	1.14E-02
1	Spalla 1	188	GEN RARA Vento(min)	83	-774.69	-33.28	-14.47	36.29	1.302	27.87	1.13E-02
1	Spalla 1	188	GEN RARA Frenam(min)	83	-757.17	-1.45	-39.30	39.33	1.302	30.20	1.14E-02
1	Spalla 1	188	GEN RARA Termico(min)	83	-794.70	-20.80	-19.22	28.32	1.302	21.75	1.12E-02
1	Spalla 2	190	GEN RARA Mobili(max)	78	-326.28	21.66	16.81	27.42	1.302	21.06	1.14E-02
1	Spalla 2	190	GEN RARA Vento(max)	78	-325.59	33.22	14.49	36.24	1.302	27.83	1.13E-02

1	Spalla 2	190	GEN RARA Frenam(max)	78	-349.65	1.45	39.30	39.33	1.302	30.20	1.14E-02
1	Spalla 2	190	GEN RARA Termico(max)	78	-308.20	20.77	19.24	28.31	1.302	21.74	1.12E-02
1	Spalla 2	190	GEN RARA Mobili(min)	78	-943.37	-22.11	-5.47	22.78	1.302	17.49	1.57E-02
1	Spalla 2	190	GEN RARA Vento(min)	78	-781.23	-33.73	-5.42	34.16	1.302	26.24	1.44E-02
1	Spalla 2	190	GEN RARA Frenam(min)	78	-757.17	-1.95	-30.23	30.29	1.302	23.26	1.44E-02
1	Spalla 2	190	GEN RARA Termico(min)	78	-798.62	-21.27	-10.17	23.58	1.302	18.11	1.46E-02
1	Spalla 2	191	GEN RARA Mobili(max)	80	-392.41	22.14	16.08	27.36	1.302	21.01	1.18E-02
1	Spalla 2	191	GEN RARA Vento(max)	80	-388.00	33.59	13.99	36.39	1.302	27.94	1.17E-02
1	Spalla 2	191	GEN RARA Frenam(max)	80	-390.48	1.71	38.97	39.01	1.302	29.96	1.17E-02
1	Spalla 2	191	GEN RARA Termico(max)	80	-359.25	20.88	18.91	28.17	1.302	21.63	1.15E-02
1	Spalla 2	191	GEN RARA Mobili(min)	80	-1330.43	-21.79	-4.94	22.34	1.302	17.16	1.56E-02
1	Spalla 2	191	GEN RARA Vento(min)	80	-1051.89	-33.36	-5.19	33.76	1.302	25.93	1.17E-02
1	Spalla 2	191	GEN RARA Frenam(min)	80	-1049.41	-1.48	-30.17	30.21	1.302	23.20	1.44E-02
1	Spalla 2	191	GEN RARA Termico(min)	80	-1080.63	-20.65	-10.11	22.99	1.302	17.66	1.46E-02
1	Spalla 2	192	GEN RARA Mobili(max)	82	-392.97	21.80	16.08	27.09	1.302	20.80	1.19E-02
1	Spalla 2	192	GEN RARA Vento(max)	82	-388.94	33.37	13.99	36.18	1.302	27.79	1.17E-02
1	Spalla 2	192	GEN RARA Frenam(max)	82	-390.48	1.48	38.97	39.00	1.302	29.95	1.17E-02
1	Spalla 2	192	GEN RARA Termico(max)	82	-359.82	20.65	18.91	28.00	1.302	21.50	1.15E-02
1	Spalla 2	192	GEN RARA Mobili(min)	82	-1329.86	-22.14	-4.94	22.68	1.302	17.42	1.56E-02
1	Spalla 2	192	GEN RARA Vento(min)	82	-1050.95	-33.60	-5.19	34.00	1.302	26.11	1.44E-02
1	Spalla 2	192	GEN RARA Frenam(min)	82	-1049.41	-1.71	-30.17	30.22	1.302	23.21	1.44E-02
1	Spalla 2	192	GEN RARA Termico(min)	82	-1080.07	-20.89	-10.11	23.21	1.302	17.82	1.46E-02
1	Spalla 2	193	GEN RARA Mobili(max)	84	-330.19	22.15	16.79	27.79	1.302	21.35	1.14E-02
1	Spalla 2	193	GEN RARA Vento(max)	84	-332.11	33.78	14.47	36.75	1.302	28.22	1.13E-02
1	Spalla 2	193	GEN RARA Frenam(max)	84	-349.65	1.95	39.30	39.35	1.302	30.22	1.14E-02
1	Spalla 2	193	GEN RARA Termico(max)	84	-312.11	21.30	19.23	28.70	1.302	22.04	1.12E-02
1	Spalla 2	193	GEN RARA Mobili(min)	84	-939.45	-21.69	-5.46	22.37	1.302	17.18	1.57E-02
1	Spalla 2	193	GEN RARA Vento(min)	84	-774.71	-33.28	-5.40	33.72	1.302	25.89	1.44E-02
1	Spalla 2	193	GEN RARA Frenam(min)	84	-757.17	-1.45	-30.23	30.26	1.302	23.24	1.44E-02
1	Spalla 2	193	GEN RARA Termico(min)	84	-794.70	-20.80	-10.16	23.15	1.302	17.78	1.46E-02
2	Pila 1	195	GEN RARA Mobili(max)	21	-1301.84	49.04	23.80	54.51	2.855	19.10	7.04E-04
2	Pila 1	195	GEN RARA Vento(max)	21	-1322.46	76.11	22.77	79.44	2.855	27.83	1.02E-03
2	Pila 1	195	GEN RARA Frenam(max)	21	-1403.75	4.81	76.80	76.95	2.855	26.96	1.00E-03
2	Pila 1	195	GEN RARA Termico(max)	21	-1251.63	48.61	26.83	55.52	2.855	19.45	9.81E-04
2	Pila 1	195	GEN RARA Mobili(min)	21	-2662.85	-44.97	-2.80	45.06	2.855	15.78	3.67E-03
2	Pila 1	195	GEN RARA Vento(min)	21	-2289.61	-72.46	-0.44	72.46	2.855	25.38	3.30E-03
2	Pila 1	195	GEN RARA Frenam(min)	21	-2208.32	-1.15	-54.46	54.47	2.855	19.08	3.29E-03
2	Pila 1	195	GEN RARA Termico(min)	21	-2360.44	-44.96	-4.49	45.18	2.855	15.83	3.39E-03
2	Pila 1	196	GEN RARA Mobili(max)	23	-1734.35	46.33	23.11	51.77	2.855	18.14	8.64E-04
2	Pila 1	196	GEN RARA Vento(max)	23	-1730.16	73.75	22.11	76.99	2.855	26.97	1.11E-03
2	Pila 1	196	GEN RARA Frenam(max)	23	-1739.89	2.32	76.27	76.31	2.855	26.73	1.09E-03
2	Pila 1	196	GEN RARA Termico(max)	23	-1630.69	45.46	26.22	52.48	2.855	18.38	1.05E-03
2	Pila 1	196	GEN RARA Mobili(min)	23	-3287.71	-45.09	-1.66	45.12	2.855	15.81	3.47E-03
2	Pila 1	196	GEN RARA Vento(min)	23	-2754.57	-72.63	0.40	72.63	2.855	25.44	3.13E-03
2	Pila 1	196	GEN RARA Frenam(min)	23	-2744.84	-1.20	-53.77	53.78	2.855	18.84	3.13E-03
2	Pila 1	196	GEN RARA Termico(min)	23	-2854.04	-44.34	-3.71	44.49	2.855	15.59	3.19E-03
2	Pila 1	197	GEN RARA Mobili(max)	885	-1732.28	45.10	23.11	50.68	2.855	17.75	9.70E-04
2	Pila 1	197	GEN RARA Vento(max)	885	-1726.70	72.64	22.11	75.93	2.855	26.60	1.17E-03
2	Pila 1	197	GEN RARA Frenam(max)	885	-1739.89	1.20	76.27	76.28	2.855	26.72	1.14E-03
2	Pila 1	197	GEN RARA Termico(max)	885	-1628.61	44.35	26.22	51.52	2.855	18.05	1.12E-03
2	Pila 1	197	GEN RARA Mobili(min)	885	-3289.78	-46.34	-1.66	46.37	2.855	16.24	3.44E-03
2	Pila 1	197	GEN RARA Vento(min)	885	-2758.03	-73.76	0.40	73.76	2.855	25.84	3.10E-03
2	Pila 1	197	GEN RARA Frenam(min)	885	-2744.84	-2.32	-53.77	53.82	2.855	18.85	3.11E-03
2	Pila 1	197	GEN RARA Termico(min)	885	-2856.11	-45.47	-3.71	45.62	2.855	15.98	3.17E-03
2	Pila 1	198	GEN RARA Mobili(max)	27	-1307.06	44.99	23.80	50.90	2.855	17.83	1.12E-03
2	Pila 1	198	GEN RARA Vento(max)	27	-1331.15	72.49	22.77	75.98	2.855	26.62	1.28E-03
2	Pila 1	198	GEN RARA Frenam(max)	27	-1403.75	1.15	76.80	76.81	2.855	26.91	1.22E-03
2	Pila 1	198	GEN RARA Termico(max)	27	-1256.85	44.98	26.83	52.37	2.855	18.35	1.32E-03
2	Pila 1	198	GEN RARA Mobili(min)	27	-2657.64	-49.06	-2.80	49.14	2.855	17.21	3.56E-03
2	Pila 1	198	GEN RARA Vento(min)	27	-2280.92	-76.15	-0.43	76.15	2.855	26.68	3.21E-03
2	Pila 1	198	GEN RARA Frenam(min)	27	-2208.32	-4.81	-54.46	54.67	2.855	19.15	3.21E-03
2	Pila 1	198	GEN RARA Termico(min)	27	-2355.22	-48.63	-4.49	48.84	2.855	17.11	3.28E-03
2	Pila 2	200	GEN RARA Mobili(max)	22	-1301.83	49.04	2.80	49.12	2.855	17.21	3.56E-03
2	Pila 2	200	GEN RARA Vento(max)	22	-1322.44	76.11	0.43	76.11	2.855	26.66	3.21E-03
2	Pila 2	200	GEN RARA Frenam(max)	22	-1403.79	4.81	54.46	54.67	2.855	19.15	3.21E-03
2	Pila 2	200	GEN RARA Termico(max)	22	-1251.62	48.61	4.49	48.82	2.855	17.10	3.28E-03
2	Pila 2	200	GEN RARA Mobili(min)	22	-2662.88	-44.97	-23.80	50.88	2.855	17.82	1.12E-03
2	Pila 2	200	GEN RARA Vento(min)	22	-2289.64	-72.45	-22.77	75.94	2.855	26.60	1.29E-03
2	Pila 2	200	GEN RARA Frenam(min)	22	-2208.29	-1.15	-76.80	76.81	2.855	26.91	1.22E-03
2	Pila 2	200	GEN RARA Termico(min)	22	-2360.46	-44.96	-26.83	52.36	2.855	18.34	1.32E-03
2	Pila 2	201	GEN RARA Mobili(max)	24	-1734.36	46.33	1.66	46.36	2.855	16.24	3.44E-03
2	Pila 2	201	GEN RARA Vento(max)	24	-1730.17	73.75	-0.40	73.75	2.855	25.84	3.10E-03
2	Pila 2	201	GEN RARA Frenam(max)	24	-1739.88	2.32	53.77	53.82	2.855	18.85	3.11E-03
2	Pila 2	201	GEN RARA Termico(max)	24	-1630.69	45.46	3.71	45.61	2.855	15.98	3.17E-03
2	Pila 2	201	GEN RARA Mobili(min)	24	-3287.69	-45.09	-23.11	50.67	2.855	17.75	9.71E-04
2	Pila 2	201	GEN RARA Vento(min)	24	-2754.55	-72.63	-22.11	75.92	2.855	26.60	1.17E-03
2	Pila 2	201	GEN RARA Frenam(min)	24	-2744.84	-1.20	-76.27	76.28	2.855	26.72	1.14E-03

2	Pila 2	201	GEN RARA Termico(min)	24	-2854.02	-44.34	-26.22	51.51	2.855	18.05	1.12E-03
2	Pila 2	202	GEN RARA Mobili(max)	886	-1732.28	45.10	1.66	45.13	2.855	15.81	3.47E-03
2	Pila 2	202	GEN RARA Vento(max)	886	-1726.69	72.64	-0.40	72.64	2.855	25.45	3.13E-03
2	Pila 2	202	GEN RARA Frenam(max)	886	-1739.88	1.20	53.77	53.78	2.855	18.84	3.13E-03
2	Pila 2	202	GEN RARA Termico(max)	886	-1628.61	44.35	3.71	44.50	2.855	15.59	3.19E-03
2	Pila 2	202	GEN RARA Mobili(min)	886	-3289.78	-46.34	-23.11	51.78	2.855	18.14	8.63E-04
2	Pila 2	202	GEN RARA Vento(min)	886	-2758.02	-73.76	-22.11	77.00	2.855	26.97	1.11E-03
2	Pila 2	202	GEN RARA Frenam(min)	886	-2744.84	-2.32	-76.27	76.31	2.855	26.73	1.08E-03
2	Pila 2	202	GEN RARA Termico(min)	886	-2856.11	-45.47	-26.22	52.49	2.855	18.39	1.05E-03
2	Pila 2	203	GEN RARA Mobili(max)	28	-1307.05	44.99	2.80	45.08	2.855	15.79	3.67E-03
2	Pila 2	203	GEN RARA Vento(max)	28	-1331.13	72.49	0.43	72.49	2.855	25.39	3.30E-03
2	Pila 2	203	GEN RARA Frenam(max)	28	-1403.79	1.15	54.46	54.47	2.855	19.08	3.29E-03
2	Pila 2	203	GEN RARA Termico(max)	28	-1256.84	44.98	4.49	45.20	2.855	15.84	3.39E-03
2	Pila 2	203	GEN RARA Mobili(min)	28	-2657.66	-49.06	-23.80	54.53	2.855	19.10	7.03E-04
2	Pila 2	203	GEN RARA Vento(min)	28	-2280.95	-76.15	-22.77	79.48	2.855	27.84	1.02E-03
2	Pila 2	203	GEN RARA Frenam(min)	28	-2208.29	-4.81	-76.80	76.95	2.855	26.96	1.00E-03
2	Pila 2	203	GEN RARA Termico(min)	28	-2355.24	-48.63	-26.83	55.54	2.855	19.46	9.80E-04

				Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
<b>SFORZI MASSIMI:</b>				<b>Spalle -308.17</b>	<b>33.78</b>	<b>39.30</b>	<b>39.35</b>	<b>1.302</b>	<b>30.22</b>	<b>1.32E-02</b>
				<b>Pile -1251.62</b>	<b>76.11</b>	<b>76.80</b>	<b>79.48</b>	<b>2.855</b>	<b>27.84</b>	<b>3.67E-03</b>
<b>SFORZI MINIMI:</b>				<b>Spalle -1330.65</b>	<b>-33.73</b>	<b>-39.30</b>				
				<b>Pile -3289.78</b>	<b>-76.15</b>	<b>-76.80</b>				

**STATI LIMITE DI ESERCIZIO - COMBINAZIONI FREQUENTI**

Tipo isolatore	Struttura Appoggio	No.	Load	Node	Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
1	Spalla 1	185	GEN FREQ Mobili(max)	77	-359.09	1.38	3.97	4.20	1.302	3.23	1.43E-02
1	Spalla 1	185	GEN FREQ Vento(max)	77	-415.76	6.37	2.72	6.93	1.302	5.32	1.22E-02
1	Spalla 1	185	GEN FREQ Termico(max)	77	-414.07	0.08	3.88	3.88	1.302	2.98	1.23E-02
1	Spalla 1	185	GEN FREQ Mobili(min)	77	-747.69	-1.88	-13.03	13.16	1.302	10.11	1.14E-02
1	Spalla 1	185	GEN FREQ Vento(min)	77	-496.39	-6.98	-9.50	11.79	1.302	9.05	1.17E-02
1	Spalla 1	185	GEN FREQ Termico(min)	77	-498.09	-0.69	-10.66	10.68	1.302	8.20	1.17E-02
1	Spalla 1	186	GEN FREQ Mobili(max)	79	-401.22	1.70	3.88	4.24	1.302	3.25	1.44E-02
1	Spalla 1	186	GEN FREQ Vento(max)	79	-402.35	6.34	2.83	6.94	1.302	5.33	1.24E-02
1	Spalla 1	186	GEN FREQ Termico(max)	79	-395.72	-0.02	4.05	4.05	1.302	3.11	1.25E-02
1	Spalla 1	186	GEN FREQ Mobili(min)	79	-1038.76	-1.47	-12.68	12.76	1.302	9.80	1.18E-02
1	Spalla 1	186	GEN FREQ Vento(min)	79	-481.93	-6.51	-9.56	11.57	1.302	8.88	1.19E-02
1	Spalla 1	186	GEN FREQ Termico(min)	79	-488.57	-0.15	-10.78	10.78	1.302	8.28	1.19E-02
1	Spalla 1	187	GEN FREQ Mobili(max)	81	-401.22	1.47	3.88	4.15	1.302	3.19	1.44E-02
1	Spalla 1	187	GEN FREQ Vento(max)	81	-402.54	6.52	2.83	7.11	1.302	5.46	1.24E-02
1	Spalla 1	187	GEN FREQ Termico(max)	81	-395.72	0.15	4.05	4.05	1.302	3.11	1.25E-02
1	Spalla 1	187	GEN FREQ Mobili(min)	81	-1038.76	-1.70	-12.68	12.79	1.302	9.83	1.17E-02
1	Spalla 1	187	GEN FREQ Vento(min)	81	-481.74	-6.35	-9.56	11.48	1.302	8.81	1.19E-02
1	Spalla 1	187	GEN FREQ Termico(min)	81	-488.57	0.02	-10.78	10.78	1.302	8.28	1.19E-02
1	Spalla 1	188	GEN FREQ Mobili(max)	83	-359.09	1.88	3.97	4.39	1.302	3.37	1.43E-02
1	Spalla 1	188	GEN FREQ Vento(max)	83	-417.07	6.99	2.71	7.50	1.302	5.76	1.22E-02
1	Spalla 1	188	GEN FREQ Termico(max)	83	-414.07	0.69	3.88	3.94	1.302	3.03	1.23E-02
1	Spalla 1	188	GEN FREQ Mobili(min)	83	-747.69	-1.38	-13.03	13.10	1.302	10.06	1.14E-02
1	Spalla 1	188	GEN FREQ Vento(min)	83	-495.09	-6.38	-9.49	11.44	1.302	8.78	1.17E-02
1	Spalla 1	188	GEN FREQ Termico(min)	83	-498.09	-0.08	-10.66	10.66	1.302	8.19	1.17E-02
1	Spalla 2	190	GEN FREQ Mobili(max)	78	-359.14	1.38	13.03	13.10	1.302	10.06	1.14E-02
1	Spalla 2	190	GEN FREQ Vento(max)	78	-415.76	6.37	9.50	11.44	1.302	8.78	1.17E-02
1	Spalla 2	190	GEN FREQ Termico(max)	78	-414.07	0.08	10.66	10.66	1.302	8.19	1.17E-02
1	Spalla 2	190	GEN FREQ Mobili(min)	78	-747.68	-1.88	-3.97	4.39	1.302	3.37	1.43E-02
1	Spalla 2	190	GEN FREQ Vento(min)	78	-496.40	-6.98	-2.72	7.49	1.302	5.75	1.22E-02
1	Spalla 2	190	GEN FREQ Termico(min)	78	-498.09	-0.69	-3.88	3.94	1.302	3.03	1.23E-02
1	Spalla 2	191	GEN FREQ Mobili(max)	80	-401.25	1.70	12.68	12.79	1.302	9.83	1.17E-02
1	Spalla 2	191	GEN FREQ Vento(max)	80	-402.35	6.34	9.56	11.47	1.302	8.81	1.19E-02
1	Spalla 2	191	GEN FREQ Termico(max)	80	-395.72	-0.02	10.78	10.78	1.302	8.28	1.19E-02
1	Spalla 2	191	GEN FREQ Mobili(min)	80	-1038.64	-1.47	-3.88	4.15	1.302	3.19	1.44E-02
1	Spalla 2	191	GEN FREQ Vento(min)	80	-481.93	-6.51	-2.83	7.10	1.302	5.45	1.24E-02
1	Spalla 2	191	GEN FREQ Termico(min)	80	-488.57	-0.15	-4.05	4.05	1.302	3.11	1.25E-02
1	Spalla 2	192	GEN FREQ Mobili(max)	82	-401.25	1.47	12.68	12.76	1.302	9.80	1.18E-02
1	Spalla 2	192	GEN FREQ Vento(max)	82	-402.54	6.52	9.56	11.57	1.302	8.89	1.19E-02
1	Spalla 2	192	GEN FREQ Termico(max)	82	-395.72	0.15	10.78	10.78	1.302	8.28	1.19E-02
1	Spalla 2	192	GEN FREQ Mobili(min)	82	-1038.64	-1.70	-3.88	4.24	1.302	3.25	1.44E-02
1	Spalla 2	192	GEN FREQ Vento(min)	82	-481.74	-6.35	-2.83	6.95	1.302	5.34	1.24E-02
1	Spalla 2	192	GEN FREQ Termico(min)	82	-488.57	0.02	-4.05	4.05	1.302	3.11	1.25E-02
1	Spalla 2	193	GEN FREQ Mobili(max)	84	-359.14	1.88	13.03	13.16	1.302	10.11	1.14E-02
1	Spalla 2	193	GEN FREQ Vento(max)	84	-417.06	6.99	9.49	11.79	1.302	9.05	1.17E-02



1	Spalla 2	193	GEN FREQ Termico(max)	84	-414.07	0.69	10.66	10.68	1.302	8.20	1.17E-02
1	Spalla 2	193	GEN FREQ Mobili(min)	84	-747.68	-1.38	-3.97	4.20	1.302	3.23	1.43E-02
1	Spalla 2	193	GEN FREQ Vento(min)	84	-495.09	-6.38	-2.71	6.93	1.302	5.32	1.22E-02
1	Spalla 2	193	GEN FREQ Termico(min)	84	-498.09	-0.08	-3.88	3.88	1.302	2.98	1.23E-02
2	Pila 1	195	GEN FREQ Mobili(max)	21	-1430.15	4.55	21.54	22.02	2.855	7.71	1.03E-03
2	Pila 1	195	GEN FREQ Vento(max)	21	-1476.07	17.14	17.30	24.35	2.855	8.53	2.03E-03
2	Pila 1	195	GEN FREQ Termico(max)	21	-1466.54	3.13	18.30	18.57	2.855	6.50	2.02E-03
2	Pila 1	195	GEN FREQ Mobili(min)	21	-2181.92	-0.89	0.80	1.20	2.855	0.42	3.25E-03
2	Pila 1	195	GEN FREQ Vento(min)	21	-1767.65	-13.94	6.88	15.55	2.855	5.45	2.25E-03
2	Pila 1	195	GEN FREQ Termico(min)	21	-1777.18	0.07	5.89	5.89	2.855	2.06	2.27E-03
2	Pila 1	196	GEN FREQ Mobili(max)	23	-1766.57	2.24	21.01	21.13	2.855	7.40	1.11E-03
2	Pila 1	196	GEN FREQ Vento(max)	23	-1764.47	15.14	17.22	22.93	2.855	8.03	2.01E-03
2	Pila 1	196	GEN FREQ Termico(max)	23	-1740.65	0.92	18.24	18.26	2.855	6.40	1.99E-03
2	Pila 1	196	GEN FREQ Mobili(min)	23	-2718.16	-1.12	1.49	1.86	2.855	0.65	3.10E-03
2	Pila 1	196	GEN FREQ Vento(min)	23	-2027.83	-14.15	6.86	15.73	2.855	5.51	2.17E-03
2	Pila 1	196	GEN FREQ Termico(min)	23	-2051.65	0.07	5.84	5.84	2.855	2.05	2.19E-03
2	Pila 1	197	GEN FREQ Mobili(max)	885	-1766.57	1.12	21.01	21.04	2.855	7.37	1.16E-03
2	Pila 1	197	GEN FREQ Vento(max)	885	-1763.78	14.15	17.22	22.29	2.855	7.81	2.01E-03
2	Pila 1	197	GEN FREQ Termico(max)	885	-1740.65	-0.07	18.24	18.24	2.855	6.39	2.00E-03
2	Pila 1	197	GEN FREQ Mobili(min)	885	-2718.16	-2.24	1.49	2.69	2.855	0.94	3.08E-03
2	Pila 1	197	GEN FREQ Vento(min)	885	-2028.52	-15.14	6.86	16.62	2.855	5.82	2.17E-03
2	Pila 1	197	GEN FREQ Termico(min)	885	-2051.65	-0.92	5.84	5.91	2.855	2.07	2.18E-03
2	Pila 1	198	GEN FREQ Mobili(max)	27	-1430.15	0.89	21.54	21.56	2.855	7.55	1.22E-03
2	Pila 1	198	GEN FREQ Vento(max)	27	-1477.81	13.95	17.30	22.22	2.855	7.79	2.09E-03
2	Pila 1	198	GEN FREQ Termico(max)	27	-1466.54	-0.07	18.30	18.30	2.855	6.41	2.08E-03
2	Pila 1	198	GEN FREQ Mobili(min)	27	-2181.92	-4.55	0.80	4.62	2.855	1.62	3.18E-03
2	Pila 1	198	GEN FREQ Vento(min)	27	-1765.91	-17.15	6.88	18.48	2.855	6.47	2.19E-03
2	Pila 1	198	GEN FREQ Termico(min)	27	-1777.18	-3.13	5.89	6.67	2.855	2.34	2.21E-03
2	Pila 2	200	GEN FREQ Mobili(max)	22	-1430.16	4.55	-0.80	4.62	2.855	1.62	3.18E-03
2	Pila 2	200	GEN FREQ Vento(max)	22	-1476.06	17.14	-6.88	18.47	2.855	6.47	2.19E-03
2	Pila 2	200	GEN FREQ Termico(max)	22	-1466.54	3.13	-5.89	6.67	2.855	2.34	2.21E-03
2	Pila 2	200	GEN FREQ Mobili(min)	22	-2181.92	-0.89	-21.54	21.56	2.855	7.55	1.22E-03
2	Pila 2	200	GEN FREQ Vento(min)	22	-1767.66	-13.94	-17.30	22.22	2.855	7.78	2.09E-03
2	Pila 2	200	GEN FREQ Termico(min)	22	-1777.18	0.07	-18.30	18.30	2.855	6.41	2.08E-03
2	Pila 2	201	GEN FREQ Mobili(max)	24	-1766.57	2.24	-1.49	2.69	2.855	0.94	3.08E-03
2	Pila 2	201	GEN FREQ Vento(max)	24	-1764.47	15.14	-6.86	16.62	2.855	5.82	2.17E-03
2	Pila 2	201	GEN FREQ Termico(max)	24	-1740.65	0.92	-5.84	5.91	2.855	2.07	2.18E-03
2	Pila 2	201	GEN FREQ Mobili(min)	24	-2718.15	-1.12	-21.01	21.04	2.855	7.37	1.16E-03
2	Pila 2	201	GEN FREQ Vento(min)	24	-2027.83	-14.15	-17.22	22.29	2.855	7.81	2.01E-03
2	Pila 2	201	GEN FREQ Termico(min)	24	-2051.65	0.07	-18.24	18.24	2.855	6.39	2.00E-03
2	Pila 2	202	GEN FREQ Mobili(max)	886	-1766.57	1.12	-1.49	1.86	2.855	0.65	3.10E-03
2	Pila 2	202	GEN FREQ Vento(max)	886	-1763.77	14.15	-6.86	15.73	2.855	5.51	2.17E-03
2	Pila 2	202	GEN FREQ Termico(max)	886	-1740.65	-0.07	-5.84	5.84	2.855	2.05	2.19E-03
2	Pila 2	202	GEN FREQ Mobili(min)	886	-2718.15	-2.24	-21.01	21.13	2.855	7.40	1.11E-03
2	Pila 2	202	GEN FREQ Vento(min)	886	-2028.52	-15.14	-17.22	22.93	2.855	8.03	2.01E-03
2	Pila 2	202	GEN FREQ Termico(min)	886	-2051.65	-0.92	-18.24	18.26	2.855	6.40	1.99E-03
2	Pila 2	203	GEN FREQ Mobili(max)	28	-1430.16	0.89	-0.80	1.20	2.855	0.42	3.25E-03
2	Pila 2	203	GEN FREQ Vento(max)	28	-1477.80	13.95	-6.88	15.55	2.855	5.45	2.25E-03
2	Pila 2	203	GEN FREQ Termico(max)	28	-1466.54	-0.07	-5.89	5.89	2.855	2.06	2.27E-03
2	Pila 2	203	GEN FREQ Mobili(min)	28	-2181.92	-4.55	-21.54	22.02	2.855	7.71	1.03E-03
2	Pila 2	203	GEN FREQ Vento(min)	28	-1765.92	-17.15	-17.30	24.36	2.855	8.53	2.03E-03
2	Pila 2	203	GEN FREQ Termico(min)	28	-1777.18	-3.13	-18.30	18.57	2.855	6.50	2.02E-03

		Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
<b>SFORZI MASSIMI:</b>	Spalle	-359.09	6.99	13.03	13.16	1.302	10.11	1.44E-02
	Pila	-1430.15	17.14	21.54	24.36	2.855	8.53	3.25E-03
<b>SFORZI MINIMI:</b>	Spalle	-1038.76	-6.98	-13.03				
	Pila	-2718.16	-17.15	-21.54				

**STATI LIMITE DI ESERCIZIO - COMBINAZIONI QUASI PERMANENTI**

Tipo isolatore	Struttura Appoggio	No.	Load	Node	Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
1	Spalla 1	185	GEN Q.P.(max)	77	-421.07	0.02	2.67	2.67	1.302	2.05	1.22E-02
1	Spalla 1	185	GEN Q.P.(min)	77	-491.09	-0.62	-9.45	9.47	1.302	7.27	1.17E-02
1	Spalla 1	186	GEN Q.P.(max)	79	-403.45	-0.03	2.81	2.81	1.302	2.16	1.24E-02
1	Spalla 1	186	GEN Q.P.(min)	79	-480.83	-0.14	-9.54	9.54	1.302	7.33	1.19E-02
1	Spalla 1	187	GEN Q.P.(max)	81	-403.45	0.14	2.81	2.81	1.302	2.16	1.24E-02
1	Spalla 1	187	GEN Q.P.(min)	81	-480.83	0.03	-9.54	9.54	1.302	7.33	1.19E-02
1	Spalla 1	188	GEN Q.P.(max)	83	-421.07	0.62	2.67	2.74	1.302	2.11	1.22E-02
1	Spalla 1	188	GEN Q.P.(min)	83	-491.09	-0.02	-9.45	9.45	1.302	7.26	1.17E-02

1	Spalla 2	190	GEN Q.P.(max)	78	-421.07	0.02	9.45	9.45	1.302	7.26	1.17E-02
1	Spalla 2	190	GEN Q.P.(min)	78	-491.09	-0.62	-2.67	2.74	1.302	2.11	1.22E-02
1	Spalla 2	191	GEN Q.P.(max)	80	-403.45	-0.03	9.54	9.54	1.302	7.33	1.19E-02
1	Spalla 2	191	GEN Q.P.(min)	80	-480.83	-0.14	-2.81	2.81	1.302	2.16	1.24E-02
1	Spalla 2	192	GEN Q.P.(max)	82	-403.45	0.14	9.54	9.54	1.302	7.33	1.19E-02
1	Spalla 2	192	GEN Q.P.(min)	82	-480.83	0.03	-2.81	2.81	1.302	2.16	1.24E-02
1	Spalla 2	193	GEN Q.P.(max)	84	-421.07	0.62	9.45	9.47	1.302	7.27	1.17E-02
1	Spalla 2	193	GEN Q.P.(min)	84	-491.09	-0.02	-2.67	2.67	1.302	2.05	1.22E-02
2	Pila 1	195	GEN Q.P.(max)	21	-1492.43	2.88	17.26	17.50	2.855	6.13	2.04E-03
2	Pila 1	195	GEN Q.P.(min)	21	-1751.29	0.32	6.92	6.93	2.855	2.43	2.24E-03
2	Pila 1	196	GEN Q.P.(max)	23	-1766.57	0.85	17.20	17.22	2.855	6.03	2.01E-03
2	Pila 1	196	GEN Q.P.(min)	23	-2025.73	0.14	6.87	6.87	2.855	2.41	2.17E-03
2	Pila 1	197	GEN Q.P.(max)	885	-1766.57	-0.14	17.20	17.20	2.855	6.03	2.01E-03
2	Pila 1	197	GEN Q.P.(min)	885	-2025.73	-0.85	6.87	6.92	2.855	2.42	2.17E-03
2	Pila 1	198	GEN Q.P.(max)	27	-1492.43	-0.32	17.26	17.26	2.855	6.05	2.09E-03
2	Pila 1	198	GEN Q.P.(min)	27	-1751.29	-2.88	6.92	7.50	2.855	2.63	2.19E-03
2	Pila 2	200	GEN Q.P.(max)	22	-1492.43	2.88	-6.92	7.50	2.855	2.63	2.19E-03
2	Pila 2	200	GEN Q.P.(min)	22	-1751.29	0.32	-17.26	17.26	2.855	6.05	2.09E-03
2	Pila 2	201	GEN Q.P.(max)	24	-1766.57	0.85	-6.87	6.92	2.855	2.42	2.17E-03
2	Pila 2	201	GEN Q.P.(min)	24	-2025.73	0.14	-17.20	17.20	2.855	6.03	2.01E-03
2	Pila 2	202	GEN Q.P.(max)	886	-1766.57	-0.14	-6.87	6.87	2.855	2.41	2.17E-03
2	Pila 2	202	GEN Q.P.(min)	886	-2025.73	-0.85	-17.20	17.22	2.855	6.03	2.01E-03
2	Pila 2	203	GEN Q.P.(max)	28	-1492.43	-0.32	-6.92	6.93	2.855	2.43	2.24E-03
2	Pila 2	203	GEN Q.P.(min)	28	-1751.29	-2.88	-17.26	17.50	2.855	6.13	2.04E-03

		Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
<b>SFORZI MASSIMI:</b>	<b>Spalle</b>	<b>-403.45</b>	<b>0.62</b>	<b>9.54</b>	<b>9.54</b>	<b>1.302</b>	<b>7.33</b>	<b>1.24E-02</b>
	<b>Pile</b>	<b>-1492.43</b>	<b>2.88</b>	<b>17.26</b>	<b>17.50</b>	<b>2.855</b>	<b>6.13</b>	<b>2.24E-03</b>
<b>SFORZI MINIMI:</b>	<b>Spalle</b>	<b>-491.09</b>	<b>-0.62</b>	<b>-9.54</b>				
	<b>Pile</b>	<b>-2025.73</b>	<b>-2.88</b>	<b>-17.26</b>				

**STATI LIMITE DI ESERCIZIO - SISMICI (SLD)**

Tipo isolatore	Struttura Appoggio	No.	Load	Node	Axial (kN)	Shear-y (kN)	Shear-z (kN)	Shear-tot (kN)	Kh eq. (kN/mm)	Deform. (mm)	Rotazione (rad)
1	Spalla 1	185	GEN SLD Long(max)	77	-411.14	8.37	31.31	32.41	1.302	24.89	1.23E-02
1	Spalla 1	185	GEN SLD Trasv(max)	77	-402.79	27.86	11.44	30.12	1.302	23.13	1.23E-02
1	Spalla 1	185	GEN SLD Vert(max)	77	-407.82	8.37	11.41	14.15	1.302	10.87	1.23E-02
1	Spalla 1	185	GEN SLD Long(min)	77	-501.01	-8.98	-38.09	39.13	1.302	30.05	1.17E-02
1	Spalla 1	185	GEN SLD Trasv(min)	77	-509.37	-28.47	-18.22	33.80	1.302	25.96	1.17E-02
1	Spalla 1	185	GEN SLD Vert(min)	77	-504.33	-8.98	-18.19	20.29	1.302	15.58	1.17E-02
1	Spalla 1	186	GEN SLD Long(max)	79	-397.32	8.34	31.43	32.52	1.302	24.97	1.25E-02
1	Spalla 1	186	GEN SLD Trasv(max)	79	-399.31	27.86	11.47	30.13	1.302	23.14	1.25E-02
1	Spalla 1	186	GEN SLD Vert(max)	79	-394.68	8.34	11.52	14.22	1.302	10.92	1.25E-02
1	Spalla 1	186	GEN SLD Long(min)	79	-486.97	-8.51	-38.16	39.10	1.302	30.03	1.19E-02
1	Spalla 1	186	GEN SLD Trasv(min)	79	-484.97	-28.03	-18.20	33.42	1.302	25.67	1.19E-02
1	Spalla 1	186	GEN SLD Vert(min)	79	-489.60	-8.51	-18.25	20.14	1.302	15.46	1.19E-02
1	Spalla 1	187	GEN SLD Long(max)	81	-397.32	8.51	31.43	32.56	1.302	25.01	1.25E-02
1	Spalla 1	187	GEN SLD Trasv(max)	81	-399.31	28.03	11.47	30.29	1.302	23.26	1.25E-02
1	Spalla 1	187	GEN SLD Vert(max)	81	-394.68	8.51	11.52	14.32	1.302	11.00	1.25E-02
1	Spalla 1	187	GEN SLD Long(min)	81	-486.97	-8.34	-38.16	39.06	1.302	30.00	1.19E-02
1	Spalla 1	187	GEN SLD Trasv(min)	81	-484.97	-27.86	-18.20	33.28	1.302	25.56	1.19E-02
1	Spalla 1	187	GEN SLD Vert(min)	81	-489.60	-8.34	-18.25	20.07	1.302	15.41	1.19E-02
1	Spalla 1	188	GEN SLD Long(max)	83	-411.14	8.98	31.31	32.57	1.302	25.02	1.23E-02
1	Spalla 1	188	GEN SLD Trasv(max)	83	-402.79	28.47	11.44	30.68	1.302	23.56	1.23E-02
1	Spalla 1	188	GEN SLD Vert(max)	83	-407.83	8.98	11.41	14.52	1.302	11.15	1.23E-02
1	Spalla 1	188	GEN SLD Long(min)	83	-501.01	-8.37	-38.09	39.00	1.302	29.95	1.17E-02
1	Spalla 1	188	GEN SLD Trasv(min)	83	-509.37	-27.86	-18.22	33.29	1.302	25.57	1.17E-02
1	Spalla 1	188	GEN SLD Vert(min)	83	-504.33	-8.37	-18.19	20.02	1.302	15.38	1.17E-02
1	Spalla 2	190	GEN SLD Long(max)	78	-411.14	8.37	38.09	39.00	1.302	29.95	1.17E-02
1	Spalla 2	190	GEN SLD Trasv(max)	78	-402.79	27.86	18.22	33.29	1.302	25.57	1.17E-02
1	Spalla 2	190	GEN SLD Vert(max)	78	-407.83	8.37	18.19	20.02	1.302	15.38	1.17E-02
1	Spalla 2	190	GEN SLD Long(min)	78	-501.01	-8.98	-31.31	32.57	1.302	25.02	1.23E-02
1	Spalla 2	190	GEN SLD Trasv(min)	78	-509.37	-28.47	-11.44	30.68	1.302	23.56	1.23E-02
1	Spalla 2	190	GEN SLD Vert(min)	78	-504.33	-8.98	-11.41	14.52	1.302	11.15	1.23E-02
1	Spalla 2	191	GEN SLD Long(max)	80	-397.32	8.34	38.16	39.06	1.302	30.00	1.19E-02
1	Spalla 2	191	GEN SLD Trasv(max)	80	-399.31	27.86	18.20	33.28	1.302	25.56	1.19E-02
1	Spalla 2	191	GEN SLD Vert(max)	80	-394.68	8.34	18.25	20.07	1.302	15.41	1.19E-02
1	Spalla 2	191	GEN SLD Long(min)	80	-486.97	-8.51	-31.43	32.56	1.302	25.01	1.25E-02
1	Spalla 2	191	GEN SLD Trasv(min)	80	-484.97	-28.03	-11.47	30.29	1.302	23.26	1.25E-02





## 14 GIUNTI DI DILATAZIONE

I giunti di dilatazione si assumono soggetti alle condizioni di carico di progetto con l'eccezione delle dilatazioni termiche che si considerano raddoppiate ( $\pm 30^\circ$ ) per tener conto dell'incertezza sulla posizione del punto fisso.

Segue tabulato degli spostamenti dei nodi di estremità dell'impalcato per le condizioni di carico pertinenti.

Node	Load	DX (mm)	DY (mm)	DZ (mm)	RX ([rad])	RY ([rad])	RZ ([rad])
719	Vento strutture (Y+)	0.068389	17.576833	0.002204	-0.000048	-0.000015	0.000016
760	Vento strutture (Y+)	0.021520	17.573931	-0.004683	-0.000026	-0.000006	0.000015
801	Vento strutture (Y+)	-0.023173	17.574243	0.005461	-0.000027	0.000004	0.000015
842	Vento strutture (Y+)	-0.068734	17.579361	-0.001587	-0.000060	0.000018	0.000016
719	Vento mobili (Y+)	0.036570	6.996293	-0.013120	-0.000084	-0.000042	0.000008
760	Vento mobili (Y+)	0.015985	6.982558	-0.010969	-0.000022	-0.000005	0.000009
801	Vento mobili (Y+)	-0.008582	6.980313	0.006823	-0.000014	0.000013	0.000009
842	Vento mobili (Y+)	-0.036025	6.980703	0.009808	-0.000022	0.000024	0.000010
719	Frenamento (X+)	19.251852	-0.000177	0.010642	0.000002	0.000009	0.000000
760	Frenamento (X+)	19.251739	0.000012	0.010578	0.000000	0.000008	0.000000
801	Frenamento (X+)	19.251739	-0.000012	0.010578	0.000000	0.000008	0.000000
842	Frenamento (X+)	19.251852	0.000177	0.010642	-0.000002	0.000009	0.000000
719	Termico Unif. +15°	-9.150895	0.028749	0.011428	-0.000384	-0.000086	0.000000
760	Termico Unif. +15°	-9.118708	-0.000741	-0.005659	-0.000058	0.000094	-0.000001
801	Termico Unif. +15°	-9.118708	0.000741	-0.005659	0.000058	0.000094	0.000001
842	Termico Unif. +15°	-9.150895	-0.028749	0.011428	0.000384	-0.000086	0.000000
719	Termico diff. +5°/H	-0.123520	0.002038	-0.351782	-0.000021	-0.000443	0.000000
760	Termico diff. +5°/H	-0.121938	-0.000072	-0.347577	-0.000004	-0.000428	0.000000
801	Termico diff. +5°/H	-0.121938	0.000072	-0.347577	0.000004	-0.000428	0.000000
842	Termico diff. +5°/H	-0.123520	-0.002038	-0.351782	0.000021	-0.000443	0.000000
719	SLC Long (X+)(RS)	70.659806	-0.000727	0.032927	0.000007	0.000031	0.000000
760	SLC Long (X+)(RS)	70.659263	-0.000032	0.032527	0.000000	0.000028	0.000000
801	SLC Long (X+)(RS)	70.659263	0.000032	0.032527	0.000000	0.000028	0.000000
842	SLC Long (X+)(RS)	70.659806	0.000727	0.032927	-0.000007	0.000031	0.000000
719	SLC Trasv (Y+)(RS)	0.294864	69.192101	0.011048	-0.000211	-0.000078	0.000068
760	SLC Trasv (Y+)(RS)	0.096180	69.181443	-0.022178	-0.000114	-0.000024	0.000064
801	SLC Trasv (Y+)(RS)	-0.096180	69.181443	0.022178	-0.000114	0.000024	0.000064
842	SLC Trasv (Y+)(RS)	-0.294864	69.192101	-0.011048	-0.000211	0.000078	0.000068
719	SLC Vert (Z+)(RS)	0.098816	0.001751	-0.191825	-0.000012	-0.000289	0.000000
760	SLC Vert (Z+)(RS)	0.101032	0.000471	-0.186926	-0.000003	-0.000277	0.000000
801	SLC Vert (Z+)(RS)	0.101032	-0.000473	-0.186926	0.000003	-0.000277	0.000000
842	SLC Vert (Z+)(RS)	0.098816	-0.001751	-0.191825	0.000012	-0.000289	0.000000

Ai valori determinati in direzione longitudinale si sommano 50 mm assunti come massima deformazione ammissibile delle sottostrutture, già calcolata allo SLU.

Si prevedono giunti con le seguenti caratteristiche di deformazione massime:

- escursione longitudinale:  $\Delta X = \pm 165$  mm
- escursione trasversale:  $\Delta Y = \pm 165$  mm

1) Verifica deformazioni longitudinali statiche: considerando il frenamento come carico variabile dominante si ottiene la deformazione massima allo SLU dalla seguente somma:

	$\Delta X$	$\gamma_q$	$\psi_0$	
- Frenamento:	19.3	x 1.35	x 1	= 26.1 mm
- Termico unif. $\pm 30^\circ$ :	2 x 9.2	x 1.2	x 0.6	= 13.2 mm
- Deform. max sottostrutture:				= 50 mm
- Deformazione totale:				= 89.3 mm (< 165 mm: verificato)

2) Verifica deformazioni trasversali statiche: considerando il vento come carico variabile dominante si ottiene la deformazione massima allo SLU dalla seguente somma:

	$\Delta Y$	$\gamma_q$	$\psi_0$	
- Vento strutture:	17.6	x 1.5	x 1	= 26.4 mm
- Vento carichi mobili:	7.00	x 1.5	x 1	= 10.5 mm
- Deformazione totale:				= 36.9 mm (< 165 mm: verificato)

3) Verifica deformazioni longitudinali sismiche: si ottiene la deformazione massima allo SLC dalla seguente somma:

	$\Delta X$	$\gamma_q$	$\psi_0$	
- SLC longitudinale:	70.7	x 1	x 1	= 70.7 mm
- Termico unif. $\pm 30^\circ$ :	2 x 9.2	x 1	x 0.5	= 9.2 mm
- Deform. max sottostrutture:				= 50 mm
- Deformazione totale:				= 129.9 mm (< 165 mm: verificato)

3) Verifica deformazioni trasversali sismiche: si ottiene la deformazione massima allo SLC dalla seguente somma:

	$\Delta Y$	$\gamma_q$	$\psi_0$	
- SLC trasversale:	69.2	x 1	x 1	= 69.2 mm
- Deformazione totale:				= 69.2 mm (< 165 mm: verificato)

## 15 VERIFICHE TRASVERSALI DELLA SOLETTA STRADALE DI SCORRIMENTO

### 15.1 VERIFICA IN FASE COSTRUTTIVA

In fase costruttiva la sezione resistente è rappresentata dai tralicci inseriti nelle lastre prefabbricate, sollecitati dal peso della soletta e dal carico accidentale previsto da EN 1991-1-6, § 4.11.2.

#### 15.1.1 FASE COSTRUTTIVA - SBALZO LATERALE

##### VERIFICA LASTRE IN C.A. TRALICCIATE PREFABBRICATE

LASTRE PREFABBRICATE CAVALCAVIA STRADA VIC. DELLE COSTE - CAMPATA A SBALZO: L= 135 cm

##### DATI GENERALI LASTRA PREFABBRICATA.

Schema vincolare in fase costruttiva:	<b>A) Incastro - B) Estremo libero (a sbalzo)</b>
Luce di calcolo della lastra:	L= 135 cm
Larghezza di calcolo della lastra:	B <sub>c</sub> = 240 cm
Altezza calcestruzzo lastra:	H <sub>c</sub> = 6 cm
Altezza della soletta (getto in opera):	H <sub>s</sub> = 24 cm
Peso specifico calcestruzzo:	g <sub>c</sub> = 25 kN/m <sup>3</sup>
Vincolo laterale armature offerto dalla lastra presente nella sezione di verifica:	NO

##### DATI TRALICCI E ARMATURA AGGIUNTIVA LASTRA PREFABBRICATA.

Numero tralicci per lastra:	N <sub>tr</sub> = 4
Altezza nominale del traliccio:	H <sub>tr</sub> = 20.5 cm
Altezza utile del traliccio:	H <sub>u</sub> = 18.8 cm
Larghezza traliccio:	B <sub>tr</sub> = 12.0 cm
Passo staffe:	P <sub>st</sub> = 20.0 cm
Ricoprimento tralicci e armature aggiuntive (dal bordo inferiore lastra):	R <sub>tr</sub> = 4.0 cm
Modulo elastico acciaio	E <sub>s</sub> = 210 000 N/mm <sup>2</sup>
Coefficiente parziale di sicurezza dell'acciaio armature:	γ <sub>s</sub> = 1.15

##### DATI ARMATURA LASTRA PREFABBRICATA.

Tipo di acciaio  
Resistenza caratteristica acciaio  
Resistenza di calcolo acciaio  
Diametro:  
Numero di ferri per traliccio (Arm. agg. per lastra):  
Area di armatura per lastra:  
Inclinazione ferri in direzione longitudinale:  
Inclinazione ferri in direzione trasversale:  
Coefficiente di vincolo:  
Lunghezza di calcolo per instabilità aste compresse  
Lunghezza libera d'inflessione (L<sub>0</sub> = β\*L):

	Singolo traliccio			Arm. agg. lastra
	Corr. Superiore	Corr. Inferiori	Staffe	
	B450C	B450C	B450C	B450C
f <sub>yk</sub> =	450.00	450.00	450.00	450.00
f <sub>yd</sub> =	391.30	391.30	391.30	391.30
Φ=	16	12	10	0
N <sub>F</sub> =	1	2	2	0
A <sub>F</sub> =	8.042	9.048	6.283	0.000
α=	0.00	0.00	70.97	0.00
γ=	0.00	0.00	80.93	0.00
β=	1	1	1	1
L=	20.00	20.00	20.14	40.00
L <sub>0</sub> =	20.00	20.00	20.14	40.00

##### DATI DI CARICO

NOTA: Per le azioni in fase di costruzione durante il getto di calcestruzzo ci si riferisce a UNI EN 1991-1-6, § 4.11.2.

Coefficiente parziale dei carichi permanenti strutturali (peso lastra):	γ <sub>g1</sub> = 1.35
Coefficiente parziale dei carichi permanenti portati (getto in opera):	γ <sub>g2</sub> = 1.5
Coefficiente parziale dei carichi variabili (sovraccarichi):	γ <sub>q</sub> = 1.5

Estensione del carico permanente (X1=X2 per carico concentrato):		Da X1 (cm)	A X2 (cm)
Peso strutturale lastra:	G <sub>1</sub> =	1.50 kN/mq	0 135
Permanente portato 1:            getto in opera soletta:	G <sub>2,1</sub> =	6.00 kN/mq	0 135
Permanente portato 2:	G <sub>2,2</sub> =	0.00 kN/m	0 0
Permanente portato 3:	G <sub>2,3</sub> =	0.00 kN/m	0 0
Sovraccarico all'interno dell'area di lavoro di 3 m x 3 m (L se minore):	Q <sub>1</sub> =	0.75 kN/mq	
Sovraccarico all'esterno dell'area di lavoro di 3 m x 3 m (L se minore):	Q <sub>2</sub> =	0.75 kN/mq	

##### SOLLECITAZIONI ALLO S.L.U.    NOTA: Le sollecitazioni si intendono riferite alla larghezza della lastra.

Momento flettente di progetto:	M <sub>Sd</sub> =	-26.57 KN.m
Sforzo di taglio di progetto:	V <sub>Sd</sub> =	39.37 KN

##### VERIFICA ALLO S.L.U.

Sforzi assiali nei correnti (N<sub>Sd</sub>=M<sub>Sd</sub>/H<sub>u</sub>)  
Sforzi assiali nelle staffe (N<sub>Sd</sub>=V<sub>Sd</sub>/(Sen(α)\*Sen(γ)))

Carico elastico critico (N<sub>CR</sub>=ρ<sup>2</sup>\*E<sub>s</sub>\*J/L<sub>0</sub><sup>2</sup>)  
Snellezza adimensionale (λ = (A<sub>f</sub> f<sub>yk</sub>/N<sub>CR</sub>)<sup>0.5</sup>)  
Fattore di imperfezione (NTC 2008; tab. 4.2.VI)  
Coefficiente di stabilità delle aste compresse:  
Fattore di riduzione  
Resistenza di calcolo a compressione (instabilità)  
Resistenza di calcolo a trazione e compressione  
Considerare instabilità delle aste compresse (non incluse nel cls.)  
Resistenza di calcolo considerata (ferro singolo)  
Sforzo assiale di calcolo (ferro singolo)  
Coefficiente di sfruttamento (N<sub>Sd</sub>/N<sub>Rd</sub> < 1; verificato)

	Corr. Superiore (Teso)	Corr. Inferiori (Compresso)	Staffe (Compresso)	Arm. Agg. (Compresso)
N <sub>CR</sub> =	166.690	52.742	25.088	0.000
λ=	0.737	0.982	1.187	0.000
α=	0.490	0.490	0.490	0.490
Φ=	0.903	1.174	1.446	0.451
χ=	0.702	0.550	0.440	1.000
N <sub>b,Rd</sub> =	-55.217	-24.352	-13.524	0.000
N <sub>c,Rd</sub> =	±78.676	±44.255	±30.733	±0.000
SI/NO	NO	SI	SI	SI
N <sub>Rd</sub> =	78.676	-24.352	-13.524	0.000
N <sub>Sd</sub> =	35.335	-17.668	5.271	0.000
N <sub>Sd</sub> /N <sub>Rd</sub> =	0.449	0.726	0.390	0.000

### 15.1.2 FASE COSTRUTTIVA - CAMPATA TRA LE TRAVI

La lastra è semplicemente appoggiata agli estremi sulla luce di 280 cm.

#### VERIFICA LASTRE IN C.A. TRALICCIATE PREFABBRICATE

**LASTRE PREFABBRICATE CAVALCAVIA STRADA VIC. DELLE COSTE - TRA TRAVI: L= 280 cm**

##### DATI GENERALI LASTRA PREFABBRICATA.

Schema vincolare in fase costruttiva:	<b>A) Appoggio - B) Appoggio</b>
Luce di calcolo della lastra:	<b>L= 280 cm</b>
Larghezza di calcolo della lastra:	<b>B<sub>C</sub>= 240 cm</b>
Altezza calcestruzzo lastra:	<b>H<sub>C</sub>= 6 cm</b>
Altezza della soletta (getto in opera):	<b>H<sub>S</sub>= 24 cm</b>
Peso specifico calcestruzzo:	<b>g<sub>C</sub>= 25 kN/m<sup>3</sup></b>
Vincolo laterale armature offerto dalla lastra presente nella sezione di verifica:	<b>SI</b>

##### DATI TRALICCI E ARMATURA AGGIUNTIVA LASTRA PREFABBRICATA.

Numero tralici per lastra:	<b>N<sub>T</sub>= 4</b>
Altezza nominale del traliccio:	<b>H<sub>T</sub>= 20.5 cm</b>
Altezza utile del traliccio:	<b>H<sub>U</sub>= 18.8 cm</b>
Larghezza traliccio:	<b>B<sub>T</sub>= 12.0 cm</b>
Passo staffe:	<b>P<sub>ST</sub>= 20.0 cm</b>
Ricoprimento tralici e armature aggiuntive (dal bordo inferiore lastra):	<b>R<sub>IT</sub>= 4.0 cm</b>
Modulo elastico acciaio	<b>E<sub>S</sub>= 210 000 N/mm<sup>2</sup></b>
Coefficiente parziale di sicurezza dell'acciaio armature:	<b>γ<sub>S</sub>= 1.15</b>

##### DATI ARMATURA LASTRA PREFABBRICATA.

	Singolo traliccio			Arm. agg. lastra	
	Corr. Superiore	Corr. Inferiori	Staffe		
Tipo di acciaio	B450C	B450C	B450C	B450C	
Resistenza caratteristica acciaio	450.00	450.00	450.00	450.00	N/mm <sup>2</sup>
Resistenza di calcolo acciaio	391.30	391.30	391.30	391.30	N/mm <sup>2</sup>
Diametro:	16	12	10	0	mm
Numero di ferri per traliccio (Arm. agg. per lastra):	1	2	2	0	
Area di armatura per lastra:	8.042	9.048	6.283	0.000	cm <sup>2</sup>
Inclinazione ferri in direzione longitudinale:	0.00	0.00	70.97	0.00	°
Inclinazione ferri in direzione trasversale:	0.00	0.00	80.93	0.00	°
Coefficiente di vincolo:	1	1	1	1	
Lunghezza di calcolo per instabilità aste compresse	20.00	20.00	20.14	40.00	cm
Lunghezza libera d'inflessione (L <sub>0</sub> = β*L):	20.00	20.00	20.14	40.00	cm

##### DATI DI CARICO

NOTA: Per le azioni in fase di costruzione durante il getto di calcestruzzo ci si riferisce a: UNI EN 1991-1-6, § 4.11.2.

Coefficiente parziale dei carichi permanenti strutturali (peso lastra):	γ <sub>G1</sub> =	1.35
Coefficiente parziale dei carichi permanenti portati (getto in opera):	γ <sub>G2</sub> =	1.5
Coefficiente parziale dei carichi variabili (sovraccarichi):	γ <sub>Q</sub> =	1.5

Estensione del carico permanente (X1=X2 per carico concentrato):		Da X1 (cm)	A X2 (cm)
Peso strutturale lastra:	G <sub>1</sub> =	1.50 kN/mq	0      280
Permanente portato 1:      getto in opera soletta:	G <sub>2,1</sub> =	6.00 kN/mq	0      280
Permanente portato 2:      .	G <sub>2,2</sub> =	0.00 kN/m	0      0
Permanente portato 3:      .	G <sub>2,3</sub> =	0.00 kN/m	0      0
Sovraccarico all'interno dell'area di lavoro di 3 m x 3 m (L se minore):	Q <sub>1</sub> =	0.75 kN/mq	
Sovraccarico all'esterno dell'area di lavoro di 3 m x 3 m (L se minore):	Q <sub>2</sub> =	0.75 kN/mq	

##### SOLLECITAZIONI ALLO S.L.U.      NOTA: Le sollecitazioni si intendono riferite alla larghezza della lastra.

Momento flettente di progetto:	M <sub>Sd</sub> =	28.58 KN.m
Sforzo di taglio di progetto:	V <sub>Sd</sub> =	40.82 KN

##### VERIFICA ALLO S.L.U..

Sforzi assiali nei correnti (N<sub>Sd</sub>=M<sub>Sd</sub>/H<sub>U</sub>)

Sforzi assiali nelle staffe (N<sub>Sd</sub>=V<sub>Sd</sub>/(Sen(α)\*Sen(γ)))

Carico elastico critico (N<sub>CR</sub>=p<sup>2</sup>\*E<sub>S</sub>\*J/L<sub>0</sub><sup>3</sup>)

Snellezza adimensionale (λ = (A<sub>T</sub> f<sub>yk</sub>/N<sub>CR</sub>)<sup>0.5</sup>)

Fattore di imperfezione (NTC 2008; tab. 4.2.VI)

Coefficiente di stabilità delle aste compresse:

Fattore di riduzione

Resistenza di calcolo a compressione (instabilità)

Resistenza di calcolo a trazione e compressione

Considerare instabilità delle aste compresse (non incluse nel cls.)

Resistenza di calcolo considerata (ferro singolo)

Sforzo assiale di calcolo (ferro singolo)

Coefficiente di sfruttamento (N<sub>Sd</sub>/N<sub>Rd</sub> < 1: verificato)

	Corr. Superiore (Compresso)	Corr. Inferiori (Teso)	Staffe (Compresso)	Arm. Agg. (Teso)	
N <sub>CR</sub> =	166.690	52.742	25.088	0.000	kN
λ=	0.737	0.982	1.187	0.000	
α=	0.490	0.490	0.490	0.490	
Φ=	0.903	1.174	1.446	0.451	
χ=	0.702	0.550	0.440	1.000	
N <sub>b,Rd</sub> =	-55.217	-24.352	-13.524	0.000	kN
N <sub>c,Rd</sub> =	±78.676	±44.255	±30.733	±0.000	kN
SI/NO	SI	NO	SI	NO	
N <sub>Rd</sub> =	-55.217	44.255	-13.524	0.000	kN
N <sub>Sd</sub> =	-38.001	19.001	5.466	0.000	kN
N <sub>Sd</sub> /N <sub>Rd</sub> =	0.688	0.429	0.404	0.000	



## 15.2 VERIFICA IN ESERCIZIO - SBALZO LATERALE

La verifica viene svolta per lo sbalzo avente una luce  $L = 1.35$  m dall'asse trave.

La determinazione delle sollecitazioni massime viene svolta con apposito foglio di calcolo considerando i carichi permanenti e tutte le possibili colonne di carico mobile, compatibilmente con lo sbalzo considerato.

Si considerano in alternativa:

- colonne di schemi di carico 1
- schema di carico 2

Inoltre si considera una combinazione eccezionale con i carichi di cui sopra (con coefficienti SLU unitari) e l'effetto di urto sul sicurvia

Le singole ruote dei carichi mobili vengono disposte in modo da ottenere o il momento flettente massimo o lo sforzo di taglio massimo alla sezione di incastro.

Verifica allo stato limite ultimo: si determina l'area di armatura minima necessaria (verifica a flessione) e l'area minima di armatura trasversale (verifica a taglio).

Verifica allo stato limite di esercizio: si effettua la verifica allo stato limite di fessurazione per via indiretta dimensionando l'armatura minima lavorante ad una definita tensione, funzione del diametro massimo delle barre e dell'apertura ammissibile delle fessure; nel presente caso si ha:

- condizioni ambientali: Molto aggressive
- apertura max fessure, combinazioni frequenti:  $w_1 = 0.2$  mm
- apertura max fessure, combinazioni quasi permanenti:  $w_1 = 0.2$  mm
- diametro massimo delle barre:  $\varnothing = 25$  mm

Dalla tabella C4.1.II della Circ. 02/02/2009 n°617 si ottiene:

- Tensione di progetto nell'acciaio:  $\sigma_s = 160$  N/mm<sup>2</sup>



**Verifica dello sbalzo trasversale: sezione corrente**

**L= 1.350 m**

Altezza della lastra prefabbricata									0.060 m
Altezza totale soletta all'incastro									0.300 m
Altezza totale soletta all'estremità libera									0.300 m
Lunghezza di calcolo dello sbalzo									1.350 m
Larghezza del marciapiede									1.900 m
Altezza del marciapiede									0.150 m
Peso specifico calcestruzzo armato									25.000 kN/m <sup>3</sup>
Altezza della massicciata									0.100 m
Peso della massicciata									2.500 kN/m <sup>2</sup>
Peso del sicurvìa									0.800 kN/m
Distanza asse sicurvìa da incastro									0.000 m
Peso del parapetto									0.500 kN/m
Distanza asse parapetto da incastro									1.250 m
Peso della veletta									2.000 kN/m
Distanza asse veletta da incastro									1.400 m
Carico della folla (di combinazione)									6.000 kN/m
Distanza asse folla da incastro									0.550 m
Coefficiente dinamico									1.000
Schema di carico 1:	Larghezza delle colonne di carico								3.000 m
	Interasse ruote in senso trasversale								2.000 m
	Interasse ruote in senso longitudinale								1.200 m
	Lati dell'impronta ruota: L=	0.400 m		T=	0.400 m				
			1° colonna		2° colonna		3° colonna		
	Peso singola ruota	150.000		100.000			50.000 kN		
	Carico distribuito	9.000		2.500			2.500 kN/m <sup>2</sup>		
Schema di carico 2:	Interasse ruote in senso trasversale								2.000 m
	Interasse ruote in senso longitudinale								0.000 m
	Lati dell'impronta ruota: L=	0.350 m		T=	0.600 m				
	Peso singola ruota						200.000 kN		
Angolo di ripartizione verticale dei carichi nello spessore									45.000 °
Angolo di ripartizione orizzontale dei carichi (effetto piastra)									45.000 °

Tabella dei carichi mobili

n° ruote =	numero ruote carico considerato
b=	braccio del carico in esame (risultante)
lrip =	larghezza di ripartizione del carico ruote
q=	carico distribuito sulla larghezza di ripartizione

	n° carico	n° ruote	Massimo momento flettente			Massimo sforzo di taglio		
			bm (m)	lrip (m)	qm (kN/m)	bt (m)	lrip (m)	qt (kN/m)
1° colonna	Schema 1 - distribuito		0.000	1.000	0.000	0.000	1.000	0.000
	Schema 1 - 1° fila	1	0.000	0.840	0.000	0.000	0.840	0.000
	Schema 1 - 1° fila	2	0.000	2.040	0.000	0.000	2.040	0.000
	Schema 1 - 2° fila	2	0.000	2.040	0.000	0.000	2.040	0.000
2° colonna	Schema 1 - distribuito		0.000	1.000	0.000	0.000	1.000	0.000
	Schema 1 - 1° fila	1	0.000	0.840	0.000	0.000	0.840	0.000
	Schema 1 - 1° fila	2	0.000	2.040	0.000	0.000	2.040	0.000
	Schema 1 - 2° fila	2	0.000	2.040	0.000	0.000	2.040	0.000
3° colonna	Schema 1 - distribuito		0.000	1.000	0.000	0.000	1.000	0.000
	Schema 1 - 1° fila	1	0.000	0.840	0.000	0.000	0.840	0.000
	Schema 1 - 1° fila	2	0.000	2.040	0.000	0.000	2.040	0.000
	Schema 1 - 2° fila	2	0.000	2.040	0.000	0.000	2.040	0.000
	Schema 2 - 1° fila	1	0.000	0.000	0.000	0.000	0.790	0.000
	Schema 2 - 2° fila	1	0.000	0.000	0.000	0.000	0.790	0.000

Effetto d'urto sul sicurvìa (azione eccezionale)	100.000 kN
Altezza della forza d'urto da estradosso soletta	1.150 m
Larghezza di ripartizione urto	1.040 m

**Sollecitazioni all'incastro; sezione corrente**

Tipo di carico	Tagli (kN/m)	Momenti (kNm/m)	Sf.norm (kN/m)
Peso proprio (rettangolo)	10.125	-6.834	0.000
Peso proprio (triangolo)	0.000	0.000	0.000
Marciapiede	7.125	-2.850	0.000
Massicciata	-1.375	-0.378	0.000
Sicurtà	0.800	0.000	0.000
Parapetto	0.500	-0.625	0.000
Veletta	2.000	-2.800	0.000
Folla	6.000	-3.300	0.000
Schema 1 - distribuito	0.000	0.000	0.000
Schema 1 (tandem max)	0.000	0.000	0.000
Schema 2 (max)	0.000	0.000	0.000
Urto	0.000	-125.000	96.154

Coefficienti parziali ( $\gamma \times \psi$ )	SLU	SLU ECC:	SLE (rare)	SLE (frequenti)	SLE (quasi perm.)
Carichi permanenti	1.350	1.000	1.000	1.000	1.000
Carichi mobili - Tandem	1.350	1.000	1.000	0.750	0.000
Carichi mobili - Distribuiti	1.350	1.000	1.000	0.400	0.000

**Sollecitazioni complessive all'incastro; sezione corrente**

Tipo di carico	Tagli (kN/m)	Momenti (kNm/m)	Sf.norm (kN/m)
SLU Totali perm.+ mobili	33.986	-22.663	0.000
SLU ECC. Totali perm.+ mobili + urto	19.175	-138.488	96.154
SLE (rare) Totali perm.+ mobili	25.175	-16.788	0.000
SLE (frequenti) Totali perm.+ mobili	21.575	-14.808	0.000
SLE (quasi perm.) Totali perm.+ mobili	19.175	-13.488	0.000

**Dimensionamento armature minime allo SLU**

Resistenza di calcolo calcestruzzo:	fc <sub>d</sub> =	18.13 N/mm <sup>2</sup>
Resistenza di calcolo armatura:	fs <sub>d</sub> =	391.30 N/mm <sup>2</sup>
Modulo di elasticità armatura:	Es=	210000 N/mm <sup>2</sup>
Ricoprimento armature:	Ry=	50 mm
Inclinazione delle staffe (45° <= α <= 90°)	α=	90.00 °
Inclinazione di calcolo dei puntoni di calcestruzzo	θ =	21.80 °
Altezza utile della sezione di incastro	d=	25.00 cm
Altezza minima sezione reagente:	X=	3.78 cm
Area di armatura tesa minima:	As(sup)=	16.48 cm <sup>2</sup> /m
Area di armatura compressa minima:	As(inf)=	0.00 cm <sup>2</sup> /m
Verifica resistenza al taglio, solo cls.	Vrd,c=	129.06 kN
Coefficiente di sicurezza, solo cls.	Vrd,c/Vsd=	3.797 > 1: Verificato
Area di armatura trasversale necessaria per taglio:	Ast=	N.D. cm <sup>2</sup> /mq

**Dimensionamento armature minime allo SLE**

		SLE (frequenti)	SLE (quasi perm.)
Valore limite di apertura fessure:	wd=	0.2	0.2 mm
Tensione di calcolo armatura:	Sa=	160	160 N/mm <sup>2</sup>
Tensione nel calcestruzzo	Sc=	2.59	2.46 N/mm <sup>2</sup>
Area di armatura tesa minima:	As(sup)=	3.96	3.60 cm <sup>2</sup> /m

### 15.3 VERIFICA IN ESERCIZIO - SOLETTA INTERNA TRA LE TRAVI

La soletta tra le travi si schematizza come trave semi-incastata di luce 3.20 m per una larghezza di calcolo unitaria.

<b>Verifica soletta tra le travi(o su travi a canaletta)</b>		<b>L =</b>		<b>3.200 m</b>
Altezza della lastra prefabbricata				0.060 m
Altezza della soletta				0.300 m
Luce di calcolo				3.200 m
Peso specifico calcestruzzo armato				25.000 kN/m <sup>3</sup>
Altezza della massicciata				0.100 m
Peso della massicciata				2.500 kN/m <sup>2</sup>
Coefficiente dinamico				1.000
Schema di carico 1:	Larghezza delle colonne di carico			3.000 m
	Interasse ruote in senso trasversale			2.000 m
	Interasse ruote in senso longitudinale			1.200 m
	Lati dell'impronta ruota: L=	0.400 m	T=	0.400 m
		1° colonna	2° colonna	3° colonna
	Peso singola ruota	150.000	100.000	50.000 kN
	Carico distribuito	9.000	2.500	2.500 kN/m <sup>2</sup>
Schema di carico 2:	Interasse ruote in senso trasversale			2.000 m
	Interasse ruote in senso longitudinale			0.000 m
	Lati dell'impronta ruota: L=	0.350 m	T=	0.600 m
	Peso singola ruota			200.000 kN
Angolo di ripartizione verticale dei carichi nello spessore				45.000 °
Angolo di ripartizione orizzontale dei carichi (effetto piastra)				45.000 °
Efficienza minima dei vncoli di incastro (0=appoggio; 1=incastro perfetto)				0.750

Tabella dei carichi mobili

- n° ruote = numero ruote carico considerato
- xm = distanza dall'incastro del carico in esame (disposizione di momento massimo)
- xt = distanza dall'incastro del carico in esame (disposizione di taglio massimo)
- lrip = larghezza di ripartizione del carico ruote
- q= carico distribuito sulla larghezza di ripartizione

	n° carico	n° ruote	Massimo momento flettente			Massimo sforzo di taglio		
			xm (m)	lrip (m)	qm (kN/m)	xt (m)	lrip (m)	qt (kN/m)
1° colonna	Schema 1 - distribuito		2.150	1.000	18.900	1.500	1.000	27.000
	Schema 1 - 1° fila	1	1.600	2.440	61.475	0.500	2.440	61.475
	Schema 1 - 1° fila	2	1.600	3.640	82.418	0.500	3.640	82.418
	Schema 1 - 2° fila	2	0.000	0.000	0.000	2.500	3.640	82.418
2° colonna	Schema 1 - distribuito		1.500	1.000	7.500	2.950	1.000	1.250
	Schema 1 - 1° fila	1	0.600	2.440	40.984	3.200	2.440	40.984
	Schema 1 - 1° fila	2	0.600	3.640	54.945	3.200	3.640	54.945
	Schema 1 - 2° fila	2	0.000	0.000	0.000	0.000	0.000	0.000
3° colonna	Schema 1 - distribuito		0.000	1.000	0.000	0.000	1.000	0.000
	Schema 1 - 1° fila	1	0.000	0.000	0.000	0.000	0.000	0.000
	Schema 1 - 1° fila	2	0.000	0.000	0.000	0.000	0.000	0.000
	Schema 1 - 2° fila	2	0.000	0.000	0.000	0.000	0.000	0.000
	Schema 2 - 1° fila	1	1.600	2.390	83.682	0.500	2.390	83.682
	Schema 2 - 2° fila	1	0.000	0.000	0.000	2.500	2.390	83.682

**Sollecitazioni massime/minime**

Tipo di carico	Taglio max (kN/m)	Momento max (kNm/m)	Momento min (kNm/m)
Peso soletta	12.000	4.800	-6.400
Massicciata	4.000	1.600	-2.133
Schema 1 - distribuito	15.615	6.204	-8.885
Schema 1 (max/min)	131.895	39.940	-52.522
Schema 2 (max/min)	88.176	31.846	-39.354

Coefficienti parziali (γ X ψ)	SLU	SLE (rare)	SLE (frequenti)	SLE (quasi perm.)
Carichi permanenti	1.350	1.000	1.000	1.000
Carichi mobili - Tandem	1.350	1.000	0.750	0.000
Carichi mobili - Distribuiti	1.350	1.000	0.400	0.000

**Sollecitazioni compressive massime/minime**

Tipo di carico	Taglio max (kN/m)	Momento max (kNm/m)	Momento min (kNm/m)
SLU Totali perm.+ mobili	220.739	70.935	-94.420
SLE (rare) Totali perm.+ mobili	163.510	52.545	-69.940
SLE (frequenti) Totali perm.+ mobili	121.167	38.837	-51.479
SLE (quasi perm.) Totali perm.+ mobili	16.000	6.400	-8.533

**Dimensionamento armature minime allo SLU**

Resistenza di calcolo calcestruzzo:	fcd=	19.83 N/mmq
Resistenza di calcolo armatura:	fsd=	391.30 N/mmq
Modulo di elasticità armatura:	Es=	210000.00 N/mmq
Ricoprimento armature:	Ry(sup)=	50 mm
	Ry(inf)=	80 mm
Inclinazione delle staffe (45° ≤ α ≤ 90°)	α=	90.00 °
Inclinazione di calcolo dei puntoni di calcestruzzo	θ =	21.80 °

	Momento max	Momento min
Altezza utile della sezione	d= 22.00	25.00 cm
Altezza sezione reagente:	X= 2.11	2.48 cm
Area di armatura superiore minima:	As(sup)= 0.00	10.05 cmq/m
Area di armatura inferiore minima:	As(inf)= 8.57	0.00 cmq/m
Verifica resistenza al taglio, solo cls.	Vrd,c=	137.20 kN
Coefficiente di sicurezza, solo cls.	Vrd,c/Vsd=	0.622 <1: NON Verificato
Area di armatura trasversale necessaria per ta	Ast=	10.03 cmq/mq

**Dimensionamento armature minime allo SLE (frequenti)**

	Momento max	Momento min
Valore limite di apertura fessure:	wd=	0.2
Tensione di calcolo armatura:	Sa=	160
		160 N/mmq
Tensione nel calcestruzzo	Sc=	5.39
Area di armatura superiore minima:	As(sup)=	0.00
Area di armatura inferiore minima:	As(inf)=	12.42
		14.51 cmq/m
		0.00 cmq/m

## 15.4 VERIFICA CORDOLI

Si effettua la verifica della resistenza a taglio delle armature di collegamento tra la soletta e il cordolo sotto l'effetto dell'urto sul sicurvìa; si ha:

- effetto di urto sul sicurvìa:  $H = 100 \text{ kN}$
- larghezza di applicazione:  $b = 0.50 \text{ m}$
- area armature:  $1 + 1 \text{ } \varnothing 16 / 20:$   $A_1 = 2 \times 201 \times 50 / 20 = 1005 \text{ mm}^2$   
 $1 \text{ } \varnothing 12 / 20 / 40:$   $A_2 = 4 \times 113 \times 50 / 20 = 1130 \text{ mm}^2$   
 $A = 1005 + 283 = 2135 \text{ mm}^2$
- resistenza tangenziale di calcolo:  $t_d = \frac{450}{1.15\sqrt{3}} = 226 \text{ N/mm}^2$
- resistenza di calcolo:  $R = 226 \times 2.135 = 482.5 \text{ kN}$  (Verificato:  $R > H$ )

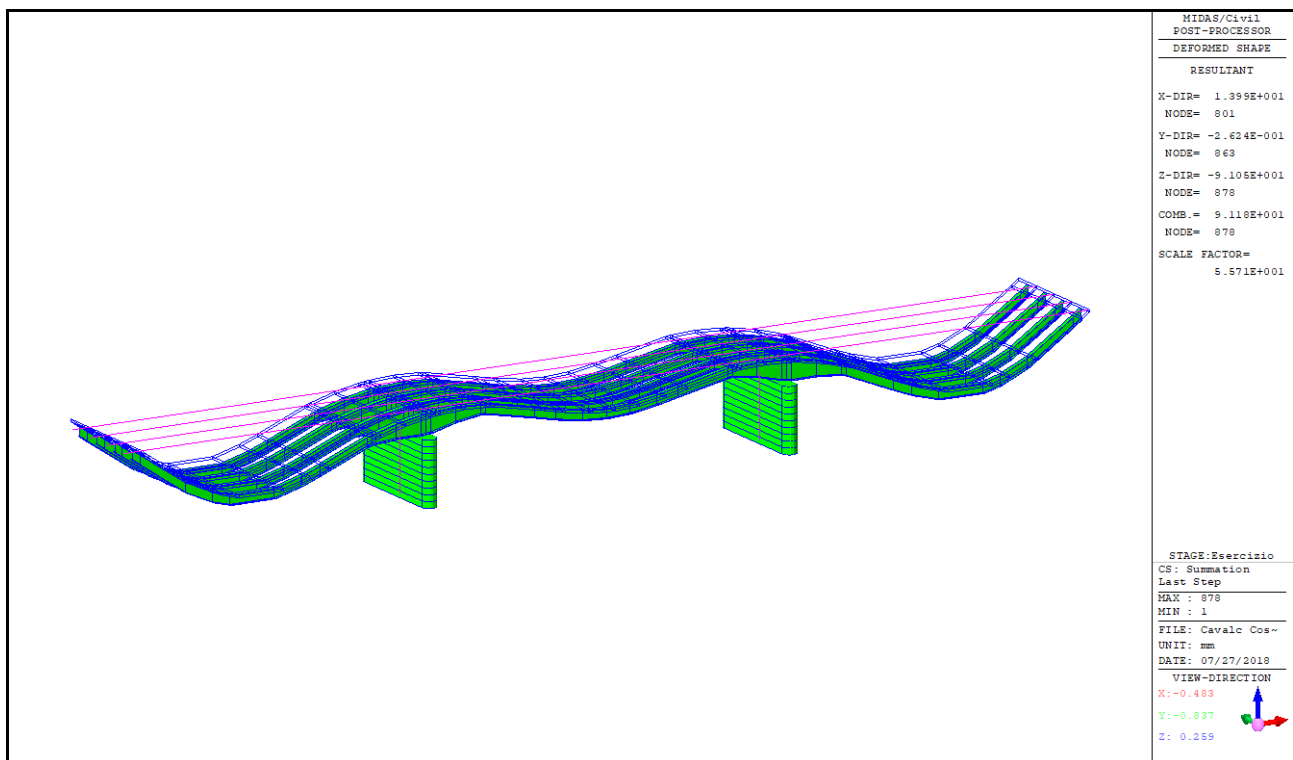
## 16 VERIFICHE DI DEFORMAZIONE TRAVI PRINCIPALI

### 16.1 DEFORMAZIONE CARICHI PERMANENTI

Si verifica che la deformata minima dovuta ai carichi permanenti sia minore di:

- campate laterali:  $\Delta Z_{lim,L} = L / 300 = 31000 / 300 = 103.3 \text{ mm.}$
- campata centrale:  $\Delta Z_{lim,C} = L / 300 = 38000 / 300 = 126.7 \text{ mm.}$

Segue diagramma delle deformazioni minime per la combinazione considerata.



**DEFORMATA MINIMA CARICHI PERMANENTI**

Il valore massimo (in modulo) della deformazione vale:

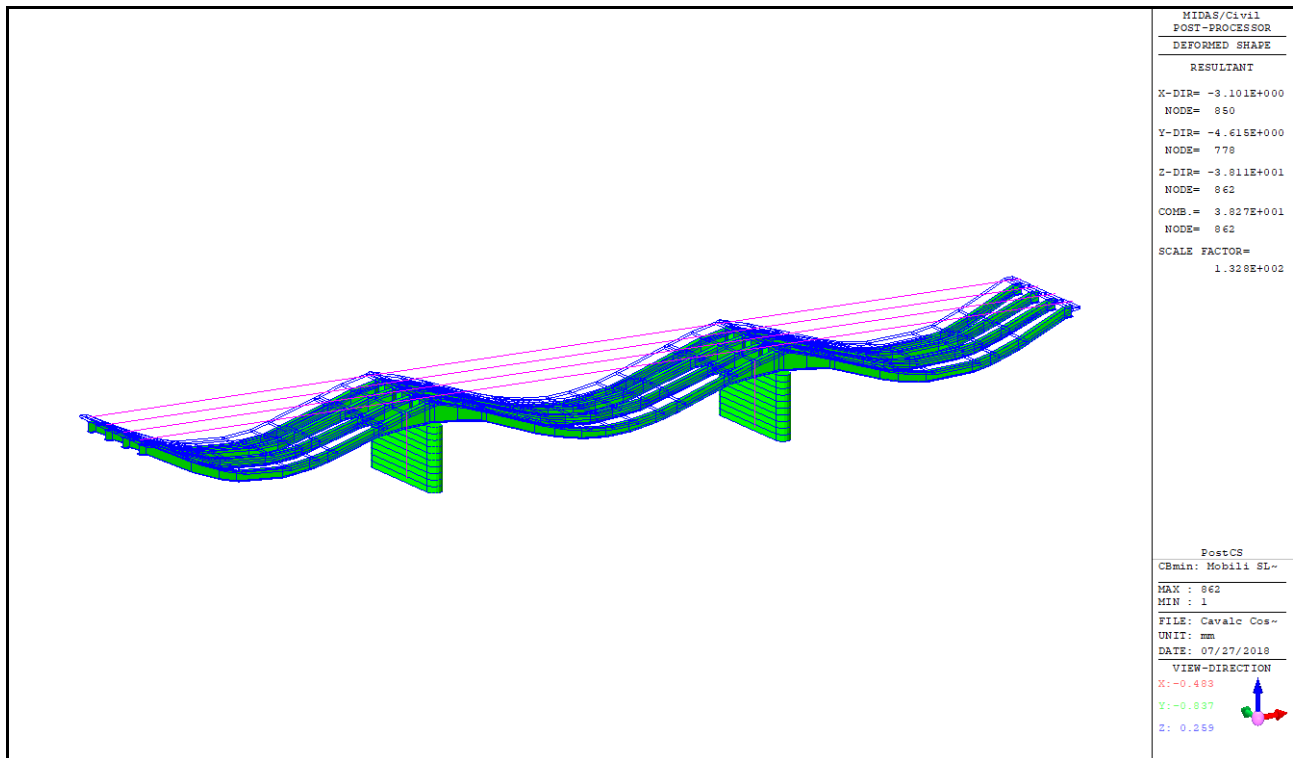
- campate laterali:  $\Delta Z = 91.1 \text{ mm, minore del valore limite } \Delta Z_{lim,L} = 103.3 \text{ mm.}$
- campata centrale:  $\Delta Z = 38.8 \text{ mm, minore del valore limite } \Delta Z_{lim,L} = 126.7 \text{ mm.}$

### 16.2 DEFORMAZIONE CARICHI VARIABILI

Si verifica che la deformata minima dei carichi variabili di progetto, nella combinazione caratteristica (rara) sia minore di:

- campate laterali:  $\Delta Z_{lim,L} = L / 700 = 31000 / 700 = 44.3 \text{ mm.}$
- campata centrale:  $\Delta Z_{lim,C} = L / 700 = 38000 / 700 = 54.3 \text{ mm.}$

Segue diagramma delle deformazioni minime per la combinazione considerata; si nota che il diagramma risulta dall'involuppo di diverse condizioni di carico mobile, quindi i valori sono corretti ma la "forma" della deformata non è significativa.



**DEFORMATA MINIMA CARICHI MOBILI (INVILUPPO MINIMI)**

Il valore massimo (in modulo) della deformazione vale:

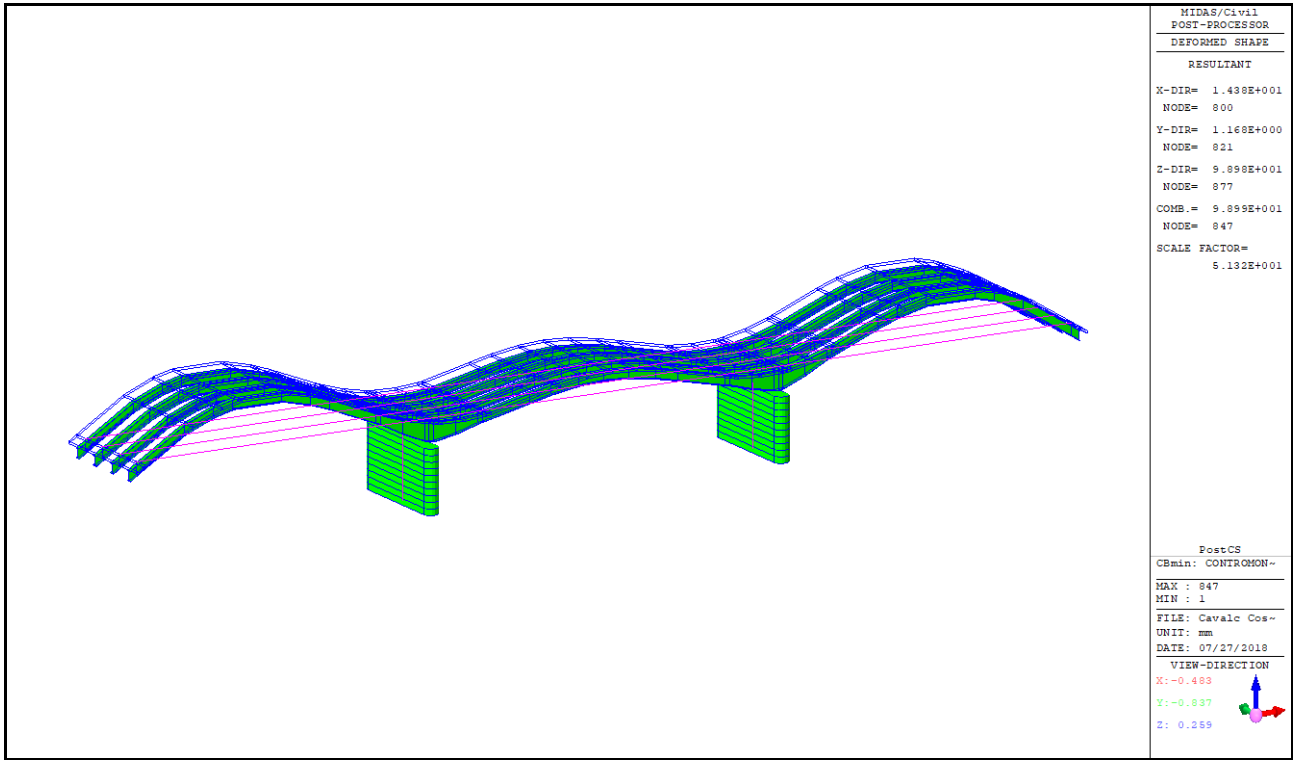
- campate laterali:  $\Delta Z = 33.1$  mm, minore del valore limite  $\Delta Z_{lim,L} = 44.3$  mm.
- campata centrale:  $\Delta Z = 38.1$  mm, minore del valore limite  $\Delta Z_{lim,L} = 54.3$  mm.

## 17 CONTROMONTE TRAVI

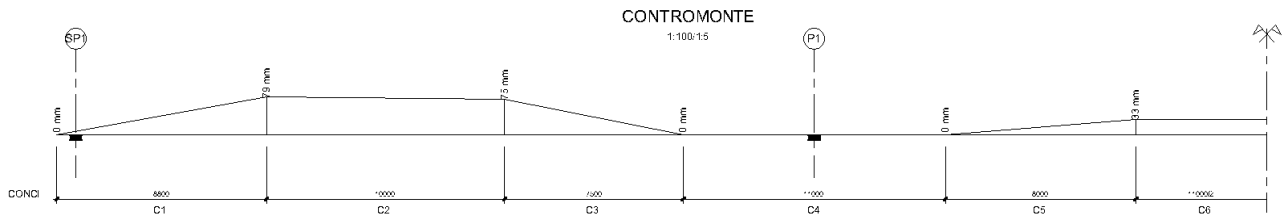
Le travature vengono realizzate con una controfreccia pari all'inverso delle deformazioni permanenti più il 25% dei carichi mobili, nella combinazione caratteristica; l'andamento dei singoli conci è rettilineo.

Nel diagramma a pagina seguente sono evidenziate le deformazioni da ottenere.





DEFORMATA - CONTROMONTE DI PROGETTO



CONTROMONTE EFFETTIVE (1/2 IMPALCATO)

## 18 VERIFICA DEI CONTENUTI DI CUI AL PAR. 10.2 DELLE N.T.C. 2008

- Tipo di analisi svolta
- Analisi strutturale condotta di tipo statico e dinamico lineare
- Metodo adottato per la risoluzione del problema strutturale: Metodo agli elementi finiti con software Midas/Civil 2015 v.1.2.
- Metodologie seguite per la verifica o per il progetto-verifica delle sezioni. Metodo semiprobabilistico agli Stati Limite
- Combinazioni di carico adottate: Le combinazioni di carico adottate sono riportate nel paragrafo 5 "Combinazioni di carico" e sono state scelte in modo da massimizzare tutte le sollecitazioni sulla struttura. L'impiego delle combinazioni adottate è esaustivo delle configurazioni studiate per la struttura in esame.
  - Origine e Caratteristiche dei Codici di Calcolo
- Vedi paragrafo 2 "Modellazione strutturale".
  - Affidabilità dei codici utilizzati
- Vedi paragrafo 2 "Modellazione strutturale".
  - Validazione dei codici.
- Nel caso in cui si rendesse necessaria una validazione indipendente del calcolo strutturale, i calcoli potranno essere eseguiti nuovamente da soggetto diverso da quello originario mediante programmi di calcolo diversi da quelli usati originariamente e ciò al fine di eseguire un effettivo controllo incrociato sui risultati delle elaborazioni. Al fondo della presente relazione si riporta il tabulato di input del modello strutturale con tutte le informazioni sufficienti a rendere ripercorribili tutti i calcoli effettuati.
  - Modalità di presentazione dei risultati.
- Il percorso che ha condotto ai risultati è stato: modellazione della struttura, analisi dei carichi e disposizione degli stessi sul modello; calcolo delle sollecitazioni; verifica degli elementi ritenuti significativi. La quantità di informazioni che ha accompagnato l'utilizzo del software in input e in output è cospicua. Per non appesantire eccessivamente la relazione di calcolo, si è operata la scelta di fornire soltanto una sintesi completa ed efficace dei risultati privilegiando schemi grafici ai tabulati. E' comunque disponibile su supporto informatico l'intero sviluppo dei tabulati di output con tutte le informazioni necessarie alla eventuale riproduzione del calcolo automatico.
  - Informazioni generali sull'elaborazione.
- Al termine della elaborazione sono stati svolti estesi controlli per l'esame dei risultati e per una valutazione complessiva dell'elaborazione dal punto di vista del corretto comportamento del modello.
  - Giudizio motivato di accettabilità dei risultati.
- I risultati della elaborazione sono stati analizzati criticamente mediante confronto con calcoli di massima eseguiti manualmente; tali controlli sommari hanno portato a confermare la validità dei risultati. I risultati delle elaborazioni sono quindi stati sottoposti a controlli che ne hanno comprovato l'attendibilità. In particolare si è svolto il controllo di equilibrio tra reazioni vincolari e carichi applicati.

## 19 ALLEGATO – DATI PROGRAMMA DI CALCOLO

A seguire l'allegato con i dati del programma di calcolo.

**20 SOTTOSCRIZIONE DELL'ELABORATO DA PARTE DEL R.T.P.**

**STUDIO CORONA S.r.l.**

---

**ECOPLAN S.r.l.**

---

**I.T. S.r.l.**

---

**E&G S.r.l.**

---

**CONSORZIO UNING**

---

**ARKE' INGEGNERIA S.r.l.**

---

**SETAC S.r.l.**

---

**ING. RENATO DEL PRETE**

---

**DOTT. DANILO GALLO**

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

```

;-----
; MIDAS/Civil Text (MCT) File.
; Date : 2017/11/15
;-----

```

\*VERSION  
8.3.5

\*UNIT ; Unit System  
KN , M, J, C

\*PROJINFO ; Project Information  
PROJECT=CASALPUSTERLENGO  
REVISION=2  
USER=Renato Vaira  
ADDRESS=Studio Corona  
TITLE=CAVALCAVIA STRADA VIC. DELLE COSTE


\*STRUCTYPE ; Structure Type  
0, 1, 1, NO, YES, 9.806, 0, NO, NO, NO

\*REBAR-MATL-CODE ; Rebar Material Code  
UNI(RC), B450C, UNI(RC), B450C

\*NODE ; Nodes

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26,	69,	0,	-2.7
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32,	69,	-4.8,	-2.3
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34,	69,	-1.6,	-2.3
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36,	69,	1.6,	-2.3
37,	31,	4.8,	-2.3
38,	69,	4.8,	-2.3
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PROJECT TITLE : CASALPUSTERLENGO

	Company	Client
	Author	File Name
	Renato Vaira	Cavalc Costa V_2.mct

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183, 37, -4.8, -1.13

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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372, 52.6, -1.6, -1.06

PROJECT TITLE : CASALPUSTERLENGO

	Company	Client
	Author	File Name
	Renato Vaira	Cavalc Costa V_2.mct

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379, 95, -1.6, -1.06  
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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

451, 95, 4.8, -1.06  
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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct


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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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229, BEAM , 5, 16, 788, 789, 0
230, BEAM , 5, 6, 789, 790, 0
231, BEAM , 5, 6, 790, 791, 0
232, BEAM , 5, 11, 791, 792, 0
233, BEAM , 5, 11, 792, 793, 0
234, BEAM , 5, 11, 793, 794, 0
235, BEAM , 5, 2, 794, 795, 0
236, BEAM , 5, 2, 795, 796, 0
237, BEAM , 5, 2, 796, 797, 0
238, BEAM , 5, 1, 797, 798, 0
239, BEAM , 5, 1, 798, 799, 0
240, BEAM , 5, 1, 799, 800, 0
301, BEAM , 5, 1, 801, 802, 0
302, BEAM , 5, 1, 802, 803, 0
303, BEAM , 5, 1, 803, 804, 0
304, BEAM , 5, 2, 804, 805, 0
305, BEAM , 5, 2, 805, 806, 0
306, BEAM , 5, 2, 806, 807, 0
307, BEAM , 5, 11, 807, 808, 0
308, BEAM , 5, 11, 808, 809, 0
309, BEAM , 5, 11, 809, 810, 0
310, BEAM , 5, 5, 810, 811, 0
311, BEAM , 5, 5, 811, 812, 0
312, BEAM , 5, 16, 812, 813, 0
313, BEAM , 5, 16, 813, 814, 0
314, BEAM , 5, 6, 814, 815, 0

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct


315, BEAM	,	5,	6,	815,	816,	0
316, BEAM	,	5,	7,	816,	817,	0
317, BEAM	,	5,	7,	817,	818,	0
318, BEAM	,	5,	7,	818,	819,	0
319, BEAM	,	5,	8,	819,	820,	0
320, BEAM	,	5,	8,	820,	821,	0
321, BEAM	,	5,	8,	821,	822,	0
322, BEAM	,	5,	8,	822,	823,	0
323, BEAM	,	5,	7,	823,	824,	0
324, BEAM	,	5,	7,	824,	825,	0
325, BEAM	,	5,	7,	825,	826,	0
326, BEAM	,	5,	5,	826,	827,	0
327, BEAM	,	5,	5,	827,	828,	0
328, BEAM	,	5,	16,	828,	829,	0
329, BEAM	,	5,	16,	829,	830,	0
330, BEAM	,	5,	6,	830,	831,	0
331, BEAM	,	5,	6,	831,	832,	0
332, BEAM	,	5,	11,	832,	833,	0
333, BEAM	,	5,	11,	833,	834,	0
334, BEAM	,	5,	11,	834,	835,	0
335, BEAM	,	5,	2,	835,	836,	0
336, BEAM	,	5,	2,	836,	837,	0
337, BEAM	,	5,	2,	837,	838,	0
338, BEAM	,	5,	1,	838,	839,	0
339, BEAM	,	5,	1,	839,	840,	0
340, BEAM	,	5,	1,	840,	841,	0
401, BEAM	,	5,	3,	842,	843,	0
402, BEAM	,	5,	3,	843,	844,	0
403, BEAM	,	5,	3,	844,	845,	0
404, BEAM	,	5,	4,	845,	846,	0
405, BEAM	,	5,	4,	846,	847,	0
406, BEAM	,	5,	4,	847,	848,	0
407, BEAM	,	5,	14,	848,	849,	0
408, BEAM	,	5,	14,	849,	850,	0
409, BEAM	,	5,	14,	850,	851,	0
410, BEAM	,	5,	9,	851,	852,	0
411, BEAM	,	5,	9,	852,	853,	0
412, BEAM	,	5,	21,	853,	854,	0
413, BEAM	,	5,	21,	854,	855,	0
414, BEAM	,	5,	10,	855,	856,	0
415, BEAM	,	5,	10,	856,	857,	0
416, BEAM	,	5,	12,	857,	858,	0
417, BEAM	,	5,	12,	858,	859,	0
418, BEAM	,	5,	12,	859,	860,	0
419, BEAM	,	5,	13,	860,	861,	0
420, BEAM	,	5,	13,	861,	862,	0
421, BEAM	,	5,	13,	862,	863,	0
422, BEAM	,	5,	13,	863,	864,	0
423, BEAM	,	5,	12,	864,	865,	0
424, BEAM	,	5,	12,	865,	866,	0
425, BEAM	,	5,	12,	866,	867,	0
426, BEAM	,	5,	9,	867,	868,	0
427, BEAM	,	5,	9,	868,	869,	0
428, BEAM	,	5,	21,	869,	870,	0
429, BEAM	,	5,	21,	870,	871,	0
430, BEAM	,	5,	10,	871,	872,	0
431, BEAM	,	5,	10,	872,	873,	0
432, BEAM	,	5,	14,	873,	874,	0
433, BEAM	,	5,	14,	874,	875,	0
434, BEAM	,	5,	14,	875,	876,	0
435, BEAM	,	5,	4,	876,	877,	0
436, BEAM	,	5,	4,	877,	878,	0
437, BEAM	,	5,	4,	878,	879,	0
438, BEAM	,	5,	3,	879,	880,	0
439, BEAM	,	5,	3,	880,	881,	0
440, BEAM	,	5,	3,	881,	882,	0
1014, BEAM	,	1,	18,	107,	111,	180
1015, BEAM	,	1,	18,	111,	115,	180
1016, BEAM	,	1,	18,	115,	119,	180
1017, BEAM	,	1,	18,	119,	123,	180
1018, BEAM	,	1,	18,	123,	127,	180
1019, BEAM	,	1,	18,	127,	131,	180
1022, BEAM	,	1,	18,	108,	112,	180
1023, BEAM	,	1,	18,	112,	116,	180
1024, BEAM	,	1,	18,	116,	120,	180
1025, BEAM	,	1,	18,	120,	124,	180
1026, BEAM	,	1,	18,	124,	128,	180
1027, BEAM	,	1,	18,	128,	132,	180

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct


1030, BEAM , 1, 18, 109, 113, 180
1031, BEAM , 1, 18, 113, 117, 180
1032, BEAM , 1, 18, 117, 121, 180
1033, BEAM , 1, 18, 121, 125, 180
1034, BEAM , 1, 18, 125, 129, 180
1035, BEAM , 1, 18, 129, 133, 180
1038, BEAM , 1, 18, 110, 114, 180
1039, BEAM , 1, 18, 114, 118, 180
1040, BEAM , 1, 18, 118, 122, 180
1041, BEAM , 1, 18, 122, 126, 180
1042, BEAM , 1, 18, 126, 130, 180
1043, BEAM , 1, 18, 130, 134, 180
1110, BEAM , 1, 18, 327, 345, 180
1111, BEAM , 1, 18, 345, 363, 180
1112, BEAM , 1, 18, 363, 381, 180
1113, BEAM , 1, 18, 381, 399, 180
1114, BEAM , 1, 18, 399, 417, 180
1115, BEAM , 1, 18, 417, 435, 180
1118, BEAM , 1, 18, 328, 346, 180
1119, BEAM , 1, 18, 346, 364, 180
1120, BEAM , 1, 18, 364, 382, 180
1121, BEAM , 1, 18, 382, 400, 180
1122, BEAM , 1, 18, 400, 418, 180
1123, BEAM , 1, 18, 418, 436, 180
1126, BEAM , 1, 18, 329, 347, 180
1127, BEAM , 1, 18, 347, 365, 180
1128, BEAM , 1, 18, 365, 383, 180
1129, BEAM , 1, 18, 383, 401, 180
1130, BEAM , 1, 18, 401, 419, 180
1131, BEAM , 1, 18, 419, 437, 180
1134, BEAM , 1, 18, 330, 348, 180
1135, BEAM , 1, 18, 348, 366, 180
1136, BEAM , 1, 18, 366, 384, 180
1137, BEAM , 1, 18, 384, 402, 180
1138, BEAM , 1, 18, 402, 420, 180
1139, BEAM , 1, 18, 420, 438, 180
1142, BEAM , 1, 18, 331, 349, 180
1143, BEAM , 1, 18, 349, 367, 180
1144, BEAM , 1, 18, 367, 385, 180
1145, BEAM , 1, 18, 385, 403, 180
1146, BEAM , 1, 18, 403, 421, 180
1147, BEAM , 1, 18, 421, 439, 180
1150, BEAM , 1, 18, 332, 350, 180
1151, BEAM , 1, 18, 350, 368, 180
1152, BEAM , 1, 18, 368, 386, 180
1153, BEAM , 1, 18, 386, 404, 180
1154, BEAM , 1, 18, 404, 422, 180
1155, BEAM , 1, 18, 422, 440, 180
1158, BEAM , 1, 18, 333, 351, 180
1159, BEAM , 1, 18, 351, 369, 180
1160, BEAM , 1, 18, 369, 387, 180
1161, BEAM , 1, 18, 387, 405, 180
1162, BEAM , 1, 18, 405, 423, 180
1163, BEAM , 1, 18, 423, 441, 180
1166, BEAM , 1, 18, 334, 352, 180
1167, BEAM , 1, 18, 352, 370, 180
1168, BEAM , 1, 18, 370, 388, 180
1169, BEAM , 1, 18, 388, 406, 180
1170, BEAM , 1, 18, 406, 424, 180
1171, BEAM , 1, 18, 424, 442, 180
1174, BEAM , 1, 18, 335, 353, 180
1175, BEAM , 1, 18, 353, 371, 180
1176, BEAM , 1, 18, 371, 389, 180
1177, BEAM , 1, 18, 389, 407, 180
1178, BEAM , 1, 18, 407, 425, 180
1179, BEAM , 1, 18, 425, 443, 180
1182, BEAM , 1, 18, 336, 354, 180
1183, BEAM , 1, 18, 354, 372, 180
1184, BEAM , 1, 18, 372, 390, 180
1185, BEAM , 1, 18, 390, 408, 180
1186, BEAM , 1, 18, 408, 426, 180
1187, BEAM , 1, 18, 426, 444, 180
1190, BEAM , 1, 18, 337, 355, 180
1191, BEAM , 1, 18, 355, 373, 180
1192, BEAM , 1, 18, 373, 391, 180
1193, BEAM , 1, 18, 391, 409, 180
1194, BEAM , 1, 18, 409, 427, 180
1195, BEAM , 1, 18, 427, 445, 180

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client
	Author	Renato Vaira	File Name
			Cavalc Costa V_2.mct

1198, BEAM , 1, 18, 338, 356, 180
1199, BEAM , 1, 18, 356, 374, 180
1200, BEAM , 1, 18, 374, 392, 180
1201, BEAM , 1, 18, 392, 410, 180
1202, BEAM , 1, 18, 410, 428, 180
1203, BEAM , 1, 18, 428, 446, 180
1206, BEAM , 1, 18, 339, 357, 180
1207, BEAM , 1, 18, 357, 375, 180
1208, BEAM , 1, 18, 375, 393, 180
1209, BEAM , 1, 18, 393, 411, 180
1210, BEAM , 1, 18, 411, 429, 180
1211, BEAM , 1, 18, 429, 447, 180
1214, BEAM , 1, 18, 340, 358, 180
1215, BEAM , 1, 18, 358, 376, 180
1216, BEAM , 1, 18, 376, 394, 180
1217, BEAM , 1, 18, 394, 412, 180
1218, BEAM , 1, 18, 412, 430, 180
1219, BEAM , 1, 18, 430, 448, 180
1222, BEAM , 1, 18, 341, 359, 180
1223, BEAM , 1, 18, 359, 377, 180
1224, BEAM , 1, 18, 377, 395, 180
1225, BEAM , 1, 18, 395, 413, 180
1226, BEAM , 1, 18, 413, 431, 180
1227, BEAM , 1, 18, 431, 449, 180
1230, BEAM , 1, 18, 342, 360, 180
1231, BEAM , 1, 18, 360, 378, 180
1232, BEAM , 1, 18, 378, 396, 180
1233, BEAM , 1, 18, 396, 414, 180
1234, BEAM , 1, 18, 414, 432, 180
1235, BEAM , 1, 18, 432, 450, 180
1238, BEAM , 1, 18, 343, 361, 180
1239, BEAM , 1, 18, 361, 379, 180
1240, BEAM , 1, 18, 379, 397, 180
1241, BEAM , 1, 18, 397, 415, 180
1242, BEAM , 1, 18, 415, 433, 180
1243, BEAM , 1, 18, 433, 451, 180
1246, BEAM , 1, 18, 344, 362, 180
1247, BEAM , 1, 18, 362, 380, 180
1248, BEAM , 1, 18, 380, 398, 180
1249, BEAM , 1, 18, 398, 416, 180
1250, BEAM , 1, 18, 416, 434, 180
1251, BEAM , 1, 18, 434, 452, 180
1254, BEAM , 1, 64, 495, 111, 180
1255, BEAM , 1, 64, 111, 519, 180
1256, BEAM , 1, 64, 519, 119, 180
1257, BEAM , 1, 64, 119, 543, 180
1258, BEAM , 1, 64, 543, 127, 180
1259, BEAM , 1, 64, 127, 567, 180
1262, BEAM , 1, 64, 497, 112, 180
1263, BEAM , 1, 64, 112, 521, 180
1264, BEAM , 1, 64, 521, 120, 180
1265, BEAM , 1, 64, 120, 545, 180
1266, BEAM , 1, 64, 545, 128, 180
1267, BEAM , 1, 64, 128, 569, 180
1270, BEAM , 1, 64, 504, 113, 180
1271, BEAM , 1, 64, 113, 528, 180
1272, BEAM , 1, 64, 528, 121, 180
1273, BEAM , 1, 64, 121, 552, 180
1274, BEAM , 1, 64, 552, 129, 180
1275, BEAM , 1, 64, 129, 576, 180
1278, BEAM , 1, 64, 506, 114, 180
1279, BEAM , 1, 64, 114, 530, 180
1280, BEAM , 1, 64, 530, 122, 180
1281, BEAM , 1, 64, 122, 554, 180
1282, BEAM , 1, 64, 554, 130, 180
1283, BEAM , 1, 64, 130, 578, 180
1286, BEAM , 1, 64, 489, 345, 180
1287, BEAM , 1, 64, 345, 513, 180
1288, BEAM , 1, 64, 513, 381, 180
1289, BEAM , 1, 64, 381, 537, 180
1290, BEAM , 1, 64, 537, 417, 180
1291, BEAM , 1, 64, 417, 561, 180
1294, BEAM , 1, 64, 490, 346, 180
1295, BEAM , 1, 64, 346, 514, 180
1296, BEAM , 1, 64, 514, 382, 180
1297, BEAM , 1, 64, 382, 538, 180
1298, BEAM , 1, 64, 538, 418, 180
1299, BEAM , 1, 64, 418, 562, 180

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

1302, BEAM , 1, 64, 491, 347, 180
1303, BEAM , 1, 64, 347, 515, 180
1304, BEAM , 1, 64, 515, 383, 180
1305, BEAM , 1, 64, 383, 539, 180
1306, BEAM , 1, 64, 539, 419, 180
1307, BEAM , 1, 64, 419, 563, 180
1310, BEAM , 1, 64, 492, 348, 180
1311, BEAM , 1, 64, 348, 516, 180
1312, BEAM , 1, 64, 516, 384, 180
1313, BEAM , 1, 64, 384, 540, 180
1314, BEAM , 1, 64, 540, 420, 180
1315, BEAM , 1, 64, 420, 564, 180
1318, BEAM , 1, 64, 493, 349, 180
1319, BEAM , 1, 64, 349, 517, 180
1320, BEAM , 1, 64, 517, 385, 180
1321, BEAM , 1, 64, 385, 541, 180
1322, BEAM , 1, 64, 541, 421, 180
1323, BEAM , 1, 64, 421, 565, 180
1326, BEAM , 1, 64, 494, 350, 180
1327, BEAM , 1, 64, 350, 518, 180
1328, BEAM , 1, 64, 518, 386, 180
1329, BEAM , 1, 64, 386, 542, 180
1330, BEAM , 1, 64, 542, 422, 180
1331, BEAM , 1, 64, 422, 566, 180
1334, BEAM , 1, 64, 498, 351, 180
1335, BEAM , 1, 64, 351, 522, 180
1336, BEAM , 1, 64, 522, 387, 180
1337, BEAM , 1, 64, 387, 546, 180
1338, BEAM , 1, 64, 546, 423, 180
1339, BEAM , 1, 64, 423, 570, 180
1342, BEAM , 1, 64, 499, 352, 180
1343, BEAM , 1, 64, 352, 523, 180
1344, BEAM , 1, 64, 523, 388, 180
1345, BEAM , 1, 64, 388, 547, 180
1346, BEAM , 1, 64, 547, 424, 180
1347, BEAM , 1, 64, 424, 571, 180
1350, BEAM , 1, 64, 500, 353, 180
1351, BEAM , 1, 64, 353, 524, 180
1352, BEAM , 1, 64, 524, 389, 180
1353, BEAM , 1, 64, 389, 548, 180
1354, BEAM , 1, 64, 548, 425, 180
1355, BEAM , 1, 64, 425, 572, 180
1358, BEAM , 1, 64, 501, 354, 180
1359, BEAM , 1, 64, 354, 525, 180
1360, BEAM , 1, 64, 525, 390, 180
1361, BEAM , 1, 64, 390, 549, 180
1362, BEAM , 1, 64, 549, 426, 180
1363, BEAM , 1, 64, 426, 573, 180
1366, BEAM , 1, 64, 502, 355, 180
1367, BEAM , 1, 64, 355, 526, 180
1368, BEAM , 1, 64, 526, 391, 180
1369, BEAM , 1, 64, 391, 550, 180
1370, BEAM , 1, 64, 550, 427, 180
1371, BEAM , 1, 64, 427, 574, 180
1374, BEAM , 1, 64, 503, 356, 180
1375, BEAM , 1, 64, 356, 527, 180
1376, BEAM , 1, 64, 527, 392, 180
1377, BEAM , 1, 64, 392, 551, 180
1378, BEAM , 1, 64, 551, 428, 180
1379, BEAM , 1, 64, 428, 575, 180
1382, BEAM , 1, 64, 507, 357, 180
1383, BEAM , 1, 64, 357, 531, 180
1384, BEAM , 1, 64, 531, 393, 180
1385, BEAM , 1, 64, 393, 555, 180
1386, BEAM , 1, 64, 555, 429, 180
1387, BEAM , 1, 64, 429, 579, 180
1390, BEAM , 1, 64, 508, 358, 180
1391, BEAM , 1, 64, 358, 532, 180
1392, BEAM , 1, 64, 532, 394, 180
1393, BEAM , 1, 64, 394, 556, 180
1394, BEAM , 1, 64, 556, 430, 180
1395, BEAM , 1, 64, 430, 580, 180
1398, BEAM , 1, 64, 509, 359, 180
1399, BEAM , 1, 64, 359, 533, 180
1400, BEAM , 1, 64, 533, 395, 180
1401, BEAM , 1, 64, 395, 557, 180
1402, BEAM , 1, 64, 557, 431, 180
1403, BEAM , 1, 64, 431, 581, 180



PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct


1406, BEAM , 1, 64, 510, 360, 180
1407, BEAM , 1, 64, 360, 534, 180
1408, BEAM , 1, 64, 534, 396, 180
1409, BEAM , 1, 64, 396, 558, 180
1410, BEAM , 1, 64, 558, 432, 180
1411, BEAM , 1, 64, 432, 582, 180
1414, BEAM , 1, 64, 511, 361, 180
1415, BEAM , 1, 64, 361, 535, 180
1416, BEAM , 1, 64, 535, 397, 180
1417, BEAM , 1, 64, 397, 559, 180
1418, BEAM , 1, 64, 559, 433, 180
1419, BEAM , 1, 64, 433, 583, 180
1422, BEAM , 1, 64, 512, 362, 180
1423, BEAM , 1, 64, 362, 536, 180
1424, BEAM , 1, 64, 536, 398, 180
1425, BEAM , 1, 64, 398, 560, 180
1426, BEAM , 1, 64, 560, 434, 180
1427, BEAM , 1, 64, 434, 584, 180
1430, BEAM , 1, 17, 609, 631, 0
1431, BEAM , 1, 17, 631, 653, 0
1432, BEAM , 1, 17, 653, 675, 0
1434, BEAM , 1, 17, 610, 632, 0
1435, BEAM , 1, 17, 632, 654, 0
1436, BEAM , 1, 17, 654, 676, 0
1438, BEAM , 1, 17, 611, 633, 0
1439, BEAM , 1, 17, 633, 655, 0
1440, BEAM , 1, 17, 655, 677, 0
1442, BEAM , 1, 17, 612, 634, 0
1443, BEAM , 1, 17, 634, 656, 0
1444, BEAM , 1, 17, 656, 678, 0
1446, BEAM , 1, 17, 613, 635, 0
1447, BEAM , 1, 17, 635, 657, 0
1448, BEAM , 1, 17, 657, 679, 0
1450, BEAM , 1, 17, 614, 636, 0
1451, BEAM , 1, 17, 636, 658, 0
1452, BEAM , 1, 17, 658, 680, 0
1454, BEAM , 1, 17, 615, 637, 0
1455, BEAM , 1, 17, 637, 659, 0
1456, BEAM , 1, 17, 659, 681, 0
1458, BEAM , 1, 17, 616, 638, 0
1459, BEAM , 1, 17, 638, 660, 0
1460, BEAM , 1, 17, 660, 682, 0
1462, BEAM , 1, 17, 617, 639, 0
1463, BEAM , 1, 17, 639, 661, 0
1464, BEAM , 1, 17, 661, 683, 0
1466, BEAM , 1, 17, 618, 640, 0
1467, BEAM , 1, 17, 640, 662, 0
1468, BEAM , 1, 17, 662, 684, 0
1470, BEAM , 1, 17, 619, 641, 0
1471, BEAM , 1, 17, 641, 663, 0
1472, BEAM , 1, 17, 663, 685, 0
1474, BEAM , 1, 17, 620, 642, 0
1475, BEAM , 1, 17, 642, 664, 0
1476, BEAM , 1, 17, 664, 686, 0
1478, BEAM , 1, 17, 621, 643, 0
1479, BEAM , 1, 17, 643, 665, 0
1480, BEAM , 1, 17, 665, 687, 0
1482, BEAM , 1, 17, 622, 644, 0
1483, BEAM , 1, 17, 644, 666, 0
1484, BEAM , 1, 17, 666, 688, 0
1486, BEAM , 1, 17, 623, 645, 0
1487, BEAM , 1, 17, 645, 667, 0
1488, BEAM , 1, 17, 667, 689, 0
1490, BEAM , 1, 17, 624, 646, 0
1491, BEAM , 1, 17, 646, 668, 0
1492, BEAM , 1, 17, 668, 690, 0
1494, BEAM , 1, 17, 625, 647, 0
1495, BEAM , 1, 17, 647, 669, 0
1496, BEAM , 1, 17, 669, 691, 0
1498, BEAM , 1, 17, 626, 648, 0
1499, BEAM , 1, 17, 648, 670, 0
1500, BEAM , 1, 17, 670, 692, 0
1502, BEAM , 1, 17, 627, 649, 0
1503, BEAM , 1, 17, 649, 671, 0
1504, BEAM , 1, 17, 671, 693, 0
1506, BEAM , 1, 17, 628, 650, 0
1507, BEAM , 1, 17, 650, 672, 0
1508, BEAM , 1, 17, 672, 694, 0

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

1510, BEAM , 1, 17, 629, 651, 0
1511, BEAM , 1, 17, 651, 673, 0
1512, BEAM , 1, 17, 673, 695, 0
1514, BEAM , 1, 17, 630, 652, 0
1515, BEAM , 1, 17, 652, 674, 0
1516, BEAM , 1, 17, 674, 696, 0
2001, BEAM , 1, 65, 51, 53, 180
2002, BEAM , 1, 65, 53, 55, 180
2003, BEAM , 1, 65, 55, 57, 180
2005, BEAM , 1, 65, 52, 54, 180
2006, BEAM , 1, 65, 54, 56, 180
2007, BEAM , 1, 65, 56, 58, 180
2009, BEAM , 1, 20, 51, 169, 180
2010, BEAM , 1, 20, 169, 53, 180
2011, BEAM , 1, 20, 53, 171, 180
2012, BEAM , 1, 20, 171, 55, 180
2013, BEAM , 1, 20, 55, 173, 180
2014, BEAM , 1, 20, 173, 57, 180
2017, BEAM , 1, 20, 52, 170, 180
2018, BEAM , 1, 20, 170, 54, 180
2019, BEAM , 1, 20, 54, 172, 180
2020, BEAM , 1, 20, 172, 56, 180
2021, BEAM , 1, 20, 56, 174, 180
2022, BEAM , 1, 20, 174, 58, 180
2025, BEAM , 1, 20, 496, 169, 180
2026, BEAM , 1, 20, 169, 520, 180
2027, BEAM , 1, 20, 520, 171, 180
2028, BEAM , 1, 20, 171, 544, 180
2029, BEAM , 1, 20, 544, 173, 180
2030, BEAM , 1, 20, 173, 568, 180
2033, BEAM , 1, 20, 505, 170, 180
2034, BEAM , 1, 20, 170, 529, 180
2035, BEAM , 1, 20, 529, 172, 180
2036, BEAM , 1, 20, 172, 553, 180
2037, BEAM , 1, 20, 553, 174, 180
2038, BEAM , 1, 20, 174, 577, 180
2041, BEAM , 1, 19, 496, 520, 0
2042, BEAM , 1, 19, 520, 544, 0
2043, BEAM , 1, 19, 544, 568, 0
2045, BEAM , 1, 19, 505, 529, 0
2046, BEAM , 1, 19, 529, 553, 0
2047, BEAM , 1, 19, 553, 577, 0
3001, BEAM , 1, 15, 177, 195, 90
3002, BEAM , 1, 15, 195, 210, 90
3003, BEAM , 1, 15, 210, 228, 90
3004, BEAM , 1, 15, 228, 243, 90
3005, BEAM , 1, 15, 243, 261, 90
3006, BEAM , 1, 15, 261, 276, 90
3009, BEAM , 1, 15, 178, 195, 90
3010, BEAM , 1, 15, 195, 211, 90
3011, BEAM , 1, 15, 211, 228, 90
3012, BEAM , 1, 15, 228, 244, 90
3013, BEAM , 1, 15, 244, 261, 90
3014, BEAM , 1, 15, 261, 277, 90
3017, BEAM , 1, 15, 178, 196, 90
3018, BEAM , 1, 15, 196, 211, 90
3019, BEAM , 1, 15, 211, 229, 90
3020, BEAM , 1, 15, 229, 244, 90
3021, BEAM , 1, 15, 244, 262, 90
3022, BEAM , 1, 15, 262, 277, 90
3025, BEAM , 1, 15, 179, 196, 90
3026, BEAM , 1, 15, 196, 212, 90
3027, BEAM , 1, 15, 212, 229, 90
3028, BEAM , 1, 15, 229, 245, 90
3029, BEAM , 1, 15, 245, 262, 90
3030, BEAM , 1, 15, 262, 278, 90
3033, BEAM , 1, 15, 179, 197, 90
3034, BEAM , 1, 15, 197, 212, 90
3035, BEAM , 1, 15, 212, 230, 90
3036, BEAM , 1, 15, 230, 245, 90
3037, BEAM , 1, 15, 245, 263, 90
3038, BEAM , 1, 15, 263, 278, 90
3041, BEAM , 1, 15, 180, 197, 90
3042, BEAM , 1, 15, 197, 213, 90
3043, BEAM , 1, 15, 213, 230, 90
3044, BEAM , 1, 15, 230, 246, 90
3045, BEAM , 1, 15, 246, 263, 90
3046, BEAM , 1, 15, 263, 279, 90

PROJECT TITLE : CASALPUSTERLENGO

	Company						Client
	Author	Renato Vaira					File Name
							Cavalc Costa V_2.mct

3049, BEAM , 1, 15, 180, 198, 90
3050, BEAM , 1, 15, 198, 213, 90
3051, BEAM , 1, 15, 213, 231, 90
3052, BEAM , 1, 15, 231, 246, 90
3053, BEAM , 1, 15, 246, 264, 90
3054, BEAM , 1, 15, 264, 279, 90
3057, BEAM , 1, 15, 181, 198, 90
3058, BEAM , 1, 15, 198, 214, 90
3059, BEAM , 1, 15, 214, 231, 90
3060, BEAM , 1, 15, 231, 247, 90
3061, BEAM , 1, 15, 247, 264, 90
3062, BEAM , 1, 15, 264, 280, 90
3065, BEAM , 1, 15, 181, 199, 90
3066, BEAM , 1, 15, 199, 214, 90
3067, BEAM , 1, 15, 214, 232, 90
3068, BEAM , 1, 15, 232, 247, 90
3069, BEAM , 1, 15, 247, 265, 90
3070, BEAM , 1, 15, 265, 280, 90
3073, BEAM , 1, 15, 182, 199, 90
3074, BEAM , 1, 15, 199, 215, 90
3075, BEAM , 1, 15, 215, 232, 90
3076, BEAM , 1, 15, 232, 248, 90
3077, BEAM , 1, 15, 248, 265, 90
3078, BEAM , 1, 15, 265, 281, 90
3081, BEAM , 1, 15, 182, 143, 90
3082, BEAM , 1, 15, 143, 215, 90
3083, BEAM , 1, 15, 215, 147, 90
3084, BEAM , 1, 15, 147, 248, 90
3085, BEAM , 1, 15, 248, 151, 90
3086, BEAM , 1, 15, 151, 281, 90
3089, BEAM , 1, 15, 87, 143, 90
3090, BEAM , 1, 15, 143, 91, 90
3091, BEAM , 1, 15, 91, 147, 90
3092, BEAM , 1, 15, 147, 95, 90
3093, BEAM , 1, 15, 95, 151, 90
3094, BEAM , 1, 15, 151, 99, 90
3097, BEAM , 1, 15, 87, 61, 90
3098, BEAM , 1, 15, 61, 91, 90
3099, BEAM , 1, 15, 91, 65, 90
3100, BEAM , 1, 15, 65, 95, 90
3101, BEAM , 1, 15, 95, 69, 90
3102, BEAM , 1, 15, 69, 99, 90
3105, BEAM , 1, 15, 41, 61, 90
3106, BEAM , 1, 15, 61, 43, 90
3107, BEAM , 1, 15, 43, 65, 90
3108, BEAM , 1, 15, 65, 45, 90
3109, BEAM , 1, 15, 45, 69, 90
3110, BEAM , 1, 15, 69, 47, 90
3113, BEAM , 1, 15, 41, 62, 90
3114, BEAM , 1, 15, 62, 43, 90
3115, BEAM , 1, 15, 43, 66, 90
3116, BEAM , 1, 15, 66, 45, 90
3117, BEAM , 1, 15, 45, 70, 90
3118, BEAM , 1, 15, 70, 47, 90
3121, BEAM , 1, 15, 88, 62, 90
3122, BEAM , 1, 15, 62, 92, 90
3123, BEAM , 1, 15, 92, 66, 90
3124, BEAM , 1, 15, 66, 96, 90
3125, BEAM , 1, 15, 96, 70, 90
3126, BEAM , 1, 15, 70, 100, 90
3129, BEAM , 1, 15, 88, 144, 90
3130, BEAM , 1, 15, 144, 92, 90
3131, BEAM , 1, 15, 92, 148, 90
3132, BEAM , 1, 15, 148, 96, 90
3133, BEAM , 1, 15, 96, 152, 90
3134, BEAM , 1, 15, 152, 100, 90
3137, BEAM , 1, 15, 183, 144, 90
3138, BEAM , 1, 15, 144, 216, 90
3139, BEAM , 1, 15, 216, 148, 90
3140, BEAM , 1, 15, 148, 249, 90
3141, BEAM , 1, 15, 249, 152, 90
3142, BEAM , 1, 15, 152, 282, 90
3145, BEAM , 1, 15, 183, 200, 90
3146, BEAM , 1, 15, 200, 216, 90
3147, BEAM , 1, 15, 216, 233, 90
3148, BEAM , 1, 15, 233, 249, 90
3149, BEAM , 1, 15, 249, 266, 90
3150, BEAM , 1, 15, 266, 282, 90

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

3153, BEAM	,	1,	15,	184,	200,	90
3154, BEAM	,	1,	15,	200,	217,	90
3155, BEAM	,	1,	15,	217,	233,	90
3156, BEAM	,	1,	15,	233,	250,	90
3157, BEAM	,	1,	15,	250,	266,	90
3158, BEAM	,	1,	15,	266,	283,	90
3161, BEAM	,	1,	15,	184,	201,	90
3162, BEAM	,	1,	15,	201,	217,	90
3163, BEAM	,	1,	15,	217,	234,	90
3164, BEAM	,	1,	15,	234,	250,	90
3165, BEAM	,	1,	15,	250,	267,	90
3166, BEAM	,	1,	15,	267,	283,	90
3169, BEAM	,	1,	15,	185,	201,	90
3170, BEAM	,	1,	15,	201,	218,	90
3171, BEAM	,	1,	15,	218,	234,	90
3172, BEAM	,	1,	15,	234,	251,	90
3173, BEAM	,	1,	15,	251,	267,	90
3174, BEAM	,	1,	15,	267,	284,	90
3177, BEAM	,	1,	15,	185,	202,	90
3178, BEAM	,	1,	15,	202,	218,	90
3179, BEAM	,	1,	15,	218,	235,	90
3180, BEAM	,	1,	15,	235,	251,	90
3181, BEAM	,	1,	15,	251,	268,	90
3182, BEAM	,	1,	15,	268,	284,	90
3185, BEAM	,	1,	15,	186,	202,	90
3186, BEAM	,	1,	15,	202,	219,	90
3187, BEAM	,	1,	15,	219,	235,	90
3188, BEAM	,	1,	15,	235,	252,	90
3189, BEAM	,	1,	15,	252,	268,	90
3190, BEAM	,	1,	15,	268,	285,	90
3193, BEAM	,	1,	15,	186,	203,	90
3194, BEAM	,	1,	15,	203,	219,	90
3195, BEAM	,	1,	15,	219,	236,	90
3196, BEAM	,	1,	15,	236,	252,	90
3197, BEAM	,	1,	15,	252,	269,	90
3198, BEAM	,	1,	15,	269,	285,	90
3201, BEAM	,	1,	15,	187,	203,	90
3202, BEAM	,	1,	15,	203,	220,	90
3203, BEAM	,	1,	15,	220,	236,	90
3204, BEAM	,	1,	15,	236,	253,	90
3205, BEAM	,	1,	15,	253,	269,	90
3206, BEAM	,	1,	15,	269,	286,	90
3209, BEAM	,	1,	15,	187,	204,	90
3210, BEAM	,	1,	15,	204,	220,	90
3211, BEAM	,	1,	15,	220,	237,	90
3212, BEAM	,	1,	15,	237,	253,	90
3213, BEAM	,	1,	15,	253,	270,	90
3214, BEAM	,	1,	15,	270,	286,	90
3217, BEAM	,	1,	15,	188,	204,	90
3218, BEAM	,	1,	15,	204,	221,	90
3219, BEAM	,	1,	15,	221,	237,	90
3220, BEAM	,	1,	15,	237,	254,	90
3221, BEAM	,	1,	15,	254,	270,	90
3222, BEAM	,	1,	15,	270,	287,	90
3225, BEAM	,	1,	15,	188,	145,	90
3226, BEAM	,	1,	15,	145,	221,	90
3227, BEAM	,	1,	15,	221,	149,	90
3228, BEAM	,	1,	15,	149,	254,	90
3229, BEAM	,	1,	15,	254,	153,	90
3230, BEAM	,	1,	15,	153,	287,	90
3233, BEAM	,	1,	15,	89,	145,	90
3234, BEAM	,	1,	15,	145,	93,	90
3235, BEAM	,	1,	15,	93,	149,	90
3236, BEAM	,	1,	15,	149,	97,	90
3237, BEAM	,	1,	15,	97,	153,	90
3238, BEAM	,	1,	15,	153,	101,	90
3241, BEAM	,	1,	15,	89,	63,	90
3242, BEAM	,	1,	15,	63,	93,	90
3243, BEAM	,	1,	15,	93,	67,	90
3244, BEAM	,	1,	15,	67,	97,	90
3245, BEAM	,	1,	15,	97,	71,	90
3246, BEAM	,	1,	15,	71,	101,	90
3249, BEAM	,	1,	15,	42,	63,	90
3250, BEAM	,	1,	15,	63,	44,	90
3251, BEAM	,	1,	15,	44,	67,	90
3252, BEAM	,	1,	15,	67,	46,	90
3253, BEAM	,	1,	15,	46,	71,	90
3254, BEAM	,	1,	15,	71,	48,	90

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

3257, BEAM	,	1,	15,	42,	64,	90
3258, BEAM	,	1,	15,	64,	44,	90
3259, BEAM	,	1,	15,	44,	68,	90
3260, BEAM	,	1,	15,	68,	46,	90
3261, BEAM	,	1,	15,	46,	72,	90
3262, BEAM	,	1,	15,	72,	48,	90
3265, BEAM	,	1,	15,	90,	64,	90
3266, BEAM	,	1,	15,	64,	94,	90
3267, BEAM	,	1,	15,	94,	68,	90
3268, BEAM	,	1,	15,	68,	98,	90
3269, BEAM	,	1,	15,	98,	72,	90
3270, BEAM	,	1,	15,	72,	102,	90
3273, BEAM	,	1,	15,	90,	146,	90
3274, BEAM	,	1,	15,	146,	94,	90
3275, BEAM	,	1,	15,	94,	150,	90
3276, BEAM	,	1,	15,	150,	98,	90
3277, BEAM	,	1,	15,	98,	154,	90
3278, BEAM	,	1,	15,	154,	102,	90
3281, BEAM	,	1,	15,	189,	146,	90
3282, BEAM	,	1,	15,	146,	222,	90
3283, BEAM	,	1,	15,	222,	150,	90
3284, BEAM	,	1,	15,	150,	255,	90
3285, BEAM	,	1,	15,	255,	154,	90
3286, BEAM	,	1,	15,	154,	288,	90
3289, BEAM	,	1,	15,	189,	205,	90
3290, BEAM	,	1,	15,	205,	222,	90
3291, BEAM	,	1,	15,	222,	238,	90
3292, BEAM	,	1,	15,	238,	255,	90
3293, BEAM	,	1,	15,	255,	271,	90
3294, BEAM	,	1,	15,	271,	288,	90
3297, BEAM	,	1,	15,	190,	205,	90
3298, BEAM	,	1,	15,	205,	223,	90
3299, BEAM	,	1,	15,	223,	238,	90
3300, BEAM	,	1,	15,	238,	256,	90
3301, BEAM	,	1,	15,	256,	271,	90
3302, BEAM	,	1,	15,	271,	289,	90
3305, BEAM	,	1,	15,	190,	206,	90
3306, BEAM	,	1,	15,	206,	223,	90
3307, BEAM	,	1,	15,	223,	239,	90
3308, BEAM	,	1,	15,	239,	256,	90
3309, BEAM	,	1,	15,	256,	272,	90
3310, BEAM	,	1,	15,	272,	289,	90
3313, BEAM	,	1,	15,	191,	206,	90
3314, BEAM	,	1,	15,	206,	224,	90
3315, BEAM	,	1,	15,	224,	239,	90
3316, BEAM	,	1,	15,	239,	257,	90
3317, BEAM	,	1,	15,	257,	272,	90
3318, BEAM	,	1,	15,	272,	290,	90
3321, BEAM	,	1,	15,	191,	207,	90
3322, BEAM	,	1,	15,	207,	224,	90
3323, BEAM	,	1,	15,	224,	240,	90
3324, BEAM	,	1,	15,	240,	257,	90
3325, BEAM	,	1,	15,	257,	273,	90
3326, BEAM	,	1,	15,	273,	290,	90
3329, BEAM	,	1,	15,	192,	207,	90
3330, BEAM	,	1,	15,	207,	225,	90
3331, BEAM	,	1,	15,	225,	240,	90
3332, BEAM	,	1,	15,	240,	258,	90
3333, BEAM	,	1,	15,	258,	273,	90
3334, BEAM	,	1,	15,	273,	291,	90
3337, BEAM	,	1,	15,	192,	208,	90
3338, BEAM	,	1,	15,	208,	225,	90
3339, BEAM	,	1,	15,	225,	241,	90
3340, BEAM	,	1,	15,	241,	258,	90
3341, BEAM	,	1,	15,	258,	274,	90
3342, BEAM	,	1,	15,	274,	291,	90
3345, BEAM	,	1,	15,	193,	208,	90
3346, BEAM	,	1,	15,	208,	226,	90
3347, BEAM	,	1,	15,	226,	241,	90
3348, BEAM	,	1,	15,	241,	259,	90
3349, BEAM	,	1,	15,	259,	274,	90
3350, BEAM	,	1,	15,	274,	292,	90
3353, BEAM	,	1,	15,	193,	209,	90
3354, BEAM	,	1,	15,	209,	226,	90
3355, BEAM	,	1,	15,	226,	242,	90
3356, BEAM	,	1,	15,	242,	259,	90
3357, BEAM	,	1,	15,	259,	275,	90
3358, BEAM	,	1,	15,	275,	292,	90

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

3361, BEAM , 1, 15, 194, 209, 90
3362, BEAM , 1, 15, 209, 227, 90
3363, BEAM , 1, 15, 227, 242, 90
3364, BEAM , 1, 15, 242, 260, 90
3365, BEAM , 1, 15, 260, 275, 90
3366, BEAM , 1, 15, 275, 293, 90
4001, BEAM , 3, 46, 1, 895, 0
4002, BEAM , 3, 46, 895, 887, 0
4003, BEAM , 3, 46, 887, 896, 0
4004, BEAM , 3, 46, 896, 888, 0
4005, BEAM , 3, 46, 888, 897, 0
4006, BEAM , 3, 46, 897, 889, 0
4007, BEAM , 3, 46, 889, 898, 0
4008, BEAM , 3, 46, 898, 890, 0
4009, BEAM , 3, 46, 890, 899, 0
4010, BEAM , 3, 46, 899, 25, 0
4011, BEAM , 3, 46, 2, 900, 0
4012, BEAM , 3, 46, 900, 891, 0
4013, BEAM , 3, 46, 891, 901, 0
4014, BEAM , 3, 46, 901, 892, 0
4015, BEAM , 3, 46, 892, 902, 0
4016, BEAM , 3, 46, 902, 893, 0
4017, BEAM , 3, 46, 893, 903, 0
4018, BEAM , 3, 46, 903, 894, 0
4019, BEAM , 3, 46, 894, 904, 0
4020, BEAM , 3, 46, 904, 26, 0
5009, BEAM , 2, 22, 720, 761, 0
5010, BEAM , 2, 22, 761, 802, 0
5011, BEAM , 2, 22, 802, 843, 0
5013, BEAM , 2, 23, 721, 762, 0
5014, BEAM , 2, 23, 762, 803, 0
5015, BEAM , 2, 23, 803, 844, 0
5017, BEAM , 2, 23, 723, 764, 0
5018, BEAM , 2, 23, 764, 805, 0
5019, BEAM , 2, 23, 805, 846, 0
5021, BEAM , 2, 23, 724, 765, 0
5022, BEAM , 2, 23, 765, 806, 0
5023, BEAM , 2, 23, 806, 847, 0
5025, BEAM , 2, 23, 726, 767, 0
5026, BEAM , 2, 23, 767, 808, 0
5027, BEAM , 2, 23, 808, 849, 0
5029, BEAM , 2, 24, 727, 768, 0
5030, BEAM , 2, 24, 768, 809, 0
5031, BEAM , 2, 24, 809, 850, 0
5033, BEAM , 2, 25, 729, 770, 0
5034, BEAM , 2, 25, 770, 811, 0
5035, BEAM , 2, 25, 811, 852, 0
5037, BEAM , 2, 25, 731, 772, 0
5038, BEAM , 2, 25, 772, 813, 0
5039, BEAM , 2, 25, 813, 854, 0
5041, BEAM , 2, 25, 733, 774, 0
5042, BEAM , 2, 25, 774, 815, 0
5043, BEAM , 2, 25, 815, 856, 0
5045, BEAM , 2, 26, 735, 776, 0
5046, BEAM , 2, 26, 776, 817, 0
5047, BEAM , 2, 26, 817, 858, 0
5049, BEAM , 2, 27, 736, 777, 0
5050, BEAM , 2, 27, 777, 818, 0
5051, BEAM , 2, 27, 818, 859, 0
5053, BEAM , 2, 27, 738, 779, 0
5054, BEAM , 2, 27, 779, 820, 0
5055, BEAM , 2, 27, 820, 861, 0
5070, BEAM , 2, 27, 863, 822, 0
5071, BEAM , 2, 27, 822, 781, 0
5072, BEAM , 2, 27, 781, 740, 0
5074, BEAM , 2, 27, 865, 824, 0
5075, BEAM , 2, 27, 824, 783, 0
5076, BEAM , 2, 27, 783, 742, 0
5078, BEAM , 2, 26, 866, 825, 0
5079, BEAM , 2, 26, 825, 784, 0
5080, BEAM , 2, 26, 784, 743, 0
5082, BEAM , 2, 25, 868, 827, 0
5083, BEAM , 2, 25, 827, 786, 0
5084, BEAM , 2, 25, 786, 745, 0
5086, BEAM , 2, 25, 870, 829, 0
5087, BEAM , 2, 25, 829, 788, 0
5088, BEAM , 2, 25, 788, 747, 0
5090, BEAM , 2, 25, 872, 831, 0



PROJECT TITLE : CASALPUSTERLENGO

	Company	Client
	Author	File Name
	Renato Vaira	Cavalc Costa V_2.mct

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5091, BEAM , 2, 25, 831, 790, 0
5092, BEAM , 2, 25, 790, 749, 0
5094, BEAM , 2, 24, 874, 833, 0
5095, BEAM , 2, 24, 833, 792, 0
5096, BEAM , 2, 24, 792, 751, 0
5098, BEAM , 2, 23, 875, 834, 0
5099, BEAM , 2, 23, 834, 793, 0
5100, BEAM , 2, 23, 793, 752, 0
5102, BEAM , 2, 23, 877, 836, 0
5103, BEAM , 2, 23, 836, 795, 0
5104, BEAM , 2, 23, 795, 754, 0
5110, BEAM , 2, 23, 878, 837, 0
5111, BEAM , 2, 23, 837, 796, 0
5112, BEAM , 2, 23, 796, 755, 0
5114, BEAM , 2, 23, 880, 839, 0
5115, BEAM , 2, 23, 839, 798, 0
5116, BEAM , 2, 23, 798, 757, 0
5118, BEAM , 2, 22, 881, 840, 0
5119, BEAM , 2, 22, 840, 799, 0
5120, BEAM , 2, 22, 799, 758, 0
    
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*GROUP ; Group
TRAVI PRINCIPALI, 719to882, 101to140 201to240 301to340 401to440, 0
TRASVERSI INTERMEDI, 107to134 327to452 489to495 497to504 506to519 521to528 \
530to543 545to552 554to567 569to576 578to584 609to696, 1014to1019 \
1022to1027 1030to1035 1038to1043 1110to1422by8 1111to1423by8 \
1112to1424by8 1113to1425by8 1114to1426by8 1115to1427by8 1430to1514by4 \
1431to1515by4 1432to1516by4, 0
TRASVERSI PILE, 51to58 169to174 496to568by24 505to577by24, 2001to2003 \
2005to2007 2009to2014 2017to2022 2025to2030 2033to2038 2041to2043 \
2045to2047, 0
PILE , 1 2 25 26, 4001to4020, 0
NODI , 1 2 21to28 31to38 41to48 51to58 61to72 77to84 87to102 107to134 \
143to154 159to166 169to174 177to293 327to452 489to584 609to696 719to886, \
, 0
CONTROVENTI, 41to48 61to72 87to102 143to154 177to293, 3001to3361by8 \
3002to3362by8 3003to3363by8 3004to3364by8 3005to3365by8 3006to3366by8, 0
SOLETTA FITTIZIA, , 5009to5053by4 5010to5054by4 5011to5055by4 5070to5102by4 \
5071to5103by4 5072to5104by4 5110to5112 5114to5116 5118to5120, 0
    
```

```

*BNDR-GROUP ; Boundary Group
RIGID LINK, 0
APPOGGI SPALLE, 0
SUPPORTI, 0
CONTR.FITTIZI SOLETTA, 0
APPOGGI PILE, 6492008
    
```

```

*LOAD-GROUP ; Load Group
PESO SOLETTA
PESO FINITURE
PESO ACCIAIO E PILE
    
```

```

*MATERIAL ; Material
1, STEEL, S355 (g x1.15) , 0, 0, , C, YES, 0.05, 2, 2.1000e+008, 0.3, 1.2000e-005, 88.53, 9.025
2, CONC , C32/40 SOL.TRASV , 0, 0, , C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e-005, 0, 0
3, CONC , C28/35 PILE , 0, 0, , C, NO, 0.05, 1, NTC08(RC) , , C28/35
4, CONC , C32/40 SOLETTA , 0, 0, , C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e-005, 0, 0
5, SRC , COMPOSITO , 0, 0, , C, YES, 0.05, 2, 2.1000e+008, 0.3, 1.2000e-005, 88.53, 9.02, \
2, 3.3345e+007, 0.2, 1.0000e-005, 0, 0
    
```

```

*MATL-COLOR
1, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
2, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
3, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
4, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
5, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
    
```

```

*TDM-TYPE ; Time Dependent Material
NAME=SOLETTA, European, 32000, 70, 0.3, Class N, 1, 1, NO
    
```

```

*TDM-ELAST ; Time Dependent Material (Comp. Strength)
NAME=SOLETTA, CODE, EUROPEAN, 40000, 2
    
```

```

*TDM-LINK ; Time Dependent Material Link
4, SOLETTA, SOLETTA
    
```

```

*SECTION ; Section
    
```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

```

1, COMPOSITE , C1 Interno          , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2, COMPOSITE , C2 Interno          , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3, COMPOSITE , C1 Esterno          , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
4, COMPOSITE , C2 esterno          , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
5, TAPERED   , C4 Interno Sx       , CT, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0
6, TAPERED   , C4 Interno Dx       , CT, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0
7, COMPOSITE , C5 Interno          , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
8, COMPOSITE , C6 Interno          , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
9, TAPERED   , C4 Esterno Sx       , CT, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0

```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

```

0
0

10, TAPERED , C4 Esterno Dx , CT, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

11, COMPOSITE , C3 Interno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
12, COMPOSITE , C5 Esterno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
13, COMPOSITE , C6 Esterno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
14, COMPOSITE , C3 Esterno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
15, DBUSER , CONTROV 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0,
0, 0, 0, 0
16, COMPOSITE , C4 Interno Centr , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
17, DBUSER , HSUP_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
18, DBUSER , HINF_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
19, DBUSER , HSUP_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,
0, 0, 0, 0
20, DBUSER , DIAG_PILE 2L80x8d20, CC, 0, 1, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.02, 0
, 0, 0, 0, 0
21, COMPOSITE , C4 Esterno Centr , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
22, DBUSER , 330x30 , RT, 0, 0, 1, 0.8, 0, 0, NO, SB , 2, 0.3, 3.3, 0, 0, 0, 0, 0, 0, 0, 0
23, DBUSER , 500x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5, 0, 0, 0, 0, 0, 0, 0, 0
24, DBUSER , 400x30 , LT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0, 0
25, DBUSER , 300x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 3, 0, 0, 0, 0, 0, 0, 0, 0
26, DBUSER , 400x30 dx , RT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0, 0
27, DBUSER , 520x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5.2, 0, 0, 0, 0, 0, 0, 0, 0
46, DBUSER , PILA , CC, 0, 0, 0, 0, 0, 0, YES, STRK, 2, 1.5, 12, 0, 0, 0, 0, 0, 0, 0, 0
64, DBUSER , DIAG_CORR 2L80x8d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.015,
0, 0, 0, 0, 0
65, DBUSER , HINF_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,

```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

0, 0, 0, 0

\*SECT-COLOR

```

1, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
2, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
3, 3, 111, 0, 93, 255, 87, 3, 111, 0, NO, 0.5
4, 70, 70, 0, 192, 255, 0, 70, 70, 0, NO, 0.5
5, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
6, 0, 13, 13, 0, 128, 128, 0, 13, 13, NO, 0.5
7, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
8, 123, 40, 0, 255, 192, 160, 123, 40, 0, NO, 0.5
9, 3, 0, 108, 163, 160, 255, 3, 0, 108, NO, 0.5
10, 3, 111, 66, 163, 255, 160, 3, 111, 66, NO, 0.5
11, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
12, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
13, 0, 72, 72, 0, 192, 192, 0, 72, 72, NO, 0.5
14, 102, 69, 0, 192, 128, 0, 102, 69, 0, NO, 0.5
15, 65, 165, 65, 192, 192, 192, 65, 165, 65, NO, 0.5
16, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
17, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
18, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
19, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
20, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
21, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
22, 70, 70, 0, 192, 255, 0, 70, 70, 0, NO, 0.5
23, 121, 0, 91, 255, 0, 192, 121, 0, 91, NO, 0.5
24, 89, 0, 162, 212, 160, 255, 89, 0, 162, NO, 0.5
25, 0, 43, 19, 0, 128, 57, 0, 43, 19, NO, 0.5
26, 115, 0, 0, 255, 87, 87, 115, 0, 0, NO, 0.5
27, 34, 77, 0, 85, 192, 0, 34, 77, 0, NO, 0.5
46, 85, 32, 0, 192, 72, 0, 85, 32, 0, NO, 0.5
64, 85, 32, 0, 192, 72, 0, 85, 32, 0, NO, 0.5
65, 85, 32, 0, 192, 72, 0, 85, 32, 0, NO, 0.5
    
```

\*COMP-GEN-SECT-PSC-DESIGN ; Composite Section for PSC Design

```

1, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
2, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
3, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
4, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
5, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
6, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
7, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
8, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
9, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
10, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
11, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
12, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
13, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
14, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
16, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
21, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
    
```

\*DGN-SECT

```

1, COMPOSITE , C1 Interno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2, COMPOSITE , C2 Interno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3, COMPOSITE , C1 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
4, COMPOSITE , C2 esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
    
```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

```

0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
5, TAPERED , C4 Interno Sx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

6, TAPERED , C4 Interno Dx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0


7, COMPOSITE , C5 Interno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
8, COMPOSITE , C6 Interno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
9, TAPERED , C4 Esterno Sx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

10, TAPERED , C4 Esterno Dx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

11, COMPOSITE , C3 Interno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
12, COMPOSITE , C5 Esterno , CT, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04

```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

```

0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
13, COMPOSITE , C6 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
14, COMPOSITE , C3 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
15, DBUSER , CONTROV 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0,
0, 0, 0, 0
16, COMPOSITE , C4 Interno Centr , CT, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
17, DBUSER , HSUP_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
18, DBUSER , HINF_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
19, DBUSER , HSUP_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,
0, 0, 0, 0
20, DBUSER , DIAG_PILE 2L80x8d20, CC, 0, 1, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.02, 0
, 0, 0, 0, 0
21, COMPOSITE , C4 Esterno Centr , CT, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
22, DBUSER , 330x30 , RT, 0, 0, 1, 0.8, 0, 0, NO, SB , 2, 0.3, 3.3, 0, 0, 0, 0, 0, 0, 0, 0
23, DBUSER , 500x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5, 0, 0, 0, 0, 0, 0, 0, 0
24, DBUSER , 400x30 , LT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0, 0
25, DBUSER , 300x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 3, 0, 0, 0, 0, 0, 0, 0, 0
26, DBUSER , 400x30 dx , RT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0, 0
27, DBUSER , 520x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5.2, 0, 0, 0, 0, 0, 0, 0, 0
46, DBUSER , PILA , CC, 0, 0, 0, 0, 0, 0, YES, STRK, 2, 1.5, 12, 0, 0, 0, 0, 0, 0, 0
64, DBUSER , DIAG_CORR 2L80x8d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.015,
0, 0, 0, 0, 0
65, DBUSER , HINF_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,
0, 0, 0, 0

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*REBAR-PSC ; Reinforcement of Section
SECT=1, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=2, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=3, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=4, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
SECT=5, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
SECT=6, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
SECT=7, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
SECT=8, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=9, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
SECT=10, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
SECT=11, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=12, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=13, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
SECT=14, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=16, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
SECT=21, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2

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\*TS-GROUP ; Tapered Section Group  
T3 SX, 310 311, LINEAR, , , LINEAR, , , 0

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T3 DX, 314 315, LINEAR, , , , LINEAR, , , , 0  
 T1 SX, 110 111, LINEAR, , , , LINEAR, , , , 0  
 T1 DX, 114 115, LINEAR, , , , LINEAR, , , , 0  
 T2 SX, 210 211, LINEAR, , , , LINEAR, , , , 0  
 T2 DX, 214 215, LINEAR, , , , LINEAR, , , , 0  
 T4 SX, 410 411, LINEAR, , , , LINEAR, , , , 0  
 T4 DX, 414 415, LINEAR, , , , LINEAR, , , , 0  
 T1 SX 2, 126 127, LINEAR, , , , LINEAR, , , , 0  
 T1 DX 2, 130 131, LINEAR, , , , LINEAR, , , , 0  
 T2 SX 2, 226 227, LINEAR, , , , LINEAR, , , , 0  
 T2 DX 2, 230 231, LINEAR, , , , LINEAR, , , , 0  
 T3 SX 2, 326 327, LINEAR, , , , LINEAR, , , , 0  
 T3 DX 2, 330 331, LINEAR, , , , LINEAR, , , , 0  
 T4 SX 2, 426 427, LINEAR, , , , LINEAR, , , , 0  
 T4 DX 2, 430 431, LINEAR, , , , LINEAR, , , , 0

\*STLDCASE ; Static Load Cases

Peso acciaio e pile, D ,  
 Peso soletta, D ,  
 Finiture, DW,  
 Vento strutture (Y+), W ,  
 Vento mobili (Y+), WL,  
 Frenamento (X+), BRK,  
 Termico Unif. +15°, T ,  
 Termico diff. +5°/H, TPG,

\*DGN-CTRL ; General Design Data

NO, NO, YES, , , , , 0, 3D, YES, NO, NO, 0, 1, NO, 2

\*DGN-STEEL ; Steel Design Code

CODE=Eurocode3-2:05, NO, 0  
 1.05, 1.1, 1.25, 0, 0

\*CONSTRAINT ; Supports

1 2 883 884, 111111, SUPPORTI

\*ELASTICLINK

185, 77, 159, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 186, 79, 161, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 187, 81, 163, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 188, 83, 165, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 190, 78, 160, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 191, 80, 162, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 192, 82, 164, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 193, 84, 166, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE  
 195, 21, 31, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE  
 196, 23, 33, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE  
 197, 885, 35, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE  
 198, 27, 37, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE  
 200, 22, 32, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE  
 201, 24, 34, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE  
 202, 886, 36, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE  
 203, 28, 38, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE

\*MEMBERTYPE ; Modify Member Type

1014to1019 1022to1027 1030to1035 1038to1043 1110to1422by8 , BRACE, 0  
 1111to1423by8 1112to1424by8 1113to1425by8 1114to1426by8 , BRACE, 0  
 1115to1427by8 1430to1514by4 1431to1515by4 1432to1516by4 2001to2003 , BRACE, 0  
 2005to2007 2009to2014 2017to2022 2025to2030 2033to2038 2041to2043 , BRACE, 0  
 2045to2047 3001to3361by8 3002to3362by8 3003to3363by8 3004to3364by8 , BRACE, 0  
 3005to3365by8 3006to3366by8, BRACE, 0

\*RIGIDLINK ; Rigid Link

25, 111111, 21 23 27 885, RIGID LINK  
 26, 111111, 22 24 28 886, RIGID LINK  
 720, 111111, 159 177 327 489 609, RIGID LINK  
 721, 111111, 178 328 490 610, RIGID LINK  
 723, 111111, 179 329 491 611, RIGID LINK  
 724, 111111, 180 330 492 612, RIGID LINK  
 726, 111111, 181 331 493 613, RIGID LINK  
 727, 111111, 182 332 494 614, RIGID LINK  
 729, 111111, 87 107 495 615, RIGID LINK  
 731, 111111, 31 41 51 496, RIGID LINK  
 733, 111111, 88 108 497 616, RIGID LINK  
 735, 111111, 183 333 498 617, RIGID LINK  
 736, 111111, 184 334 499 618, RIGID LINK  
 738, 111111, 185 335 500 619, RIGID LINK

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
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740, 111111, 186 336 501 620, RIGID LINK  
 742, 111111, 187 337 502 621, RIGID LINK  
 743, 111111, 188 338 503 622, RIGID LINK  
 745, 111111, 89 109 504 623, RIGID LINK  
 747, 111111, 32 42 52 505, RIGID LINK  
 749, 111111, 90 110 506 624, RIGID LINK  
 751, 111111, 189 339 507 625, RIGID LINK  
 752, 111111, 190 340 508 626, RIGID LINK  
 754, 111111, 191 341 509 627, RIGID LINK  
 755, 111111, 192 342 510 628, RIGID LINK  
 757, 111111, 193 343 511 629, RIGID LINK  
 758, 111111, 160 194 344 512 630, RIGID LINK  
 761, 111111, 161 210 363 513 631, RIGID LINK  
 762, 111111, 211 364 514 632, RIGID LINK  
 764, 111111, 212 365 515 633, RIGID LINK  
 765, 111111, 213 366 516 634, RIGID LINK  
 767, 111111, 214 367 517 635, RIGID LINK  
 768, 111111, 215 368 518 636, RIGID LINK  
 770, 111111, 91 115 519 637, RIGID LINK  
 772, 111111, 33 43 53 520, RIGID LINK  
 774, 111111, 92 116 521 638, RIGID LINK  
 776, 111111, 216 369 522 639, RIGID LINK  
 777, 111111, 217 370 523 640, RIGID LINK  
 779, 111111, 218 371 524 641, RIGID LINK  
 781, 111111, 219 372 525 642, RIGID LINK  
 783, 111111, 220 373 526 643, RIGID LINK  
 784, 111111, 221 374 527 644, RIGID LINK  
 786, 111111, 93 117 528 645, RIGID LINK  
 788, 111111, 34 44 54 529, RIGID LINK  
 790, 111111, 94 118 530 646, RIGID LINK  
 792, 111111, 222 375 531 647, RIGID LINK  
 793, 111111, 223 376 532 648, RIGID LINK  
 795, 111111, 224 377 533 649, RIGID LINK  
 796, 111111, 225 378 534 650, RIGID LINK  
 798, 111111, 226 379 535 651, RIGID LINK  
 799, 111111, 162 227 380 536 652, RIGID LINK  
 802, 111111, 163 243 399 537 653, RIGID LINK  
 803, 111111, 244 400 538 654, RIGID LINK  
 805, 111111, 245 401 539 655, RIGID LINK  
 806, 111111, 246 402 540 656, RIGID LINK  
 808, 111111, 247 403 541 657, RIGID LINK  
 809, 111111, 248 404 542 658, RIGID LINK  
 811, 111111, 95 123 543 659, RIGID LINK  
 813, 111111, 35 45 55 544, RIGID LINK  
 815, 111111, 96 124 545 660, RIGID LINK  
 817, 111111, 249 405 546 661, RIGID LINK  
 818, 111111, 250 406 547 662, RIGID LINK  
 820, 111111, 251 407 548 663, RIGID LINK  
 822, 111111, 252 408 549 664, RIGID LINK  
 824, 111111, 253 409 550 665, RIGID LINK  
 825, 111111, 254 410 551 666, RIGID LINK  
 827, 111111, 97 125 552 667, RIGID LINK  
 829, 111111, 36 46 56 553, RIGID LINK  
 831, 111111, 98 126 554 668, RIGID LINK  
 833, 111111, 255 411 555 669, RIGID LINK  
 834, 111111, 256 412 556 670, RIGID LINK  
 836, 111111, 257 413 557 671, RIGID LINK  
 837, 111111, 258 414 558 672, RIGID LINK  
 839, 111111, 259 415 559 673, RIGID LINK  
 840, 111111, 164 260 416 560 674, RIGID LINK  
 843, 111111, 165 276 435 561 675, RIGID LINK  
 844, 111111, 277 436 562 676, RIGID LINK  
 846, 111111, 278 437 563 677, RIGID LINK  
 847, 111111, 279 438 564 678, RIGID LINK  
 849, 111111, 280 439 565 679, RIGID LINK  
 850, 111111, 281 440 566 680, RIGID LINK  
 852, 111111, 99 131 567 681, RIGID LINK  
 854, 111111, 37 47 57 568, RIGID LINK  
 856, 111111, 100 132 569 682, RIGID LINK  
 858, 111111, 282 441 570 683, RIGID LINK  
 859, 111111, 283 442 571 684, RIGID LINK  
 861, 111111, 284 443 572 685, RIGID LINK  
 863, 111111, 285 444 573 686, RIGID LINK  
 865, 111111, 286 445 574 687, RIGID LINK  
 866, 111111, 287 446 575 688, RIGID LINK  
 868, 111111, 101 133 576 689, RIGID LINK  
 870, 111111, 38 48 58 577, RIGID LINK  
 872, 111111, 102 134 578 690, RIGID LINK

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	Company		Client	
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874, 111111, 288 447 579 691, RIGID LINK  
 875, 111111, 289 448 580 692, RIGID LINK  
 877, 111111, 290 449 581 693, RIGID LINK  
 878, 111111, 291 450 582 694, RIGID LINK  
 880, 111111, 292 451 583 695, RIGID LINK  
 881, 111111, 166 293 452 584 696, RIGID LINK  
 883, 111111, 77to83by2, RIGID LINK  
 884, 111111, 78to84by2, RIGID LINK

\*LOADTOMASS, XYZ, YES, YES, YES, YES, 9.806  
 Peso soletta, 1, Finiture, 1

\*USE-STLD, Peso acciaio e pile

\*SELFWEIGHT, 0, 0, -1, PESO ACCIAIO E PILE

; End of data for load case [Peso acciaio e pile] -----

\*USE-STLD, Peso soletta

\*BEAMLOAD ; Element Beam Loads

101, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 102, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 103, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 104, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 105, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 106, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 107, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 108, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 109, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 110, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 111, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 112, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 113, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 114, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 115, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 116, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 117, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 118, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 119, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 120, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 121, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 122, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 123, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 124, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 125, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 126, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 127, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 128, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,  
 0, NO  
 129, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,

PROJECT TITLE : CASALPUSTERLENGO

	Company	Client	
	Author	File Name	
	Renato Vaira	Cavalc Costa V_2.mct	

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0, NO
130, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
131, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
132, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
133, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
134, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
135, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
136, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
137, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
138, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
139, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
140, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
201, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
202, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
203, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
204, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
205, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
206, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
207, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
208, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
209, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
210, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
211, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
212, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
213, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
214, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
215, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
216, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
217, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
218, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
219, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
220, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
221, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
222, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
223, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
224, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
225, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
226, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
227, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
228, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -29.6, 1, -29.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0


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PROJECT TITLE : CASALPUSTERLENGO

	Company	Client	
	Author	File Name	
	Renato Vaira	Cavalc Costa V_2.mct	

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0, NO
427, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
428, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
429, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
430, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
431, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
432, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
433, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
434, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
435, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
436, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
437, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
438, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
439, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
440, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.23, 1, -23.23, 0, 0, 0, 0, PESO SOLETTA, NO, 0,
0, NO
    
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; End of data for load case [Peso soletta] -----

\*USE-STLD, Finiture

\*BEAMLOAD ; Element Beam Loads


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101, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
102, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
103, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
104, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
105, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
106, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
107, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
108, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
109, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
110, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
111, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
112, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
113, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
114, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
115, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
116, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
117, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
118, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
119, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
120, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
121, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
122, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.25, -0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINI
    
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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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RE, NO, 0, 0, NO
421, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
422, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
423, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
424, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
425, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
426, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
427, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
428, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
429, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
430, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
431, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
432, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
433, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
434, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
435, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
436, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
437, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
438, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
439, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
440, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.25, 0.25, NO, 0, -13.06, 1, -13.06, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO

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; End of data for load case [Finiture] -----

\*USE-STLD, Vento strutture (Y+)

\*BEAMLOAD ; Element Beam Loads

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101, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
102, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
103, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
104, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
105, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
106, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
107, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
108, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
109, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
110, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.51, YES, 0, 3.03, 1, 3.71, 0, 0, 0, 0, , NO, 0, 0,
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111, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.51, -0.76, YES, 0, 3.71, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0,
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112, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.76, NO, 0, 4.38, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0, N
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114, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.51, YES, 0, 4.38, 1, 3.71, 0, 0, 0, 0, , NO, 0, 0,
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115, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.51, -0.25, YES, 0, 3.71, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0,
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PROJECT TITLE : CASALPUSTERLENGO

	Company	Client	
	Author	File Name	
	Renato Vaira	Cavalc Costa V_2.mct	

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424, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
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; End of data for load case [Frenamento (X+)] -----

\*USE-STLD, Termico Unif. +15°

\*ELTEMPER ; Element Temperatures

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct


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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct


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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	Company	Client	
	Author	File Name	
		Renato Vaira	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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3324, 15,  
3325, 15,  
3326, 15,  
3329, 15,  
3330, 15,  
3331, 15,  
3332, 15,  
3333, 15,  
3334, 15,  
3337, 15,  
3338, 15,  
3339, 15,  
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3342, 15,  
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3346, 15,  
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3348, 15,  
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3355, 15,  
3356, 15,  
3357, 15,  
3358, 15,  
3361, 15,  
3362, 15,  
3363, 15,  
3364, 15,  
3365, 15,  
3366, 15,

; End of data for load case [Termico Unif. +15°] -----

\*USE-STLD, Termico diff. +5°/H

\*THERGRAD ; Temperature Gradient

101, 1, 5, YES, 0, 0, YES, 0,  
102, 1, 5, YES, 0, 0, YES, 0,  
103, 1, 5, YES, 0, 0, YES, 0,  
104, 1, 5, YES, 0, 0, YES, 0,  
105, 1, 5, YES, 0, 0, YES, 0,  
106, 1, 5, YES, 0, 0, YES, 0,  
107, 1, 5, YES, 0, 0, YES, 0,  
108, 1, 5, YES, 0, 0, YES, 0,  
109, 1, 5, YES, 0, 0, YES, 0,  
110, 1, 5, YES, 0, 0, YES, 0,  
111, 1, 5, YES, 0, 0, YES, 0,  
112, 1, 5, YES, 0, 0, YES, 0,  
113, 1, 5, YES, 0, 0, YES, 0,  
114, 1, 5, YES, 0, 0, YES, 0,  
115, 1, 5, YES, 0, 0, YES, 0,  
116, 1, 5, YES, 0, 0, YES, 0,  
117, 1, 5, YES, 0, 0, YES, 0,  
118, 1, 5, YES, 0, 0, YES, 0,  
119, 1, 5, YES, 0, 0, YES, 0,  
120, 1, 5, YES, 0, 0, YES, 0,  
121, 1, 5, YES, 0, 0, YES, 0,  
122, 1, 5, YES, 0, 0, YES, 0,  
123, 1, 5, YES, 0, 0, YES, 0,  
124, 1, 5, YES, 0, 0, YES, 0,  
125, 1, 5, YES, 0, 0, YES, 0,  
126, 1, 5, YES, 0, 0, YES, 0,  
127, 1, 5, YES, 0, 0, YES, 0,

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

128, 1, 5, YES, 0, 0, YES, 0,  
 129, 1, 5, YES, 0, 0, YES, 0,  
 130, 1, 5, YES, 0, 0, YES, 0,  
 131, 1, 5, YES, 0, 0, YES, 0,  
 132, 1, 5, YES, 0, 0, YES, 0,  
 133, 1, 5, YES, 0, 0, YES, 0,  
 134, 1, 5, YES, 0, 0, YES, 0,  
 135, 1, 5, YES, 0, 0, YES, 0,  
 136, 1, 5, YES, 0, 0, YES, 0,  
 137, 1, 5, YES, 0, 0, YES, 0,  
 138, 1, 5, YES, 0, 0, YES, 0,  
 139, 1, 5, YES, 0, 0, YES, 0,  
 140, 1, 5, YES, 0, 0, YES, 0,  
 201, 1, 5, YES, 0, 0, YES, 0,  
 202, 1, 5, YES, 0, 0, YES, 0,  
 203, 1, 5, YES, 0, 0, YES, 0,  
 204, 1, 5, YES, 0, 0, YES, 0,  
 205, 1, 5, YES, 0, 0, YES, 0,  
 206, 1, 5, YES, 0, 0, YES, 0,  
 207, 1, 5, YES, 0, 0, YES, 0,  
 208, 1, 5, YES, 0, 0, YES, 0,  
 209, 1, 5, YES, 0, 0, YES, 0,  
 210, 1, 5, YES, 0, 0, YES, 0,  
 211, 1, 5, YES, 0, 0, YES, 0,  
 212, 1, 5, YES, 0, 0, YES, 0,  
 213, 1, 5, YES, 0, 0, YES, 0,  
 214, 1, 5, YES, 0, 0, YES, 0,  
 215, 1, 5, YES, 0, 0, YES, 0,  
 216, 1, 5, YES, 0, 0, YES, 0,  
 217, 1, 5, YES, 0, 0, YES, 0,  
 218, 1, 5, YES, 0, 0, YES, 0,  
 219, 1, 5, YES, 0, 0, YES, 0,  
 220, 1, 5, YES, 0, 0, YES, 0,  
 221, 1, 5, YES, 0, 0, YES, 0,  
 222, 1, 5, YES, 0, 0, YES, 0,  
 223, 1, 5, YES, 0, 0, YES, 0,  
 224, 1, 5, YES, 0, 0, YES, 0,  
 225, 1, 5, YES, 0, 0, YES, 0,  
 226, 1, 5, YES, 0, 0, YES, 0,  
 227, 1, 5, YES, 0, 0, YES, 0,  
 228, 1, 5, YES, 0, 0, YES, 0,  
 229, 1, 5, YES, 0, 0, YES, 0,  
 230, 1, 5, YES, 0, 0, YES, 0,  
 231, 1, 5, YES, 0, 0, YES, 0,  
 232, 1, 5, YES, 0, 0, YES, 0,  
 233, 1, 5, YES, 0, 0, YES, 0,  
 234, 1, 5, YES, 0, 0, YES, 0,  
 235, 1, 5, YES, 0, 0, YES, 0,  
 236, 1, 5, YES, 0, 0, YES, 0,  
 237, 1, 5, YES, 0, 0, YES, 0,  
 238, 1, 5, YES, 0, 0, YES, 0,  
 239, 1, 5, YES, 0, 0, YES, 0,  
 240, 1, 5, YES, 0, 0, YES, 0,  
 301, 1, 5, YES, 0, 0, YES, 0,  
 302, 1, 5, YES, 0, 0, YES, 0,  
 303, 1, 5, YES, 0, 0, YES, 0,  
 304, 1, 5, YES, 0, 0, YES, 0,  
 305, 1, 5, YES, 0, 0, YES, 0,  
 306, 1, 5, YES, 0, 0, YES, 0,  
 307, 1, 5, YES, 0, 0, YES, 0,  
 308, 1, 5, YES, 0, 0, YES, 0,  
 309, 1, 5, YES, 0, 0, YES, 0,  
 310, 1, 5, YES, 0, 0, YES, 0,  
 311, 1, 5, YES, 0, 0, YES, 0,  
 312, 1, 5, YES, 0, 0, YES, 0,  
 313, 1, 5, YES, 0, 0, YES, 0,  
 314, 1, 5, YES, 0, 0, YES, 0,  
 315, 1, 5, YES, 0, 0, YES, 0,  
 316, 1, 5, YES, 0, 0, YES, 0,  
 317, 1, 5, YES, 0, 0, YES, 0,  
 318, 1, 5, YES, 0, 0, YES, 0,  
 319, 1, 5, YES, 0, 0, YES, 0,  
 320, 1, 5, YES, 0, 0, YES, 0,  
 321, 1, 5, YES, 0, 0, YES, 0,  
 322, 1, 5, YES, 0, 0, YES, 0,  
 323, 1, 5, YES, 0, 0, YES, 0,  
 324, 1, 5, YES, 0, 0, YES, 0,  
 325, 1, 5, YES, 0, 0, YES, 0,

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

```

326, 1, 5, YES, 0, 0, YES, 0,
327, 1, 5, YES, 0, 0, YES, 0,
328, 1, 5, YES, 0, 0, YES, 0,
329, 1, 5, YES, 0, 0, YES, 0,
330, 1, 5, YES, 0, 0, YES, 0,
331, 1, 5, YES, 0, 0, YES, 0,
332, 1, 5, YES, 0, 0, YES, 0,
333, 1, 5, YES, 0, 0, YES, 0,
334, 1, 5, YES, 0, 0, YES, 0,
335, 1, 5, YES, 0, 0, YES, 0,
336, 1, 5, YES, 0, 0, YES, 0,
337, 1, 5, YES, 0, 0, YES, 0,
338, 1, 5, YES, 0, 0, YES, 0,
339, 1, 5, YES, 0, 0, YES, 0,
340, 1, 5, YES, 0, 0, YES, 0,
401, 1, 5, YES, 0, 0, YES, 0,
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403, 1, 5, YES, 0, 0, YES, 0,
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408, 1, 5, YES, 0, 0, YES, 0,
409, 1, 5, YES, 0, 0, YES, 0,
410, 1, 5, YES, 0, 0, YES, 0,
411, 1, 5, YES, 0, 0, YES, 0,
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414, 1, 5, YES, 0, 0, YES, 0,
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416, 1, 5, YES, 0, 0, YES, 0,
417, 1, 5, YES, 0, 0, YES, 0,
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419, 1, 5, YES, 0, 0, YES, 0,
420, 1, 5, YES, 0, 0, YES, 0,
421, 1, 5, YES, 0, 0, YES, 0,
422, 1, 5, YES, 0, 0, YES, 0,
423, 1, 5, YES, 0, 0, YES, 0,
424, 1, 5, YES, 0, 0, YES, 0,
425, 1, 5, YES, 0, 0, YES, 0,
426, 1, 5, YES, 0, 0, YES, 0,
427, 1, 5, YES, 0, 0, YES, 0,
428, 1, 5, YES, 0, 0, YES, 0,
429, 1, 5, YES, 0, 0, YES, 0,
430, 1, 5, YES, 0, 0, YES, 0,
431, 1, 5, YES, 0, 0, YES, 0,
432, 1, 5, YES, 0, 0, YES, 0,
433, 1, 5, YES, 0, 0, YES, 0,
434, 1, 5, YES, 0, 0, YES, 0,
435, 1, 5, YES, 0, 0, YES, 0,
436, 1, 5, YES, 0, 0, YES, 0,
437, 1, 5, YES, 0, 0, YES, 0,
438, 1, 5, YES, 0, 0, YES, 0,
439, 1, 5, YES, 0, 0, YES, 0,
440, 1, 5, YES, 0, 0, YES, 0,

```

; End of data for load case [Termico diff. +5°/H] -----

\*STAGE ; Define Construction Stage

```

NAME=Costruzione, 30, YES, NO
STEP=3, 10
AELEM=TRAVI PRINCIPALI, 1, TRASVERSI INTERMEDI, 1, TRASVERSI PILE, 1, PILE, 1, NODI, 1
      CONTROVENTI, 1
ABNDR=RIGID LINK, DEFORMED, APPOGGI SPALLE, DEFORMED, SUPPORTI, DEFORMED
      APPOGGI PILE, DEFORMED
ALOAD=PESO SOLETTA, LAST, PESO ACCIAIO E PILE, FIRST
NAME=Esercizio, 10000, YES, NO
STEP=3, 10, 32, 100, 316, 1000, 3162
AELEM=SOLETTA FITTIZIA, 1
ALOAD=PESO FINITURE, FIRST

```

\*SFUNCTION ; Spectrum Function

```

FUNC=SLD Orizz, 1, 0, 1, 9.806, 0.05, , 1.000000
USER
      0.000000,      0.06887,      0.140750,      0.17485
      0.422250,      0.17485,      0.487080,      0.15158
      0.551910,      0.13377,      0.616740,      0.11971
      0.681570,      0.10832,      0.746390,      0.09891

```



PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

```

0.811220,      0.09101,      0.876050,      0.08427
0.940880,      0.07847,      1.005710,      0.07341
1.070540,      0.06896,      1.135370,      0.06503
1.171200,      0.06304,      1.172200,      0.05143
1.200200,      0.05023,      1.265030,      0.04765
1.329860,      0.04533,      1.394690,      0.04322
1.459520,      0.0413,       1.524340,      0.03955
1.589170,      0.03793,      1.654000,      0.03645
1.718830,      0.03507,      1.783660,      0.0338
1.889200,      0.03013,      1.994740,      0.02702
2.100280,      0.02437,      2.205820,      0.0221
2.311360,      0.02013,      2.416900,      0.01841
2.522440,      0.0169,       2.627980,      0.01557
2.733520,      0.01439,      2.839060,      0.01334
2.944600,      0.0124,       3.050140,      0.01156
3.155680,      0.0108,       3.261220,      0.01011
3.366760,      0.00949,      3.472300,      0.00892
3.577840,      0.0084,       3.683380,      0.00793
3.788920,      0.00749,      3.894460,      0.00709
4.000000,      0.00672

```

FUNC=SLD Vert, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000,      0.01328,      0.050000,      0.03372
0.150000,      0.03372,      0.235000,      0.02152
0.320000,      0.01581,      0.405000,      0.01249
0.490000,      0.01032,      0.575000,      0.00888
0.660000,      0.00766,      0.745000,      0.00679
0.830000,      0.00609,      0.915000,      0.00553
1.000000,      0.00506,      1.094000,      0.00423
1.188000,      0.00359,      1.281000,      0.00308
1.375000,      0.00268,      1.469000,      0.00234
1.563000,      0.00207,      1.656000,      0.00184
1.750000,      0.00165,      1.844000,      0.00149
1.938000,      0.00135,      2.031000,      0.00123
2.125000,      0.00112,      2.219000,      0.00103
2.313000,      0.00095,      2.406000,      0.00087
2.500000,      0.00081,      2.594000,      0.00075
2.688000,      0.0007,      2.781000,      0.00065
2.875000,      0.00061,      2.969000,      0.00057
3.063000,      0.00054,      3.156000,      0.00051
3.250000,      0.00048,      3.344000,      0.00045
3.438000,      0.00043,      3.531000,      0.00041
3.625000,      0.00038,      3.719000,      0.00037
3.813000,      0.00035,      3.906000,      0.00033
4.000000,      0.00032

```

FUNC=SLV Orizz, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000,      0.15104,      0.155180,      0.38361
0.465530,      0.38361,      0.538730,      0.33148
0.611930,      0.29183,      0.685130,      0.26065
0.758330,      0.23549,      0.831540,      0.21476
0.904740,      0.19738,      0.977940,      0.18261
1.051140,      0.16989,      1.124350,      0.15883
1.171200,      0.15248,      1.172200,      0.12439
1.197550,      0.12176,      1.270750,      0.11474
1.343950,      0.10849,      1.417150,      0.10289
1.490360,      0.09784,      1.563560,      0.09326
1.636760,      0.08908,      1.709960,      0.08527
1.783170,      0.08177,      1.856370,      0.07855
1.929570,      0.07557,      2.002770,      0.0728
2.097880,      0.06635,      2.192980,      0.06072
2.288090,      0.05578,      2.383200,      0.05142
2.478300,      0.04755,      2.573410,      0.0441
2.668510,      0.04101,      2.763620,      0.03824
2.858730,      0.03573,      2.953830,      0.03347
3.048940,      0.03141,      3.144050,      0.02954
3.239150,      0.02783,      3.334260,      0.02627
3.429360,      0.02483,      3.524470,      0.02351
3.619580,      0.02229,      3.714680,      0.02116
3.809790,      0.02012,      3.904890,      0.01915
4.000000,      0.01825

```

FUNC=SLV Vert, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000,      0.04314,      0.050000,      0.10955
0.150000,      0.10955,      0.235000,      0.06993
0.320000,      0.05135,      0.405000,      0.04058
0.490000,      0.03354,      0.575000,      0.02858
0.660000,      0.0249,       0.745000,      0.02206

```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

```

0.830000, 0.0198, 0.915000, 0.01796
1.000000, 0.01643, 1.094000, 0.01374
1.188000, 0.01165, 1.281000, 0.01001
1.375000, 0.00869, 1.469000, 0.00762
1.563000, 0.00673, 1.656000, 0.00599
1.750000, 0.00537, 1.844000, 0.00483
1.938000, 0.00438, 2.031000, 0.00398
2.125000, 0.00364, 2.219000, 0.00334
2.313000, 0.00307, 2.406000, 0.00284
2.500000, 0.00263, 2.594000, 0.00244
2.688000, 0.00228, 2.781000, 0.00212
2.875000, 0.00199, 2.969000, 0.00186
3.063000, 0.00175, 3.156000, 0.00165
3.250000, 0.00156, 3.344000, 0.00147
3.438000, 0.00139, 3.531000, 0.00132
3.625000, 0.00125, 3.719000, 0.00119
3.813000, 0.00113, 3.906000, 0.00108
4.000000, 0.00103

```

FUNC=SLC Orizz, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000, 0.19189, 0.157800, 0.48457
0.473410, 0.48457, 0.551420, 0.41601
0.629430, 0.36445, 0.707450, 0.32426
0.785460, 0.29205, 0.863470, 0.26567
0.941490, 0.24365, 1.019500, 0.22501
1.097520, 0.20901, 1.171200, 0.19586
1.172200, 0.15979, 1.175530, 0.15933
1.253540, 0.14942, 1.331560, 0.14066
1.409570, 0.13288, 1.487590, 0.12591
1.565600, 0.11964, 1.643610, 0.11396
1.721630, 0.10879, 1.799640, 0.10408
1.877660, 0.09975, 1.955670, 0.09577
2.033680, 0.0921, 2.111700, 0.0887
2.201620, 0.0816, 2.291540, 0.07532
2.381460, 0.06974, 2.471380, 0.06476
2.561290, 0.06029, 2.651210, 0.05627
2.741130, 0.05264, 2.831050, 0.04935
2.920970, 0.04636, 3.010890, 0.04363
3.100810, 0.04114, 3.190730, 0.03885
3.280650, 0.03675, 3.370570, 0.03481
3.460490, 0.03303, 3.550400, 0.03138
3.640320, 0.02985, 3.730240, 0.02842
3.820160, 0.0271, 3.910080, 0.02587
4.000000, 0.02472

```

FUNC=SLC Vert, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000, 0.06177, 0.050000, 0.15598
0.150000, 0.15598, 0.235000, 0.09956
0.320000, 0.07312, 0.405000, 0.05777
0.490000, 0.04775, 0.575000, 0.04069
0.660000, 0.03545, 0.745000, 0.03141
0.830000, 0.02819, 0.915000, 0.02557
1.000000, 0.0234, 1.094000, 0.01956
1.188000, 0.01659, 1.281000, 0.01425
1.375000, 0.01238, 1.469000, 0.01085
1.563000, 0.00958, 1.656000, 0.00853
1.750000, 0.00764, 1.844000, 0.00688
1.938000, 0.00623, 2.031000, 0.00567
2.125000, 0.00518, 2.219000, 0.00475
2.313000, 0.00438, 2.406000, 0.00404
2.500000, 0.00374, 2.594000, 0.00348
2.688000, 0.00324, 2.781000, 0.00302
2.875000, 0.00283, 2.969000, 0.00265
3.063000, 0.00249, 3.156000, 0.00235
3.250000, 0.00222, 3.344000, 0.00209
3.438000, 0.00198, 3.531000, 0.00188
3.625000, 0.00178, 3.719000, 0.00169
3.813000, 0.00161, 3.906000, 0.00153
4.000000, 0.00146

```

\*SPLDCASE ; Spectrum Load Cases

```

NAME=SLD Long (X+), XY, 0, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLD Orizz
NAME=SLD Trasv (Y+), XY, 90, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLD Orizz
NAME=SLD Vert (Z+), Z, 0, 1, 1, NO, NO, LOG,

```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

CQC, YES, 0, NO  
 SLD Vert  
 NAME=SLV Long (X+), XY, 0, 1, 1, NO, NO, LOG,  
 CQC, YES, 0, NO  
 SLV Orizz  
 NAME=SLV Trasv (Y+), XY, 90, 1, 1, NO, NO, LOG,  
 CQC, YES, 0, NO  
 SLV Orizz  
 NAME=SLV Vert (Z+), Z, 0, 1, 1, NO, NO, LOG,  
 CQC, YES, 0, NO  
 SLV Vert  
 NAME=SLC Long (X+), XY, 0, 1, 1, NO, NO, LOG,  
 CQC, YES, 0, NO  
 SLC Orizz  
 NAME=SLC Trasv (Y+), XY, 90, 1, 1, NO, NO, LOG,  
 CQC, YES, 0, NO  
 SLC Orizz  
 NAME=SLC Vert (Z+), Z, 0, 1, 1, NO, NO, LOG,  
 CQC, YES, 0, NO  
 SLC Vert

\*MVLDCODE ; Moving Load Code  
 CODE=EUROCODE

\*LINELANE ; Traffic Line Lanes

NAME=C. LENTA DX, LANE, , 0, 0, BOTH, 3, 2  
 201, 0.025, 0, NO, 0, 202, 0.025, 0, NO, 0, 203, 0.025, 0, NO, 0  
 204, 0.025, 0, NO, 0, 205, 0.025, 0, NO, 0, 206, 0.025, 0, NO, 0  
 207, 0.025, 0, NO, 0, 208, 0.025, 0, NO, 0, 209, 0.025, 0, NO, 0  
 210, 0.025, 0, NO, 0, 211, 0.025, 0, NO, 0, 212, 0.025, 0, NO, 0  
 213, 0.025, 0, NO, 0, 214, 0.025, 0, NO, 0, 215, 0.025, 0, NO, 0  
 216, 0.025, 0, NO, 0, 217, 0.025, 0, NO, 0, 218, 0.025, 0, NO, 0  
 219, 0.025, 0, NO, 0, 220, 0.025, 0, NO, 0, 221, 0.025, 0, NO, 0  
 222, 0.025, 0, NO, 0, 223, 0.025, 0, NO, 0, 224, 0.025, 0, NO, 0  
 225, 0.025, 0, NO, 0, 226, 0.025, 0, NO, 0, 227, 0.025, 0, NO, 0  
 228, 0.025, 0, NO, 0, 229, 0.025, 0, NO, 0, 230, 0.025, 0, NO, 0  
 231, 0.025, 0, NO, 0, 232, 0.025, 0, NO, 0, 233, 0.025, 0, NO, 0  
 234, 0.025, 0, NO, 0, 235, 0.025, 0, NO, 0, 236, 0.025, 0, NO, 0  
 237, 0.025, 0, NO, 0, 238, 0.025, 0, NO, 0, 239, 0.025, 0, NO, 0  
 240, 0.025, 0, NO, 0

NAME=C. LENTA SX, LANE, , 0, 0, BOTH, 3, 2  
 301, -0.025, 0, NO, 0, 302, -0.025, 0, NO, 0  
 303, -0.025, 0, NO, 0, 304, -0.025, 0, NO, 0  
 305, -0.025, 0, NO, 0, 306, -0.025, 0, NO, 0  
 307, -0.025, 0, NO, 0, 308, -0.025, 0, NO, 0  
 309, -0.025, 0, NO, 0, 310, -0.025, 0, NO, 0  
 311, -0.025, 0, NO, 0, 312, -0.025, 0, NO, 0  
 313, -0.025, 0, NO, 0, 314, -0.025, 0, NO, 0  
 315, -0.025, 0, NO, 0, 316, -0.025, 0, NO, 0  
 317, -0.025, 0, NO, 0, 318, -0.025, 0, NO, 0  
 319, -0.025, 0, NO, 0, 320, -0.025, 0, NO, 0  
 321, -0.025, 0, NO, 0, 322, -0.025, 0, NO, 0  
 323, -0.025, 0, NO, 0, 324, -0.025, 0, NO, 0  
 325, -0.025, 0, NO, 0, 326, -0.025, 0, NO, 0  
 327, -0.025, 0, NO, 0, 328, -0.025, 0, NO, 0  
 329, -0.025, 0, NO, 0, 330, -0.025, 0, NO, 0  
 331, -0.025, 0, NO, 0, 332, -0.025, 0, NO, 0  
 333, -0.025, 0, NO, 0, 334, -0.025, 0, NO, 0  
 335, -0.025, 0, NO, 0, 336, -0.025, 0, NO, 0  
 337, -0.025, 0, NO, 0, 338, -0.025, 0, NO, 0  
 339, -0.025, 0, NO, 0, 340, -0.025, 0, NO, 0

NAME=DX 1, LANE, , 0, 0, BOTH, 3, 2  
 201, 1.15, 0, NO, 0, 202, 1.15, 0, NO, 0, 203, 1.15, 0, NO, 0  
 204, 1.15, 0, NO, 0, 205, 1.15, 0, NO, 0, 206, 1.15, 0, NO, 0  
 207, 1.15, 0, NO, 0, 208, 1.15, 0, NO, 0, 209, 1.15, 0, NO, 0  
 210, 1.15, 0, NO, 0, 211, 1.15, 0, NO, 0, 212, 1.15, 0, NO, 0  
 213, 1.15, 0, NO, 0, 214, 1.15, 0, NO, 0, 215, 1.15, 0, NO, 0  
 216, 1.15, 0, NO, 0, 217, 1.15, 0, NO, 0, 218, 1.15, 0, NO, 0  
 219, 1.15, 0, NO, 0, 220, 1.15, 0, NO, 0, 221, 1.15, 0, NO, 0  
 222, 1.15, 0, NO, 0, 223, 1.15, 0, NO, 0, 224, 1.15, 0, NO, 0  
 225, 1.15, 0, NO, 0, 226, 1.15, 0, NO, 0, 227, 1.15, 0, NO, 0  
 228, 1.15, 0, NO, 0, 229, 1.15, 0, NO, 0, 230, 1.15, 0, NO, 0  
 231, 1.15, 0, NO, 0, 232, 1.15, 0, NO, 0, 233, 1.15, 0, NO, 0  
 234, 1.15, 0, NO, 0, 235, 1.15, 0, NO, 0, 236, 1.15, 0, NO, 0  
 237, 1.15, 0, NO, 0, 238, 1.15, 0, NO, 0, 239, 1.15, 0, NO, 0  
 240, 1.15, 0, NO, 0

NAME=DX 2, LANE, , 0, 0, BOTH, 3, 2  
 301, 1.35, 0, NO, 0, 302, 1.35, 0, NO, 0, 303, 1.35, 0, NO, 0

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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304, 1.35, 0, NO, 0, 305, 1.35, 0, NO, 0, 306, 1.35, 0, NO, 0
307, 1.35, 0, NO, 0, 308, 1.35, 0, NO, 0, 309, 1.35, 0, NO, 0
310, 1.35, 0, NO, 0, 311, 1.35, 0, NO, 0, 312, 1.35, 0, NO, 0
313, 1.35, 0, NO, 0, 314, 1.35, 0, NO, 0, 315, 1.35, 0, NO, 0
316, 1.35, 0, NO, 0, 317, 1.35, 0, NO, 0, 318, 1.35, 0, NO, 0
319, 1.35, 0, NO, 0, 320, 1.35, 0, NO, 0, 321, 1.35, 0, NO, 0
322, 1.35, 0, NO, 0, 323, 1.35, 0, NO, 0, 324, 1.35, 0, NO, 0
325, 1.35, 0, NO, 0, 326, 1.35, 0, NO, 0, 327, 1.35, 0, NO, 0
328, 1.35, 0, NO, 0, 329, 1.35, 0, NO, 0, 330, 1.35, 0, NO, 0
331, 1.35, 0, NO, 0, 332, 1.35, 0, NO, 0, 333, 1.35, 0, NO, 0
334, 1.35, 0, NO, 0, 335, 1.35, 0, NO, 0, 336, 1.35, 0, NO, 0
337, 1.35, 0, NO, 0, 338, 1.35, 0, NO, 0, 339, 1.35, 0, NO, 0
340, 1.35, 0, NO, 0
NAME=DX RIM, LANE, , 0, 0, BOTH, 2.5, 0
301, -1.4, 0, NO, 0, 302, -1.4, 0, NO, 0, 303, -1.4, 0, NO, 0
304, -1.4, 0, NO, 0, 305, -1.4, 0, NO, 0, 306, -1.4, 0, NO, 0
307, -1.4, 0, NO, 0, 308, -1.4, 0, NO, 0, 309, -1.4, 0, NO, 0
310, -1.4, 0, NO, 0, 311, -1.4, 0, NO, 0, 312, -1.4, 0, NO, 0
313, -1.4, 0, NO, 0, 314, -1.4, 0, NO, 0, 315, -1.4, 0, NO, 0
316, -1.4, 0, NO, 0, 317, -1.4, 0, NO, 0, 318, -1.4, 0, NO, 0
319, -1.4, 0, NO, 0, 320, -1.4, 0, NO, 0, 321, -1.4, 0, NO, 0
322, -1.4, 0, NO, 0, 323, -1.4, 0, NO, 0, 324, -1.4, 0, NO, 0
325, -1.4, 0, NO, 0, 326, -1.4, 0, NO, 0, 327, -1.4, 0, NO, 0
328, -1.4, 0, NO, 0, 329, -1.4, 0, NO, 0, 330, -1.4, 0, NO, 0
331, -1.4, 0, NO, 0, 332, -1.4, 0, NO, 0, 333, -1.4, 0, NO, 0
334, -1.4, 0, NO, 0, 335, -1.4, 0, NO, 0, 336, -1.4, 0, NO, 0
337, -1.4, 0, NO, 0, 338, -1.4, 0, NO, 0, 339, -1.4, 0, NO, 0
340, -1.4, 0, NO, 0
NAME=MARC DX, LANE, , 0, 0, BOTH, 1.2, 0
101, 0.55, 0, NO, 0, 102, 0.55, 0, NO, 0, 103, 0.55, 0, NO, 0
104, 0.55, 0, NO, 0, 105, 0.55, 0, NO, 0, 106, 0.55, 0, NO, 0
107, 0.55, 0, NO, 0, 108, 0.55, 0, NO, 0, 109, 0.55, 0, NO, 0
110, 0.55, 0, NO, 0, 111, 0.55, 0, NO, 0, 112, 0.55, 0, NO, 0
113, 0.55, 0, NO, 0, 114, 0.55, 0, NO, 0, 115, 0.55, 0, NO, 0
116, 0.55, 0, NO, 0, 117, 0.55, 0, NO, 0, 118, 0.55, 0, NO, 0
119, 0.55, 0, NO, 0, 120, 0.55, 0, NO, 0, 121, 0.55, 0, NO, 0
122, 0.55, 0, NO, 0, 123, 0.55, 0, NO, 0, 124, 0.55, 0, NO, 0
125, 0.55, 0, NO, 0, 126, 0.55, 0, NO, 0, 127, 0.55, 0, NO, 0
128, 0.55, 0, NO, 0, 129, 0.55, 0, NO, 0, 130, 0.55, 0, NO, 0
131, 0.55, 0, NO, 0, 132, 0.55, 0, NO, 0, 133, 0.55, 0, NO, 0
134, 0.55, 0, NO, 0, 135, 0.55, 0, NO, 0, 136, 0.55, 0, NO, 0
137, 0.55, 0, NO, 0, 138, 0.55, 0, NO, 0, 139, 0.55, 0, NO, 0
140, 0.55, 0, NO, 0
NAME=MARC SX, LANE, , 0, 0, BOTH, 1.2, 0
401, -0.55, 0, NO, 0, 402, -0.55, 0, NO, 0, 403, -0.55, 0, NO, 0
404, -0.55, 0, NO, 0, 405, -0.55, 0, NO, 0, 406, -0.55, 0, NO, 0
407, -0.55, 0, NO, 0, 408, -0.55, 0, NO, 0, 409, -0.55, 0, NO, 0
410, -0.55, 0, NO, 0, 411, -0.55, 0, NO, 0, 412, -0.55, 0, NO, 0
413, -0.55, 0, NO, 0, 414, -0.55, 0, NO, 0, 415, -0.55, 0, NO, 0
416, -0.55, 0, NO, 0, 417, -0.55, 0, NO, 0, 418, -0.55, 0, NO, 0
419, -0.55, 0, NO, 0, 420, -0.55, 0, NO, 0, 421, -0.55, 0, NO, 0
422, -0.55, 0, NO, 0, 423, -0.55, 0, NO, 0, 424, -0.55, 0, NO, 0
425, -0.55, 0, NO, 0, 426, -0.55, 0, NO, 0, 427, -0.55, 0, NO, 0
428, -0.55, 0, NO, 0, 429, -0.55, 0, NO, 0, 430, -0.55, 0, NO, 0
431, -0.55, 0, NO, 0, 432, -0.55, 0, NO, 0, 433, -0.55, 0, NO, 0
434, -0.55, 0, NO, 0, 435, -0.55, 0, NO, 0, 436, -0.55, 0, NO, 0
437, -0.55, 0, NO, 0, 438, -0.55, 0, NO, 0, 439, -0.55, 0, NO, 0
440, -0.55, 0, NO, 0
NAME=SX 1, LANE, , 0, 0, BOTH, 3, 2
301, -1.15, 0, NO, 0, 302, -1.15, 0, NO, 0, 303, -1.15, 0, NO, 0
304, -1.15, 0, NO, 0, 305, -1.15, 0, NO, 0, 306, -1.15, 0, NO, 0
307, -1.15, 0, NO, 0, 308, -1.15, 0, NO, 0, 309, -1.15, 0, NO, 0
310, -1.15, 0, NO, 0, 311, -1.15, 0, NO, 0, 312, -1.15, 0, NO, 0
313, -1.15, 0, NO, 0, 314, -1.15, 0, NO, 0, 315, -1.15, 0, NO, 0
316, -1.15, 0, NO, 0, 317, -1.15, 0, NO, 0, 318, -1.15, 0, NO, 0
319, -1.15, 0, NO, 0, 320, -1.15, 0, NO, 0, 321, -1.15, 0, NO, 0
322, -1.15, 0, NO, 0, 323, -1.15, 0, NO, 0, 324, -1.15, 0, NO, 0
325, -1.15, 0, NO, 0, 326, -1.15, 0, NO, 0, 327, -1.15, 0, NO, 0
328, -1.15, 0, NO, 0, 329, -1.15, 0, NO, 0, 330, -1.15, 0, NO, 0
331, -1.15, 0, NO, 0, 332, -1.15, 0, NO, 0, 333, -1.15, 0, NO, 0
334, -1.15, 0, NO, 0, 335, -1.15, 0, NO, 0, 336, -1.15, 0, NO, 0
337, -1.15, 0, NO, 0, 338, -1.15, 0, NO, 0, 339, -1.15, 0, NO, 0
340, -1.15, 0, NO, 0
NAME=SX 2, LANE, , 0, 0, BOTH, 3, 2
201, -1.35, 0, NO, 0, 202, -1.35, 0, NO, 0, 203, -1.35, 0, NO, 0
204, -1.35, 0, NO, 0, 205, -1.35, 0, NO, 0, 206, -1.35, 0, NO, 0
207, -1.35, 0, NO, 0, 208, -1.35, 0, NO, 0, 209, -1.35, 0, NO, 0
210, -1.35, 0, NO, 0, 211, -1.35, 0, NO, 0, 212, -1.35, 0, NO, 0

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PROJECT TITLE : CASALPUSTERLENGO

	Company	Client
	Author	File Name
	Renato Vaira	Cavalc Costa V_2.mct

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213, -1.35, 0, NO, 0, 214, -1.35, 0, NO, 0, 215, -1.35, 0, NO, 0
216, -1.35, 0, NO, 0, 217, -1.35, 0, NO, 0, 218, -1.35, 0, NO, 0
219, -1.35, 0, NO, 0, 220, -1.35, 0, NO, 0, 221, -1.35, 0, NO, 0
222, -1.35, 0, NO, 0, 223, -1.35, 0, NO, 0, 224, -1.35, 0, NO, 0
225, -1.35, 0, NO, 0, 226, -1.35, 0, NO, 0, 227, -1.35, 0, NO, 0
228, -1.35, 0, NO, 0, 229, -1.35, 0, NO, 0, 230, -1.35, 0, NO, 0
231, -1.35, 0, NO, 0, 232, -1.35, 0, NO, 0, 233, -1.35, 0, NO, 0
234, -1.35, 0, NO, 0, 235, -1.35, 0, NO, 0, 236, -1.35, 0, NO, 0
237, -1.35, 0, NO, 0, 238, -1.35, 0, NO, 0, 239, -1.35, 0, NO, 0
240, -1.35, 0, NO, 0

```

NAME=SX RIM, LANE, , 0, 0, BOTH, 2.5, 0

```

201, 1.4, 0, NO, 0, 202, 1.4, 0, NO, 0, 203, 1.4, 0, NO, 0
204, 1.4, 0, NO, 0, 205, 1.4, 0, NO, 0, 206, 1.4, 0, NO, 0
207, 1.4, 0, NO, 0, 208, 1.4, 0, NO, 0, 209, 1.4, 0, NO, 0
210, 1.4, 0, NO, 0, 211, 1.4, 0, NO, 0, 212, 1.4, 0, NO, 0
213, 1.4, 0, NO, 0, 214, 1.4, 0, NO, 0, 215, 1.4, 0, NO, 0
216, 1.4, 0, NO, 0, 217, 1.4, 0, NO, 0, 218, 1.4, 0, NO, 0
219, 1.4, 0, NO, 0, 220, 1.4, 0, NO, 0, 221, 1.4, 0, NO, 0
222, 1.4, 0, NO, 0, 223, 1.4, 0, NO, 0, 224, 1.4, 0, NO, 0
225, 1.4, 0, NO, 0, 226, 1.4, 0, NO, 0, 227, 1.4, 0, NO, 0
228, 1.4, 0, NO, 0, 229, 1.4, 0, NO, 0, 230, 1.4, 0, NO, 0
231, 1.4, 0, NO, 0, 232, 1.4, 0, NO, 0, 233, 1.4, 0, NO, 0
234, 1.4, 0, NO, 0, 235, 1.4, 0, NO, 0, 236, 1.4, 0, NO, 0
237, 1.4, 0, NO, 0, 238, 1.4, 0, NO, 0, 239, 1.4, 0, NO, 0
240, 1.4, 0, NO, 0

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\*VEHICLE ; Vehicles

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NAME=Fatica 2, 1, 3, Fatigue Load Model 2 (630), , , 1, , ,
NAME=Fatica 3 (1 veicolo), 1, 3, Fatigue Load Model 3 (One Vehicle), , , 1, , ,
NAME=Schema 1, 1, 1, Load Model 1, 0.75, 0.4, , 1, 1, 1, 1, 1, 1
NAME=Schema 5, 1, 2, Uniform load (Road bridge footway), , , , 0.4, 2.5,

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\*MVLDCASE (EURO) ; Moving Load Cases

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NAME=Mobili SLU DX, 1, , Schema 1, Schema 5, YES
1, DX 1, DX 2
2, DX RIM
3, MARC DX, MARC SX
NAME=Mobili SLU SX, 1, , Schema 1, Schema 5, YES
1, SX 1, SX 2
2, SX RIM
3, MARC DX, MARC SX
NAME=Mobili SLE DX, 1, , Schema 1, Schema 5, NO
1, DX 1, DX 2
2, DX RIM
3, MARC DX, MARC SX
NAME=Mobili SLE SX, 1, , Schema 1, Schema 5, NO
1, SX 1, SX 2
2, SX RIM
3, MARC DX, MARC SX
NAME=Fatica V.ILL., 2, , 1, YES
Fatica 2, 1, 0, 1, C. LENTA DX, C. LENTA SX
NAME=Fatica DANN.ACC., 2, , 1, YES
Fatica 3 (1 veicolo), 1, 0, 1, C. LENTA DX, C. LENTA SX

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\*CPOSECT4CS ; Composite Section for Construction Stage

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SEC=5, Costruzione, NORMAL, YES
1, MATL, 1, , 0, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=11, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=1, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=2, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=7, Costruzione, NORMAL, NO
1, MATL, 1, , 0, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=16, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=3, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=4, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0

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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
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2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=6, Costruzione, NORMAL, YES
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=8, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=9, Costruzione, NORMAL, YES
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=10, Costruzione, NORMAL, YES
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=12, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=13, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=14, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
SEC=21, Costruzione, NORMAL, NO
1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0
2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 1, 0.3
    
```

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*STAGE-COLOR ; Diagram Color for Construction Stage
Costruzione, 0, 157, 192
Esercizio, 255, 255, 255
    
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*STAGE-CTRL ; Construction Stage Analysis Control Data
YES, , INTERNAL, NO, YES, YES, NO, NO, NO, NO, NO, , , DW, W, DW, Erection Load 1, Erection Load 2, Erecti
on Load 3, YES, NO, , NO, NO
NO
NO
YES, YES, BOTH, 5, 0.01, YES, NO, YES, YES, 0, YES
NO, NO, 2, YES, 2, 5, 7, 10, 20
NO, , , 1, NO
Finiture
    
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*LOADCOMB ; Combinations
NAME=Vento Strutture (Y-), GEN, INACTIVE, 0, 0, , 0, 0
ST, Vento strutture (Y+), -1
NAME=Vento Mobili (Y-), GEN, INACTIVE, 0, 0, , 0, 0
ST, Vento mobili (Y+), -1
NAME=Frenamento (X-), GEN, INACTIVE, 0, 0, , 0, 0
ST, Frenamento (X+), -1
NAME=Termico unif. -15°, GEN, INACTIVE, 0, 0, , 0, 0
ST, Termico Unif. +15°, -1
NAME=Termico diff. -5°/H, GEN, INACTIVE, 0, 0, , 0, 0
ST, Termico diff. +5°/H, -1
NAME=SLD Long (X-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLD Long (X+), -1
NAME=SLD Trasv (Y-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLD Trasv (Y+), -1
NAME=SLD Vert (Z-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLD Vert (Z+), -1
NAME=SLV Long (X-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLV Long (X+), -1
NAME=SLV Trasv (Y-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLV Trasv (Y+), -1
NAME=SLV Vert (Z-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLV Vert (Z+), -1
NAME=SLC Long (X-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLC Long (X+), -1
NAME=SLC Trasv (Y-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLC Trasv (Y+), -1
NAME=SLC Vert (Z-), GEN, INACTIVE, 0, 0, , 0, 0
RS, SLC Vert (Z+), -1
NAME=Vento Strutture INV, GEN, INACTIVE, 0, 1, , 0, 0
ST, Vento strutture (Y+), 1, CB, Vento Strutture (Y-), 1
NAME=Vento Mobili INV, GEN, INACTIVE, 0, 1, , 0, 0
ST, Vento mobili (Y+), 1, CB, Vento Mobili (Y-), 1
NAME=Frenamento INV, GEN, ACTIVE, 0, 1, , 0, 0
ST, Frenamento (X+), 1, CB, Frenamento (X-), 1
NAME=Termico unif. INV, GEN, ACTIVE, 0, 1, , 0, 0
ST, Termico Unif. +15°, 1, CB, Termico unif. -15°, 1
NAME=Termico diff. INV, GEN, ACTIVE, 0, 1, , 0, 0
    
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ST, Termico diff. +5°/H, 1, CB, Termico diff. -5°/H, 1  
NAME=SLD Long INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLD Long (X+), 1, CB, SLD Long (X-), 1  
NAME=SLD Trasv INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLD Trasv (Y+), 1, CB, SLD Trasv (Y-), 1  
NAME=SLD Vert INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLD Vert (Z+), 1, CB, SLD Vert (Z-), 1  
NAME=SLV Long INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLV Long (X+), 1, CB, SLV Long (X-), 1  
NAME=SLV Trasv INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLV Trasv (Y+), 1, CB, SLV Trasv (Y-), 1  
NAME=SLV Vert INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLV Vert (Z+), 1, CB, SLV Vert (Z-), 1  
NAME=SLC Long INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLC Long (X+), 1, CB, SLC Long (X-), 1  
NAME=SLC Trasv INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLC Trasv (Y+), 1, CB, SLC Trasv (Y-), 1  
NAME=SLC Vert INV, GEN, INACTIVE, 0, 1, , 0, 0  
RS, SLC Vert (Z+), 1, CB, SLC Vert (Z-), 1  
NAME=Mobili SLU INV, GEN, ACTIVE, 0, 1, , 0, 0  
MV, Mobili SLU DX, 1, MV, Mobili SLU SX, 1  
NAME=Mobili SLE INV, GEN, ACTIVE, 0, 1, , 0, 0  
MV, Mobili SLE DX, 1, MV, Mobili SLE SX, 1  
NAME=GEN SLU Mobili, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CB, Vento Strutture INV, 0.9  
CB, Vento Mobili INV, 0.9, CB, Termico unif. INV, 0.72  
CB, Termico diff. INV, 0.72, CB, Mobili SLU INV, 1.35  
NAME=GEN SLU Vento, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CB, Vento Strutture INV, 1.5  
CB, Vento Mobili INV, 1.5, CB, Termico unif. INV, 0.72  
CB, Termico diff. INV, 0.72, CB, Mobili SLE INV, 1.35  
NAME=GEN SLU Frenamento, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CB, Frenamento INV, 1.35  
CB, Termico unif. INV, 0.72, CB, Termico diff. INV, 0.72  
CB, Mobili SLE INV, 1.35  
NAME=GEN SLU Termico, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CB, Vento Strutture INV, 0.9  
CB, Vento Mobili INV, 0.9, CB, Termico unif. INV, 1.2  
CB, Termico diff. INV, 1.2, CB, Mobili SLE INV, 1.35  
NAME=GEN SLD Long, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLD Long INV, 1, CB, SLD Trasv INV, 0.3  
CB, SLD Vert INV, 0.3  
NAME=GEN SLD Trasv, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLD Long INV, 0.3, CB, SLD Trasv INV, 1  
CB, SLD Vert INV, 0.3  
NAME=GEN SLD Vert, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLD Long INV, 0.3  
CB, SLD Trasv INV, 0.3, CB, SLD Vert INV, 1  
NAME=GEN SLV Long, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLV Long INV, 1, CB, SLV Trasv INV, 0.3  
CB, SLV Vert INV, 0.3  
NAME=GEN SLV Trasv, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLV Long INV, 0.3, CB, SLV Trasv INV, 1  
CB, SLV Vert INV, 0.3  
NAME=GEN SLV Vert, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLV Long INV, 0.3  
CB, SLV Trasv INV, 0.3, CB, SLV Vert INV, 1  
NAME=GEN SLC Long, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLC Long INV, 1, CB, SLC Trasv INV, 0.3  
CB, SLC Vert INV, 0.3  
NAME=GEN SLC Trasv, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLC Long INV, 0.3, CB, SLC Trasv INV, 1  
CB, SLC Vert INV, 0.3  
NAME=GEN SLC Vert, GEN, ACTIVE, 0, 0, , 0, 0  
CS, Summation, 1, CB, Termico unif. INV, 0.5  
CB, Termico diff. INV, 0.5, CB, SLC Long INV, 0.3




PROJECT TITLE : CASALPUSTERLENGO

	Company	Client
	Author	File Name
	Renato Vaira	Cavalc Costa V_2.mct

CB, SLC Trasv INV, 0.3, CB, SLC Vert INV, 1  
 NAME=GEN RARA Mobili, GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Vento Strutture INV, 0.6  
 CB, Vento Mobili INV, 0.6, CB, Termico unif. INV, 0.6  
 CB, Termico diff. INV, 0.6, CB, Mobili SLU INV, 1  
 NAME=GEN RARA Vento, GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Vento Strutture INV, 1, CB, Vento Mobili INV, 1  
 CB, Termico unif. INV, 0.6, CB, Termico diff. INV, 0.6  
 CB, Mobili SLE INV, 1  
 NAME=GEN RARA Frenam, GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Frenamento INV, 1, CB, Termico unif. INV, 0.6  
 CB, Termico diff. INV, 0.6, CB, Mobili SLE INV, 1  
 NAME=GEN RARA Termico, GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Vento Strutture INV, 0.6  
 CB, Vento Mobili INV, 0.6, CB, Termico unif. INV, 1  
 CB, Termico diff. INV, 1, CB, Mobili SLE INV, 1  
 NAME=GEN FREQ Mobili, GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Termico unif. INV, 0.5  
 CB, Termico diff. INV, 0.5, CB, Mobili SLE INV, 1  
 NAME=GEN FREQ Vento, GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Vento Strutture INV, 0.2  
 CB, Vento Mobili INV, 0.2, CB, Termico unif. INV, 0.5  
 CB, Termico diff. INV, 0.5  
 NAME=GEN FREQ Termico, GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Termico unif. INV, 0.6  
 CB, Termico diff. INV, 0.6  
 NAME=GEN Q.P., GEN, ACTIVE, 0, 0, , 0, 0  
 CS, Summation, 1, CB, Termico unif. INV, 0.5  
 CB, Termico diff. INV, 0.5  
 NAME=GEN SLU INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, GEN SLU Mobili, 1, CB, GEN SLU Vento, 1, CB, GEN SLU Frenamento, 1  
 CB, GEN SLU Termico, 1  
 NAME=GEN SLD INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, GEN SLD Long, 1, CB, GEN SLD Trasv, 1, CB, GEN SLD Vert, 1  
 NAME=GEN SLV INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, GEN SLV Long, 1, CB, GEN SLV Trasv, 1, CB, GEN SLV Vert, 1  
 NAME=GEN SLC INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, GEN SLC Long, 1, CB, GEN SLC Trasv, 1, CB, GEN SLC Vert, 1  
 NAME=GEN RARA INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, GEN RARA Mobili, 1, CB, GEN RARA Vento, 1, CB, GEN RARA Frenam, 1  
 CB, GEN RARA Termico, 1  
 NAME=GEN FREQ INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, GEN FREQ Mobili, 1, CB, GEN FREQ Vento, 1, CB, GEN FREQ Termico, 1  
 NAME=VENTO TOT INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, Vento Strutture INV, 1, CB, Vento Mobili INV, 1  
 NAME=GEN SLU/SLV INV, GEN, ACTIVE, 0, 1, , 0, 0  
 CB, GEN SLU Mobili, 1, CB, GEN SLU Vento, 1, CB, GEN SLU Frenamento, 1  
 CB, GEN SLU Termico, 1, CB, GEN SLV Long, 1, CB, GEN SLV Trasv, 1  
 CB, GEN SLV Vert, 1  
 NAME=Vento Strutture (Y-), STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Vento strutture (Y+), -1  
 NAME=Vento Mobili (Y-), STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Vento mobili (Y+), -1  
 NAME=Frenamento (X-), STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Frenamento (X+), -1  
 NAME=Termico unif. -15°, STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Termico Unif. +15°, -1  
 NAME=Termico diff. -5°/H, STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Termico diff. +5°/H, -1  
 NAME=SLV Long (X-), STEEL, INACTIVE, 0, 0, , 0, 0  
 RS, SLV Long (X+), -1  
 NAME=SLV Trasv (Y-), STEEL, INACTIVE, 0, 0, , 0, 0  
 RS, SLV Trasv (Y+), -1  
 NAME=SLV Vert (Z-), STEEL, INACTIVE, 0, 0, , 0, 0  
 RS, SLV Vert (Z+), -1  
 NAME=Vento Strutture INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Vento strutture (Y+), 1, CBS, Vento Strutture (Y-), 1  
 NAME=Vento Mobili INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Vento mobili (Y+), 1, CBS, Vento Mobili (Y-), 1  
 NAME=Frenamento INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Frenamento (X+), 1, CBS, Frenamento (X-), 1  
 NAME=Termico unif. INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Termico Unif. +15°, 1, CBS, Termico unif. -15°, 1  
 NAME=Termico diff. INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Termico diff. +5°/H, 1, CBS, Termico diff. -5°/H, 1  
 NAME=SLV Long INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 RS, SLV Long (X+), 1, CBS, SLV Long (X-), 1  
 NAME=SLV Trasv INV, STEEL, INACTIVE, 0, 1, , 0, 0

PROJECT TITLE : CASALPUSTERLENGO


	Company	Client
	Author	File Name
	Renato Vaira	Cavalc Costa V_2.mct

RS, SLV Trasv (Y+), 1, CBS, SLV Trasv (Y-), 1  
NAME=SLV Vert INV, STEEL, INACTIVE, 0, 1, , 0, 0  
RS, SLV Vert (Z+), 1, CBS, SLV Vert (Z-), 1  
NAME=Mobili SLU INV, STEEL, INACTIVE, 0, 1, , 0, 0  
MV, Mobili SLU DX, 1, MV, Mobili SLU SX, 1  
NAME=Mobili SLE INV, STEEL, INACTIVE, 0, 1, , 0, 0  
MV, Mobili SLE DX, 1, MV, Mobili SLE SX, 1  
NAME=ST SLU Mobili, STEEL, STRENGTH, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CBS, Vento Strutture INV, 0.9  
CBS, Vento Mobili INV, 0.9, CBS, Termico unif. INV, 0.72  
CBS, Termico diff. INV, 0.72, CBS, Mobili SLU INV, 1.35  
NAME=ST SLU Vento, STEEL, STRENGTH, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CBS, Vento Strutture INV, 1.5  
CBS, Vento Mobili INV, 1.5, CBS, Termico unif. INV, 0.72  
CBS, Termico diff. INV, 0.72, CBS, Mobili SLE INV, 1.35  
NAME=ST SLU Frenamento, STEEL, STRENGTH, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CBS, Frenamento INV, 1.35  
CBS, Termico unif. INV, 0.72, CBS, Termico diff. INV, 0.72  
CBS, Mobili SLE INV, 1.35  
NAME=ST SLU Termico, STEEL, STRENGTH, 0, 0, , 0, 0  
CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
CS, Shrinkage Secondary, 1.2, CBS, Vento Strutture INV, 0.9  
CBS, Vento Mobili INV, 0.9, CBS, Termico unif. INV, 1.2  
CBS, Termico diff. INV, 1.2, CBS, Mobili SLE INV, 1.35  
NAME=ST SLV Long, STEEL, STRENGTH, 0, 0, , 0, 0  
CS, Summation, 1, CBS, Termico unif. INV, 0.5  
CBS, Termico diff. INV, 0.5, CBS, SLV Long INV, 1  
CBS, SLV Trasv INV, 0.3, CBS, SLV Vert INV, 0.3  
NAME=ST SLV Trasv, STEEL, STRENGTH, 0, 0, , 0, 0  
CS, Summation, 1, CBS, Termico unif. INV, 0.5  
CBS, Termico diff. INV, 0.5, CBS, SLV Long INV, 0.3  
CBS, SLV Trasv INV, 1, CBS, SLV Vert INV, 0.3  
NAME=ST SLV Vert, STEEL, STRENGTH, 0, 0, , 0, 0  
CS, Summation, 1, CBS, Termico unif. INV, 0.5  
CBS, Termico diff. INV, 0.5, CBS, SLV Long INV, 0.3  
CBS, SLV Trasv INV, 0.3, CBS, SLV Vert INV, 1  
NAME=ST FATICA VITA ILL., STEEL, SERVICE, 0, 0, , 0, 0  
MV, Fatica V.ILL., 1, CS, Summation, 1  
NAME=ST FATICA DANN.ACC., STEEL, SERVICE, 0, 0, , 0, 0  
MV, Fatica DANN.ACC., 1, CS, Summation, 1  
NAME=ST RARA Mobili, STEEL, SERVICE, 0, 0, , 3, 0  
CS, Summation, 1, CBS, Vento Strutture INV, 0.6  
CBS, Vento Mobili INV, 0.6, CBS, Termico unif. INV, 0.6  
CBS, Termico diff. INV, 0.6, CBS, Mobili SLU INV, 1  
NAME=ST RARA Vento, STEEL, SERVICE, 0, 0, , 3, 0  
CS, Summation, 1, CBS, Vento Strutture INV, 1, CBS, Vento Mobili INV, 1  
CBS, Termico unif. INV, 0.6, CBS, Termico diff. INV, 0.6  
CBS, Mobili SLE INV, 1  
NAME=ST RARA Frenamento, STEEL, SERVICE, 0, 0, , 3, 0  
CS, Summation, 1, CBS, Frenamento INV, 1, CBS, Termico unif. INV, 0.6  
CBS, Termico diff. INV, 0.6, CBS, Mobili SLE INV, 1  
NAME=ST RARA Termico, STEEL, SERVICE, 0, 0, , 3, 0  
CS, Summation, 1, CBS, Vento Strutture INV, 0.6  
CBS, Vento Mobili INV, 0.6, CBS, Termico unif. INV, 1  
CBS, Termico diff. INV, 1, CBS, Mobili SLE INV, 1  
NAME=ST FREQ Mobili, STEEL, SERVICE, 0, 0, , 2, 0  
CS, Summation, 1, CBS, Termico unif. INV, 0.5  
CBS, Termico diff. INV, 0.5, CBS, Mobili SLE INV, 1  
NAME=ST FREQ Vento, STEEL, SERVICE, 0, 0, , 2, 0  
CS, Summation, 1, CBS, Vento Strutture INV, 0.2  
CBS, Vento Mobili INV, 0.2, CBS, Termico unif. INV, 0.5  
CBS, Termico diff. INV, 0.5  
NAME=ST FREQ Termico, STEEL, SERVICE, 0, 0, , 2, 0  
CS, Summation, 1, CBS, Termico unif. INV, 0.6  
CBS, Termico diff. INV, 0.6  
NAME=ST Q.P., STEEL, SERVICE, 0, 0, , 1, 0  
CS, Summation, 1, CBS, Termico unif. INV, 0.5  
CBS, Termico diff. INV, 0.5  
NAME=FATICA VITA ILL., CONC, STRENGTH, 0, 0, , 0, 0  
CS, Summation, 1, MV, Fatica V.ILL., 1

\*LC-COLOR ; Diagram Color for Load Case

ST, Peso acciaio e pile, 0, 192, 128, 255, 0, 192, 192, 192, 0  
ST, Peso soletta, 0, 128, 255, 255, 255, 87, 192, 0, 128  
ST, Finiture, 255, 128, 0, 192, 192, 192, 192, 192, 192

PROJECT TITLE : CASALPUSTERLENGO

	Company	Client
	Author	File Name
	Renato Vaira	
		Cavalc Costa V_2.mct

ST, Vento strutture (Y+), 255, 0, 128, 0, 128, 57, 192, 0, 192  
 ST, Vento mobili (Y+), 0, 192, 128, 255, 0, 192, 212, 160, 255  
 ST, Frenamento (X+), 255, 128, 0, 85, 0, 192, 160, 255, 255  
 ST, Termico Unif. +15°, 192, 0, 192, 0, 128, 57, 255, 87, 128  
 MV, Mobili SLU DX, 85, 192, 0, 163, 255, 160, 192, 192, 0  
 CS, Dead Load, 192, 128, 0, 255, 255, 255, 255, 160, 255  
 CS, Tendon Secondary, 163, 160, 255, 148, 87, 255, 255, 0, 192  
 CS, Creep Primary, 192, 0, 192, 163, 160, 255, 255, 0, 192  
 CS, Creep Secondary, 78, 0, 255, 255, 0, 192, 160, 192, 255  
 CS, Shrinkage Primary, 0, 192, 192, 192, 0, 192, 160, 192, 255  
 CS, Shrinkage Secondary, 0, 192, 192, 255, 255, 255, 210, 210, 210  
 CS, Summation, 0, 192, 128, 160, 255, 255, 192, 192, 0  
 RS, SLD Long (X+), 0, 128, 57, 255, 0, 192, 255, 87, 128  
 RS, SLD Trasv (Y+), 255, 0, 192, 192, 192, 192, 255, 255, 255  
 RS, SLD Vert (Z+), 192, 0, 128, 255, 192, 87, 0, 128, 57  
 RS, SLV Long (X+), 85, 0, 192, 192, 192, 0, 255, 192, 87  
 RS, SLV Trasv (Y+), 192, 72, 0, 192, 192, 0, 255, 0, 192  
 RS, SLV Vert (Z+), 192, 192, 192, 255, 192, 87, 210, 210, 210  
 RS, SLC Long (X+), 0, 128, 128, 85, 0, 192, 192, 192, 0  
 RS, SLC Trasv (Y+), 192, 0, 128, 255, 0, 192, 255, 0, 128  
 RS, SLC Vert (Z+), 255, 0, 128, 0, 157, 192, 192, 72, 0  
 ST, Termico diff. +5°/H, 0, 128, 192, 78, 0, 255, 0, 192, 128  
 MV, Mobili SLU SX, 160, 255, 255, 192, 72, 0, 192, 72, 0  
 MV, Mobili SLE DX, 78, 0, 255, 160, 255, 255, 255, 87, 87  
 CB, Vento Strutture (Y-), 255, 87, 87, 0, 192, 128, 0, 157, 192  
 CB, Vento Mobili (Y-), 192, 0, 192, 146, 0, 255, 192, 0, 128  
 MV, Mobili SLE SX, 85, 192, 0, 163, 160, 255, 0, 192, 192  
 CB, Frenamento (X-), 78, 0, 255, 85, 0, 192, 85, 192, 0  
 CB, Termico unif. -15°, 255, 255, 255, 128, 192, 0, 128, 192, 0  
 CB, Termico diff. -5°/H, 163, 160, 255, 255, 0, 192, 255, 0, 192  
 CB, SLD Long (X-), 210, 210, 210, 0, 128, 255, 0, 128, 128  
 CB, SLD Trasv (Y-), 255, 255, 87, 78, 0, 255, 85, 0, 192  
 CB, SLD Vert (Z-), 192, 0, 128, 128, 192, 0, 255, 0, 128  
 CB, SLV Long (X-), 0, 128, 192, 192, 0, 128, 0, 128, 192  
 CB, SLV Trasv (Y-), 148, 87, 255, 255, 0, 128, 192, 72, 0  
 CB, SLV Vert (Z-), 192, 72, 0, 0, 128, 192, 192, 0, 128  
 CB, SLC Long (X-), 212, 160, 255, 212, 160, 255, 85, 0, 192  
 CB, SLC Trasv (Y-), 255, 87, 87, 192, 128, 0, 0, 192, 128  
 CB, SLC Vert (Z-), 0, 157, 192, 146, 0, 255, 255, 128, 0  
 CB, Vento Strutture INV, 0, 192, 128, 255, 0, 128, 255, 255, 87  
 CB, Vento Mobili INV, 255, 87, 128, 192, 0, 128, 128, 192, 0  
 CB, Frenamento INV, 0, 128, 192, 192, 0, 192, 212, 160, 255  
 CB, Termico unif. INV, 255, 87, 128, 78, 0, 255, 192, 72, 0  
 CB, Termico diff. INV, 85, 0, 192, 255, 255, 255, 0, 128, 128  
 CB, SLD Long INV, 210, 210, 210, 255, 87, 87, 192, 192, 192  
 CB, SLD Trasv INV, 93, 255, 87, 0, 192, 128, 0, 128, 192  
 CB, SLD Vert INV, 192, 72, 0, 163, 160, 255, 192, 0, 192  
 CB, SLV Long INV, 192, 192, 0, 255, 128, 0, 255, 0, 192  
 CB, SLV Trasv INV, 0, 128, 192, 210, 210, 210, 93, 255, 87  
 CB, SLV Vert INV, 128, 192, 0, 255, 255, 255, 255, 192, 87  
 CB, SLC Long INV, 0, 128, 128, 255, 192, 160, 212, 160, 255  
 CB, SLC Trasv INV, 255, 192, 160, 255, 87, 87, 0, 192, 192  
 MV, Fatica V.I.LL., 255, 87, 87, 255, 87, 87, 255, 255, 87  
 MV, Fatica DANN.ACC., 0, 157, 192, 148, 87, 255, 85, 0, 192  
 CB, SLC Vert INV, 163, 255, 160, 255, 87, 128, 255, 87, 87  
 CB, Mobili SLU INV, 160, 192, 255, 146, 0, 255, 255, 160, 255  
 CB, Mobili SLE INV, 255, 87, 128, 192, 0, 192, 93, 255, 87  
 CB, GEN SLU Mobili, 210, 210, 210, 255, 128, 0, 163, 255, 160  
 CB, GEN SLU Vento, 160, 255, 255, 128, 192, 0, 192, 0, 192  
 CB, GEN SLU Frenamento, 255, 128, 0, 0, 192, 128, 255, 192, 87  
 CB, GEN SLU Termico, 192, 192, 0, 0, 128, 128, 85, 0, 192  
 CB, GEN SLD Long, 0, 128, 192, 148, 87, 255, 85, 0, 192  
 CB, GEN SLD Trasv, 192, 0, 192, 93, 255, 87, 255, 160, 255  
 CB, GEN SLD Vert, 163, 255, 160, 78, 0, 255, 85, 192, 0  
 CB, GEN SLV Long, 255, 255, 87, 192, 192, 192, 192, 72, 0  
 CB, GEN SLV Trasv, 255, 0, 192, 212, 160, 255, 85, 0, 192  
 CB, GEN SLV Vert, 210, 210, 210, 160, 255, 255, 85, 192, 0  
 CB, GEN SLC Long, 0, 128, 128, 160, 255, 255, 78, 0, 255  
 CB, GEN SLC Trasv, 0, 192, 128, 255, 0, 128, 163, 255, 160  
 CB, GEN SLC Vert, 93, 255, 87, 85, 0, 192, 78, 0, 255  
 CB, GEN RARA Mobili, 255, 87, 128, 0, 192, 192, 255, 255, 255  
 CB, GEN RARA Vento, 212, 160, 255, 255, 192, 160, 255, 87, 87  
 CB, GEN RARA Frenam, 255, 87, 87, 255, 0, 128, 163, 160, 255  
 CB, GEN RARA Termico, 146, 0, 255, 255, 192, 160, 192, 128, 0  
 CB, GEN FREQ Mobili, 210, 210, 210, 93, 255, 87, 212, 160, 255  
 CB, GEN FREQ Vento, 85, 192, 0, 78, 0, 255, 93, 255, 87  
 CB, GEN FREQ Termico, 160, 255, 255, 85, 192, 0, 146, 0, 255  
 CB, GEN Q.P., 192, 0, 128, 255, 255, 255, 163, 255, 160



PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

0, 0, 0,\  
 0, 0, 0,\  
 0, 0, 0,\  
 0.0000e+000, 0,\  
 , 0, 0,\  
 0, 0, 0,\  
 0, 0, 0,\  
 0, 0, 0,\  
 0, 0, 0,\  
 0, 0, 0,\

\*DGN-REBAR-PSC

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SECT=1, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, , , NO, , ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=2, YES, NO, NO
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=3, YES, NO, NO
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=4, YES, NO, NO
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
SECT=5, YES, NO, YES
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
SECT=6, YES, NO, YES
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
SECT=7, YES, NO, NO
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
SECT=8, YES, NO, NO
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=9, YES, NO, YES
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
SECT=10, YES, NO, YES
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
SECT=11, YES, NO, NO
NO, , , NO, , , , NO, , , , NO, , , NO, , ,
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
    
```

PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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SECT=12, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=13, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
SECT=14, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=16, YES, NO, YES
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
SECT=21, YES, NO, YES
NO, , , NO, , , , NO, , , NO, , NO, ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
    
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\*DGN-TRANS\_STIFFENER ; Transverse Stiffener of Element

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ELEM=101, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=102, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=103, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=104, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=105, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=106, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=107, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=108, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=109, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=110, YES
I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=111, YES
I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=112, YES
I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=113, YES
I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=114, YES
I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=115, YES
I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=116, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=117, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=118, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=119, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=120, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=121, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=122, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=123, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
    
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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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ELEM=124, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=125, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=126, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=127, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=128, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=129, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=130, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=131, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=132, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=133, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=134, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=135, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=136, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=137, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=138, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=139, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=140, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=201, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=202, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=203, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=204, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=205, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=206, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=207, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=208, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=209, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=210, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=211, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=212, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=213, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=214, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=215, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=216, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=217, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=218, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=219, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=220, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=221, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=222, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
    
```



PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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ELEM=223, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=224, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=225, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=226, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=227, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=228, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=229, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=230, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=231, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=232, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=233, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=234, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=235, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=236, YES
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ELEM=237, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=238, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=239, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=240, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=301, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=302, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=303, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=304, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=305, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=306, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=307, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=308, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=309, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=310, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=311, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=312, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=313, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=314, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=315, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=316, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=317, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=318, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=319, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=320, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=321, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
    
```

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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ELEM=322, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=323, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=324, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=325, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=326, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=327, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=328, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=329, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=330, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=331, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=332, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=333, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=334, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=335, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=336, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=337, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=338, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=339, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=340, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=401, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=402, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=403, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=404, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=405, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=406, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=407, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=408, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=409, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=410, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=411, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=412, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=413, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=414, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=415, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=416, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=417, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=418, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=419, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=420, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
    
```

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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ELEM=421, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=422, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=423, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=424, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=425, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=426, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=427, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=428, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=429, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=430, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=431, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=432, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=433, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=434, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=435, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=436, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=437, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=438, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=439, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=440, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
    
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\*DGN-SHEAR\_CONNECTOR ; Shear Connector of Element

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ELEM=101, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=102, YES
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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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ELEM=317, YES
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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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\*DGN-POSITION ; Design Position of Element  
101to140 201to240 301to340 401to440, 2

\*DGN-DESIGN OUTPUT ; Composite Position for Design Output  
208, 1

\*DGN-PLATEGIRDER ; Composite Plate Girder Design  
1901, YES, YES, YES, YES, YES, NO, YES, YES, \  
1.5, 1.15, 1.05, 1.1, 1.25, 1, 1.15, 1, 50, 1, 0.6, 0.45, 0.8, 0.6, 0.95, 0.8, 0.9, 1, 1, 0.85, 1, 1.25,  
1.25, 1.25, 1.25, 1.25, 1, 0, YES, YES, YES, YES, YES, YES, YES, 0, NO, YES, NO, NO

\*DGN-PLATE-LT-BUCKLING ; Composite Girder Lateral-Torsional Buckling

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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 ELEM=302, YES  
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 ELEM=305, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=306, YES

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

I, YES, 0.1, 0.01, 3, 3.2  
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 ELEM=312, YES  
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 ELEM=405, YES

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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I, YES, 0.1, 0.01, 2, 3.2
ELEM=406, YES
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*DGN-PLATE-FTG ; Composite Girder Fatigue Resistance
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ELEM=103, YES
    
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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct


I, YES, 1.1, 1  
 ELEM=104, YES  
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 ELEM=202, YES

PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

I, YES, 1.1, 1  
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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

I, YES, 1.1, 1  
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 ELEM=306, YES  
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 ELEM=340, YES



PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

I, YES, 1.1, 1  
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 ELEM=438, YES  
 I, YES, 1.1, 1  
 ELEM=439, YES





PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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SECT=6, YES, YES, YES
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1, 0, -0.261942
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.328865, 0, -0.261942, 1.55, 0.328865, 0
1, 0, -0.261942
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=7, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, 0, 1.5, 0.141273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, 0, 1.5, 0.311273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, 0, 1.5, 0.141273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, 0, 1.5, 0.311273, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=8, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=9, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, 0, 1.4, 0.172355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, 0, 1.4, 0.342355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0.272916, 1.4, 0.445271, 0
1, 0, 0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0.272916, 1.4, 0.615271, 0
1, 0, 0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=10, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, -0.272916, 1.4, 0.342355, 0
1, 0, -0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, -0.272916, 1.4, 0.172355, 0
1, 0, -0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=11, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=12, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=13, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.305564, 0, -6.65975e-007, 1.4, 0.305564, 0

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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.135564, 0, -6.65975e-007, 1.4, 0.135564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.305564, 0, -6.65975e-007, 1.4, 0.305564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.135564, 0, -6.65975e-007, 1.4, 0.135564, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=14, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=16, YES, YES, YES
2, 2
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=21, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO

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\*SECTION MANAGER-REBAR DESIGN ; Section Manager - Reinforcement Design

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SECT=1, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.268069, 0, 0, 1.5, 0.268069, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.0980688, 0, 0, 1.5, 0.0980688, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.268069, 0, 0, 1.5, 0.268069, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.0980688, 0, 0, 1.5, 0.0980688, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO

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PROJECT TITLE : CASALPUSTERLENGO

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>	Renato Vaira	<b>File Name</b>	Cavalc Costa V_2.mct

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SECT=2, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.299742, 0, 6.16954e-007, 1.5, 0.299742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.129742, 0, 6.16954e-007, 1.5, 0.129742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.299742, 0, 6.16954e-007, 1.5, 0.299742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.129742, 0, 6.16954e-007, 1.5, 0.129742, 0
1, 1.61, 0.990523
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=3, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.109898, 0, 6.16954e-007, 1.4, 0.109898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.279898, 0, 6.16954e-007, 1.4, 0.279898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.109898, 0, 6.16954e-007, 1.4, 0.109898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.279898, 0, 6.16954e-007, 1.4, 0.279898, 0
1, 1.61, 0.990523
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=4, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.312978, 0, 6.16954e-007, 1.4, 0.312978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.142978, 0, 6.16954e-007, 1.4, 0.142978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.312978, 0, 6.16954e-007, 1.4, 0.312978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.142978, 0, 6.16954e-007, 1.4, 0.142978, 0
1, 1.61, 0.990523
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=5, YES, YES, YES
2, 2
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.328865, 0, 0, 1.55, 0.328865, 0
1, 0, 0
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.158865, 0, 0, 1.55, 0.158865, 0
1, 0, 0
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0.261942, 1.55, 0.590807, 0
1, 0, 0.261942
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0.261942, 1.55, 0.420807, 0
1, 0, 0.261942
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=6, YES, YES, YES
2, 2
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.158865, 0, -0.261942, 1.55, 0.158865, 0
1, 0, -0.261942
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.328865, 0, -0.261942, 1.55, 0.328865, 0
1, 0, -0.261942

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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=7, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, 0, 1.5, 0.141273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, 0, 1.5, 0.311273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, 0, 1.5, 0.141273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, 0, 1.5, 0.311273, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=8, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=9, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, 0, 1.4, 0.172355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, 0, 1.4, 0.342355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0.272916, 1.4, 0.445271, 0
1, 0, 0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0.272916, 1.4, 0.615271, 0
1, 0, 0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=10, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, -0.272916, 1.4, 0.342355, 0
1, 0, -0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, -0.272916, 1.4, 0.172355, 0
1, 0, -0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=11, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2

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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

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-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=12, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=13, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.305564, 0, -6.65975e-007, 1.4, 0.305564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.135564, 0, -6.65975e-007, 1.4, 0.135564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.305564, 0, -6.65975e-007, 1.4, 0.305564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.135564, 0, -6.65975e-007, 1.4, 0.135564, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=14, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=16, YES, YES, YES
2, 2
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=21, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0

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PROJECT TITLE : CASALPUSTERLENGO

	Company		Client	
	Author	Renato Vaira	File Name	Cavalc Costa V_2.mct

1, 0, 0  
 1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1  
 -1.4, 0.615271, 0, 0, 1.4, 0.615271, 0  
 1, 0, 0  
 NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO  
 NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO

\*SECTION MANAGER-GROUP & PART ; Section Manager - Group & Part

SECT=1, YES  
 0, 0, 0, 0  
 SECT=2, YES  
 0, 0, 0, 0  
 SECT=3, YES  
 0, 0, 0, 0  
 SECT=4, YES  
 0, 0, 0, 0  
 SECT=5, YES  
 0, 0, 0, 0  
 SECT=6, YES  
 0, 0, 0, 0  
 SECT=7, YES  
 0, 0, 0, 0  
 SECT=8, YES  
 0, 0, 0, 0  
 SECT=9, YES  
 0, 0, 0, 0  
 SECT=10, YES  
 0, 0, 0, 0  
 SECT=11, YES  
 0, 0, 0, 0  
 SECT=12, YES  
 0, 0, 0, 0  
 SECT=13, YES  
 0, 0, 0, 0  
 SECT=14, YES  
 0, 0, 0, 0  
 SECT=16, YES  
 0, 0, 0, 0  
 SECT=21, YES  
 0, 0, 0, 0

\*SECTION MANAGER-STIFFENER ; Section Manager - Stiffener

SECT=1, YES  
 0, 0  
 SECT=2, YES  
 0, 0  
 SECT=3, YES  
 0, 0  
 SECT=4, YES  
 0, 0  
 SECT=5, YES  
 0, 0  
 SECT=6, YES  
 0, 0  
 SECT=7, YES  
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 SECT=8, YES  
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 SECT=9, YES  
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 SECT=10, YES  
 0, 0  
 SECT=11, YES  
 0, 0  
 SECT=12, YES  
 0, 0  
 SECT=13, YES  
 0, 0  
 SECT=14, YES  
 0, 0  
 SECT=16, YES  
 0, 0  
 SECT=21, YES  
 0, 0

\*ENDDATA