

**S.S. N. 9 "VIA EMILIA"**  
**VARIANTE DI CASALPUSTERLENGO ED ELIMINAZIONE**  
**PASSAGGIO A LIVELLO SULLA S.P. EX S.S. N.234**  
**PROGETTO ESECUTIVO**

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<h1>HP02</h1>	<h2>H - PROGETTO STRUTTURALE OPERE PRINCIPALI</h2> <h3>CV02 - CAVALCAVIA ASSE 80</h3> <h4>RELAZIONE DI CALCOLO IMPALCATO</h4>
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CODICE PROGETTO		NOME FILE		REVISIONE	SCALA:
PROGETTO	LIV. PROG.	N. PROG.	HP02-P80CV02STRRE02_B.dwg		
COMI	E	1701	CODICE ELAB. P80CV02STRRE02	B	----
D					
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B	EMISSIONE A SEGUITO DI ISTRUTTORIA	LUGLIO 2018	ING. NICOLA LIGAS	PROF. ING. LUIGI MONTERISI	ING. VALERIO BAJETTI
A	EMISSIONE	DICEMBRE 2017	ING. NICOLA LIGAS	PROF. ING. LUIGI MONTERISI	ING. VALERIO BAJETTI
REV.	DESCRIZIONE	DATA	REDATTO	VERIFICATO	APPROVATO

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## 1 GENERALITÀ

### 1.1. PREMESSE

La presente relazione riporta i calcoli statici relativi alla progettazione esecutiva del Cavalcavia Asse 80, intersecante alla Pk. 6+539.97 la Variante di Casalpusterlengo alla S.S. n. 9 "Via Emilia". L'opera in oggetto presenta uno schema statico di trave continua su tre campate aventi luci tra gli assi di appoggio pari a 31+38+31 m.

L'impalcato è composto, da quattro travi continue in acciaio a doppio T ad altezza variabile sostenenti la soletta di scorrimento stradale in calcestruzzo armato ordinario.

In riguardo agli aspetti sismici, l'impalcato è isolato alla sommità delle pile e spalle mediante isolatori elastomerici armati.

#### Dimensioni dell'impalcato:

Larghezza marciapiedi	= 1.90 m	(larghezza utile 1.20 m)
Larghezza carreggiata, categoria C2	= 9.50 m	
Larghezza totale soletta	= 13.30 m	
Numero delle travi	= 4	
Interasse travi	= 3.20 m	
Altezza delle travi (variabile)	= 1.00÷2.00 m	
Spessore soletta	= 30÷42 cm	(lastra 6 cm + getto 24÷36 cm)

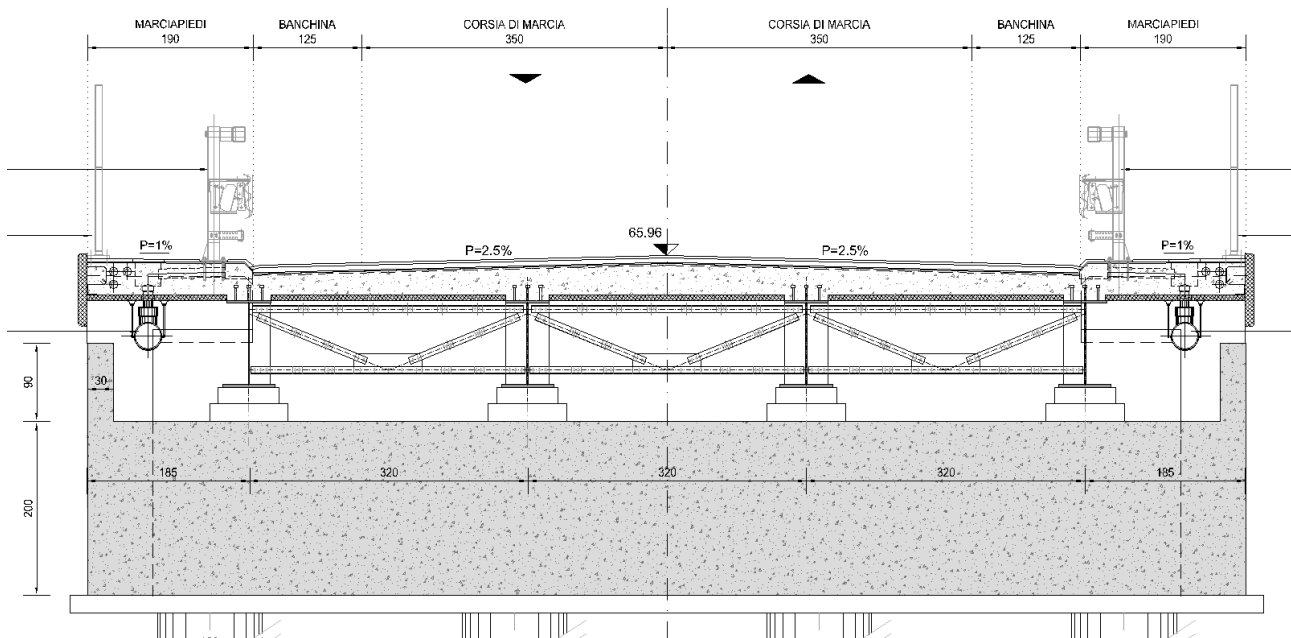


FIGURA 1 - SEZIONE TIPICA IMPALCATO (SU SPALLA)



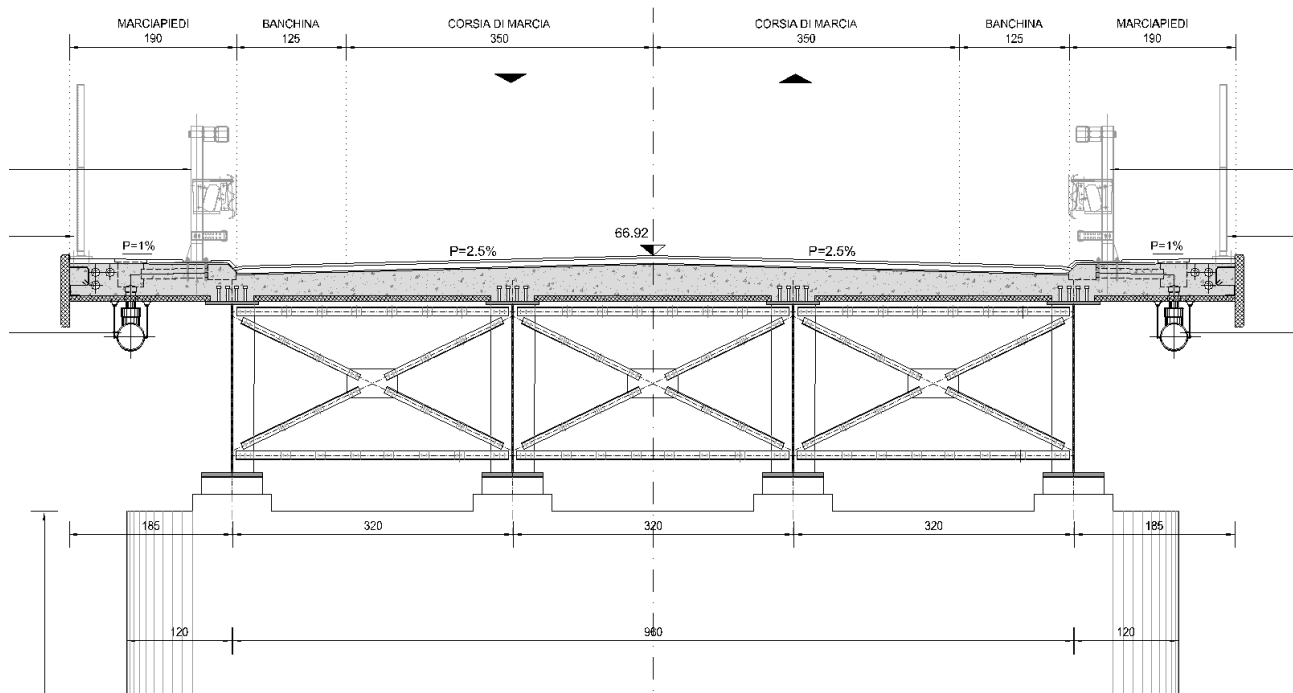


FIGURA 2- SEZIONE IMPALCATO (SU PILA)

## 1.2. NORMATIVA

Nella redazione dei calcoli statici ci si è attenuti alle prescrizioni della Normativa vigente; in particolare:

- **Legge n°1086 del 05/11/1971**  
 “Norme per la disciplina delle opere in conglomerato cementizio armato, normale e precompresso ed a struttura metallica”
- **Legge n°64 del 02/02/1974:**“Provvedimenti per le costruzioni con particolari prescrizioni per le zone sismiche”
- **Decreto Ministeriale 14/01/2008**  
 “Norme tecniche per le costruzioni”
- **Circolare Min. 02/02/2009, n° 617**  
 “Istruzioni per l’applicazione delle Norme tecniche per le costruzioni di cui al D.M. 14/01/2008”
- **UNI EN 1993-1-1:2005**  
 “Eurocodice 3 – Progettazione delle strutture in acciaio – Parte 1-1: Regole generali e regole per gli edifici”
- **UNI EN 1993-1-5:2007**  
 “Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 1-5: Elementi strutturali a lastra”
- **UNI EN 1993-1-9:2005**  
 “Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 1-9: Fatica”
- **UNI EN 1993-1-10:2005**

“Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 1-10: Resilienza del materiale e proprietà attraverso lo spessore”

**- UNI EN 1993-2:2007**

“Eurocodice 3 - Progettazione delle strutture in acciaio – Parte 2: Ponti in acciaio”

**- UNI EN 1994-1-1:2005**

“Eurocodice 4 - Progettazione delle strutture composte acciaio-calcestruzzo – Parte 1-1: Regole generali e regole per gli edifici”

**- UNI EN 1994-2:2006**

“Eurocodice 4 - Progettazione delle strutture composte acciaio-calcestruzzo – Parte 2: Regole generali e regole per i ponti”

**1.3. MATERIALI**

I materiali di previsto impiego sono elencati nelle seguenti tabelle:

<b>Calcestruzzo</b> (rif. par. 11.2.10 DM 14.01.2008)		
Resistenza caratteristica cubica a 28 gg del calcestruzzo <b>R<sub>ck</sub></b>	<b>40</b>	N/mm <sup>2</sup>
Resistenza caratteristica cilindrica	<b>32</b>	N/mm <sup>2</sup>
Resistenza cilindrica media <b>f<sub>cm</sub></b> = f <sub>ck</sub> +8	40.00	N/mm <sup>2</sup>
Resistenza media a trazione semplice assiale <b>f<sub>ctm</sub></b> = 0,30*f <sub>ck</sub> ^(2/3)	3.02	N/mm <sup>2</sup>
Resistenza caratteristica a trazione semplice assiale <b>f<sub>ctk</sub></b> = 0,7*f <sub>ctm</sub>	2.12	N/mm <sup>2</sup>
Resistenza media a trazione per flessione <b>f<sub>ctm</sub></b> = 1,2*f <sub>ctm</sub>	3.63	N/mm <sup>2</sup>
Modulo elastico secante tra 0 e 0,40 f <sub>cm</sub> <b>E<sub>cm</sub></b> = 22000 (f <sub>cm</sub> /10)^0,3	33346	N/mm <sup>2</sup>
Coefficiente di Poisson (cls fessurato/non fessurato)	0/0,2	
Coefficiente di dilatazione termica <b>α</b>	1.0E-5	1/C°
Coefficiente riduttivo per le resistenze di lunga durata <b>α<sub>cc</sub></b>	0.85	
Coefficiente parziale di sicurezza relativo al calcestruzzo <b>γ<sub>c</sub></b>	1.5	
Resistenza di calcolo a compressione del calcestruzzo <b>f<sub>cd</sub></b> = α <sub>cc</sub> f <sub>ck</sub> / γ <sub>c</sub> (per spessori > 50 mm)	18.13	N/mm <sup>2</sup>
Resistenza di calcolo a compressione del calcestruzzo <b>f<sub>cd</sub></b> = 0.8 α <sub>cc</sub> f <sub>ck</sub> / γ <sub>c</sub> (per spessori ≤ 50 mm)	14.51	N/mm <sup>2</sup>
Resistenza di calcolo a trazione del calcestruzzo <b>f<sub>ctd</sub></b> = f <sub>ctk</sub> / γ <sub>c</sub> (per spessori > 50 mm)	1.41	N/mm <sup>2</sup>
Resistenza di calcolo a trazione del calcestruzzo <b>f<sub>ctd</sub></b> = 0.8 f <sub>ctk</sub> / γ <sub>c</sub> (per spessori ≤ 50 mm)	1.13	N/mm <sup>2</sup>
Stati Limite di Esercizio: Tensione massima di compressione per combinazioni caratteristiche <b>σ<sub>c</sub></b> = 0.6 f <sub>ck</sub>	19.20	N/mm <sup>2</sup>
Stati Limite di Esercizio: Tensione massima di compressione per combinazioni quasi permanenti <b>σ<sub>c</sub></b> = 0.45 f <sub>ck</sub>	14.40	N/mm <sup>2</sup>
Tensione tangenziale di aderenza caratteristica acciaio-calcestruzzo <b>f<sub>bk</sub></b> = 2,25 η f <sub>ctk</sub> (η =1)	4.76	N/mm <sup>2</sup>
Tensione tangenziale di aderenza di calcolo acciaio-calcestruzzo <b>f<sub>bd</sub></b> = f <sub>bk</sub> / γ <sub>c</sub>	3.18	N/mm <sup>2</sup>
<b>Acciaio per cemento armato B450C</b> (rif. par. 11.3.2.1 DM 14.01.2008)		
Valore nominale della tensione caratteristica di snervamento <b>f<sub>y,nom</sub></b>	<b>450</b>	N/mm <sup>2</sup>
Valore nominale della tensione caratteristica di rottura <b>f<sub>t,nom</sub></b>	<b>540</b>	N/mm <sup>2</sup>
Coefficiente parziale di sicurezza relativo all'acciaio <b>γ<sub>s</sub></b>	1.15	
Resistenza di calcolo dell'acciaio <b>f<sub>yd</sub></b> = f <sub>y</sub> / γ <sub>s</sub>	391.3	N/mm <sup>2</sup>
Stati Limite di Esercizio: Tensione massima per combinazioni caratteristiche <b>σ<sub>s</sub></b> = 0.8 f <sub>y</sub>	360.0	N/mm <sup>2</sup>

Acciaio per strutture (rif. par. 11.3.4 DM 14.01.2008)		
<b>Tipo acciaio</b>	<b>S 355 W</b>	
Modulo elastico <b>E</b>	210000	N/mm <sup>2</sup>
Coefficiente di Poisson <b>v</b>	0.3	
Modulo di elasticità trasversale <b>G</b>	80769	N/mm <sup>2</sup>
Coefficiente di dilatazione termica <b>α</b>	1.2E-05	1/C°
Tensione caratteristica di snervamento <b>f<sub>yk</sub></b> (spessore ≤ 40 mm)	355	N/mm <sup>2</sup>
Tensione caratteristica di rottura <b>f<sub>tk</sub></b> (spessore ≤ 40 mm)	510	N/mm <sup>2</sup>
Tensione caratteristica di snervamento <b>f<sub>yk</sub></b> (40 mm < spessore ≤ 80 mm)	335	N/mm <sup>2</sup>
Tensione caratteristica di rottura <b>f<sub>tk</sub></b> (40 mm < spessore ≤ 80 mm)	490	N/mm <sup>2</sup>

Coefficienti di omogeneizzazione:

- sezioni composte acciaio/cls:  $m = E_s / E_c = 210000 / 33346 = 6.3$
- sezioni in cls. armato:  $m = 15$

Acciaio connettori trave/soletta tipo "Nelson"

tensione caratteristica di snervamento  
tensione caratteristica di rottura:

tipo: S 235 J2+C450  
 $f_{yk} \geq 350 \text{ N/mm}^2$   
 $f_{tk} \geq 450 \text{ N/mm}^2$

## 2 MODELLAZIONE STRUTTURALE

Il viadotto è stato schematizzato in un modello ad elementi finiti di tipo "trave" (beam), risolto mediante il programma di calcolo "MIDAS/Civil 2015", versione 1.2, realizzato da "MIDAS Information Technology, Co., Ltd.; Areum B/D 4° floor, 258-1 Seohyeon-dong, Bundang-gu, Seongnam, Gyeonggi-do, 463-824, Korea"; distribuito in Italia da "CSPFea s.c., via Zuccherificio 5/D, 35042 Este (PD)".

L'affidabilità del codice di calcolo è assicurata da una vasta documentazione teorica e di supporto e da una serie di esempi di verifica in cui i risultati ottenuti sono confrontati con risultati teorici tratti dalla letteratura specialistica o, in mancanza, con risultati di altri codici di calcolo indipendenti.

Il programma, ancorché utilizzabile come codice agli "elementi finiti" di tipo generale, è stato scelto perché volto soprattutto al calcolo di ponti (semplici, continui, strallati, sospesi, ecc.) consentendo tra l'altro la gestione di fasi costruttive successive, l'introduzione di cavi di precompressione, il calcolo delle relative perdite immediate (attrito) e differite (ritiro, fluage, rilassamento) e una gestione semi-automatica di ricerca degli effetti massimi e minimi dovuti ai carichi mobili, nonché la possibilità di effettuare analisi dinamiche lineari con spettro di risposta o dinamiche non lineari (time-history).

La modellazione è stata estesa all'impalcato e ai fusti delle pile, la base dei fusti è considerata incastrata; le spalle sono solamente descritte con un vincolo di incastro.

L'analisi globale della struttura è condotta secondo il metodo "Elastico"; tutti gli elementi strutturali sono considerati indefinitamente elastici.

Viene condotta un'analisi statica per le condizioni di carico relative a permanenti, carichi mobili, frenamento, vento, carichi termici, e un'analisi dinamica lineare per gli effetti delle azioni sismiche.

Si allegano i dati della modellazione strutturale e la spiegazione delle convenzioni usate nei files di dati del programma.

L'impalcato segue longitudinalmente l'asse coordinato X (da sinistra verso destra), le pile seguono l'asse Z (dall'alto verso il basso); l'asse Y (trasversale) forma con i precedenti una terna destrorsa.

I nodi degli elementi strutturali sono posizionati secondo la disposizione reale; quando necessario sono tra loro collegati da "vincoli rigidi".

La numerazione degli elementi è la seguente (xxx: numeri variabili):

- travi principali: da 101÷401 a 140÷440 ordinati per X crescente (+1) e quindi per Y crescente (+100 per ogni trave)
- trasversi intermedi: 1xxx, ordinati per Y, Z e X crescenti
- trasversi su pile: 2xxx, ordinati per Y, Z, e X crescenti
- controventi: 3xxx, ordinati per X, Y e Z crescenti
- pile: 4xxx, ordinati per Z crescente e quindi per X crescente

Per la ripartizione trasversale dei carichi si sono inoltre schematizzati elementi trasversali fittizi rappresentanti la soletta.

La soletta considerata nelle sezioni composte è limitata alla parte gettata in opera, esclusa quindi la lastra prefabbricata inferiore, di 6 cm di spessore.

Si è assunta un'accelerazione gravitazionale pari a  $g = 9,806 \text{ m/s}^2$ .

Le unità di misura utilizzate sono coerenti con il Sistema Internazionale:

lunghezze:	m	(metri)
masse:	t	(tonnellate)
forze	kN	(kilo-Newton)

I materiali usati nella modellazione hanno le caratteristiche di calcolo seguenti:

b) calcestruzzo (C32/40):

- modulo di elasticità:  $E = 33345 \text{ N/mm}^2$
- coefficiente di Poisson:  $\nu = 0,20$
- coefficiente di dilatazione termica:  $\alpha = 0.00001 \text{ C}^{-1}$
- peso specifico:  $\gamma = 25 \text{ kN/m}^3$
- massa specifica:  $m = 2.55 \text{ t/m}^3$

c) acciaio strutturale viadotto (S355W):

- modulo di elasticità:  $E = 210000 \text{ N/mm}^2$
- coefficiente di Poisson:  $\nu = 0.30$
- coefficiente di dilatazione termica:  $\alpha = 0.000012 \text{ C}^{-1}$
- peso specifico(\*):  $\gamma = 88.6 \text{ kN/m}^3$
- massa specifica(\*):  $m = 9.03 \text{ t/m}^3$

NOTA (\*): le caratteristiche volumiche dell'acciaio son aumentate del 15% per tener conto degli elementi non schematizzati nel modello (irrigidimenti, piastre, bulloni, saldature, connettori).

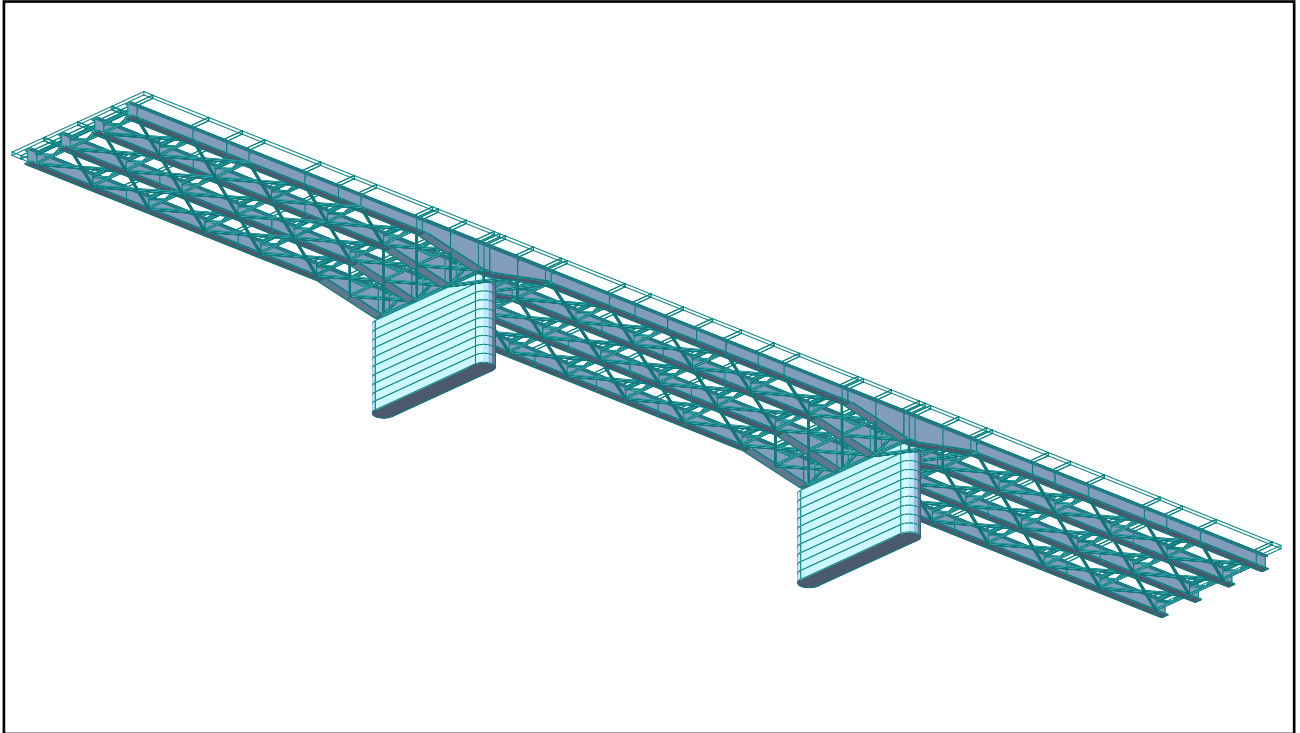


FIGURA 3 - SCHEMA DI CALCOLO – ASSONOMETRIA

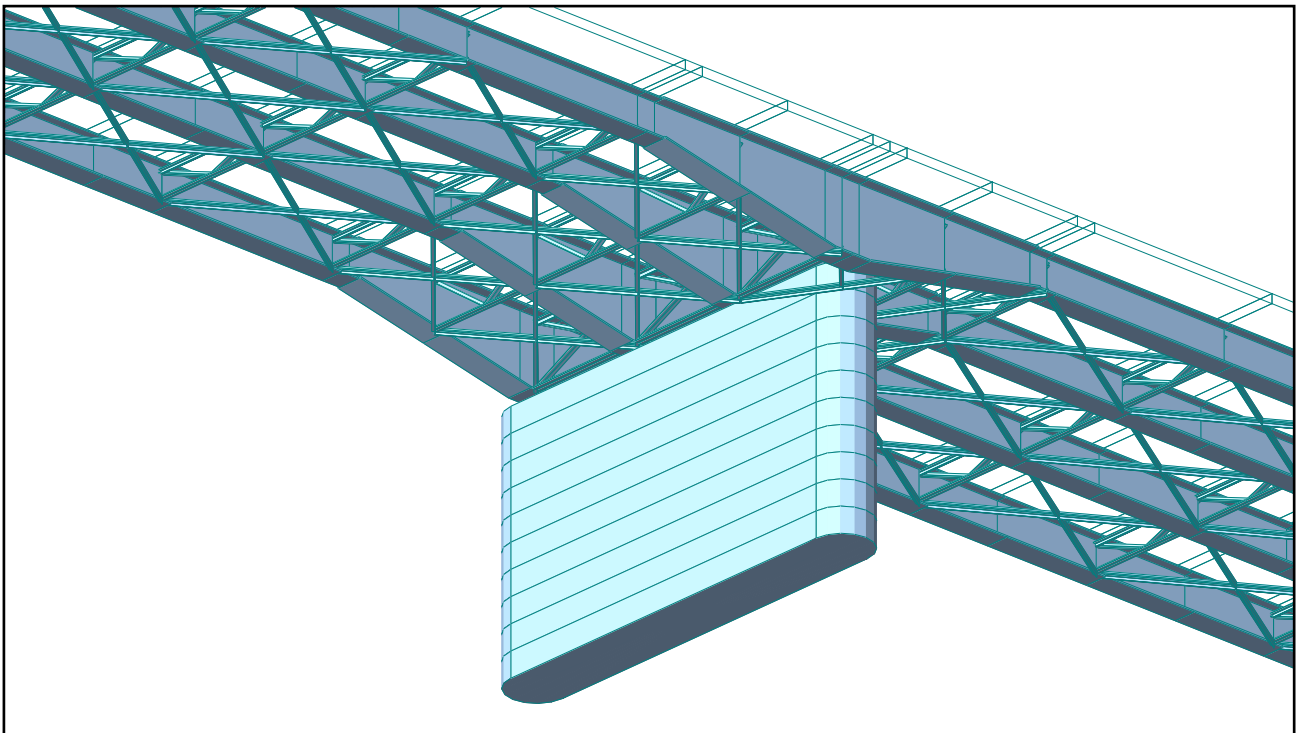


FIGURA 4 - SCHEMA DI CALCOLO – PARTICOLARE SU PILA

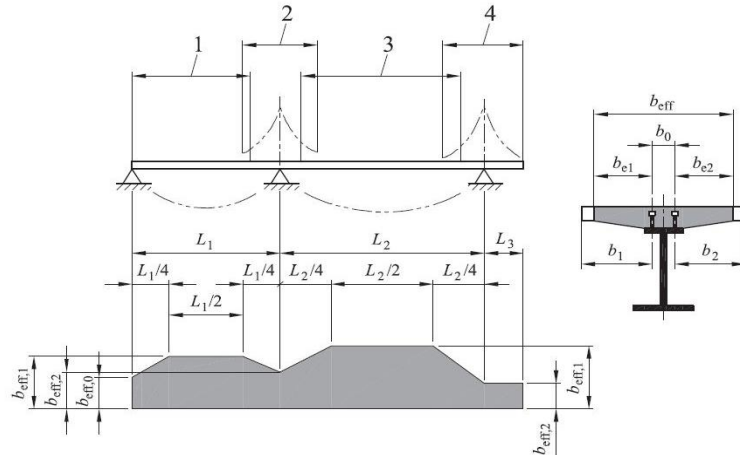
## 2.1 LARGHEZZE "EFFICACI" DI SOLETTA COLLABORANTE

Nel calcolo delle sezioni composte si tiene conto di una larghezza di soletta collaborante determinata secondo NTC 2008 # 4.3.2.3; il calcolo è riassunto nella seguente tabella.

TRAVE CONTINUA - CALCOLO DELLE LARGHEZZE EFFICACI DI SOLETTA COLLABORANTE - NTC 2008, § 4.3.2.3.

Legenda

- 1  $L_e = 0,85L_1$  per  $b_{eff,1}$
- 2  $L_e = 0,25(L_1 + L_2)$  per  $b_{eff,2}$
- 3  $L_e = 0,70L_2$  per  $b_{eff,1}$
- 4  $L_e = 2L_3$  per  $b_{eff,2}$



CAVALCAVIA ASSE 80 TRAVI ESTERNE	Appoggio / Campata	Estremità / Intermedio	Luce (m)	b <sub>0</sub> (m)	b <sub>1</sub> (m)	b <sub>2</sub> (m)	L <sub>e</sub> (m)	L <sub>e</sub> / 8 (m)	b <sub>e1</sub> (m)	b <sub>e2</sub> (m)	β <sub>1</sub>	β <sub>2</sub>	b <sub>eff</sub> (m)
Spalla 1	Appoggio	Estremità		0.300	1.450	1.700	26.350	3.294	1.450	1.700	1.000	0.938	3.344
	Campata	Estremità	31.000	0.300	1.450	1.700	26.350	3.294	1.450	1.700	1.000	1.000	3.450
Pila 1	Appoggio	Intermedio		0.300	1.450	1.700	17.250	2.156	1.450	1.700	1.000	1.000	3.450
	Campata	Intermedio	38.000	0.300	1.450	1.700	26.600	3.325	1.450	1.700	1.000	1.000	3.450
Pila 2	Appoggio	Intermedio		0.300	1.450	1.700	17.250	2.156	1.450	1.700	1.000	1.000	3.450
	Campata	Estremità	31.000	0.300	1.450	1.700	26.350	3.294	1.450	1.700	1.000	1.000	3.450
Spalla 2	Appoggio	Estremità		0.300	1.450	1.700	26.350	3.294	1.450	1.700	1.000	0.938	3.344

CAVALCAVIA ASSE 80 TRAVI INTERNE	Appoggio / Campata	Estremità / Intermedio	Luce (m)	b <sub>0</sub> (m)	b <sub>1</sub> (m)	b <sub>2</sub> (m)	L <sub>e</sub> (m)	L <sub>e</sub> / 8 (m)	b <sub>e1</sub> (m)	b <sub>e2</sub> (m)	β <sub>1</sub>	β <sub>2</sub>	b <sub>eff</sub> (m)
Spalla 1	Appoggio	Estremità		0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200
	Campata	Estremità	31.000	0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200
Pila 1	Appoggio	Intermedio		0.300	1.450	1.450	17.250	2.156	1.450	1.450	1.000	1.000	3.200
	Campata	Intermedio	38.000	0.300	1.450	1.450	26.600	3.325	1.450	1.450	1.000	1.000	3.200
Pila 2	Appoggio	Intermedio		0.300	1.450	1.450	17.250	2.156	1.450	1.450	1.000	1.000	3.200
	Campata	Estremità	31.000	0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200
Spalla 2	Appoggio	Estremità		0.300	1.450	1.450	26.350	3.294	1.450	1.450	1.000	1.000	3.200

### 3 DISPOSITIVI DI APPOGGIO E ISOLAMENTO

Alla sommità delle pile e spalle sono disposti apparecchi di appoggio antisismici costituiti da isolatori elastomerici armati, costituiti da strati alternati di lamiera di acciaio ed elastomero, collegati mediante vulcanizzazione.

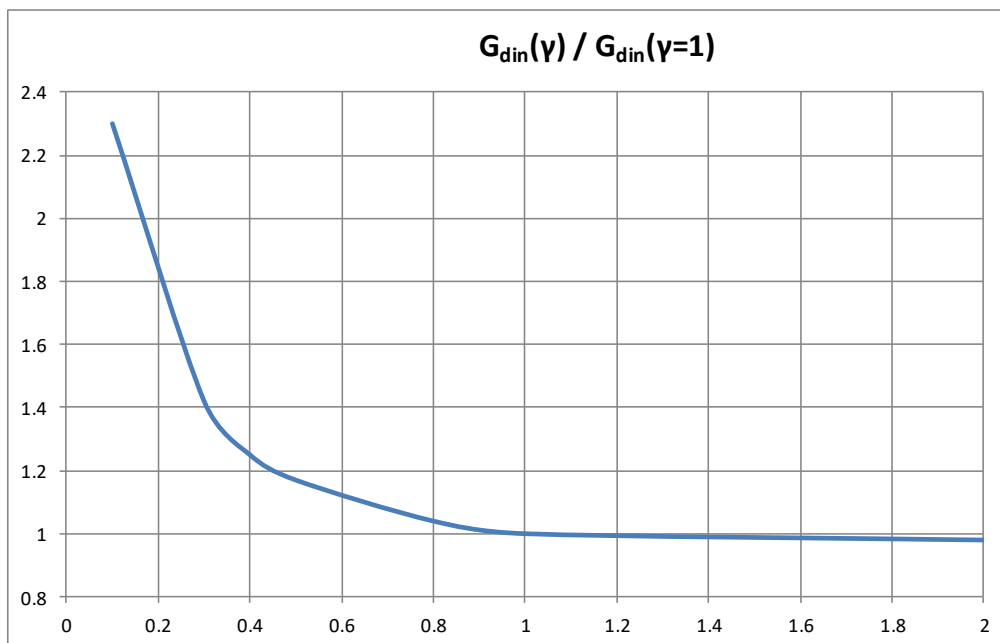
Gli isolatori sono caratterizzati da una elevata rigidità verticale, una ridotta rigidità orizzontale e una modesta capacità dissipativa; queste caratteristiche consentono, rispettivamente, di sostenere i carichi verticali senza apprezzabili cedimenti, di contenere gli spostamenti sismici orizzontali dell'impalcato e aumentare i periodi di vibrazione dell'impalcato in modo da limitare le forze dinamiche orizzontali trasmesse dall'impalcato alle pile/spalle.

Le caratteristiche di progetto degli isolatori sono riportate nella seguente tabella



Caratteristiche di progetto isolatore	SPALLE		PILE	
Diametro elastomero	Øg=	300	450	mm
Spessore totale elastomero	te=	76	78	mm
Altezza totale escluse piastre di ancoraggio	h=	152	154	mm
Altezza totale comprese piastre di ancoraggio	H=	202	204	mm
Lato piastre di ancoraggio	Z=	350	500	mm
Modulo di elasticità tang. dinamico elastomero ( $\gamma = d/te = 1$ )	G(din)=	1.4	1.4	N/mm <sup>2</sup>
Deformazione massima statica di progetto SLU/SLD ( $d/te \leq 1$ )	du=	76	78	mm
Deformazione massima sismica di progetto SLC ( $d/te \leq 2$ )	dc=	152	156	mm
Rigidezza orizzontale equivalente ( $\gamma = d/te = 1$ )	Ke=	1.30	2.85	kN/mm
Rigidezza verticale	Kv=	768	1794	kN/mm

Il modulo tangenziale dinamico e quindi la rigidezza orizzontale equivalente sono definiti per una deformazione di taglio  $\gamma = \frac{\Delta}{t_e} = 1$ ;  $G_{din}$  varia molto nel campo  $\gamma < 1$  mentre per valori  $1 < \gamma < 2$  risulta pressoché costante; la variazione media del modulo dinamico tangenziale in funzione della deformazione di taglio è rappresentata nel seguente grafico.



## 4 ANALISI DEI CARICHI

Qui di seguito si riporta l'analisi dei carichi eseguita distinguendo tra carichi permanenti ed accidentali.

### 4.1 CARICHI PERMANENTI

a) calcestruzzo pile:

- peso specifico:  $\gamma = 25 \text{ kN/m}^3$
- massa specifica:  $m = 2.55 \text{ t/m}^3$

b) acciaio strutturale: si definisce un incremento forfaitario del 15% per tener conto degli elementi non rappresentati nel modello (piastrame, bulloneria, saldature, ecc.):

- peso specifico:  $\gamma = 88.5 \text{ kN/m}^3$

- massa specifica:  $m = 9.03 \text{ t/m}^3$

c) peso soletta: il peso viene assegnato sulle singole travi (sezione reagente solo acciaio):

- travi esterne:  $g_1 = 25 \times (0.30 + 0.34) / 2 \times (1.85 + 1.60) = 27.60 \text{ kN/m}$

- travi interne:  $g_1 = 25 \times (0.34 + 0.42) / 2 \times 3.20 = 30.40 \text{ kN/m}$

#### 4.2 SOVRACCARICHI PERMANENTI (FINITURE)

Le finiture vengono assegnate sulle singole travi (con soletta collaborante) con la loro eccentricità Y rispetto all'asse trave:

- trave esterna:

- pavimentazione:	$g_2 = 2.5 \times 1.55 = 3.88 \text{ kN/m}$	$e = -0.825 \text{ m}$
- cordolo:	$g_2 = 25 \times 0.15 \times 1.90 = 7.13 \text{ kN/m}$	$e = +0.90 \text{ m}$
- veletta:	$g_2 = 25 \times 0.1 \times 0.8 = 2 \text{ kN/m}$	$e = +1.875 \text{ m}$
- sicurvia:	$g_2 = 0.8 \text{ kN/m}$	$e = +0.35 \text{ m}$
- parapetto:	$g_2 = 0.5 \text{ kN/m}$	$e = +1.75 \text{ m}$
- Totale finiture:	$g_2 = 14.31 \text{ kN/m}$	$e = +0.57 \text{ m}$

- travi interne: - pavimentazione:  $g_2 = 2.5 \times 3.2 = 8 \text{ kN/m}$   $e = 0$

#### 4.3 CARICHI MOBILI DI ESERCIZIO

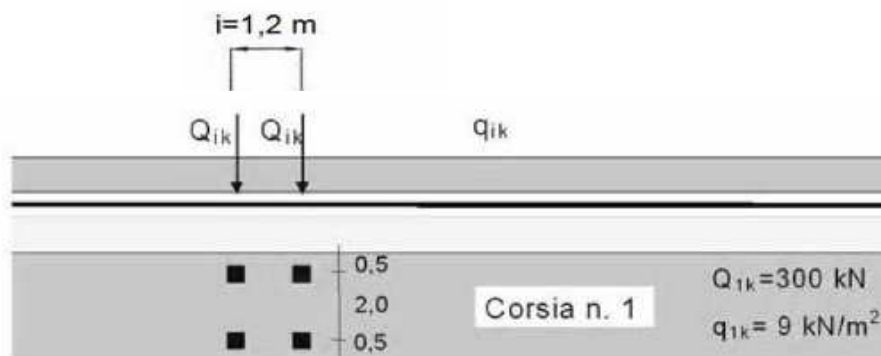
L'impalcato si considera caricato secondo lo schema di carico 1 previsto dal D.M. 14/02/2008:

- una colonna di carico costituita da:

$Q_{1k}$  mezzo convenzionale da 600 kN a due assi

$q_{1k}$  carico ripartito pari a  $9 \text{ kN/m}^2$  ( $27 \text{ kN/m}$ )

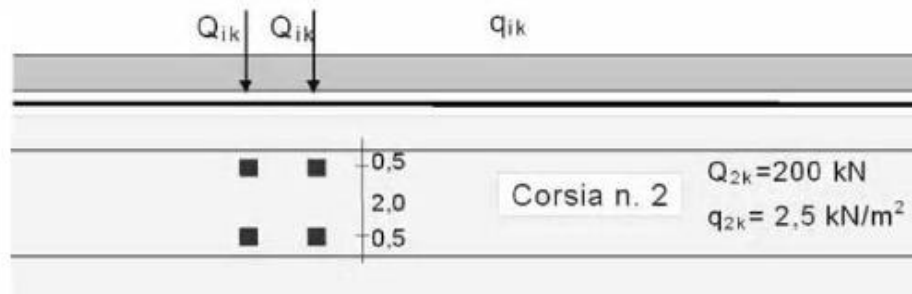
Lo schema longitudinale della colonna è il seguente:



- una seconda colonna di carico analoga alla precedente ma con carichi pari a:

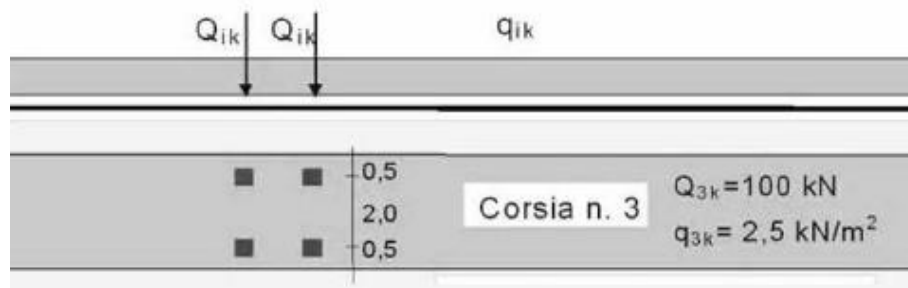
$Q_{2k}$  mezzo convenzionale da 400 kN a due assi

$q_{2k}$  carico ripartito pari a  $2.5 \text{ kN/m}^2$  ( $7.5 \text{ kN/m}$ )



- una terza colonna di carico analoga alla precedente ma con carichi pari a:

- $Q_{2k}$       mezzo convenzionale da 200 kN a due assi
- $q_{2k}$       carico ripartito pari a 2.5 kN/m<sup>2</sup> (7.5 kN/m)



L'area rimanente viene caricata con un carico  $q_{rk}$  pari a 2.5 kN/m<sup>2</sup>.

Sui marciapiedi si assume lo schema di carico 5, pari a 5.0 kN/m<sup>2</sup> che, in combinazione con i precedenti, assume il valore di 2.5 kN/m<sup>2</sup>.

Lo schema complessivo di carico è rappresentato nelle seguenti figure, fermo restando che le corsie di carico possono essere scambiate o trascurate se più gravose per l'elemento considerato.

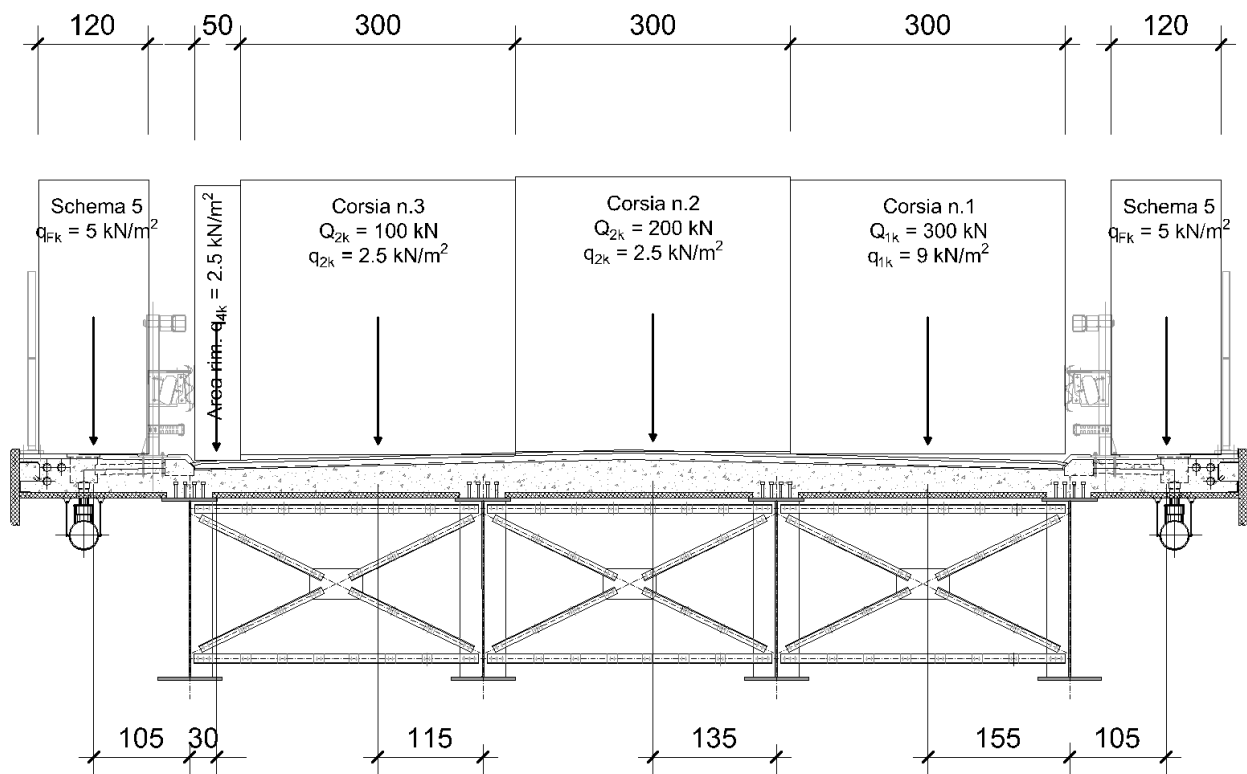


FIGURA 5 – CARICHI VARIABILI – DISPOSIZIONE A DX

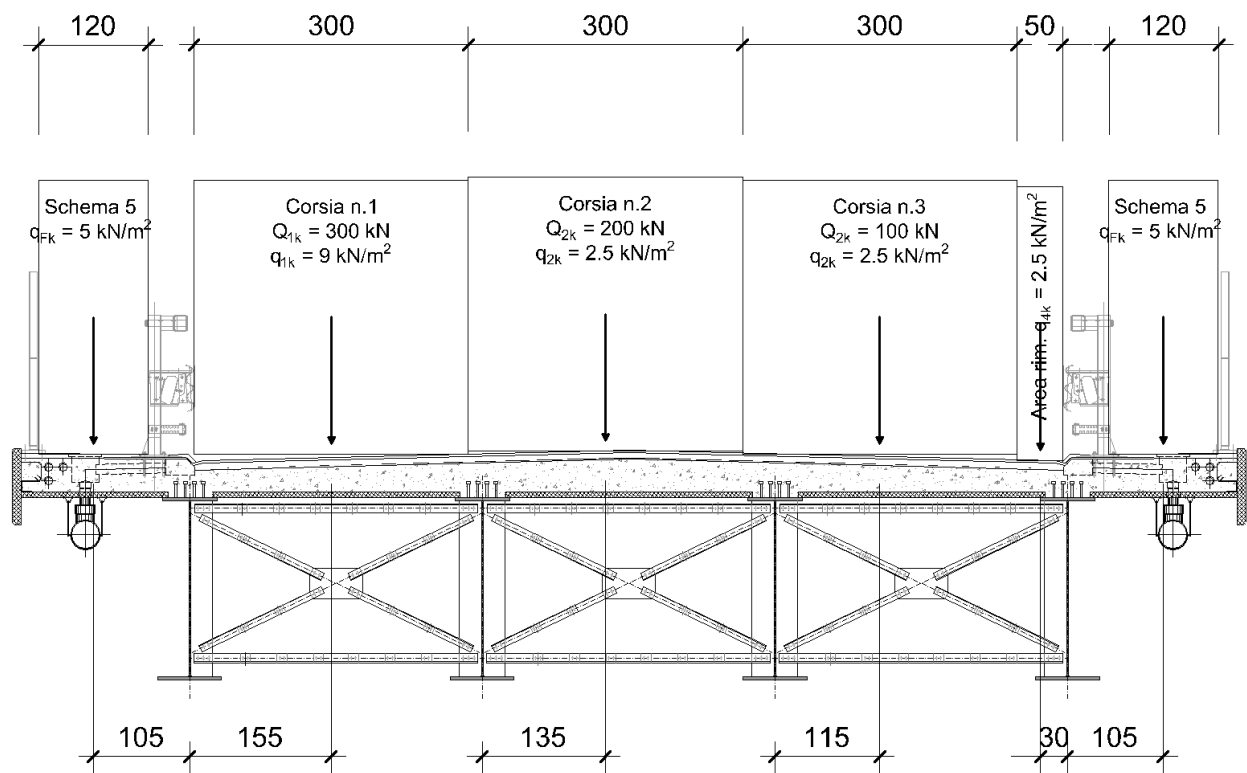


FIGURA 6 – CARICHI VARIABILI – DISPOSIZIONE A Sx

Il D.M. del 2008, in accordo con quanto previsto dagli eurocodici, considera il coefficiente dinamico già compreso nel valore dei carichi mobili. La disposizione longitudinale e trasversale più gravosa dei carichi viene determinata automaticamente dal programma di calcolo per ogni sezione e componente di sollecitazione massima e minima.

Per la valutazione degli effetti dei carichi mobili agli stati limite di esercizio si applicano ai carichi i coefficienti di combinazione (NTC2008):

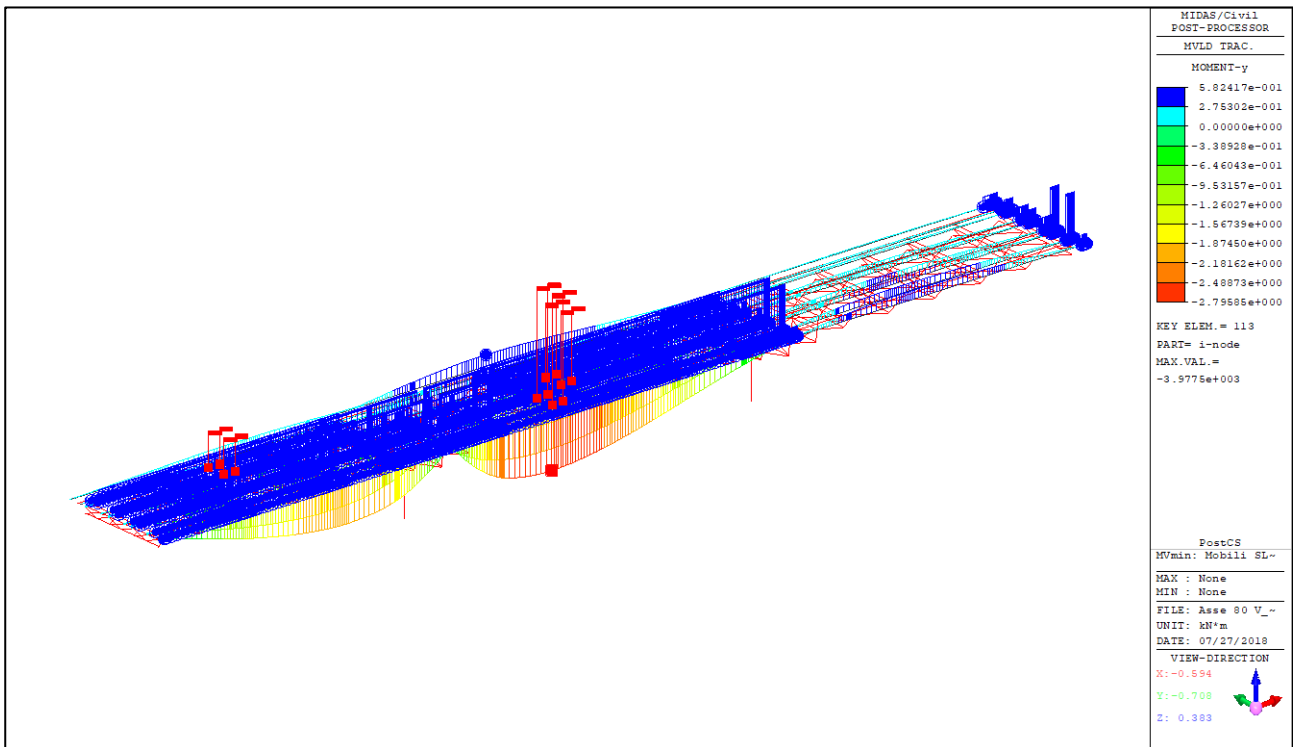
- Carichi tandem:  $\psi_0 = 0.75$      $\psi_1 = 0.75$      $\psi_2 = 0$
- Carichi distribuiti:  $\psi_0 = 0.4$      $\psi_1 = 0.4$      $\psi_2 = 0$

L'applicazione dei carichi mobili al modello strutturale è composta dei seguenti passi:

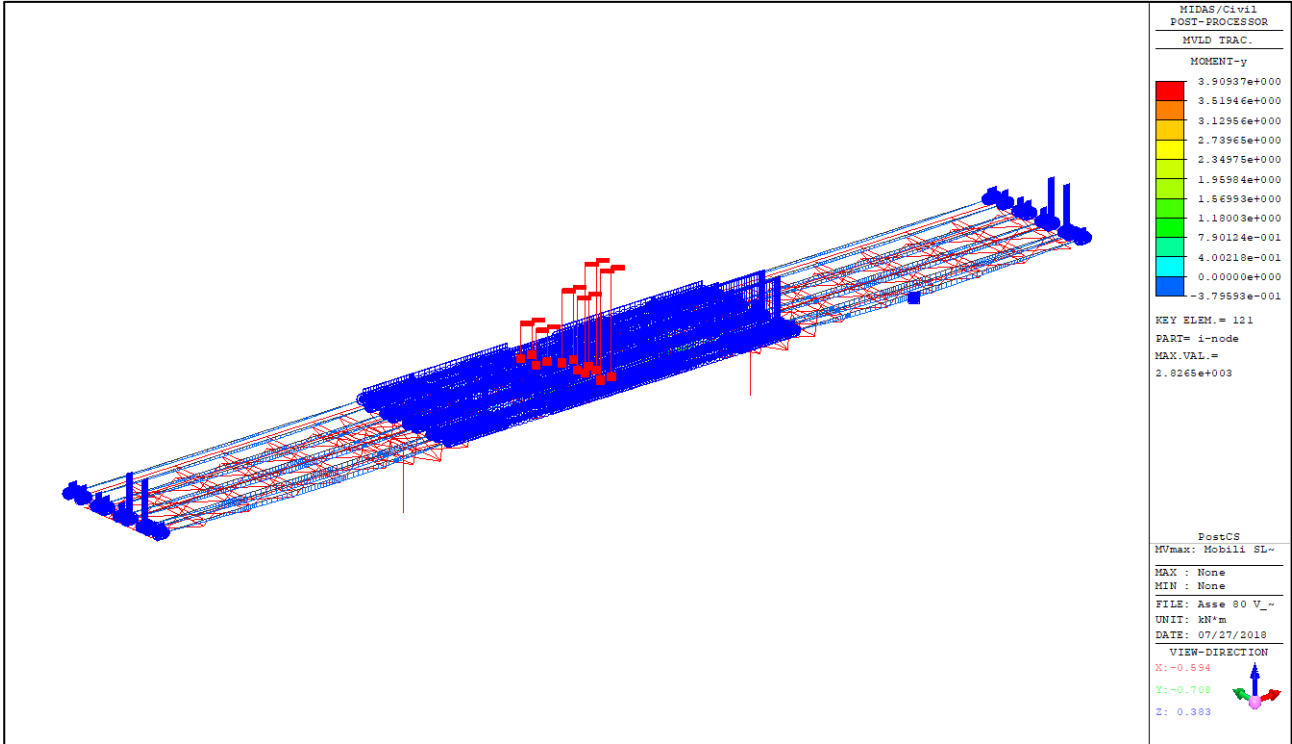
- definizione delle corsie di carico (Traffic Line Lanes) come sequenza di elementi (travi composte), caricate con la effettiva eccentricità trasversale;
- definizione degli schemi di carico transitanti sulle corsie e/o marciapiedi (Vehicles)
- definizione dei casi di carico (Moving Load Cases); per ognuno di essi si definiscono le corsie da caricare e gli schemi di carico transitanti e se considerare i coefficienti  $\psi$  (SLE) o no (SLU);
- definizione della discretizzazione dei punti di carico, assunto pari a 1/10 della lunghezza del singolo elemento caricato, nel presente caso quindi 25÷50 cm.

Nel calcolo vengono quindi generate le linee d'influenza per ogni colonna di carico, ognuna delle quali provoca il massimo o il minimo di una singola componente di sforzo (N, Vy, Vz, Mx, My, Mz) per un nodo di un elemento del modello; ciò è esteso a tutti i nodi ed elementi presenti nel modello di calcolo; ovviamente se viene ricercato il massimo vengono caricati solo i tratti aventi linea d'influenza positiva e viceversa per i minimi.

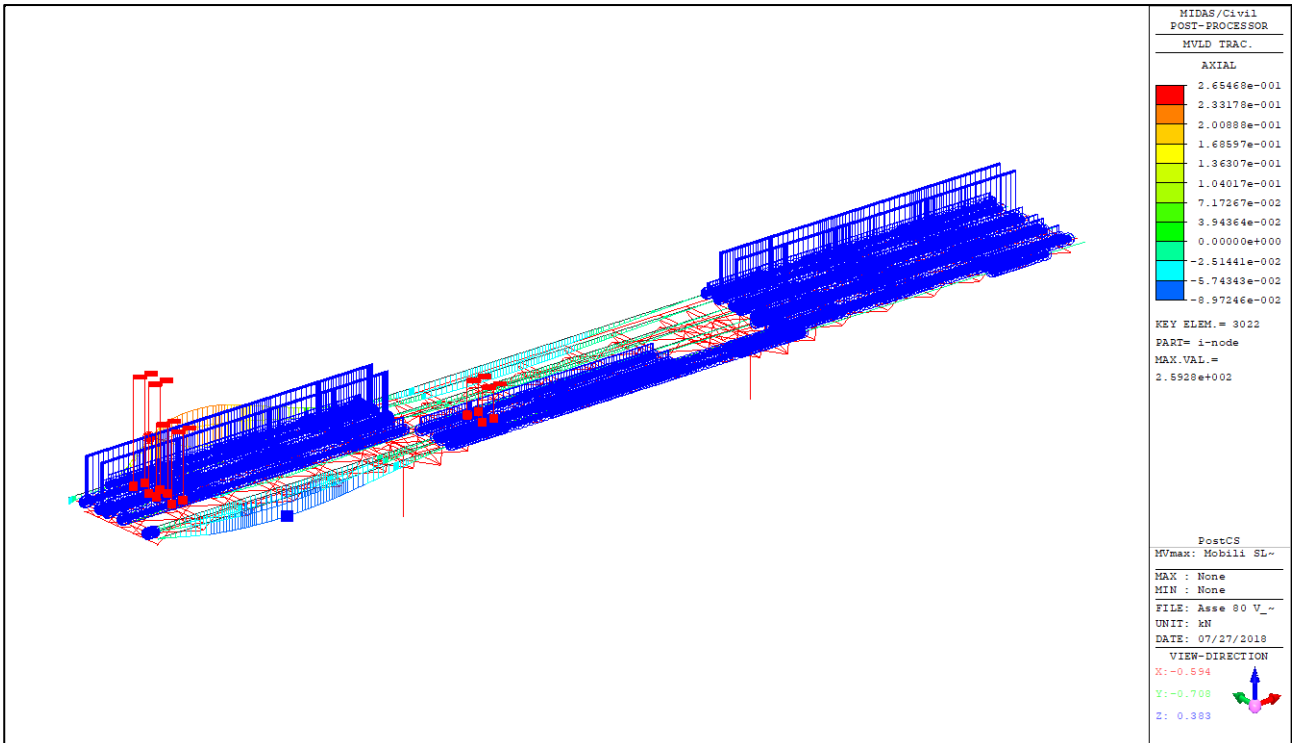
Seguono alcuni esempi grafici.



CARICHI MOBILI SLU A DX - DISPOSIZIONE MINIMO MY ELEMENTO 113, NODO INIZIALE (TRAVE ESTERNA, ASSE PILA)



CARICHI MOBILI SLU A SX - DISPOSIZIONE MASSIMO My ELEMENTO 121, NODO INIZIALE (TRAVE ESTERNA MEZZERIA)



CARICHI MOBILI SLU A SX - DISPOSIZIONE MASSIMO Fx ELEMENTO 3022, NODO INIZIALE (CONTROVENTO)



#### 4.4 CARICHI MOBILI PER VERIFICHE A FATICA

##### 4.4.1 VERIFICHE PER VITA ILLIMITATA

Le verifiche a fatica per vita illimitata (NTC 5.1.4.3) vengono eseguite per le anime e le saldature delle travi.

L'impalcato si considera caricato secondo il modello di carico a fatica 2, applicato sulla corsia lenta; il veicolo più gravoso viene determinato automaticamente considerando i massimi e minimi dell'inviluppo delle sollecitazioni ottenute per ciascun veicolo.

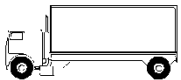




SAGOMA del VEICOLO		Distanza tra gli assi (m)	Carico frequente per asse (kN)	Tipo di ruota (Tab. 5.1.IX)
		4,5	90	A
			190	B
		4,20	80	A
		1,30	140	B
		3,20	90	A
		5,20	180	B
		1,30	120	C
		1,30	120	C
		1,30	120	C
		3,40	90	A
		6,00	190	B
		1,80	140	B
		1,80	140	B
		4,80	90	A
		3,60	180	B
		4,40	120	C
		1,30	110	C
		1,30	110	C

FIGURA 7 – MODELLO DI CARICO AI FATICA N. 2

##### 4.4.2 VERIFICHE A DANNEGGIAMENTO

Le verifiche a danneggiamento si effettuano per i connettori trave/soletta (pioli Nelson).

L'impalcato si considera caricato secondo il modello di carico a fatica 3, applicato sulla corsia lenta; il modello consiste in 4 assi di 120 kN di peso (480 kN totali).

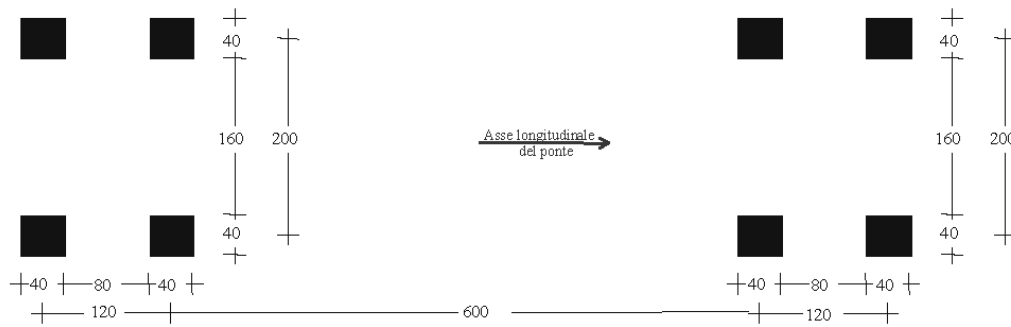


FIGURA 8 – MODELLO DI CARICO AI FATICA N.3

#### 4.5 AZIONE DEL VENTO TRASVERSALE

Nel D.M. 2008 si valuta l'azione del vento in base alla zona considerata e non ad un valore convenzionale; l'opera si trova in zona 1) Valle d'Aosta, Piemonte, Lombardia, Trentino Alto Adige, Veneto, Friuli Venezia Giulia (con l'eccezione della provincia di Trieste).



NTC 2008 - Figura 3.3.1

La pressione esterna è definita da:

$$p = q \cdot c_e \cdot c_p \cdot c_d$$

La pressione cinetica di riferimento  $q$  (in  $N/m^2$ ) in zona 1 è data dall'espressione:

$$q_b = \frac{1}{2} \cdot \rho \cdot v_b^2 = \frac{1}{2} \cdot 1.25 \cdot 25^2 = 391 \text{ N/m}^2 \quad (0.391 \text{ kN/m}^2)$$

dove:

$$\rho = 1.25 \text{ kg/m}^3 \quad \text{densità dell'aria, assunta costante}$$

$$v_b(T_R) \text{ è la velocità di riferimento del vento (in m/s)}$$

$$v_{b,0} = 25 \text{ m/s} \quad (a_s < a_0)$$

considerando un'altitudine sul livello del mare del sito:  $a_s = 70 \text{ m}$  e un periodo di ritorno di 50 anni.

$c_e$  è il coefficiente di esposizione; avendo una classe di rugosità D

Classi di rugosità del terreno

Classe di rugosità del terreno	Descrizione
A	Aree urbane in cui almeno il 15% della superficie sia coperto da edifici la cui altezza media superi i 15 m
B	Aree urbane (non di classe A), suburbane, industriali e boschive
C	Aree con ostacoli diffusi (alberi, case, muri, recinzioni); aree con rugosità non riconducibile alle classi A, B, D
D	Aree prive di ostacoli (aperta campagna, aeroporti, aree agricole, pascoli, zone paludose o sabbiose, superfici innevate o ghiacciate, mare, laghi,....)

in zona 1, si ottiene una categoria di esposizione II

ZONE 1,2,3,4,5						
	2 km	10 km	30 km	500m	750m	
A	--	IV	IV	V	V	V
B	--	III	III	IV	IV	IV
C	--	*	III	III	IV	IV
D	I	II	II	II	III	**
* Categoria II in zona 1,2,3,4 Categoria III in zona 5						
** Categoria III in zona 2,3,4,5 Categoria IV in zona 1						

a cui corrispondono i seguenti parametri per la definizione del coefficiente di esposizione

Parametri per la definizione del coefficiente di esposizione

Categoria di esposizione del sito	$k_r$	$Z_0$ (m)	$Z_{min}$ (m)
I	0,17	0,01	2
II	0,19	0,05	4
III	0,20	0,10	5
IV	0,22	0,30	8
V	0,23	0,70	12

Con l'opera in costruzione avente altezza massima sul terreno (compresi carichi mobili):

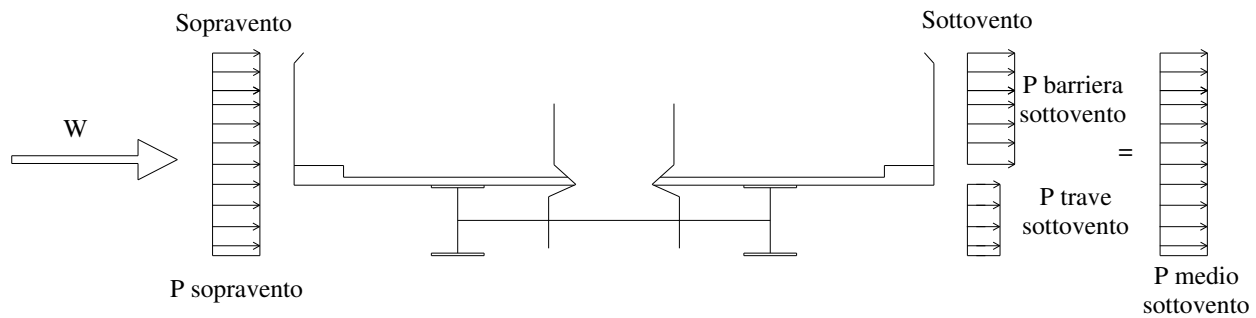
$$z \approx 12 \text{ m} > Z_{min} = 4 \text{ m}$$

$$c_e(z) = k_r^2 \cdot c_t \cdot \alpha \cdot [7 + c_t \cdot \alpha] = 0.19^2 \cdot 68.4 = 2.47$$

dove  $\alpha = \ln(z/Z_0) = \ln(12/0.05) = 5.48$ ;  $c_t = 1$

$c_d$  è il coefficiente dinamico; posto pari ad uno

$c_p$  è il coefficiente di forma; nel caso di travi multiple (Circ. 02/02/2009, C3.3.10.4) si ha, per i vari elementi:



- travi principali, altezza  $h = 1$  m, distanza reciproca media  $d = 3.2$  m:

a) lato sopravento:  $c_{p1} = 1.4$

b) lato sottovento:  $d / h = 3.2 / 1 = 3.2 \quad (2 < d/h < 5)$

$$\mu = 0.52$$

$$c_{p2} = 1.4 \times (0.52 + 0.52^2 + 0.52^3 + 0.52^4) = 1.41$$

- travi principali, altezza  $h = 2$  m, distanza reciproca media  $d = 3.2$  m:

a) lato sopravento:  $c_{p1} = 1.4$

b) lato sottovento:  $d / h = 3.2 / 2 = 1.6 \quad (d/h < 2)$

$$\mu = 0.2$$

$$c_{p2} = 1.4 \times (0.2 + 0.2^2 + 0.2^3 + 0.2^4) = 0.35$$

- soletta, cordolo, sicurvia: per il solo lato sopravento si ha:

$$c_p = 1.2$$

La pressione esterna sulle strutture è quindi definita dalle espressioni seguenti:

a) impalcato con travi  $h = 1$  m:

a1) lato sopravento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (1.4 \cdot 1.0 + 1.2 \cdot 1.45) = 1.35 + 1.68 = 3.03 \text{ kN/m}$

con eccentricità rispetto al piano stradale di riferimento:

$$e_z = (-1.35 \cdot 0.90 + 1.68 \cdot 0.27) / 3.03 = -0.25 \text{ m}$$

a2) lato sottovento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (1.41 \cdot 1.0 + 1.2 \cdot 1.15) = 1.36 + 1.33 = 2.69 \text{ kN/m}$

con eccentricità rispetto al piano stradale:

$$e_z = (-1.36 \cdot 0.90 + 1.33 \cdot 1.15) / 2.69 = +0.11 \text{ m}$$

b) impalcato con travi  $h = 2$  m:

b1) lato sopravento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (1.4 \cdot 2.0 + 1.2 \cdot 1.45) = 2.70 + 1.68 = 4.38 \text{ kN/m}$

con eccentricità rispetto al piano stradale di riferimento:

$$e_z = (-2.70 \cdot 1.40 + 1.68 \cdot 0.27) / 4.38 = -0.76 \text{ m}$$

b2) lato sottovento:  $p = q \cdot c_e \cdot c_p \cdot c_d = 0.391 \cdot 2.47 \cdot (0.35 \cdot 2.0 + 1.2 \cdot 1.15) = 0.68 + 1.33 = 2.01 \text{ kN/m}$

con eccentricità rispetto al piano stradale:

$$e_z = (-0.68 \cdot 1.40 + 1.33 \cdot 1.15) / 2.01 = +0.29 \text{ m}$$

Sulle pile si ha:  $p = 0.391 \cdot 2.47 \cdot 1.50 = 1.45 \text{ kN/m}$

La pressione dovuta ai carichi mobili, per l'altezza pari a 2.0 vale:

$$p = 0.391 \cdot 2.47 \cdot 1.2 \cdot 2.00 = 2.32 \text{ kN/m}$$

con eccentricità rispetto al piano stradale:

$$e_z = 1 + 2.00 / 2 = +2.00 \text{ m}$$

Vento su pile:  $0.391 \times 1.2 \times 2.47 \times 1.5 = 1.74 \text{ kN/m}$

#### 4.6 AZIONE DI FRENAMENTO

L'azione di frenamento è funzione del carico verticale totale agente sulla corsia convenzionale n. 1 ed è uguale a:

$$F_f = 0.6 \times (2 \times Q_{1k}) + 0.10 q_{1k} \times w_1 \times L =$$

$$= 0.6 \times (2 \times 300) + 0.10 \times 9 \times 3 \times 101.60 = 634.3 \text{ kN} \quad (180 \text{ kN} \leq F_f \leq 900 \text{ kN})$$

L'azione si considera uniformemente distribuita sulle travi principali:

$$q_x = \frac{634.3}{4 \cdot 101.6} = 1.56 \text{ kN/m/trave}$$

#### 4.7 DILATAZIONE TERMICA DIFFERENZIALE

Si assume che le travi dell'impalcato (sezioni miste acciaio/cls.) siano soggette alla variazione termica differenziale:  $\Delta T/H = \pm 5^\circ$ .

#### 4.8 DILATAZIONE TERMICA UNIFORME

Si assume che le travi dell'impalcato (sezioni miste acciaio/cls.) siano soggette alla variazione termica uniforme:  $\Delta T = \pm 15^\circ$ .

#### 4.9 AZIONI SISMICHE

La stima dei parametri spettrali necessari per la definizione dell'azione sismica è stata effettuata utilizzando le informazioni disponibili nel reticolo di riferimento (tabella 1 – Allegato B – D.M. 14 gennaio 2008).

Considerando l'ubicazione del sito in oggetto (Lat: 45.1671; Long: 9.6526) ed ipotizzando una costruzione caratterizzata da:

- una vita nominale di 50 anni, ricadente in classe d'uso pari a IV (ponti di importanza critica per il mantenimento delle vie di comunicazione...);
- una categoria topografica T1;
- una categoria C per il sottosuolo;

Si hanno i seguenti valori dei **parametri spettrali**:

STATO LIMITE	$T_R$ [anni]	$a_g$ [g]	$F_o$ [-]	$T_C$ [s]
SLO	60	0.0379	2.575	0.229
SLD	101	0.0463	2.541	0.257
SLV	949	0.1011	2.541	0.298
SLC	1950	0.1283	2.528	0.305

Le espressioni dello spettro elastico  $S_e$  di risposta secondo le NTC-08 sono le seguenti:

$$0 \leq T < T_B \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0 \cdot \left[ \frac{T}{T_B} + \frac{1}{\eta \cdot F_0} \left( 1 - \frac{T}{T_B} \right) \right]$$

$$T_B \leq T < T_C \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0$$

$$T_C \leq T < T_D \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0 \cdot \left( \frac{T_C}{T} \right)$$

$$T_D \leq T \quad S_e(T) = a_g \cdot S \cdot \eta \cdot F_0 \cdot \left( \frac{T_C \cdot T_D}{T^2} \right)$$

Trattandosi di struttura sismicamente isolata si assume:

- coefficiente di smorzamento viscoso convenzionale:  $\xi = 5 \%$
- fattore di smorzamento viscoso:  $\eta = \sqrt{\frac{10}{5 + \xi}} = 1$
- fattore di struttura:  $q = 1$

Lo spettro elastico definito viene ridotto per tutto il campo di periodi  $T \geq 0,8T_{is}$ , assumendo:

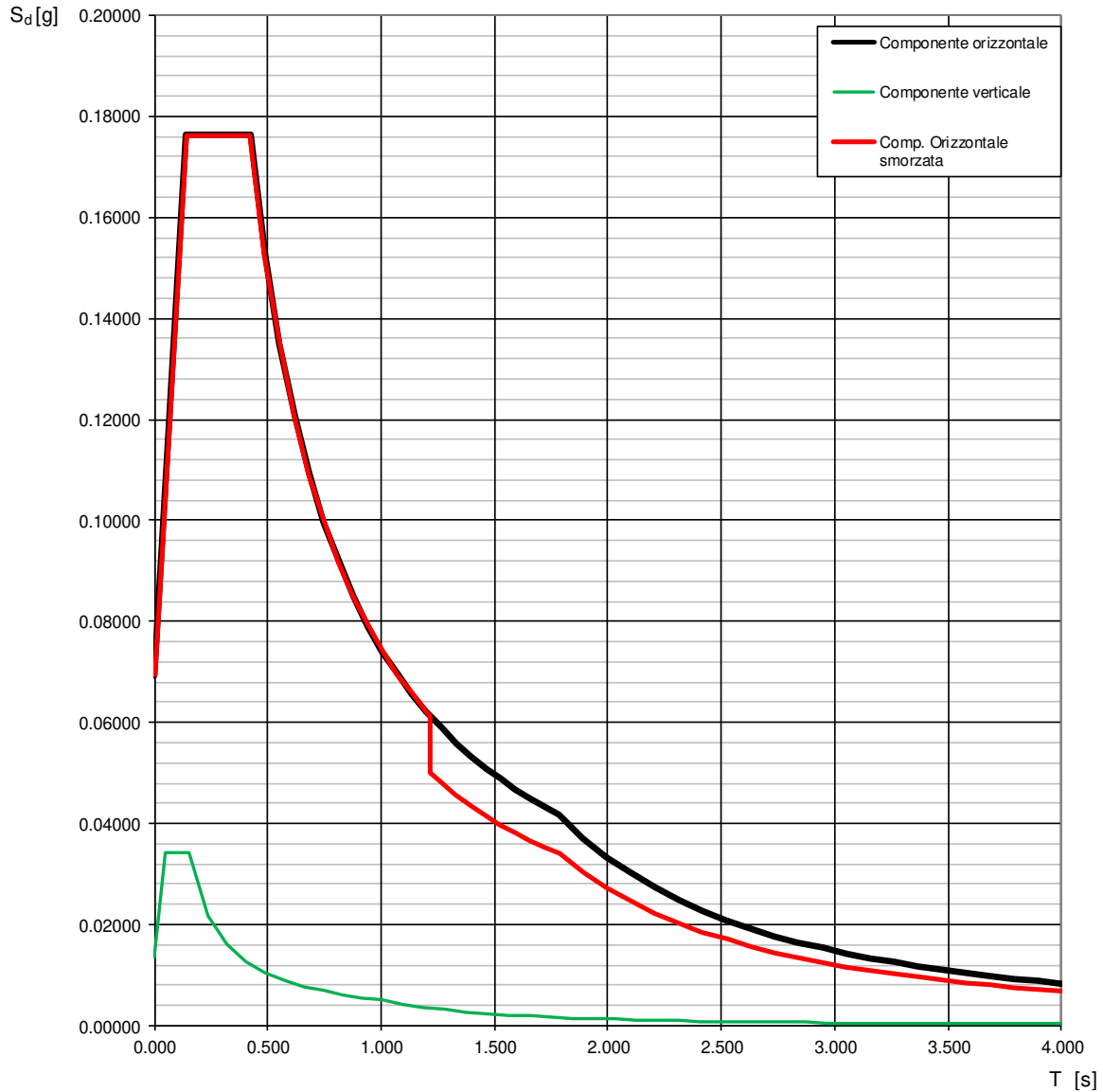
- periodo di vibrazione struttura isolata:  $T_{is} = 1.517 \text{ s}$
- coefficiente di smorzamento viscoso isolatori:  $\xi = 10 \%$

-coefficiente riduttivo  $\eta = \sqrt{\frac{10}{5 + \xi}} = \sqrt{\frac{10}{5 + 10}} = 0.816 \quad (\geq 0.55).$

Si ottengono i seguenti andamenti degli spettri. Noto il periodo (ascissa) si ricava il relativo coefficiente sismico (ordinata).

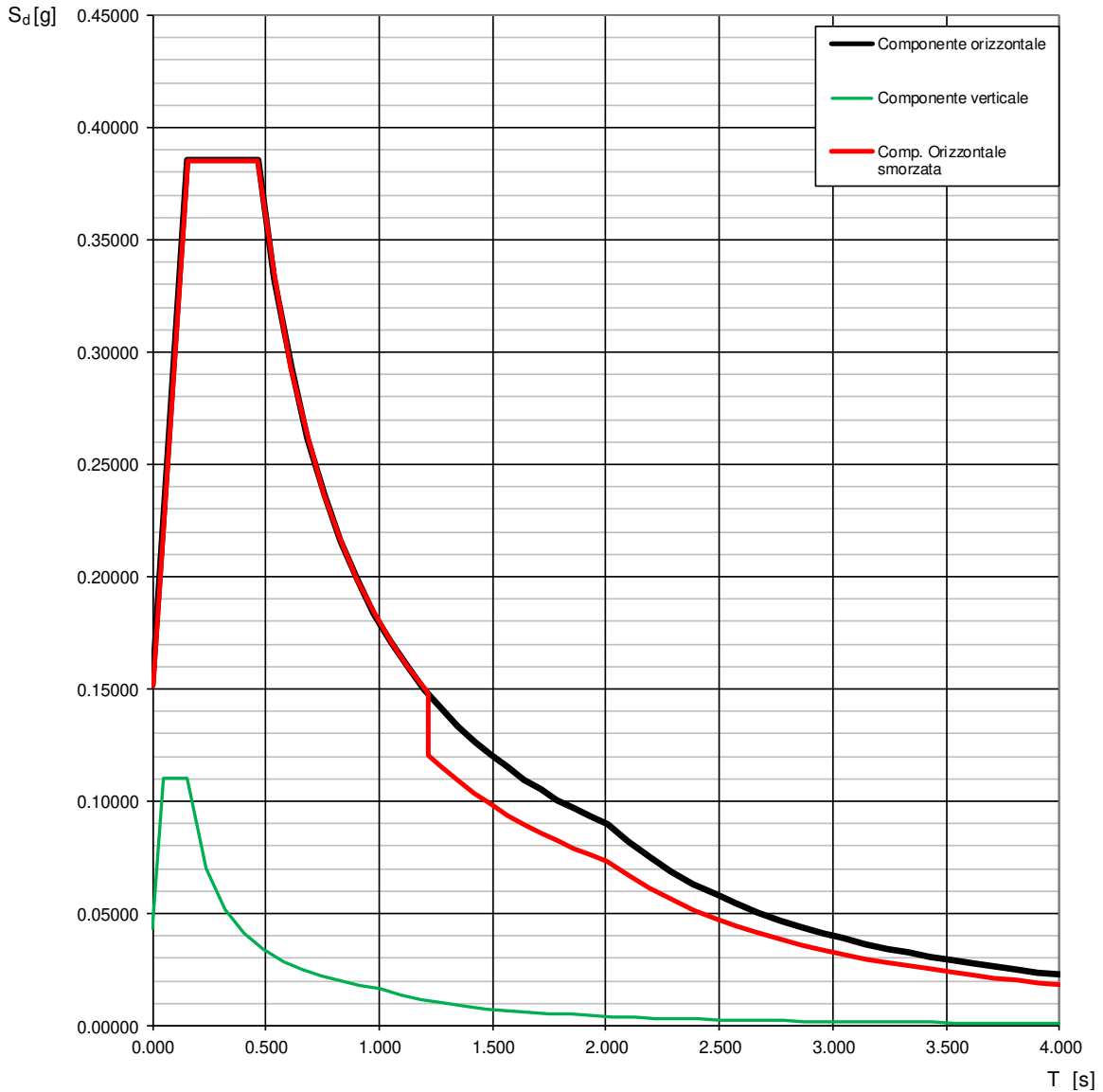


Spettri di risposta elastici (componenti orizz. e vert.) per lo stato limite: **SLD**



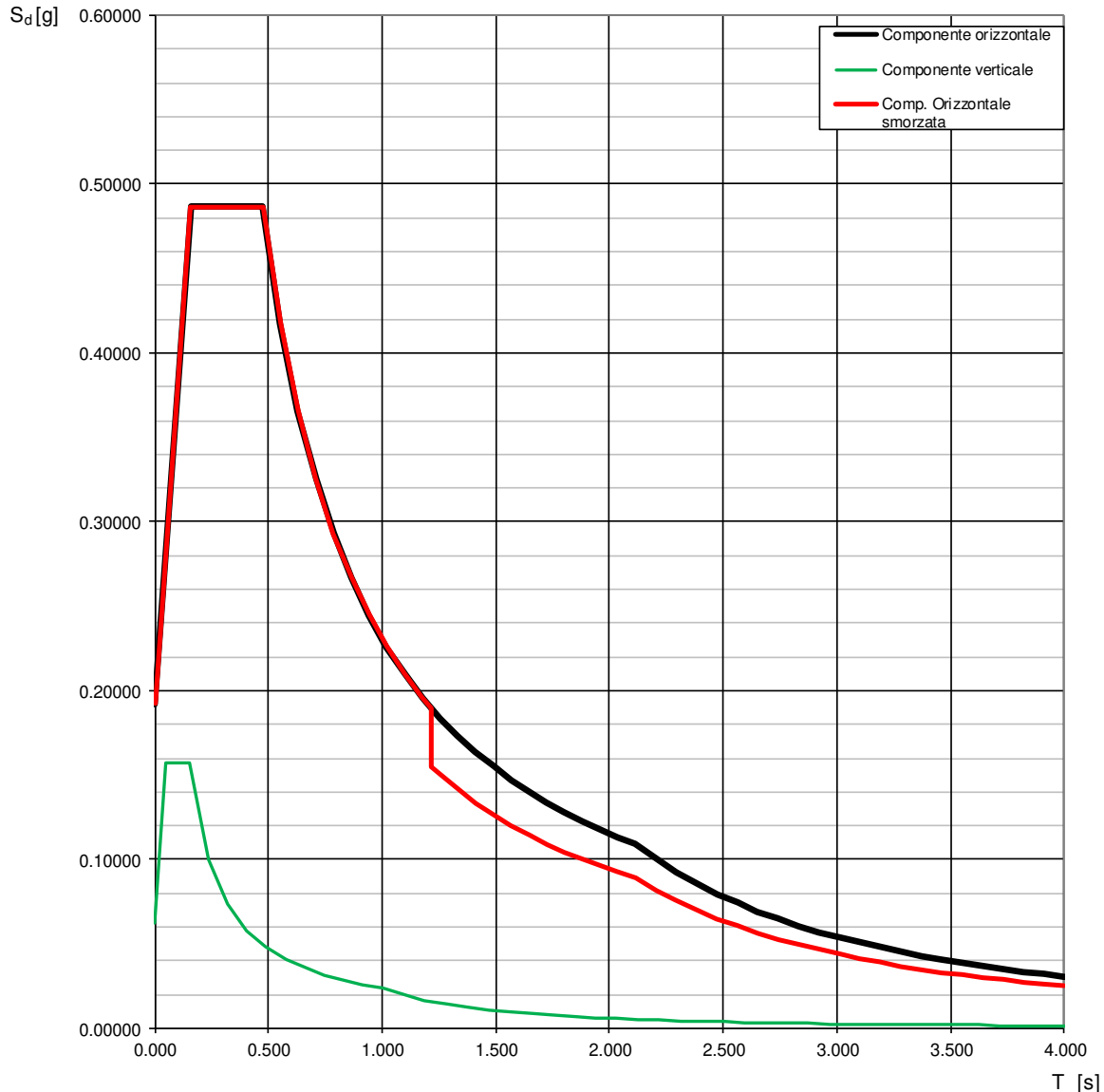
Spettri di risposta elastici e smorzati SLD

Spettri di risposta elastici (componenti orizz. e vert.) per lo stato limite: **SLV**



Spettri di risposta elastici e smorzati SLV

Spettri di risposta elastici (componenti orizz. e vert.) per lo stato limite: **SLC**



Spettri di risposta elastici e smorzati SLC

**4.10 FENOMENI DEFORMATIVI LENTI DELLA SOLETTA IN CALCESTRUZZO**

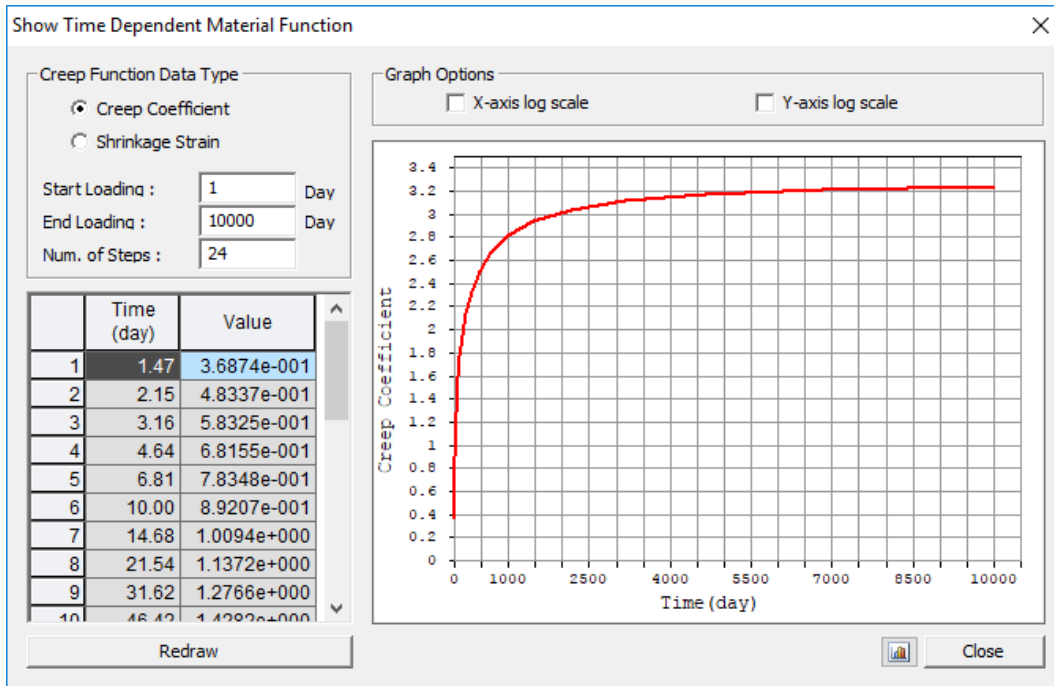
Come già detto, l'influenza di ritiro e fluage della soletta sulla struttura metallica viene considerata automaticamente dal programma di calcolo sulla base delle relazioni stabilite dal codice EN 1992-2 con i seguenti dati:

- resistenza caratteristica cilindrica a 28 gg:  $f_{ck} = 32 \text{ N/mm}^2$
- umidità relativa ambiente: 70 %
- perimetro esposto all'atmosfera:  $h = 2 \times A_c / u = 0.30 \text{ m}$
- calcestruzzo a indurimento normale: N
- età del calcestruzzo iniziale per il ritiro: 1 gg

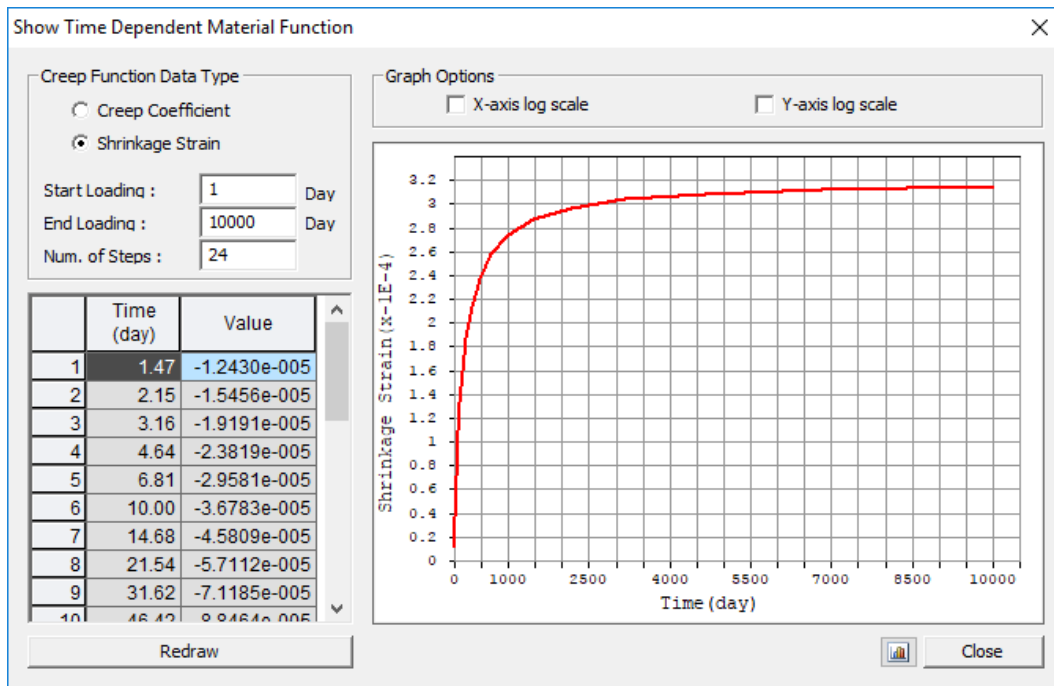
La variazione della resistenza caratteristica del calcestruzzo nel tempo viene determinata con la formula:

$$f_c(t) = f_{ck} \times e^{0,25 \times \left[ 1 - \left( \frac{28}{t} \right)^{0,5} \right]}$$

Si ottengono i seguenti diagrammi di fluage e ritiro:



CREEP (FLUAGE)



SHRINKAGE (RITIRO)

## 5 COMBINAZIONI DI CARICO

I coefficienti moltiplicativi delle singole azioni sono riassunti nelle tabelle seguenti, per le condizioni di carico statiche e sismiche.

Nelle seguenti tabelle i carichi permanenti (pesi e finiture) non appaiono esplicitamente in combinazione perché il loro contributo alle sollecitazioni deriva dagli schemi statici in fase costruttiva (diversi che nella fase di esercizio) e sono quindi accumulati nei "Dead Load" e "Erection Load 1".

Le combinazioni di carico "GEN" (generali) sono utilizzate per le reazioni, gli appoggi e comunque tutto ciò che non è di acciaio e/o composto acciaio/cls.

Le combinazioni di carico "ST" sono viceversa utilizzate per le strutture in acciaio e/o acciaio/cls. Ogni combinazione di verifica viene generata in rispetto dei par. 2.5.3, 5.1.3.12 e 7.3.5 delle NTC 2008; segue una descrizione dettagliata di esempio.

Combinazione: "ST SLU Mobili": combinazione di verifica strutturale allo SLU in cui i "Carichi Mobili" sono il carico variabile dominante mentre gli altri (Vento, Frenamento, Termici) sono assunti attraverso i coefficienti di combinazione  $\psi_0$ .

Preliminarmente per i carichi variabili che possono avere segno opposto si generano le combinazioni "negative" che vengono poi involupate in modo da ottenere i massimi e i minimi.

- 1) "Termico unif. -15°": combinazione: "Termico unif. +15°" x (-1)
- 2) "Termico diff. -5°/H": combinazione: "Termico diff. +5°/H" x (-1)
- 6) "Termico unif. INV": involuppo delle comb. "Termico unif. +15°" e "Termico unif. -15°"
- 7) "Termico diff. INV": involuppo delle comb. "Termico diff. +5°/H" e "Termico diff. -5°/H"

Analogamente si involuppano le condizioni dei carichi mobili disposti "a sinistra" e "a destra":

- 11) "Mobili SLU INV": involuppo delle comb. "Mobili SLU Dx" e "Mobili SLU Sx"

La generazione della combinazione di cui sopra comprende quindi la somma dei seguenti casi di carico (riferimento alle tabelle NTC 2008):

- "Dead Load" x 1.35: pesi strutturali permanenti (tab. 5.1.V)
- "Erection Load 1" x 1.5: peso finiture (permanenti non strutturali), (tab. 5.1.V)
- "Creep secondary" x 1.2: parte reale (iperstatica) dell'effetto del fluage della soletta in cls. (tab. 5.1.V)
- "Shrinkage secondary" x 1.2: parte reale (iperstatica) del ritiro della soletta in cls. (tab. 5.1.V)
- "Mobili SLU INV" x 1.35: involuppo carichi mobili; carico variabile dominante (tab. 5.1.V)
- "Vento strutture" x (1.5 x 0.6) = 0.9: carico variabile (tab. 5.1.V e tab. 5.1.VI)
- "Vento Mobili" x (1.5 x 0.6) = 0.9: carico variabile (tab. 5.1.V e tab. 5.1.VI)
- "Termico unif. INV" x (1.2 x 0.6) = 0.72: carico variabile (tab. 5.1.V e tab. 5.1.VI)
- "Termico diff. INV" x (1.2 x 0.6) = 0.72: carico variabile (tab. 5.1.V e tab. 5.1.VI)

La combinazione così composta è di tipo "Strength/Stress", utilizzata per le verifiche allo SLU; le altre combinazioni dello stesso tipo sono ottenute per rotazione del carico variabile dominante.







## 6 CALCOLO DELLE SOLLECITAZIONI

Le travi composte acciaio-calcestruzzo vengono analizzate dal programma secondo le fasi di costruzione (construction stage):

- fase 1: corrisponde alla fase costruttiva fino al getto della soletta (30 giorni):

sezione reagente: trave di acciaio (soletta non reagente)

carichi applicati: peso strutture acciaio e peso soletta

- fase 2: corrisponde a una fase costruttiva di lunga durata (10000 giorni) in cui la soletta ha caratteristiche elastiche variabili nel tempo e si tiene conto dei fenomeni di interazione lenti di ritiro e fluage, valutati secondo gli schemi EN 1992-2:

sezione reagente: trave di acciaio e soletta (con modulo variabile)

carichi applicati: finiture, ritiro, fluage della soletta

- fase 3: corrisponde alla fase di esercizio:

sezione reagente: trave acciaio + soletta (modulo costante)

carichi applicati: carichi di esercizio (vento, carichi mobili, termici, frenamento, sisma)

Si è assunta un'accelerazione gravitazionale pari a  $g = 9,806 \text{ m/s}^2$ .

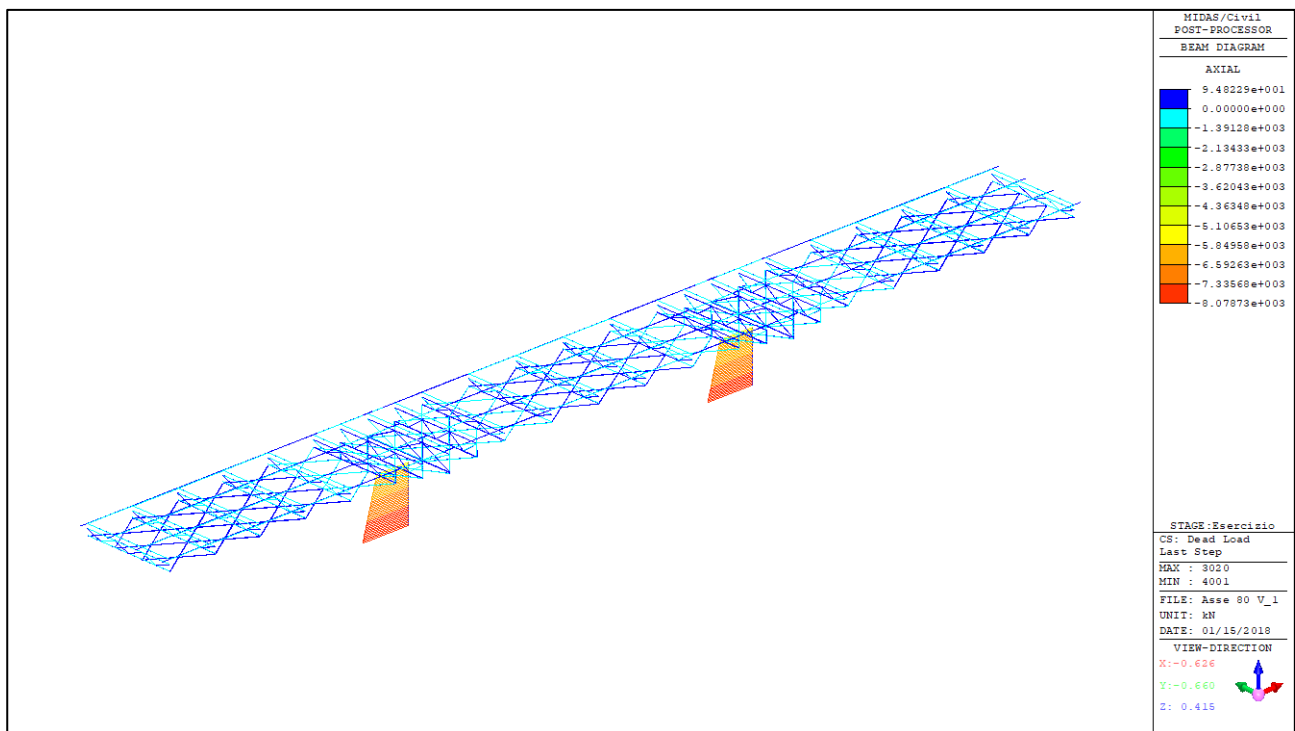
Le unità di misura utilizzate sono coerenti con il Sistema Internazionale:

lunghezze: m (metri)

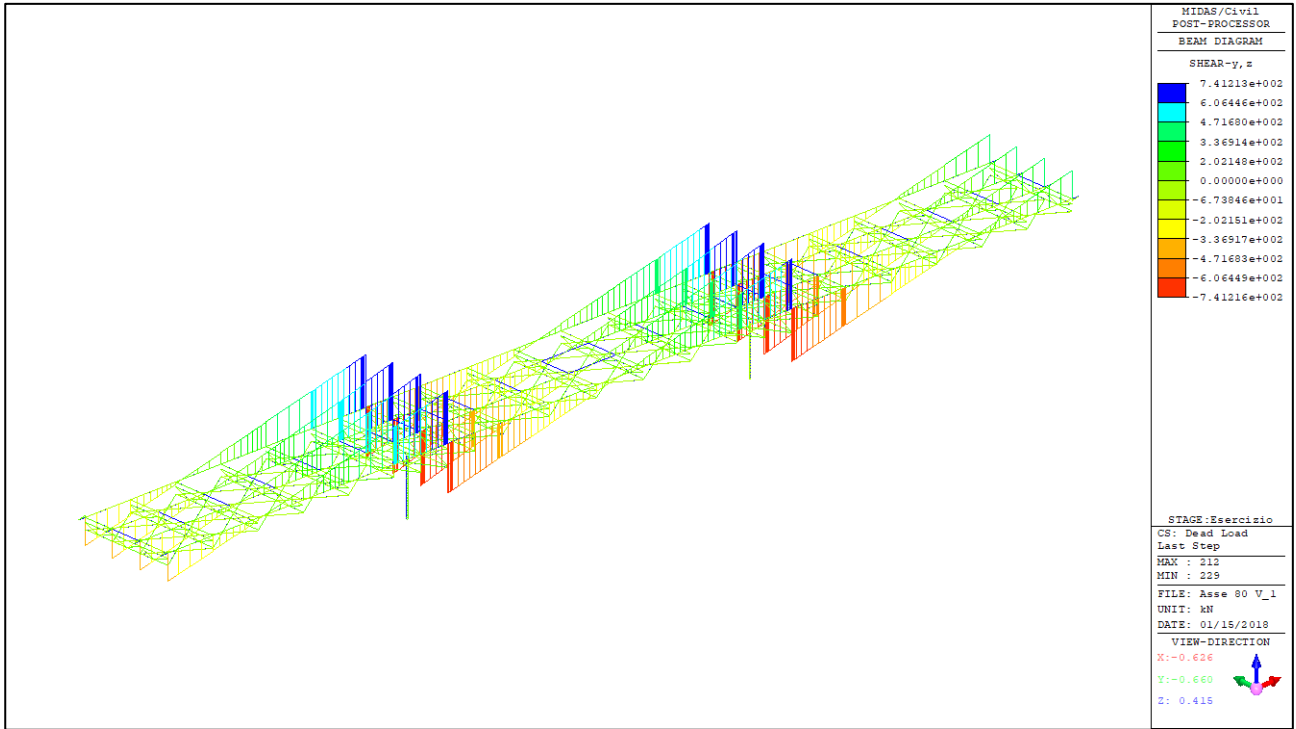
masse: t (tonnellate)

forze kN (kilo-Newton)

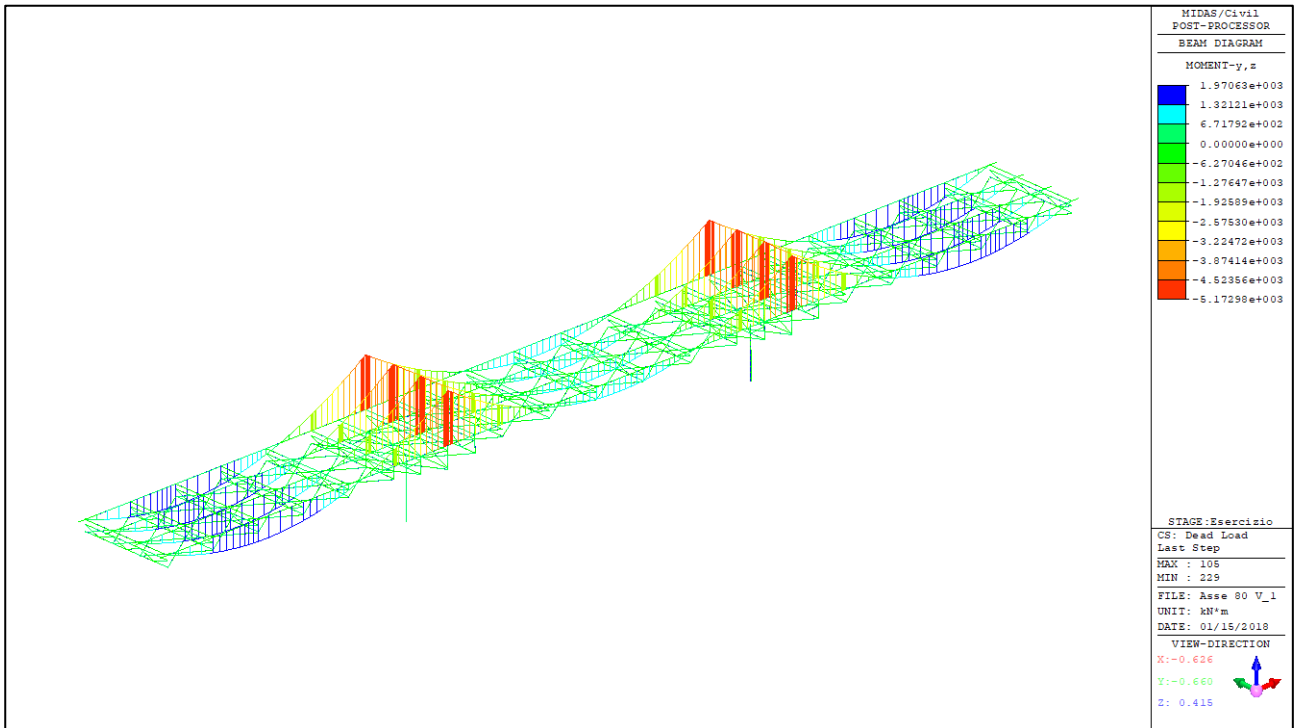
Seguono diagrammi delle azioni calcolate sulle travi composte dell'impalcato e le pile per le singole condizioni di carico (variabili involuppati  $\pm$ ).



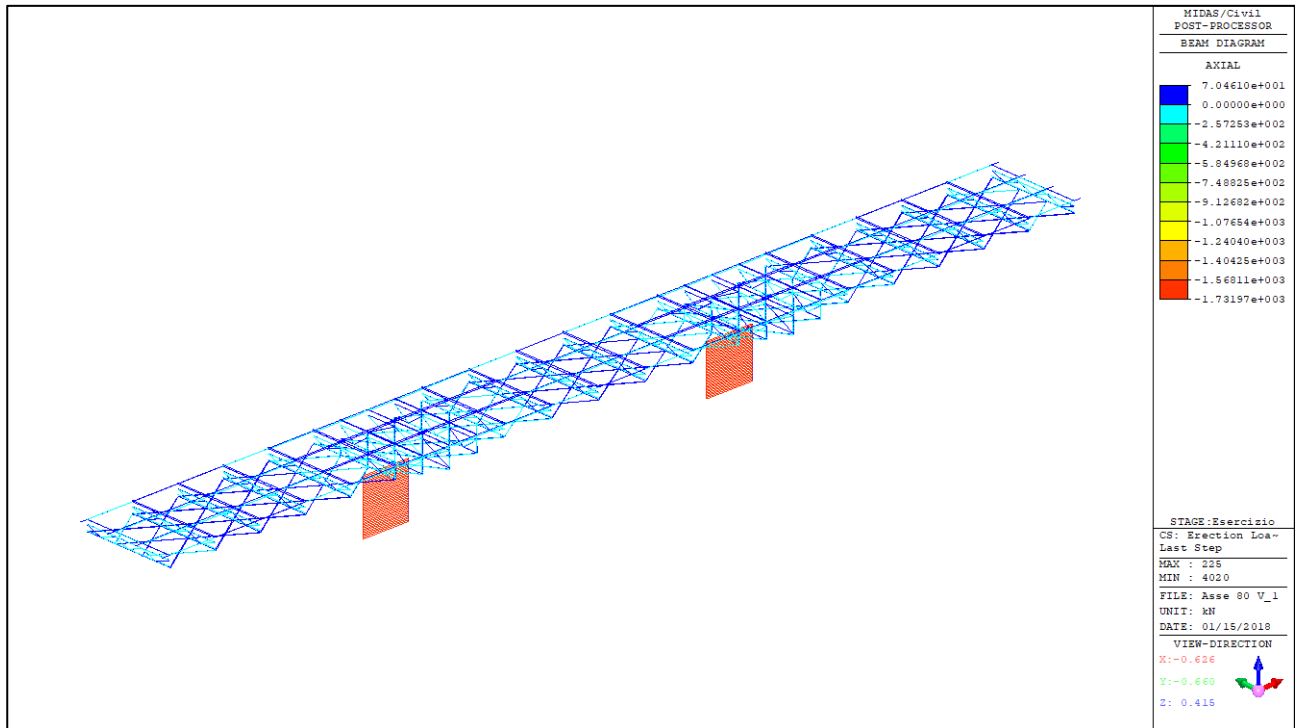
Peso travi e soletta (dead loads) - Diagramma sforzi assiali



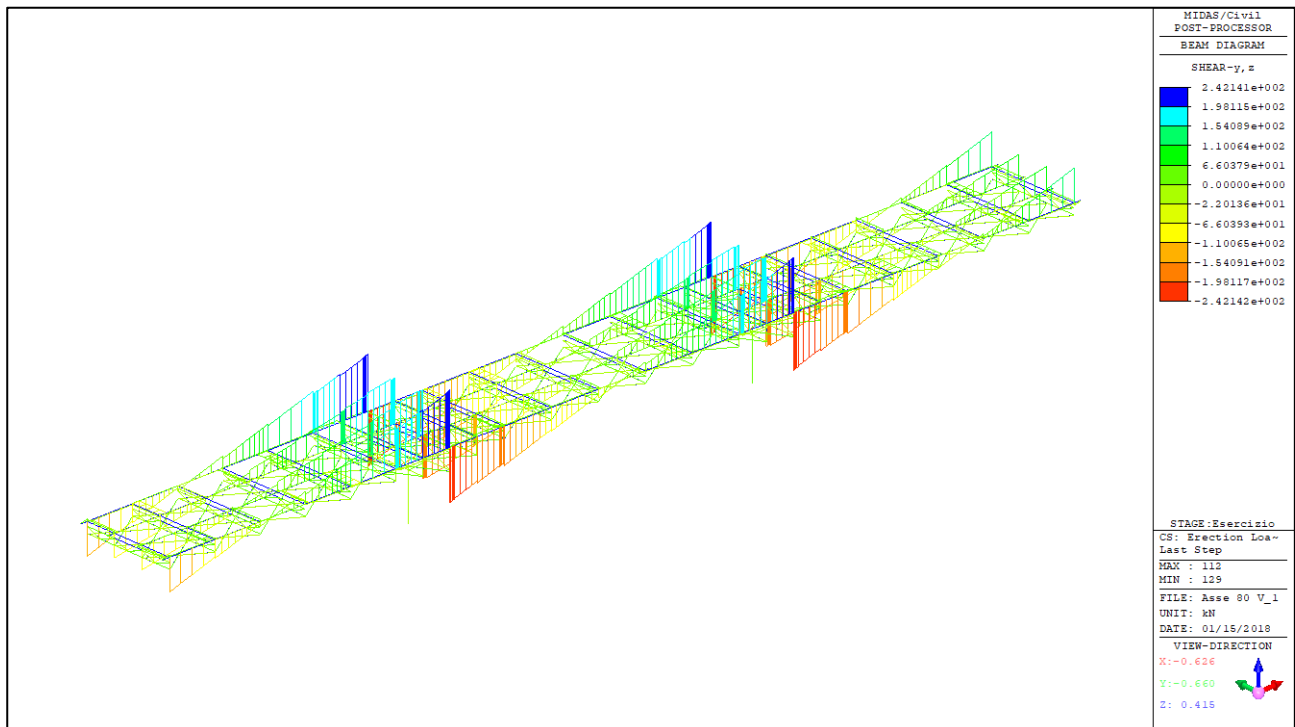
Peso travi e soletta (dead loads)- Diagramma sforzi taglianti



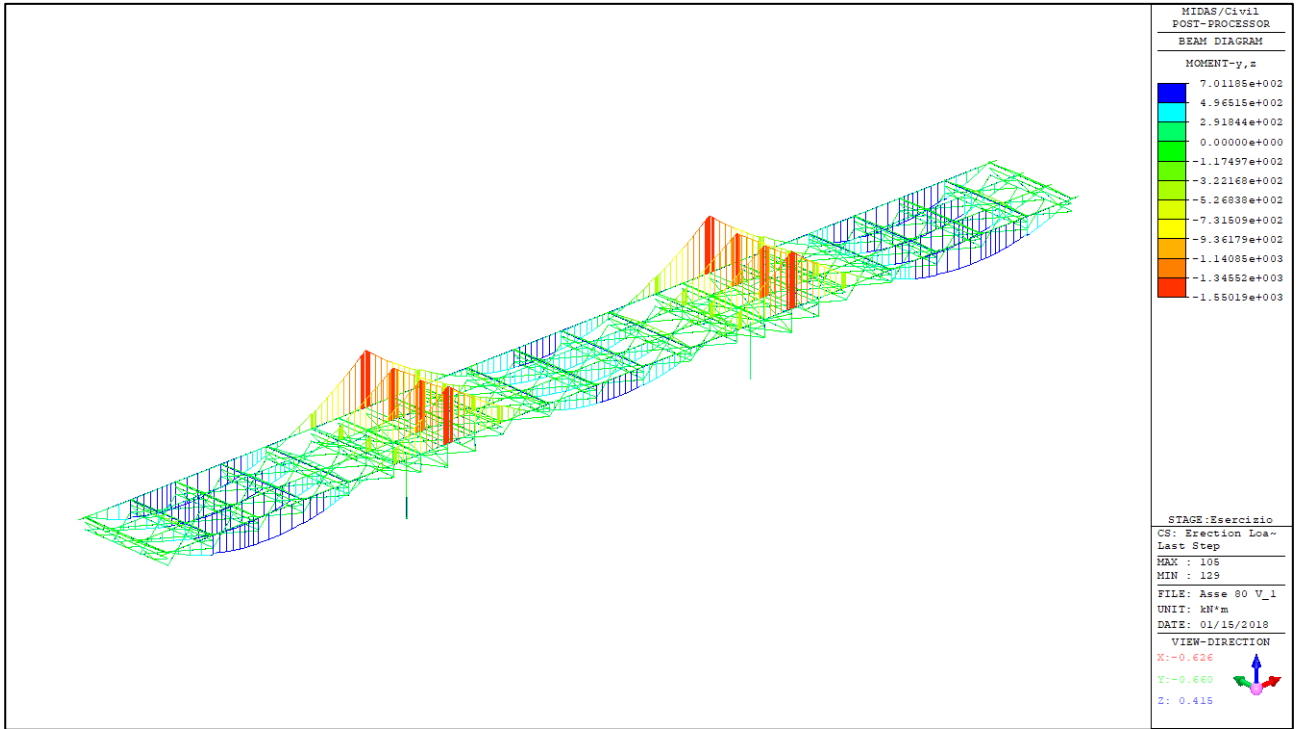
Peso travi e soletta (dead loads) - Diagramma momenti flettenti



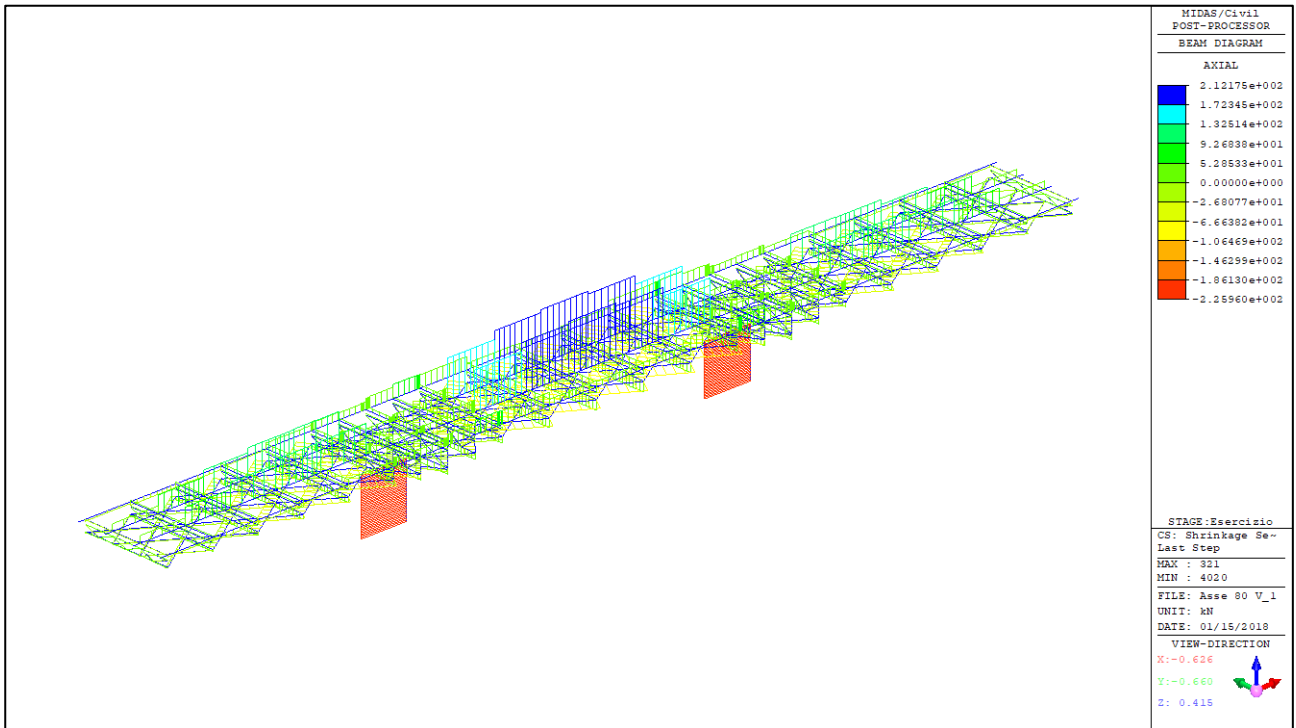
Finiture – Diagramma sforzi assiali



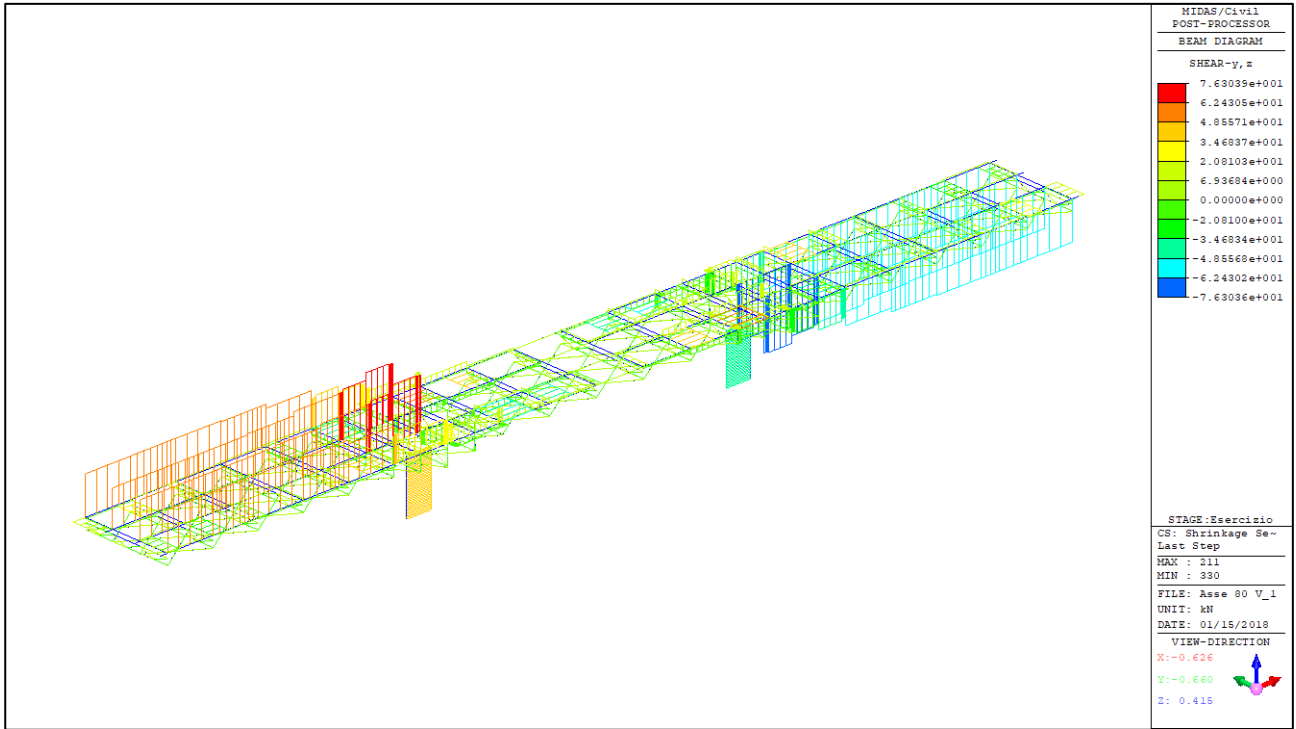
Finiture – Diagramma sforzi taglianti



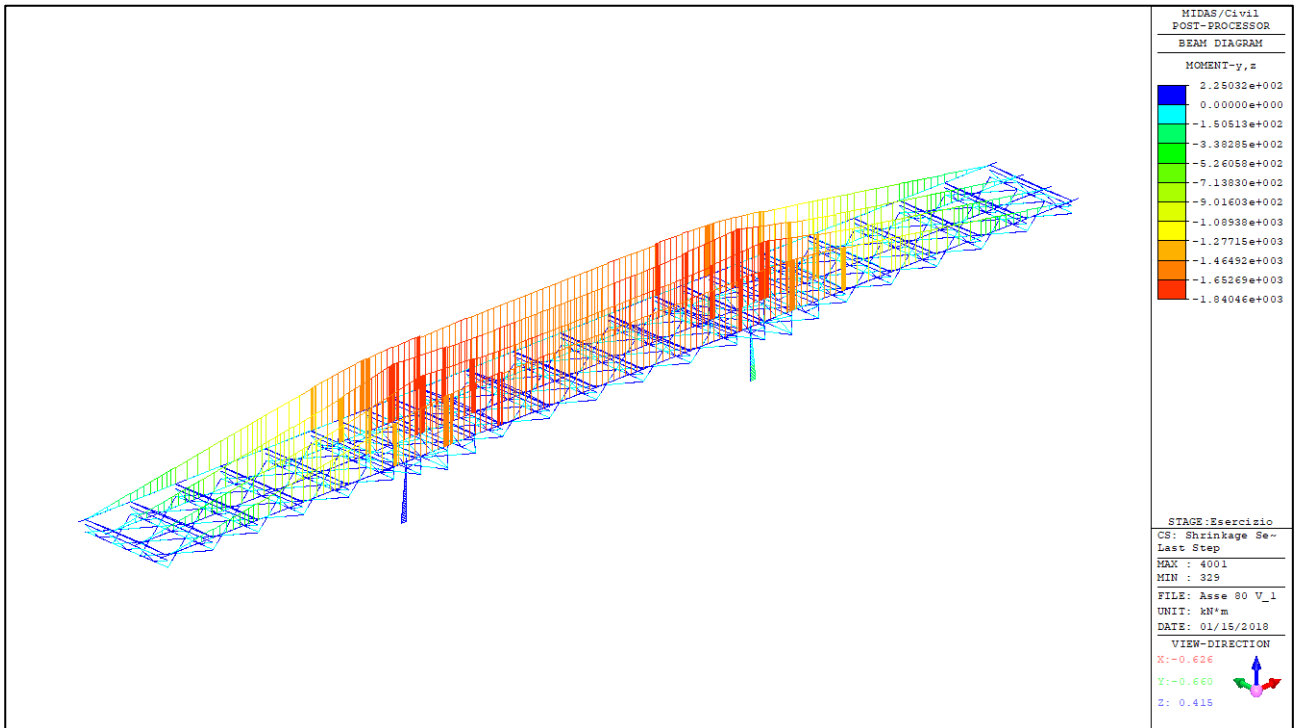
Finiture – Diagramma momenti flettenti



Ritiro soletta - Diagramma sforzi assiali

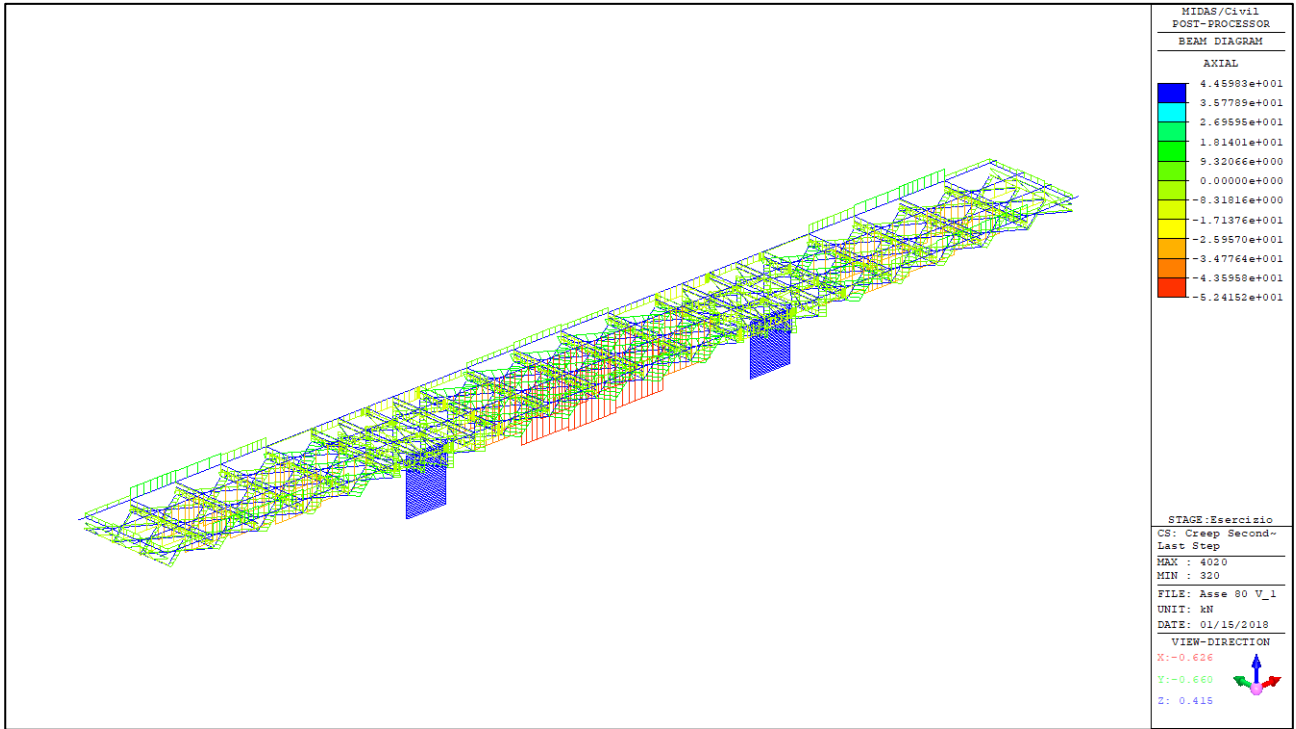


Ritiro soletta - Diagramma sforzi taglianti

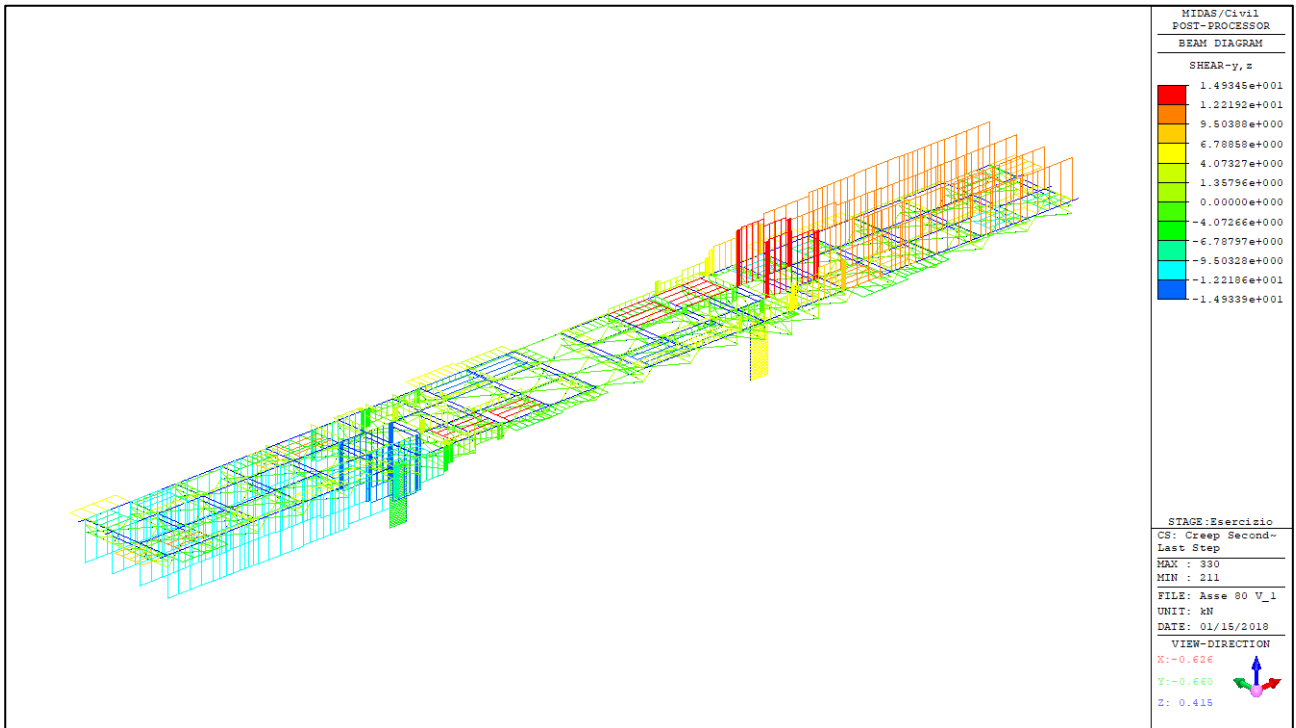


Ritiro soletta - Diagramma momenti flettenti



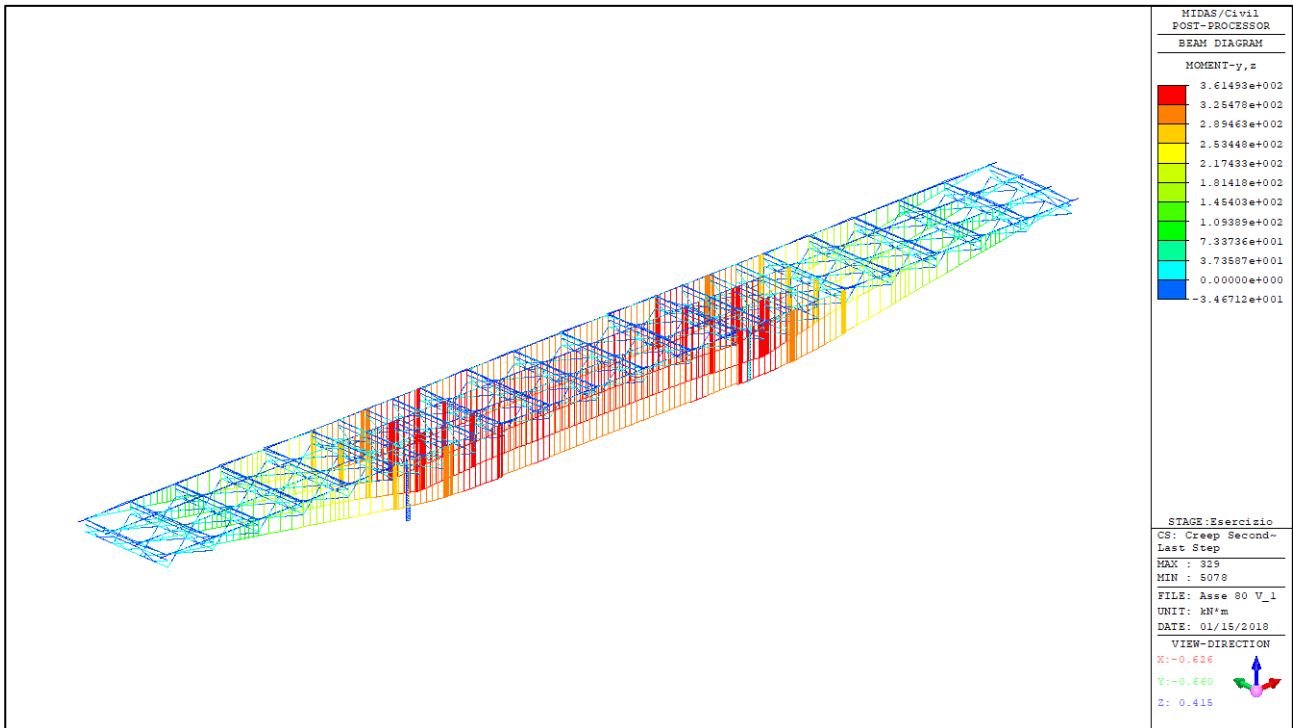


Fluage soletta - Diagramma sforzi assiali

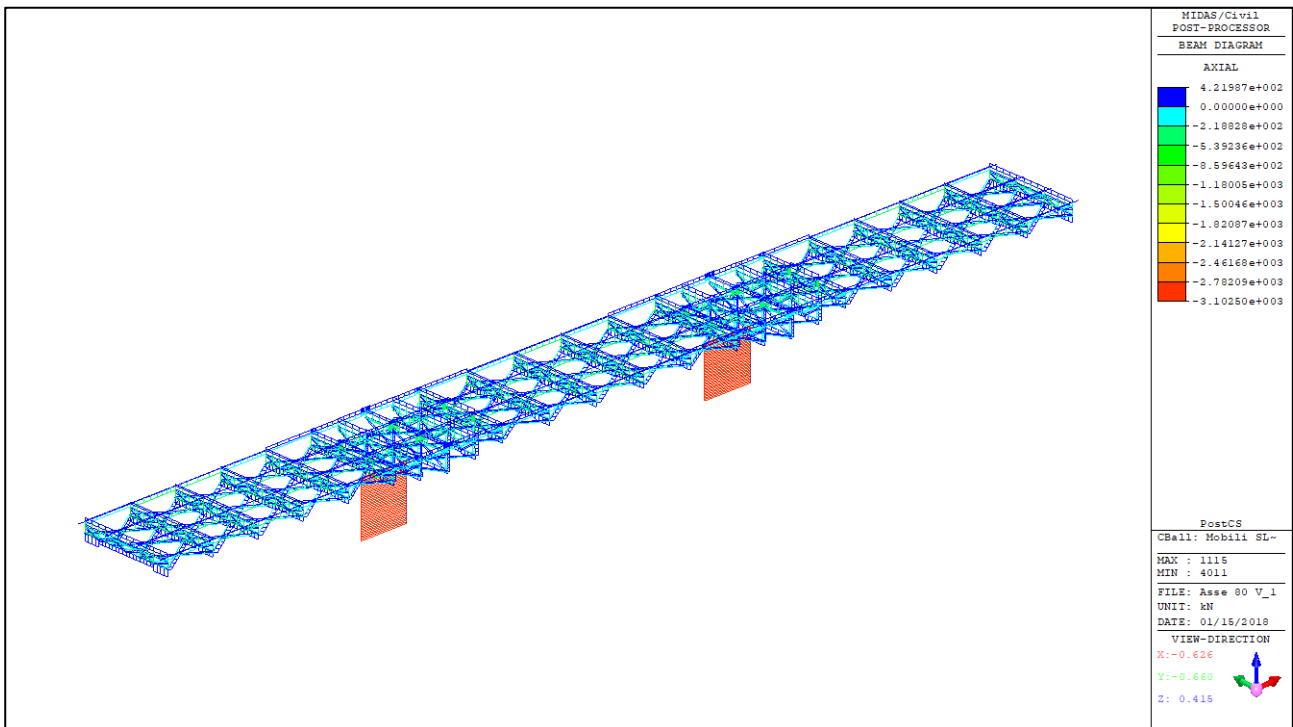


Fluage soletta - Diagramma sforzi taglianti

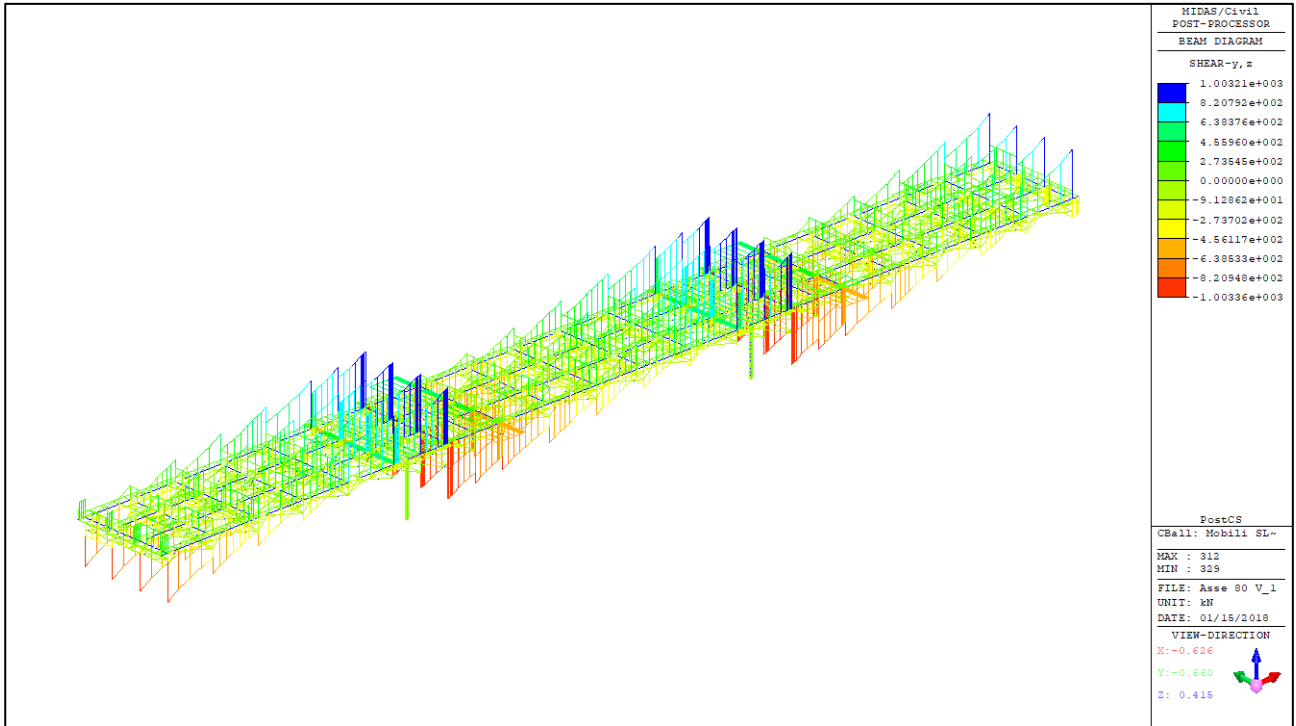




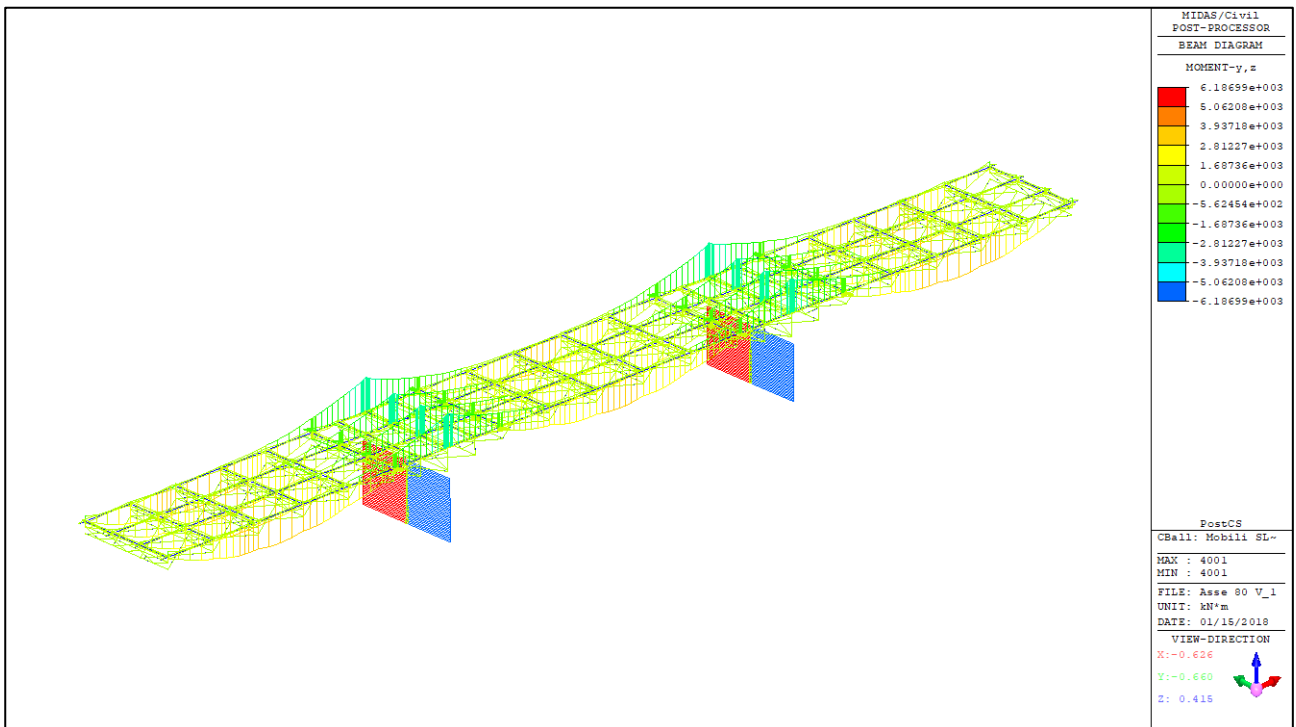
Fluage soletta - Diagramma momenti flettenti



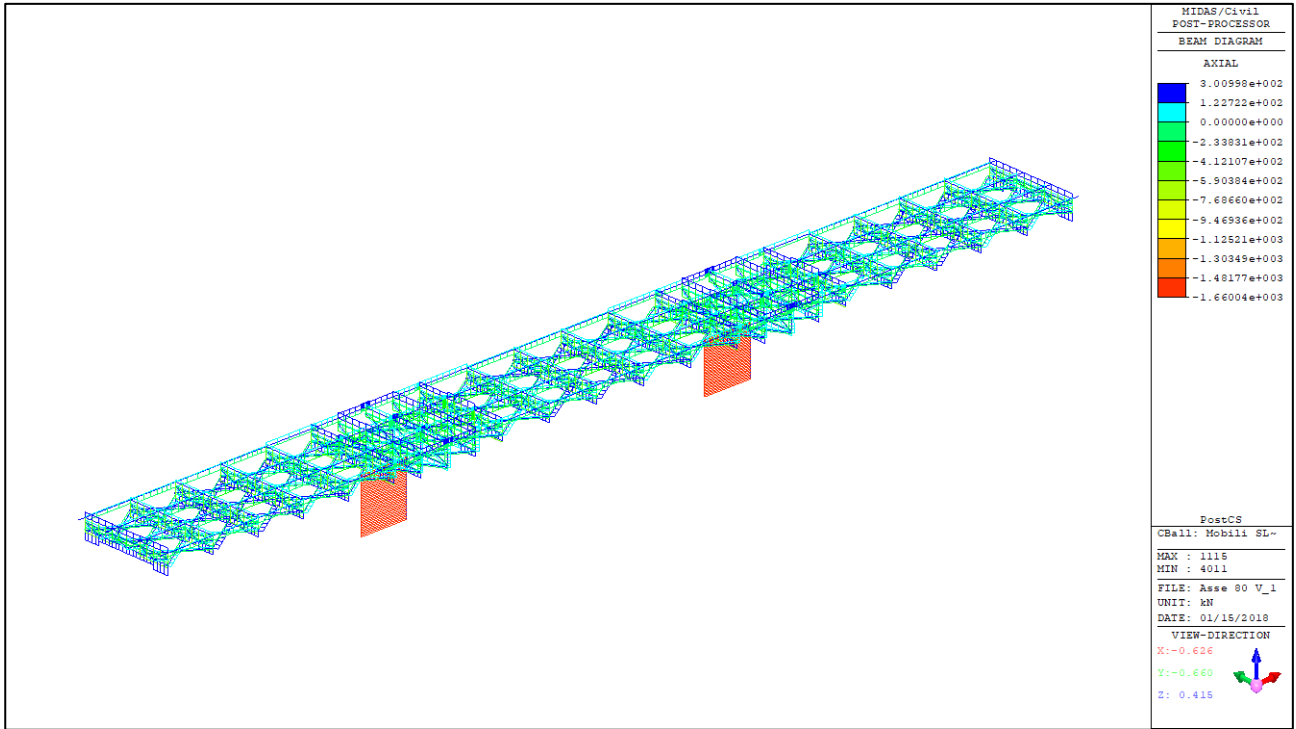
Involuppo carichi mobili SLU - Diagramma sforzi assiali



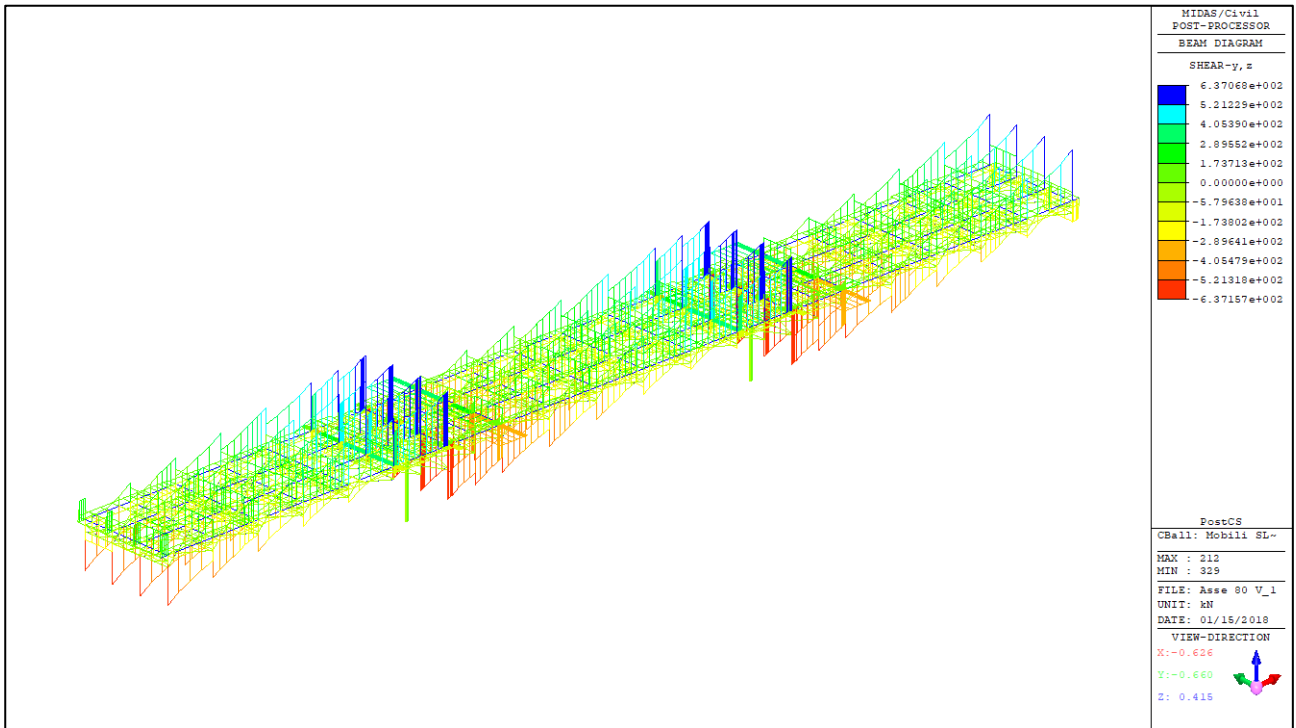
Inviluppo carichi mobili SLU - Diagramma sforzi taglianti



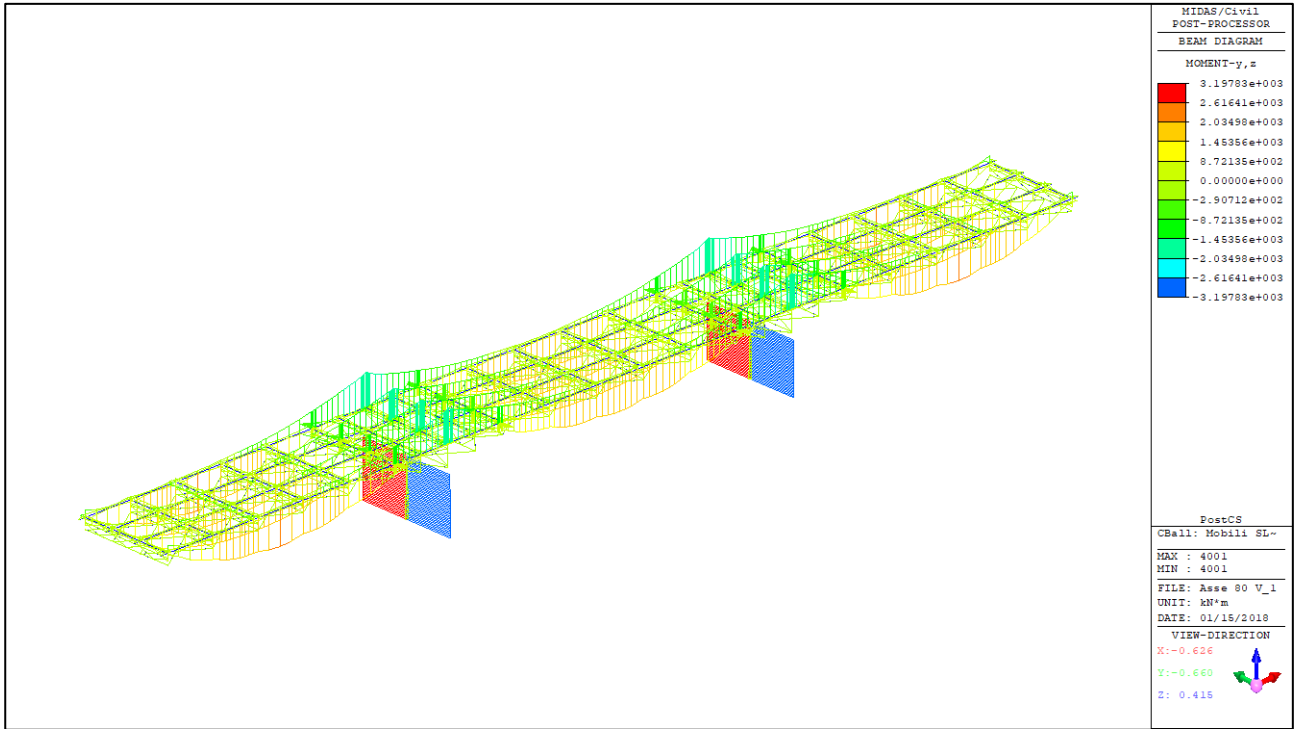
Inviluppo carichi mobili SLU - Diagramma momenti flettenti



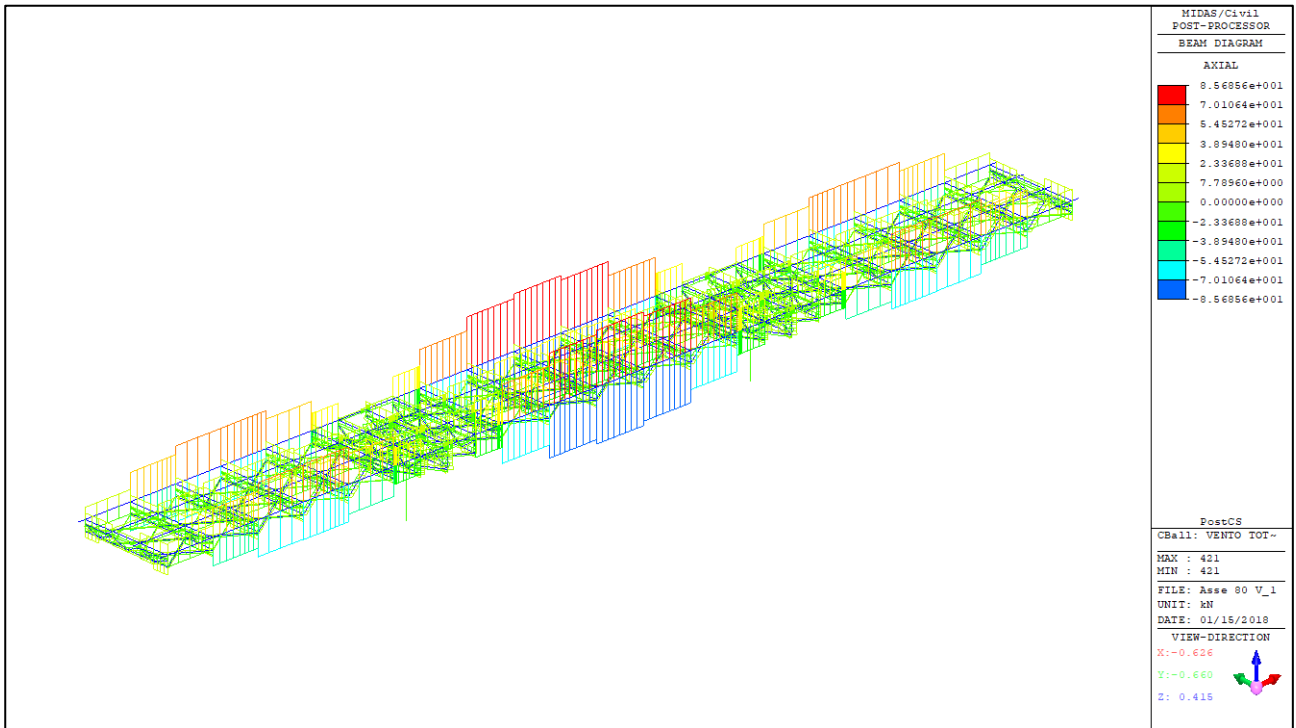
Inviluppo carichi mobili SLE - Diagramma sforzi assiali



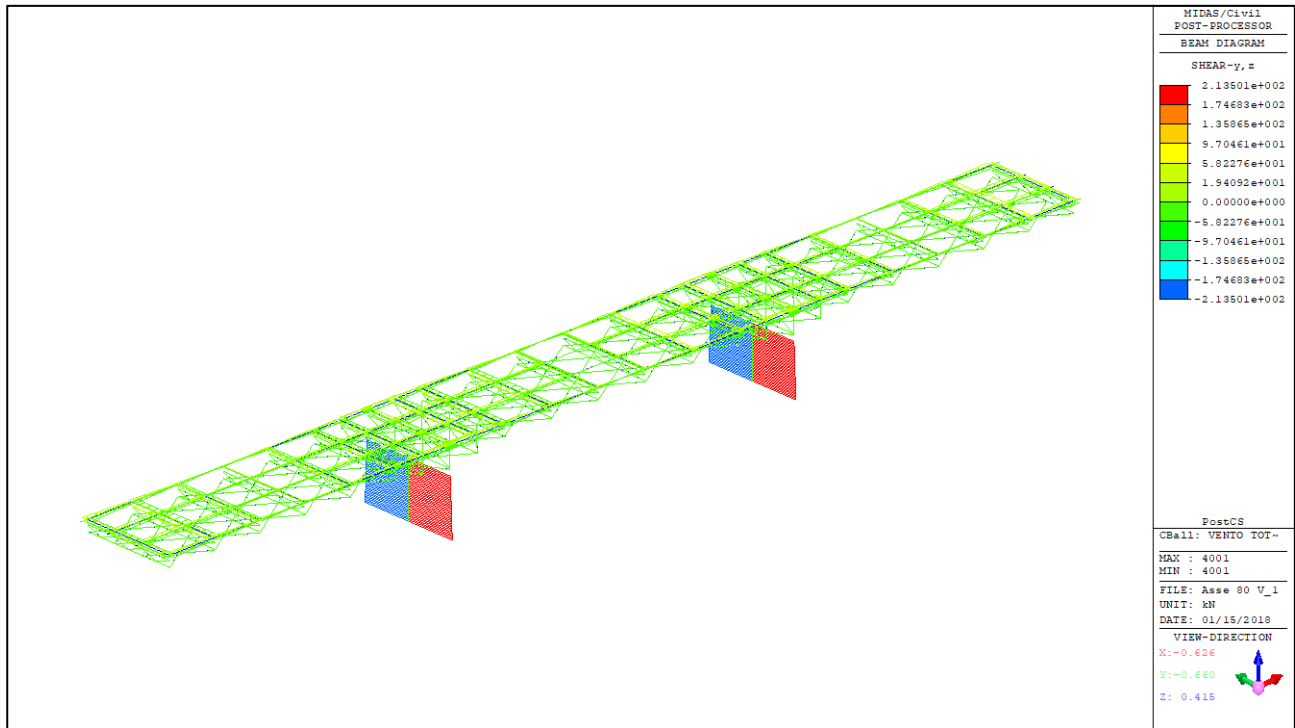
Inviluppo carichi mobili SLE - Diagramma sforzi taglianti



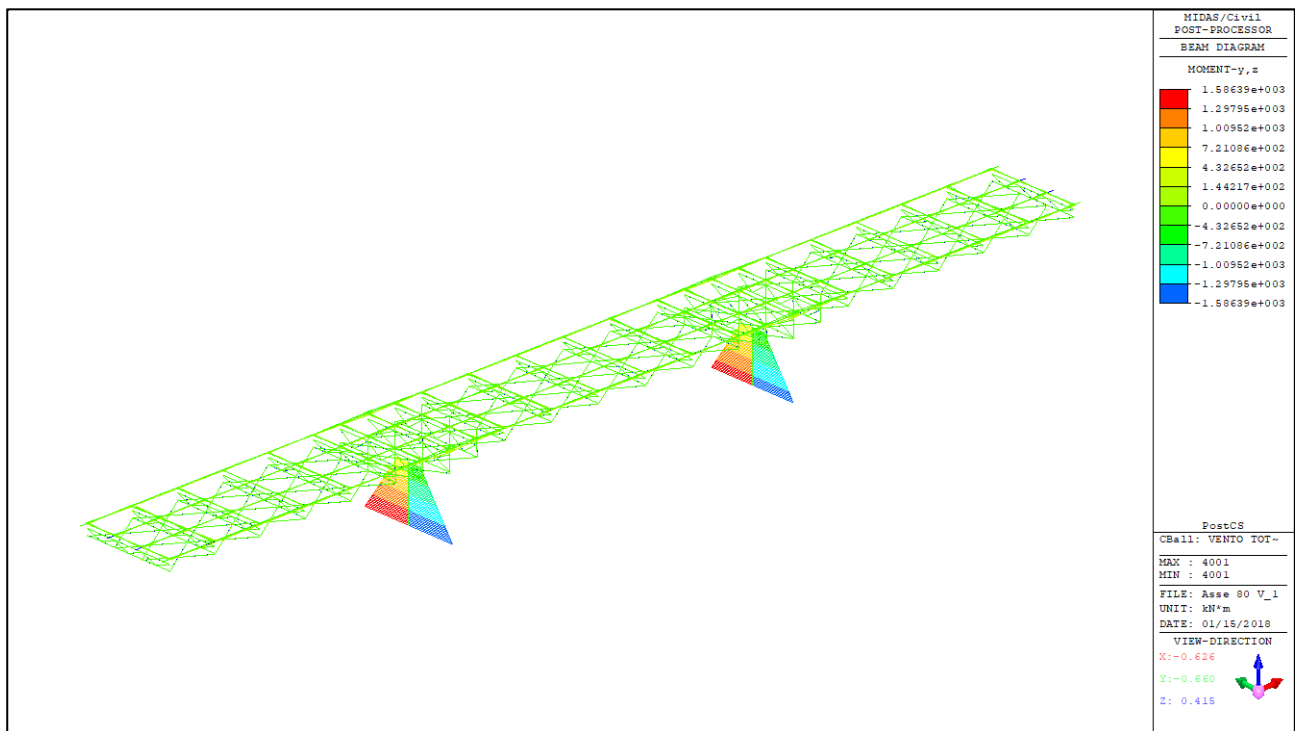
Inviluppo carichi mobili SLE - Diagramma momenti flettenti



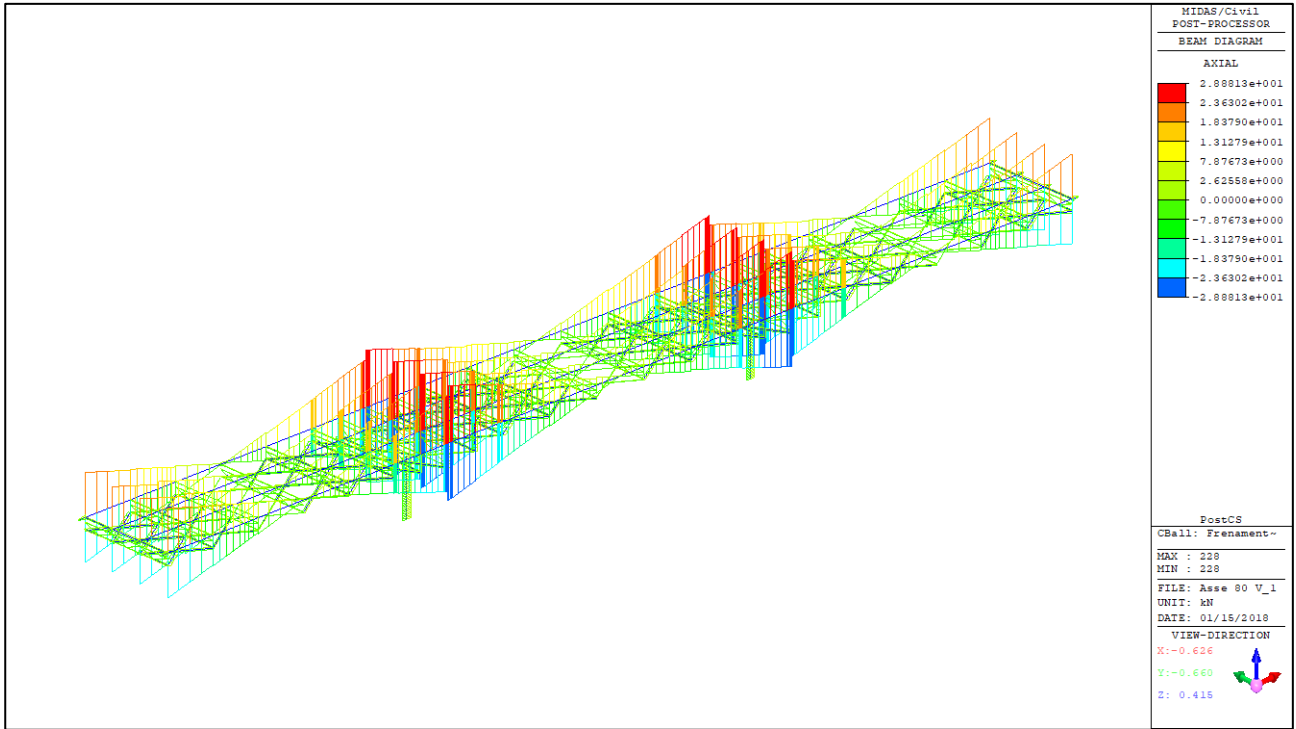
Vento trasversale (strutture e mobili) - Diagramma sforzi assiali



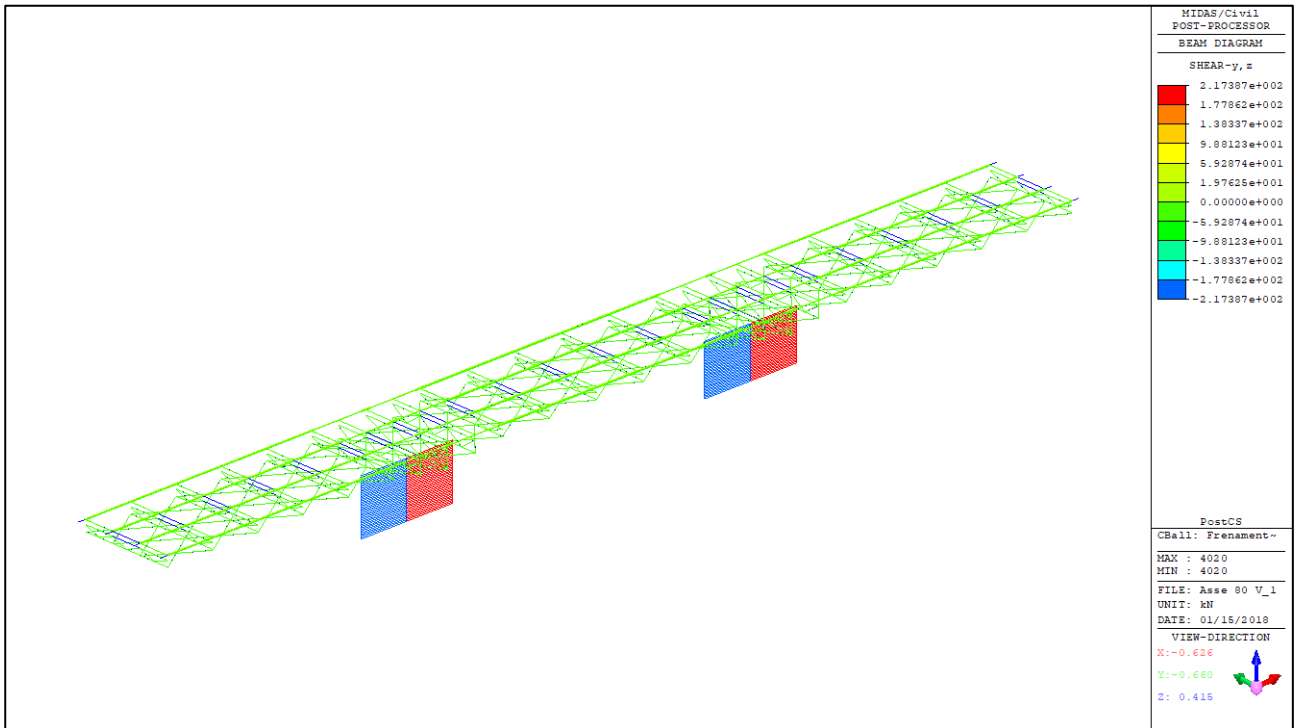
Vento trasversale (strutture e mobili) - Diagramma sforzi taglianti



Vento trasversale (strutture e mobili) - Diagramma momenti flettenti

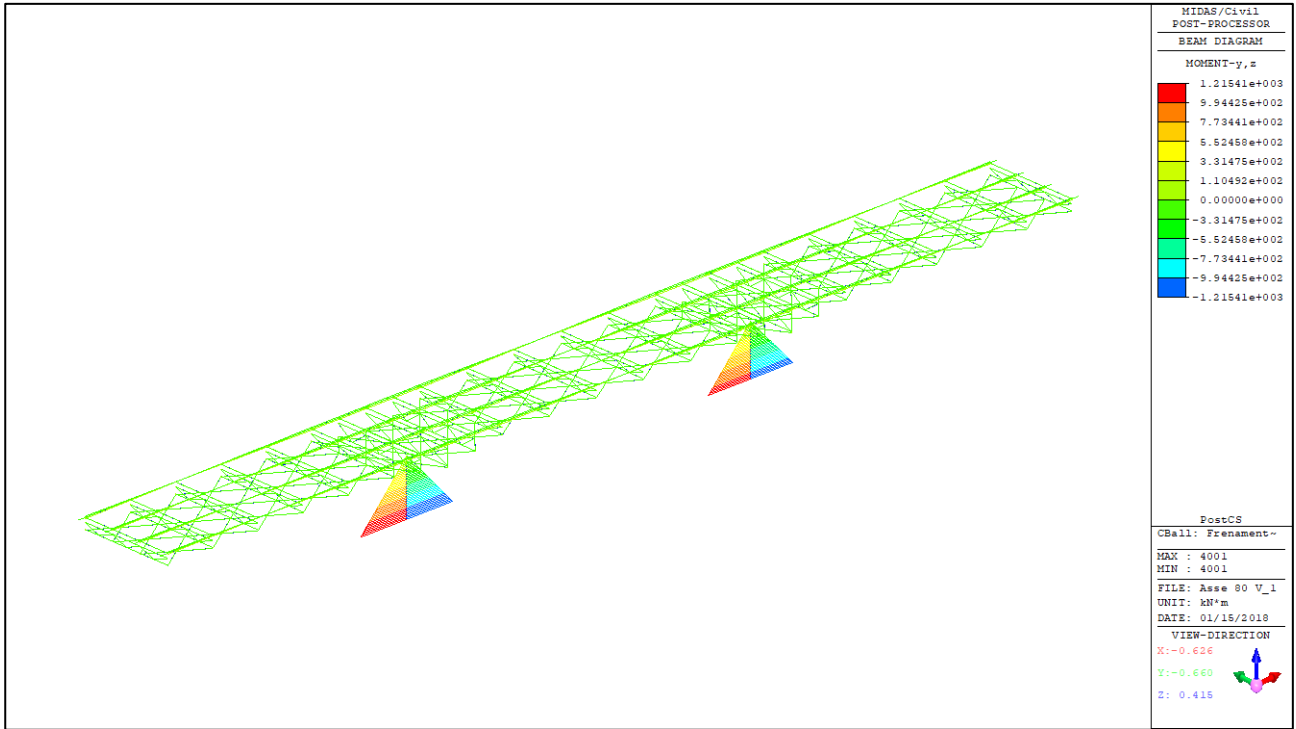


Frenamento - Diagramma sforzi assiali

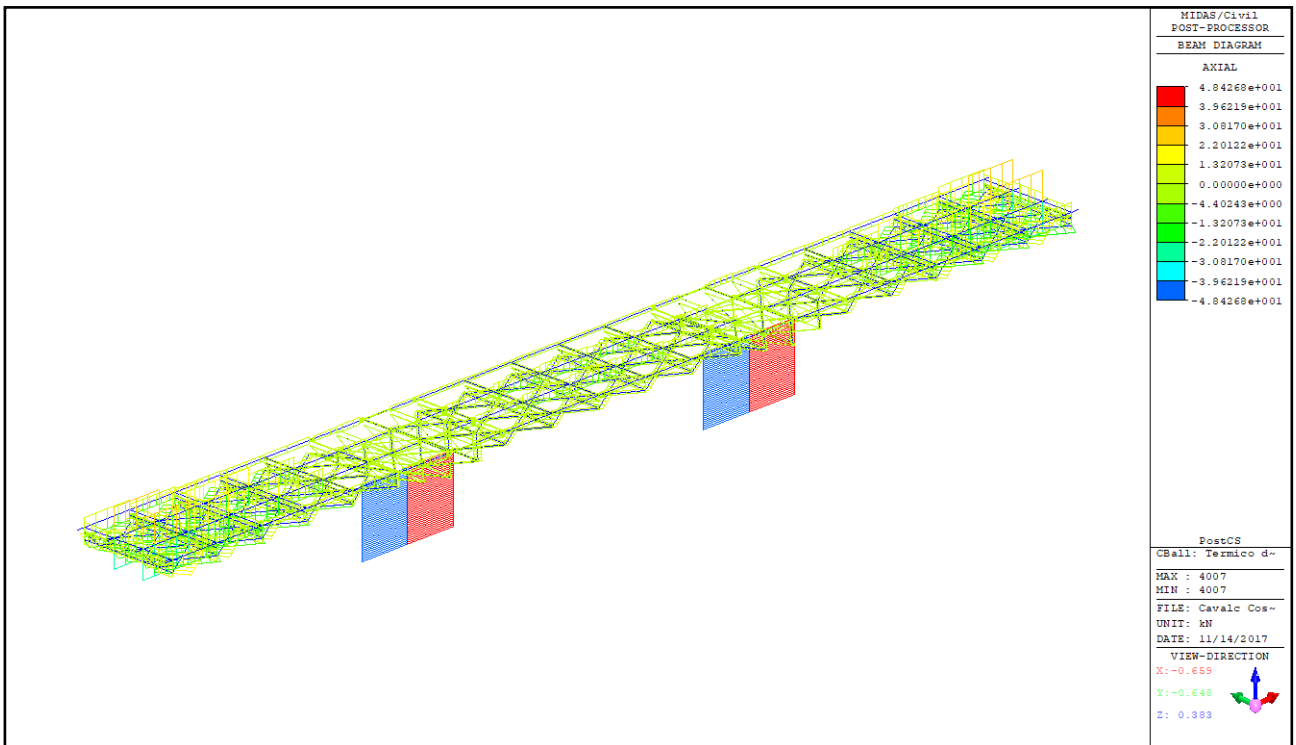


Frenamento - Diagramma sforzi taglianti



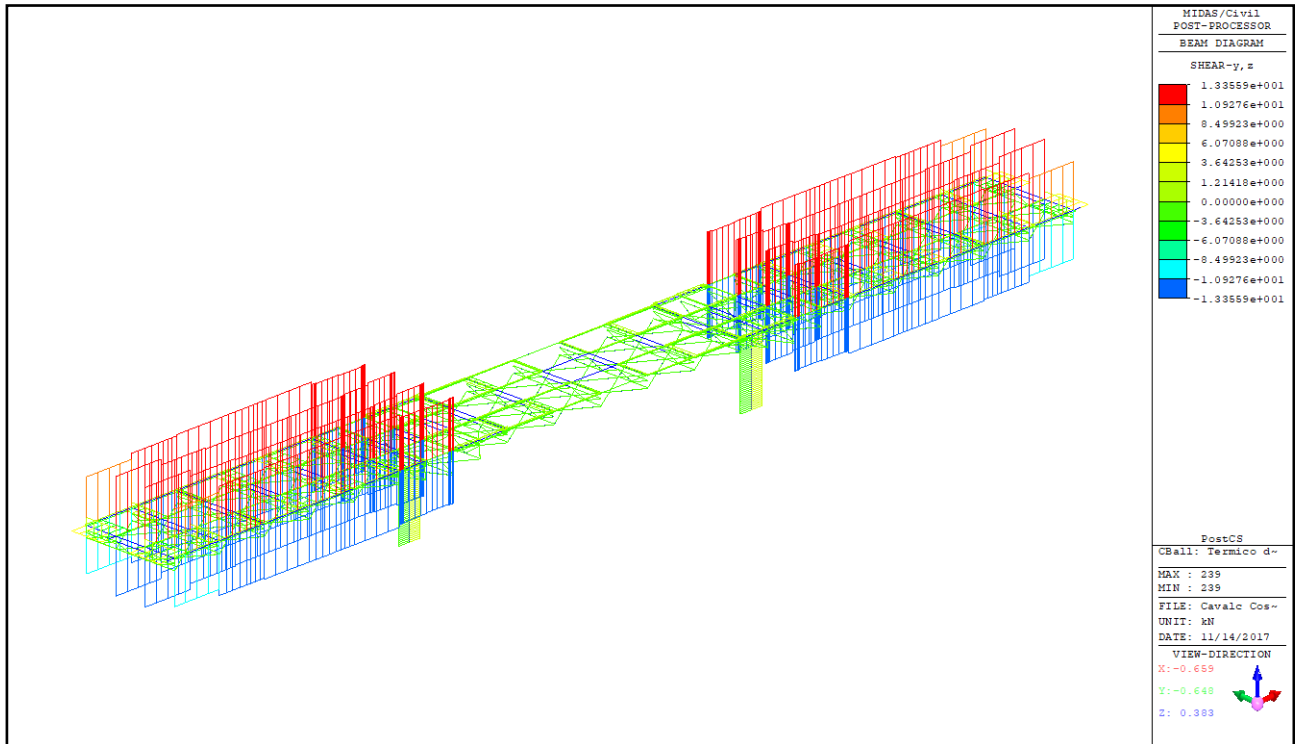


Frenamento - Diagramma momenti flettenti

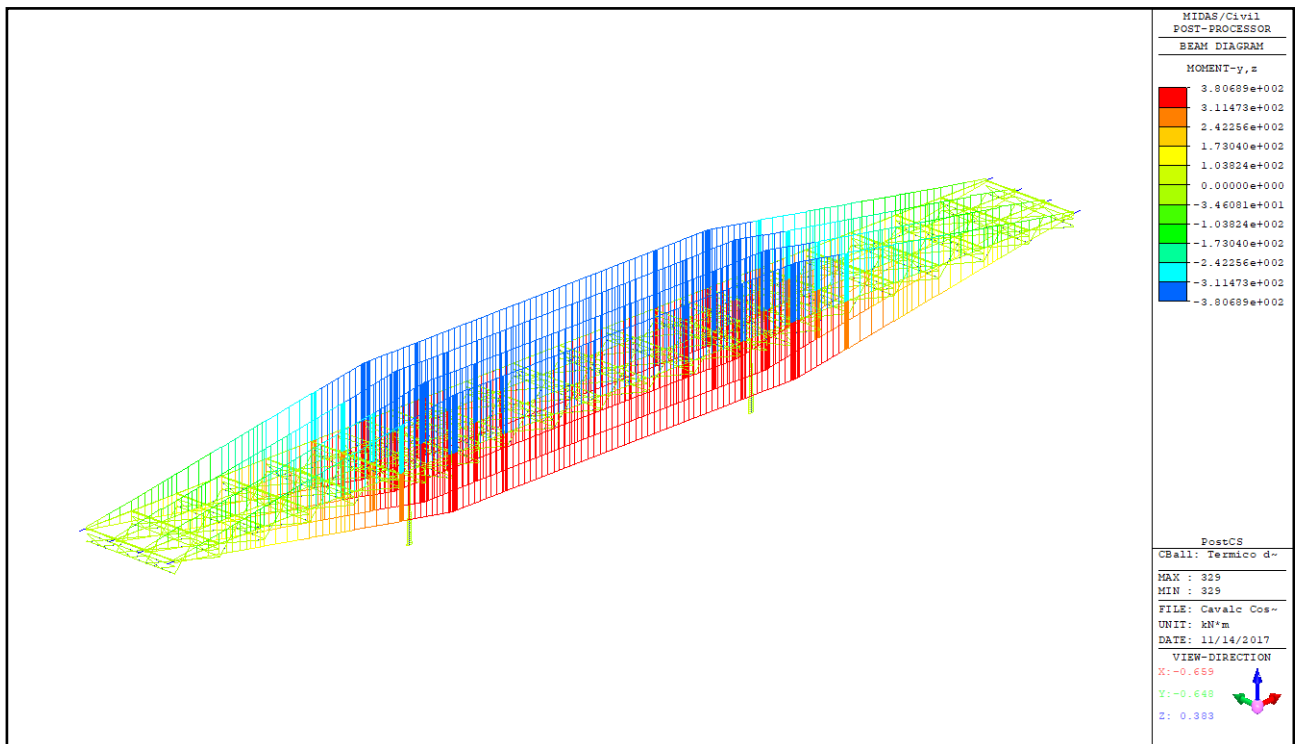


Inviluppo differenziale termico  $\pm 5^\circ/H$  - Diagramma sforzi assiali

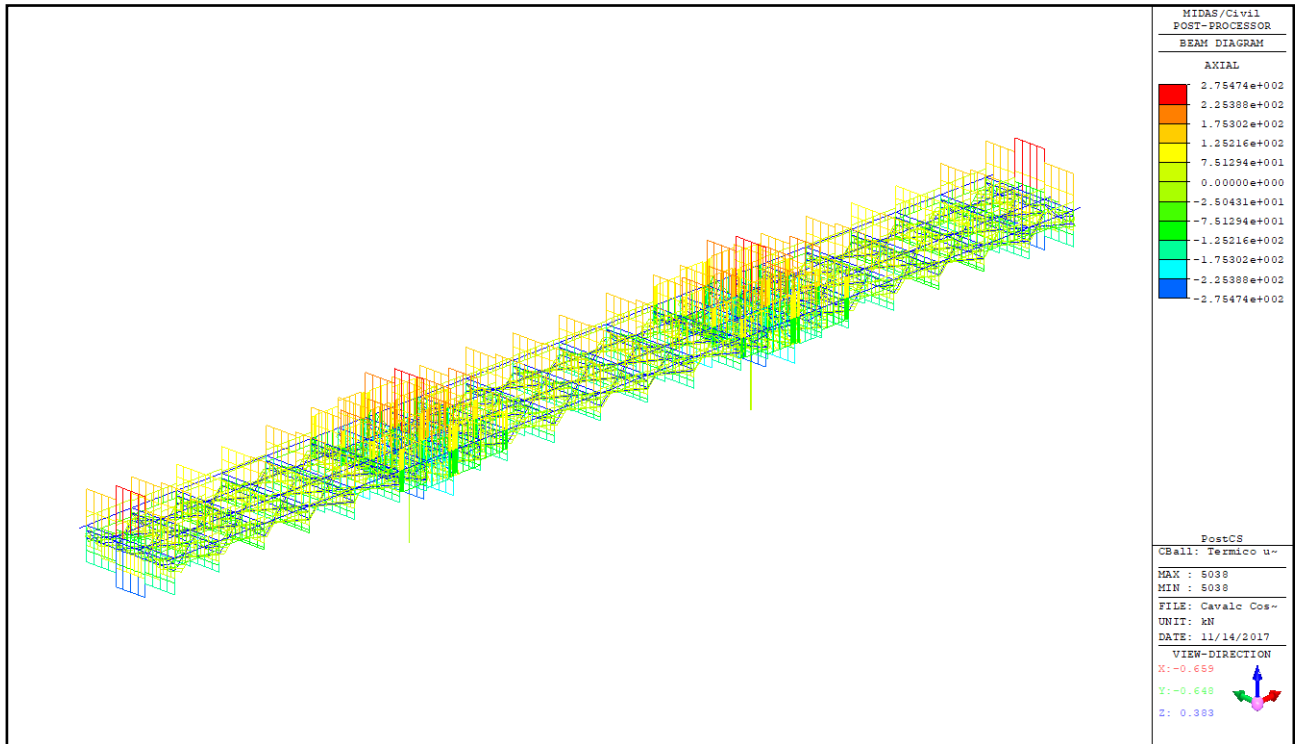




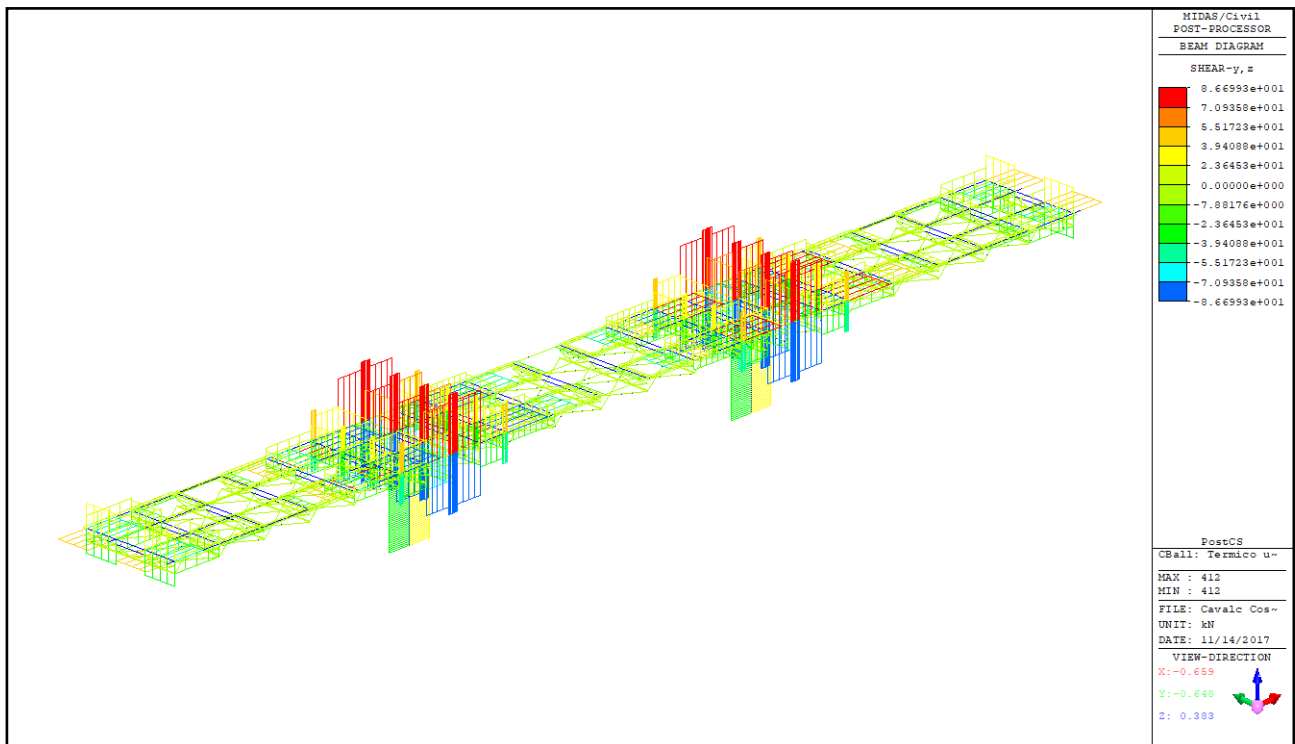
Inviluppo differenziale termico  $\pm 5^\circ/H$  - Diagramma sforzi taglianti



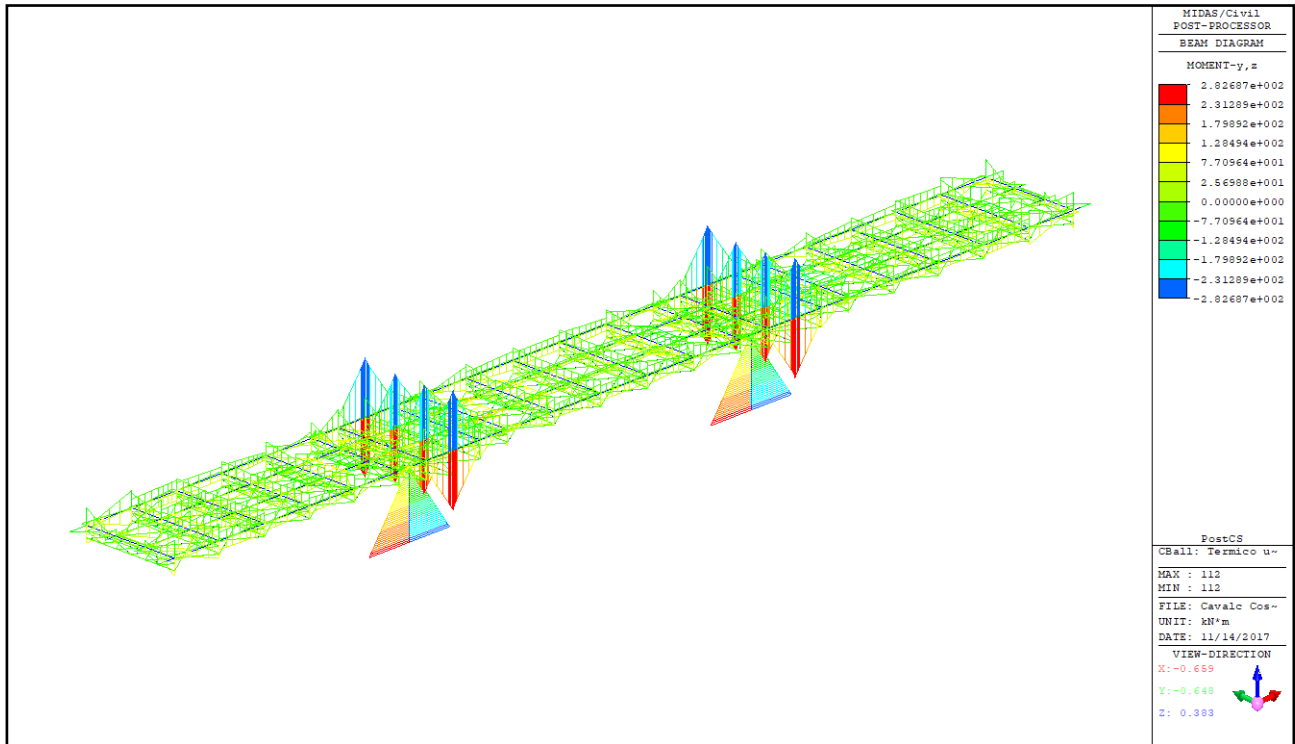
Inviluppo differenziale termico  $\pm 5^\circ/H$  - Diagramma momenti flettenti



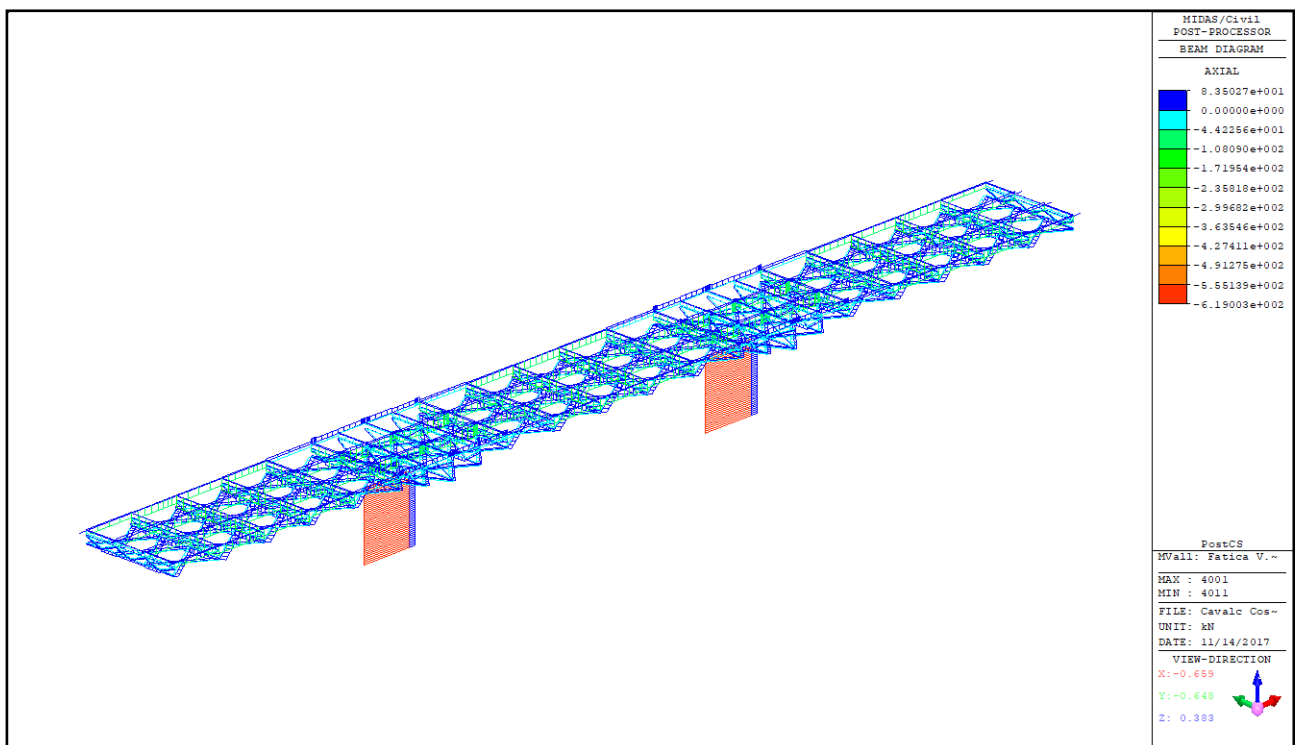
Inviluppo termico uniforme  $\pm 15^\circ$ - Diagramma sforzi assiali



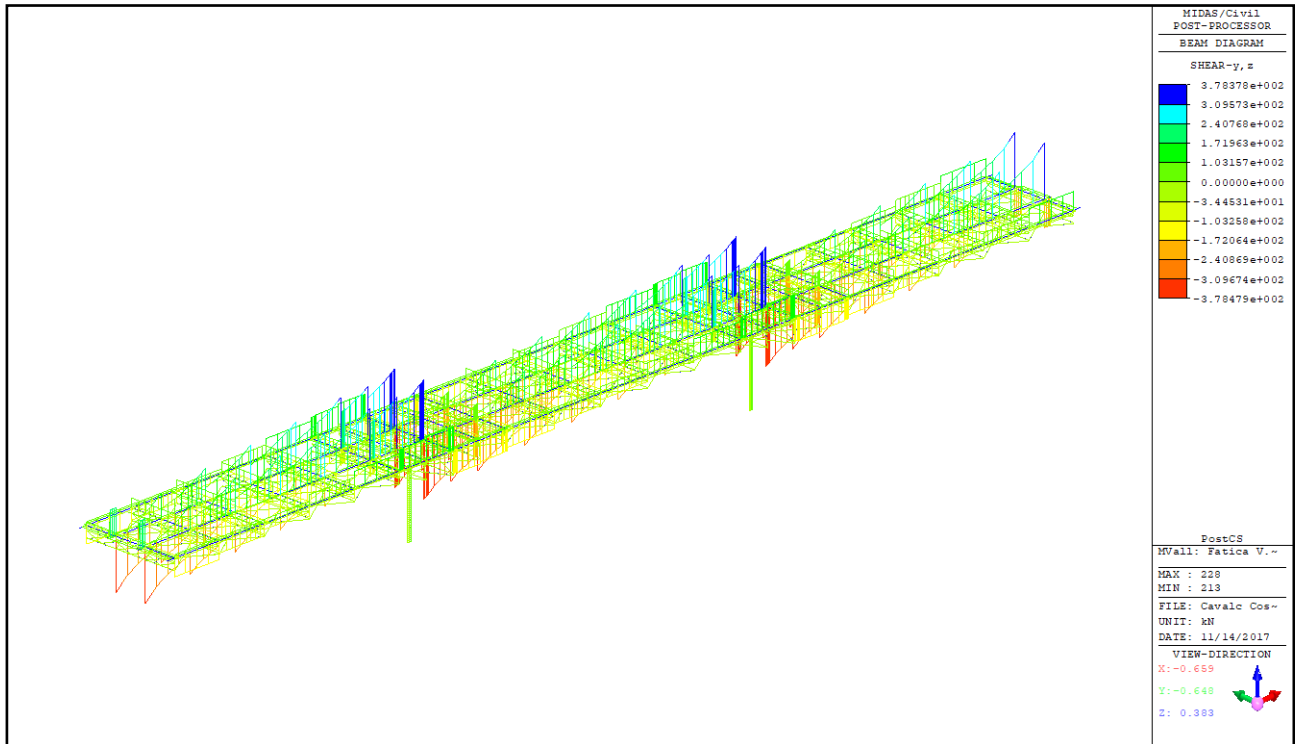
Inviluppo termico uniforme  $\pm 15^\circ$ - Diagramma sforzi taglianti



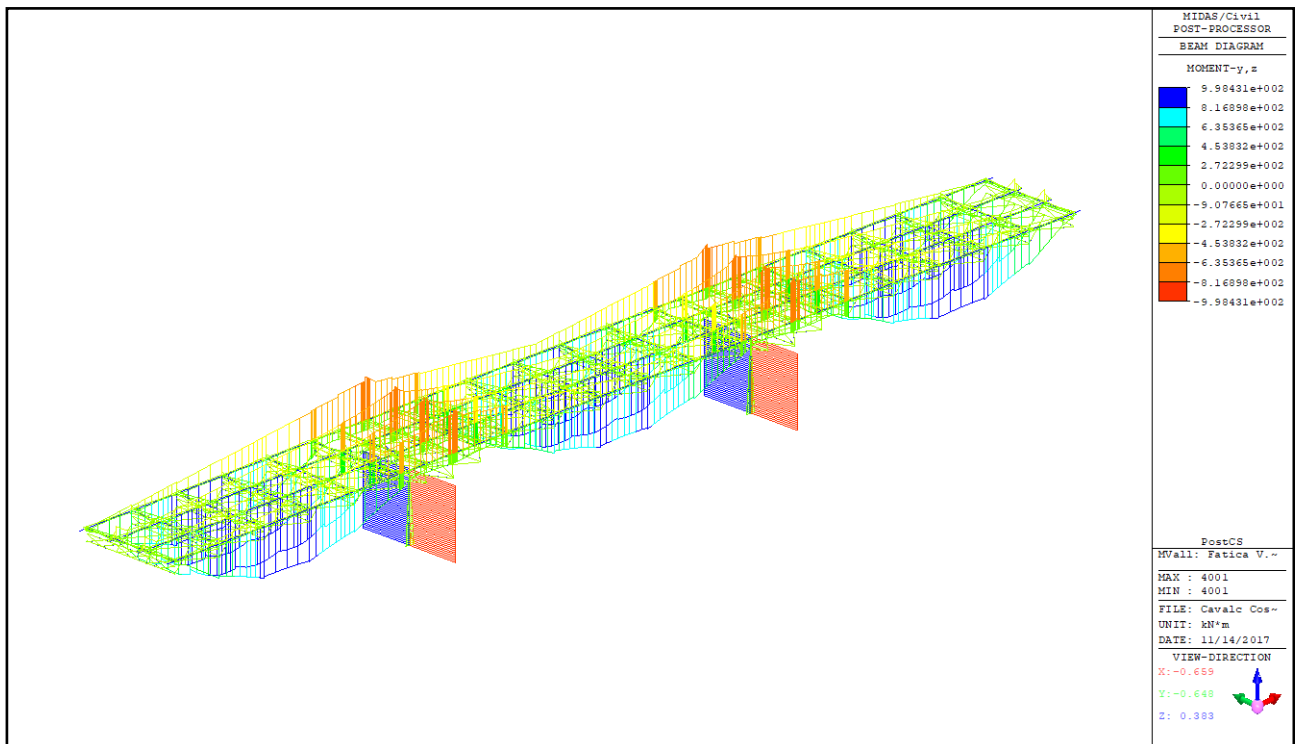
Inviluppo termico uniforme  $\pm 15^\circ$  - Diagramma momenti flettenti



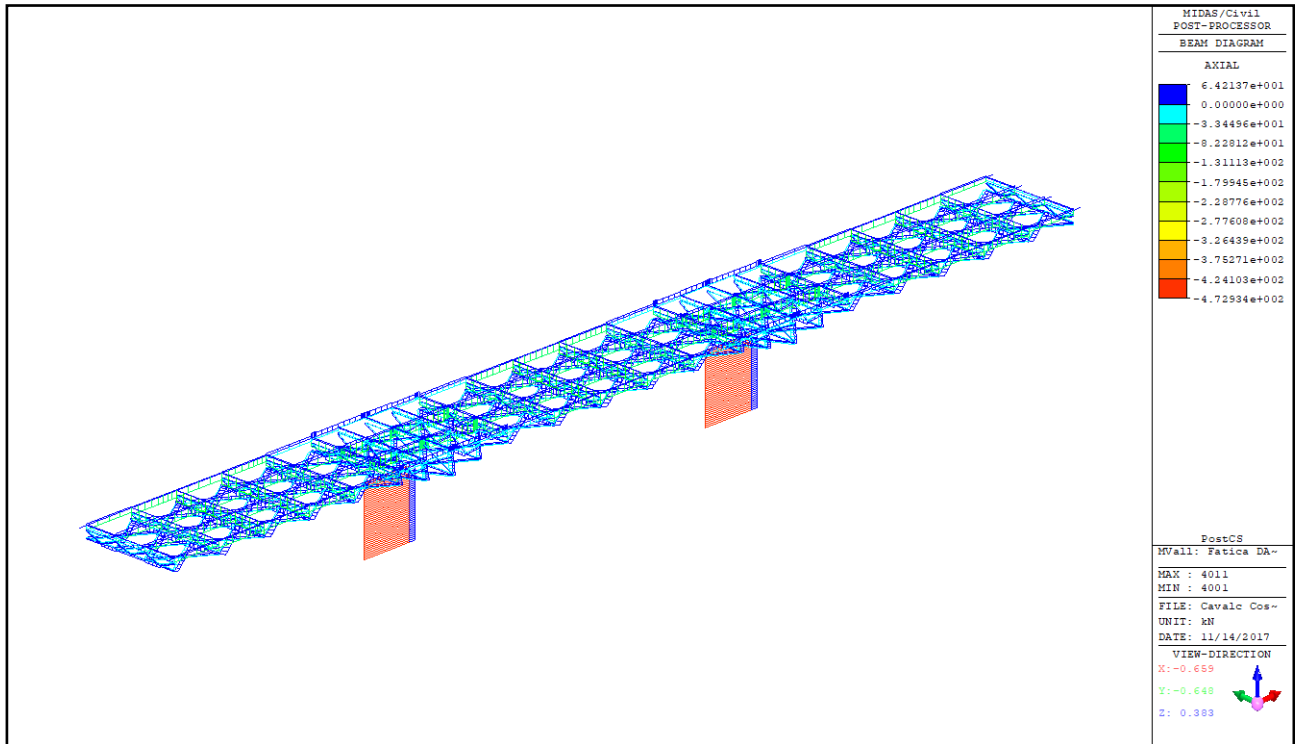
Carichi mobili fatica schema 2 (vita ill.) - Diagramma sforzi assiali



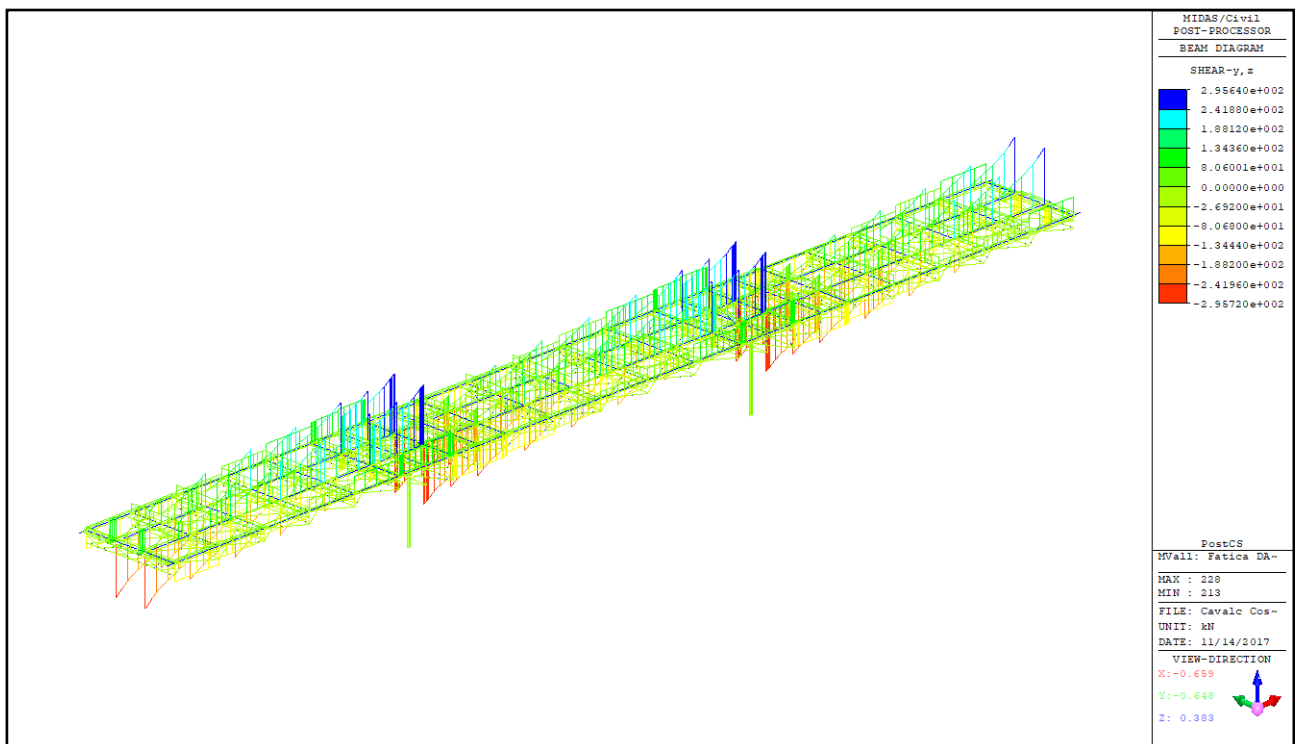
Carichi mobili fatica schema 2 (vita ill.) - Diagramma sforzi taglienti



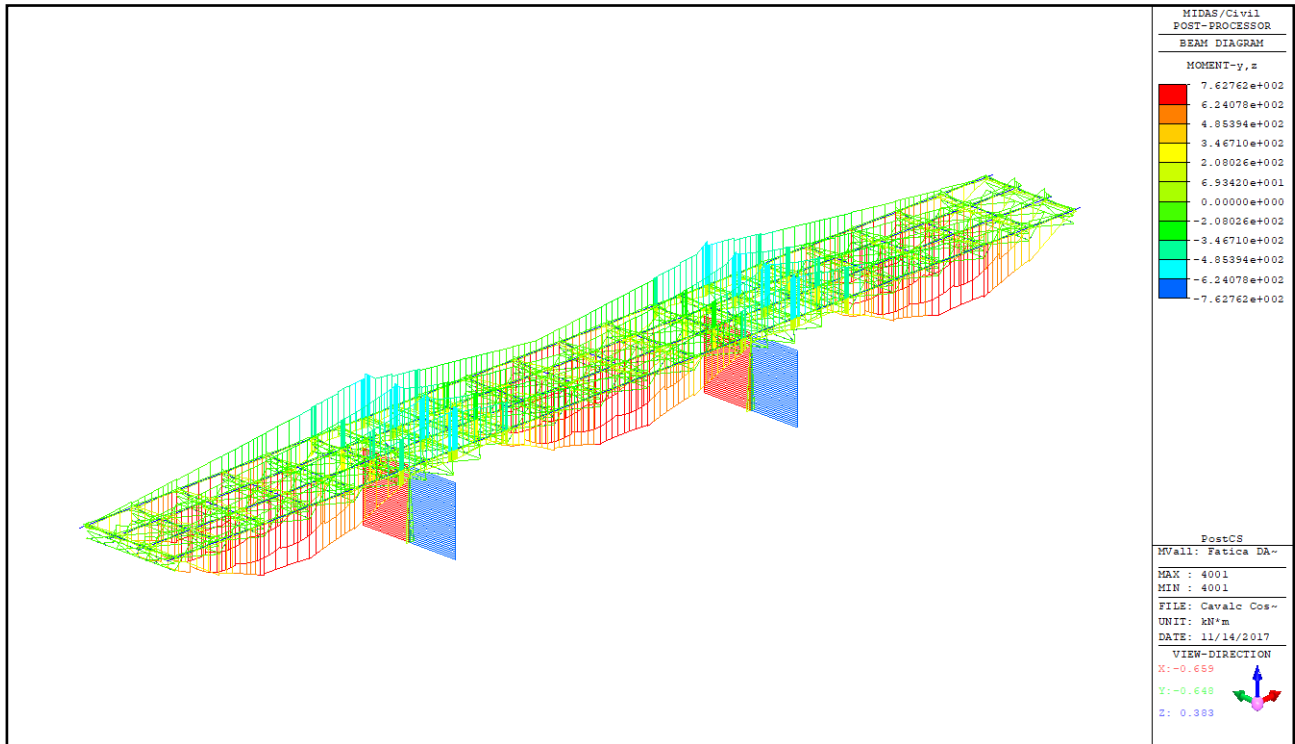
Carichi mobili fatica schema 2 (vita ill.) - Diagramma momenti flettenti



Involuppo carichi mobili fatica schema 3 (dann.) - Diagramma sforzi assiali



Involuppo carichi mobili fatica schema 3 (dann.) - Diagramma sforzi taglianti



Inviluppo carichi mobili fatica schema 3 (dann.) - Diagramma momenti flettenti

## 7 RISPOSTA SISMICA DELLA STRUTTURA

Sono stati determinati 40 modi di vibrazione con il metodo "Ritz Vectors"; i periodi determinati variano da 1.517 s a 0.0057 s; le masse eccitate risultano:

- dir. X: 98.36 %
- dir. Y: 96.91 %
- dir. Z: 96.63 %

Seguono schermate programma di calcolo.



EIGENVALUE ANALYSIS				
Mode No	Frequency		Period	Tolerance
	(rad/sec)	(cycle/sec)	(sec)	
1	4.141684	0.659169	1.517061	0.0000e+000
2	4.147372	0.660075	1.514980	0.0000e+000
3	4.552548	0.724561	1.380147	0.0000e+000
4	10.501335	1.671339	0.598323	0.0000e+000
5	15.100786	2.403365	0.416083	0.0000e+000
6	15.886037	2.528341	0.395516	0.0000e+000
7	19.046502	3.031345	0.329887	0.0000e+000
8	20.484184	3.260159	0.306733	0.0000e+000
9	20.969351	3.337376	0.299637	0.0000e+000
10	25.464820	4.052852	0.246740	0.0000e+000
11	40.266623	6.408632	0.156040	0.0000e+000
12	42.855605	6.820681	0.146613	0.0000e+000
13	46.950097	7.472340	0.133827	0.0000e+000
14	52.050809	8.284144	0.120713	0.0000e+000
15	57.142138	9.094454	0.109957	0.0000e+000
16	57.597127	9.166867	0.109089	0.0000e+000
17	61.402911	9.772577	0.102327	0.0000e+000
18	63.513330	10.108460	0.098927	0.0000e+000
19	63.776428	10.150334	0.098519	0.0000e+000
20	64.061995	10.195783	0.098080	0.0000e+000
21	71.126530	11.320139	0.088338	0.0000e+000
22	76.209149	12.129063	0.082447	0.0000e+000
23	85.226812	13.564268	0.073723	0.0000e+000
24	86.883004	13.827860	0.072318	0.0000e+000
25	90.620751	14.422740	0.069335	0.0000e+000
26	102.596331	16.328713	0.061242	0.0000e+000
27	105.190062	16.741518	0.059732	0.0000e+000
28	111.828694	17.798089	0.056186	0.0000e+000
29	117.778402	18.745015	0.053348	0.0000e+000
30	130.295025	20.737097	0.048223	0.0000e+000
31	164.179470	26.129974	0.038270	0.0000e+000
32	167.012164	26.580811	0.037621	0.0000e+000
33	180.060468	28.657514	0.034895	0.0000e+000
34	196.388404	31.256185	0.031994	0.0000e+000
35	215.635282	34.319421	0.029138	0.0000e+000
36	252.700206	40.218487	0.024864	0.0000e+000
37	473.458694	75.353291	0.013271	0.0000e+000
38	613.508881	97.642971	0.010241	0.0000e+000
39	921.890297	146.723398	0.006816	0.0000e+000
40	1096.957683	174.586238	0.005728	0.0000e+000





- ricoprimento armature inferiori: 8 cm
- ricoprimento armature superiori: 5 cm

Si riporta la verifica dettagliata di una singola sezione, presa ad esempio di calcolo; seguono i tabulati delle verifiche condensate delle singole sezioni, asta per asta, eseguite secondo NTC 2008 ed EuroCodice 4-2.

**Element Number : 104**  
**Position Information : J**

**1 Design Condition**

1.1 Design Parameters

- Partial factors

$\gamma_C$ for concrete	1.50	$\gamma_V$ for headed stud	1.25
$\gamma_S$ for reinforcing steel	1.15	$\gamma_{Ff}$ for equivalent constant Amplitude stress range	1.00
$\gamma_{M0}$ for structural steel	1.05	$\gamma_M$ for fatigue strength	1.15
$\gamma_{M1}$ for structural steel	1.10	$\gamma_{Mf,s}$ for fatigue strength of studs in shear	1.00

1.2 Material Information

- Structural steel

$$f_{sk} = 355.000 \text{ MPa} , E_s = 210000.000 \text{ MPa}$$

- Concrete

$$f_{ck} = 32.000 \text{ MPa} , E_{cm} = 33000.000 \text{ MPa}$$

- Reinforcement

$$f_{yk} = 450.000 \text{ MPa} , E_r = 210000.000 \text{ MPa}$$

1.3 Sectional Information

Slab

$B_c$	2950.000	mm	$t_c$	240.000	mm	$H_h$	60.000	mm
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Girder

$H_w$	930.000	mm	$B_1$	500.000	mm	$B_2$	600.000	mm
$t_w$	15.000	mm	$t_{f1}$	30.000	mm	$t_{f2}$	40.000	mm

Before

$A_a$	52950.000	mm <sup>2</sup>
$I_{y,a}$	9738865300.992	mm <sup>4</sup>
$I_{z,a}$	1032761562.500	mm <sup>4</sup>
$C_{y,a}$	300.000	mm
$C_{z,a}$	421.147	mm

After

$A_c$	165370.210	mm <sup>2</sup>
$I_{y,c}$	31006989969.831	mm <sup>4</sup>
$I_{z,c}$	82560834857.937	mm <sup>4</sup>
$C_{y,c}$	1475.000	mm
$C_{z,c}$	937.022	mm

Crack

$A_c$	58980.000	mm <sup>2</sup>
$I_{y,c}$	12777911732.514	mm <sup>4</sup>
$I_{z,c}$	5535258072.226	mm <sup>4</sup>
$C_{y,c}$	1475.000	mm
$C_{z,c}$	497.197	mm

## 2 Bending Resistance

### 2.1 Positive Moment

- Design load

Load combination name : ST SLU Mobili

$N_{a,Ed} = -139.922$  kN  
 $N_{c,Ed} = -33.949$  kN  
 $M_{a,Ed} = 2654.006$  kN · m  
 $M_{c,Ed} = 4677.637$  kN · m

- Stress

Top Flange

Left	$y_1$	-250.000	mm	$z_1$	62.978	mm	$\sigma_1$	-169.764	MPa
	$y_2$	-7.500	mm	$z_2$	62.978	mm	$\sigma_2$	-170.086	MPa
Right	$y_1$	250.000	mm	$z_1$	62.978	mm	$\sigma_1$	-170.427	MPa
	$y_2$	7.500	mm	$z_2$	62.978	mm	$\sigma_2$	-170.106	MPa

Bottom Flange

Left	$y_1$	-300.000	mm	$z_1$	-937.022	mm	$\sigma_1$	253.677	MPa
	$y_2$	-7.500	mm	$z_2$	-937.022	mm	$\sigma_2$	253.289	MPa
Right	$y_1$	300.000	mm	$z_1$	-937.022	mm	$\sigma_1$	252.881	MPa
	$y_2$	7.500	mm	$z_2$	-937.022	mm	$\sigma_2$	253.269	MPa

Web

Right	$y_1$	0.000	mm	$z_1$	32.978	mm	$\sigma_1$	-157.394	MPa
	$y_2$	0.000	mm	$z_2$	-897.022	mm	$\sigma_2$	236.344	MPa

- Classification of sections

Part	Class
Top flange	1
Web	1
Bottom flange	1
Section	1

- Plastic resistance moment,  $M_{pl,Rd}$

Plastic NA = 985.023 mm

$N_{slab} = 12838.400$  kN  
 $N_{g,top} = 2531.871$  kN (Upper side of PNA)  
 $N_{g,bot} = 15370.271$  kN (Lower side of PNA)

$M_{pl,Rd} = 12635.695$  kN · m  
 $x_{pl} = 314.977$  mm  
 $M_{Rd} = \beta M_{pl,Rd} = 12635.695$  kN · m  
 here,  $\beta = 1.000$

$M_{Rd} = 12635.695$  kN · m >  $M_{Ed} = 7331.643$  kN · m ...OK

### 3 Resistance to Vertical Shear

- Design load

Load combination name : ST SLU Mobili

$N_{Ed} = 48.061$  kN  
 $M_{a,Ed} = 2654.006$  kN · m  
 $M_{c,Ed} = 3482.839$  kN · m  
 $V_{Ed,a} = -47.615$  kN  
 $V_{Ed,c} = 581.580$  kN  
 $V_{Ed} = 533.964$  kN

- Stress

Top Flange

Left	$y_1$	-250.000	mm	$z_1$	62.978	mm	$\sigma_1$	-167.523	MPa
	$y_2$	-7.500	mm	$z_2$	62.978	mm	$\sigma_2$	-167.183	MPa
Right	$y_1$	250.000	mm	$z_1$	62.978	mm	$\sigma_1$	-166.823	MPa
	$y_2$	7.500	mm	$z_2$	62.978	mm	$\sigma_2$	-167.162	MPa

Bottom Flange

Left	$y_1$	-300.000	mm	$z_1$	-937.022	mm	$\sigma_1$	217.248	MPa
	$y_2$	-7.500	mm	$z_2$	-937.022	mm	$\sigma_2$	217.658	MPa
Right	$y_1$	300.000	mm	$z_1$	-937.022	mm	$\sigma_1$	218.088	MPa
	$y_2$	7.500	mm	$z_2$	-937.022	mm	$\sigma_2$	217.679	MPa

Web

Right	$y_1$	0.000	mm	$z_1$	32.978	mm	$\sigma_1$	-155.628	MPa
	$y_2$	0.000	mm	$z_2$	-897.022	mm	$\sigma_2$	202.275	MPa

- Classification of sections

Part	Class
Top flange	1
Web	1
Bottom flange	1
Section	1

- Plastic resistance moment,  $M_{pl,Rd}$

Plastic NA = 985.023 mm

$N_{slab} = 12838.400$  kN  
 $N_{g,top} = 2531.871$  kN  
 $N_{g,bot} = 15370.271$  kN

$M_{pl,Rd} = 12635.695$  kN · m

- Calculation.  $V_{bw,Rd}$

Web

$$k_{\tau} = 5.34 + 4.00 \cdot (h_w/a)^2 + k_{st} = 5.478 \quad \text{when } a/h_w = 5.376 \geq 1$$

$$\text{where, } h_w = 930.000 \text{ mm}$$

$$2.1/t \cdot (I_{sl}/h_w)^{1/3} = 0.000$$

$$k_{st} = 9 \cdot (h_w/a)^2 \cdot ((I_{sl} / (t^3 \cdot h_w))^3)^{1/4} > 2.1/t \cdot (I_{sl}/h_w)^{1/3} = 0.000$$

$$I_{sl} = 0.000 \text{ mm}^4$$

$$t = 15.000 \text{ mm}$$

$$\lambda_w = h_w / (37.4 \cdot t \cdot \epsilon \cdot \sqrt{k_{\tau}}) = 0.871$$

Contribution from the web

$$X_w = 0.83 / \lambda_w = 0.953 \quad 0.83/\eta \leq \lambda_w < 1.08$$

$$V_{bw,Rd} = \frac{X_w \cdot f_{yw} \cdot h_w \cdot t}{\sqrt{3} \cdot \gamma_{M1}} = 2478.296 \text{ kN}$$

$$V_{Rd} = 2478.296 \text{ kN}$$

$$V_{Ed} = V_{Ed} / \text{Num. of Web} = 533.964 \text{ kN}$$

$$\eta_3 = V_{Ed} / V_{bw,Rd} = 0.215 \leq 1.0$$

Contribution from the flange

$$M_{f,Rd0} = 10338.805 \text{ kN} \cdot \text{m}$$

$M_{f,Rd0}$  is calculated as  $M_{pl,Rd}$  but neglecting the web contribution.

$$\text{Reduction factor for } N_{Ed} = 1 - \frac{N_{Ed}}{(A_{t1} + A_{t2}) \cdot f_{yt} / \gamma_{M0}} = 1.000$$

$$M_{f,Rd} = \text{Reduction factor for } N_{Ed} \cdot M_{f,Rd0} = 10338.805 \text{ kN} \cdot \text{m}$$

$$V_{bf,Rd} = \frac{b_f \cdot t_f^2 \cdot f_{yt}}{c \cdot \gamma_{M1}} \left( 1 - \left( \frac{M_{Ed}}{M_{f,Rd}} \right)^2 \right) = 0.000 \text{ kN}$$

$$\text{where, } M_{f,Rd} = 10338.805 \text{ kN} \cdot \text{m}$$

$$M_{Ed} = 82307.578 \text{ kN} \cdot \text{m} \quad (\text{Taken as the greatest value of } (\sum \sigma_j)W)$$

$$c = a \cdot \left( 0.25 + \frac{1.6 \cdot b_f \cdot t_f^2 \cdot f_{yt}}{t \cdot h_w^2 \cdot f_{yw}} \right) = 1527.489$$

$$V_{Ed} / (V_{bw,Rd} + V_{bf,Rd}) = 0.215 \leq 1.0 \quad \dots \text{ OK}$$

#### 4 Resistance to Lateral Torsional Buckling

- Design load

Load combination name : ST SLU Mobili

$$\begin{aligned} N_{Ed} &= -173.871 \text{ kN} \\ M_{Ed} &= 4677.637 \text{ kN} \cdot \text{m} \\ V_1 &= -206.883 \text{ kN} \\ V_2 &= 128.218 \text{ kN} \\ M_1 &= 7065.803 \text{ kN} \cdot \text{m} \\ M_2 &= 7331.643 \text{ kN} \cdot \text{m} \\ M_{pl,Rd} &= 12635.695 \text{ kN} \cdot \text{m} \\ M_{el,Rd} &= 10044.064 \text{ kN} \cdot \text{m} \end{aligned}$$

-  $M_{b,Rd}$  Buckling resistance moment

$$\begin{aligned} L &= 2000.000 \text{ m} \\ c &= C_d / I = 0.000 \\ \gamma &= c \cdot L^4 / (E \cdot I) = 0.000 \\ \mu &= V_2 / V_1 = 0.620 \\ \Phi &= 2 \cdot (1 - M_2 / M_1) / (1 + \mu) = 0.045 \\ m_1 &= 1 + 0.44 \cdot (1 + \mu) \cdot \Phi^{1.5} + (3 + 2 \cdot \Phi) \cdot \sqrt{(350 - 50 \cdot \mu)} = 1.007 \\ m_2 &= 1 + 0.44 \cdot (1 + \mu) \cdot \Phi^{1.5} + (0.195 + (0.05 + \mu / 100) \cdot \Phi) \cdot \sqrt{0.5} = 1.007 \\ m &= \text{Min}(m_1, m_2) = 1.007 \\ \alpha_{LT} &= 0.490 \\ \lambda_{LT} &= 1.103 \cdot L / b \cdot \sqrt{(f_y / E_m)} \cdot \sqrt{(1 + A_{we} / (3 \cdot I))} = 0.182 \\ \Phi_{LT} &= 0.5 \cdot (1 + \alpha_{LT} \cdot (\lambda_{LT} - 0.2) + \lambda_{LT}^2) = 0.512 \\ X_{LT} &= \frac{1}{\Phi_{LT} + \sqrt{(\Phi_{LT}^2 - \lambda_{LT}^2)}} = 1 \end{aligned}$$

$$\begin{aligned} M_{Rd} &= 12635.695 \text{ kN} \cdot \text{m} \\ M_{b,Rd} &= X_{LT} \cdot M_{Rd} = 12635.695 \text{ kN} \cdot \text{m} \end{aligned}$$

-  $N_{b,Rd}$  Buckling resistance moment

$$\begin{aligned} X_{T,N} &= 0.996 \\ N_{b,Rd} &= X_{T,N} \cdot \text{Area} \cdot f_{y,d} = 55693.514 \text{ kN} \end{aligned}$$

$$\text{Combined Ratio} = \frac{N_{Ed}}{N_{b,Rd}} + \frac{M_{Ed}}{M_{b,Rd}} = 0.373314224$$

### 5 Resistance to Longitudinal Shear

- Design load

Load combination name : ST SLU Mobili

$$\begin{aligned} N_{c,el} &= 6442.914 \text{ kN} \\ N_{c,f} &= 12838.400 \text{ kN} \\ M_{Ed} &= 6136.844 \text{ kN} \cdot \text{m} \\ V_{Ed} &= 581.580 \text{ kN} \\ M_{pl,Rd} &= 12635.695 \text{ kN} \cdot \text{m} \\ M_{el,Rd} &= 10044.064 \text{ kN} \cdot \text{m} \end{aligned}$$

- Shear resistance of a single connector

$$\begin{aligned} P_{Rd,1} &= 0.8 \cdot f_u \cdot \pi \cdot d^2 / 4 / \gamma_V = 111.478 \text{ kN} \\ P_{Rd,2} &= 0.29 \cdot \alpha \cdot d^2 \cdot \sqrt{f_{ck} \cdot E_{cm}} / \gamma_V = 117.497 \text{ kN} \\ P_{Rd} &= \text{Min}(P_{Rd,1}, P_{Rd,2}) = 111.478 \text{ kN} \end{aligned}$$

$$\begin{aligned} \text{where, } f_u &= 450.000 \text{ MPa} \\ \alpha &= 1 \quad \text{for } h_{sc}/d > 4 \\ \text{Num.} &= 3 \\ d &= 22.200 \text{ mm} \\ h_{sc} &= 175.000 \text{ mm} \\ \text{Space} &= 200.000 \text{ mm} \end{aligned}$$

- Verification

$$\begin{aligned} v_{L,Ed} &= V_{Ed} \cdot (A \cdot z / l) = 515.263 \text{ kN/m} \\ v_{L,Rd} &= P_{Rd} \cdot \text{Num.} / \text{Space} = 1672.167 \text{ kN/m} \\ v_{L,Ed} &\leq v_{L,Rd} \quad \dots \text{ OK} \end{aligned}$$

### 6 Resistance to Fatigue

- Design load

Load comt ST FATICA DANN.ACC.

$$M_y = 2041.722 \text{ kN} \cdot \text{m}$$

- Shear stress range for the connector

$$\begin{aligned} \Delta\tau &= F_{sc} / A_{sc} = 9.092 \text{ MPa} \\ \text{where, } F_{sc} &= v_{L,Ed} \cdot \text{space of stud} / \text{number of stud} = 3.519 \text{ kN} \\ A_{sc} &= 387.076 \text{ mm}^2 \end{aligned}$$

- Damage equivalent factor

$$\begin{aligned} \lambda_v &= \lambda_{v,1} \cdot \lambda_{v,2} \cdot \lambda_{v,3} \cdot \lambda_{v,4} = 1.484 \\ \text{where, } \lambda_{v,1} &= 1.550 \\ \lambda_{v,2} &= 1.100 \\ \lambda_{v,3} &= 0.871 \\ \lambda_{v,4} &= 1.000 \end{aligned}$$

- Equivalent constant amplitude range of shear stress related to 2 million cycles

$$\Delta\tau_{E,2} = \lambda_v \cdot \Delta\tau = 13.495 \text{ MPa}$$

- Verification

$$\gamma_{FI} \cdot \Delta\tau_{E,2} / (\Delta\tau_c / \gamma_{Mf,s}) = 0.150 \leq 1$$

## 7 Stress Limitation

- In the structural steel

Characteristic load combination name : ST RARA Mobili  
 $\sigma_{Ed,ser} = -184.371$  MPa (Bottom-left fiber in the flange)  
 $T_{Ed,ser} = 7.302$  MPa (Neutral axis in the web)

$$\sigma_{Ed,ser} \leq f_y / \gamma_{M,ser}$$

$$-184.371 \text{ MPa} \leq 355.000 \text{ MPa} \quad \dots \text{ OK}$$

$$T_{Ed,ser} \leq f_y / (\sqrt{3} \cdot \gamma_{M,ser})$$

$$7.302 \text{ MPa} \leq 204.959 \text{ MPa} \quad \dots \text{ OK}$$

$$\sqrt{(\sigma_{Ed,ser})^2 + 3(T_{Ed,ser})^2} \leq f_y / \gamma_{M,ser}$$

$$184.805 \text{ MPa} \leq 355.000 \text{ MPa} \quad \dots \text{ OK}$$

- In the concrete of the slab

Characteristic load combination name : ST RARA Mobili  
 $\sigma_c \leq k_1 f_{ck}$   
 $6.798 \text{ MPa} \leq 19.200 \text{ MPa} \quad \dots \text{ OK}$

- In the reinforcement

Load combination name : ST RARA Mobili  
 $\sigma_s \leq k_3 f_{yk}$   
 $-37.666 \text{ MPa}$   
 Rebar is under compression. No need to check.

## 8 Longitudinal Shear for SLS(Serviceability limit state)

- Shear resistance of a single connector

Load combination name : ST RARA Mobili  
 $P_{Rd,1} = 0.8 \cdot f_u \cdot \pi \cdot d^2 / 4 / \gamma_V = 111.478 \text{ kN}$   
 $P_{Rd,2} = 0.29 \cdot \alpha \cdot d^2 \cdot \sqrt{f_{ck} \cdot E_{cm}} / \gamma_V = 117.497 \text{ kN}$   
 $P_{Rd} = \min(P_{Rd,1}, P_{Rd,2}) = 111.478 \text{ kN}$   
 $P_{Rd,ser} = k_s \cdot P_{Rd} = 66.887 \text{ kN}$

where,  $f_u = 450.000 \text{ MPa}$   
 $\alpha = 1$  for  $h_{sc}/d > 4$   
 Num. = 3  
 $d = 22.200 \text{ mm}$   
 $h_{sc} = 175.000 \text{ mm}$   
 Space = 200.000 mm  
 $k_s = 0.600$

- Verification

$V_{L,Ed} = V_{Ed} \cdot (A \cdot z / I) = 387.773 \text{ kN/m}$   
 $V_{L,Rd} = P_{Rd,ser} \cdot \text{Num.} / \text{Space} = 1003.300 \text{ kN/m}$   
 $V_{L,Ed} \leq V_{L,Rd} \quad \dots \text{ OK}$

## 8.1 VERIFICHE DI RESISTENZA A FLESSIONE ALLO SLU

Si riportano le verifiche allo stato limite ultimo per flessione delle sezioni composte travi-soletta. I coefficienti parziali per SLU assunti sono i seguenti:

$\gamma_C = 1.5$  materiale calcestruzzo  
 $\gamma_S = 1.15$  acciaio armature soletta  
 $\gamma_{M0} = 1.05$  acciaio strutturale: resistenza sezioni

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi positiva e negativa più gravose.

Dati tabulati:

Elem Property: nome delle caratteristiche geometriche dell'elemento



Elem: numero dell'elemento  
 Position: nodo iniziale (I) o finale (J) dell'elemento  
 Lcom: combinazione di carico positivo o negativo più gravosa  
 Type: sollecitazione (massima o minima)  
 Top class: classificazione flangia superiore  
 Bot class: classificazione flangia inferiore  
 Web class: classificazione anima  
 Sect. class: classificazione complessiva sezione  
 Ma,Ed: momento di calcolo applicato alla trave in acciaio isolata  
 Mc,Ed: momento di calcolo applicato alla sezione composta  
 Mpl,Rd: momento resistente plastico, per sezioni di classe 1 e 2  
 Mel,Rd: momento resistente elastico, per sezioni di classe 3, efficace per classe 4  
 M\_Rd: momento resistente effettivo della sezione

La condizione di verifica della sezione è la seguente:

$$\text{Verification ratio: } \frac{Ma,Ed + Mc,Ed}{M\_Rd} \leq 1$$

Elem property	Elem number	Position [node]	Positive/Negative	Lcom	Type	Top Class	Bot Class	Web Class	Sect. Class	Ma,Ed (kN*m)	Mc,Ed (kN*m)	Mpl,Rd (kN*m)	Mel,Rd (kN*m)	M_Rd (kN*m)	Verif. Ratio
Concio 1	101	I[719]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.60	8011.39	6350.05	6350.05	0.000
Concio 1	101	I[719]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.60	10750.90	9086.69	10750.90	0.000
Concio 1	101	J[720]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-13.72	-343.84	8011.39	6363.77	6363.77	0.056
Concio 1	101	J[720]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	102	I[720]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-34.14	-865.38	8011.39	6384.18	6384.18	0.141
Concio 1	102	I[720]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-34.14	232.04	10750.90	9101.69	10750.90	0.018
Concio 1	102	J[721]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	102	J[721]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1889.00	3524.43	10750.90	8256.38	10750.90	0.504
Concio 1	103	I[721]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	103	I[721]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1867.51	3235.71	10750.90	8265.83	10750.90	0.475
Concio 1	103	J[722]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	103	J[722]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2464.89	4593.24	10750.90	8003.25	10750.90	0.657
Concio 2	104	I[722]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	104	I[722]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2471.60	4594.21	12635.70	10122.68	12635.70	0.559
Concio 2	104	J[723]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	104	J[723]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2654.01	4677.64	12635.70	10044.06	12635.70	0.580
Concio 2	105	I[723]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	105	I[723]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2655.98	4610.79	12635.70	10043.21	12635.70	0.575
Concio 2	105	J[724]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2261.86	-292.17	9000.58	4380.05	9000.58	0.219
Concio 2	105	J[724]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2261.86	4191.53	12635.70	10213.07	12635.70	0.511
Concio 2	106	I[724]	Neg	ST SLU Frenamento	MY-MIN	2	1	1	2	2274.09	-286.90	9000.58	4367.82	9000.58	0.221
Concio 2	106	I[724]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2274.09	4299.66	12635.70	10207.80	12635.70	0.520
Concio 2	106	J[725]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1087.20	-95.99	9000.58	5554.71	9000.58	0.110
Concio 2	106	J[725]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.72	3524.92	12635.70	10555.33	12635.70	0.395
Concio 3	107	I[725]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1086.79	-97.94	10249.33	6848.87	10249.33	0.096
Concio 3	107	I[725]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.16	3525.39	13386.70	11067.30	13386.70	0.373
Concio 3	107	J[726]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	710.53	-1609.20	10249.33	7225.13	10249.33	0.088
Concio 3	107	J[726]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	710.53	2174.40	13386.70	11425.43	13386.70	0.216
Concio 3	108	I[726]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	736.76	-1558.36	10249.33	7198.90	10249.33	0.080
Concio 3	108	I[726]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.76	2432.61	13386.70	11413.01	13386.70	0.237
Concio 3	108	J[727]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1970.02	-4227.83	10249.33	8951.27	10249.33	0.605
Concio 3	108	J[727]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	109	I[727]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1960.11	-4114.23	10249.33	8952.86	10249.33	0.593
Concio 3	109	I[727]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	109	J[728]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2286.73	-4465.04	10249.33	8775.36	10249.33	0.659
Concio 3	109	J[728]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	110	I[728]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2283.89	-4447.79	13376.13	10750.38	10750.38	0.626
Concio 4 H=var	110	I[728]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	110	J[729]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.85	-6450.78	21530.83	16899.82	16899.82	0.625
Concio 4 H=var	110	J[729]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	111	I[729]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.32	-6423.14	21530.83	16899.90	16899.90	0.623
Concio 4 H=var	111	I[729]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	111	J[730]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.80	-8769.70	30530.77	23173.54	23173.54	0.646
Concio 4 H=var	111	J[730]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	112	I[730]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.80	-8769.70	30530.77	23173.28	23173.28	0.646
Concio 4 H=200	112	I[730]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000

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Concio 4 H=200	112	J[731]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6646.06	-9402.83	30530.77	23132.45	23132.45	0.694
Concio 4 H=200	112	J[731]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	113	I[731]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6638.47	-9455.35	30530.77	23133.14	23133.14	0.696
Concio 4 H=200	113	I[731]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	113	J[732]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.38	-8832.54	30530.77	23171.51	23171.51	0.649
Concio 4 H=200	113	J[732]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	114	I[732]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.38	-8832.54	30530.77	23171.56	23171.56	0.649
Concio 4 H=var	114	I[732]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	114	J[733]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4258.59	-6516.98	21530.83	16878.64	16878.64	0.638
Concio 4 H=var	114	J[733]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	115	I[733]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4254.46	-6543.31	21530.83	16879.21	16879.21	0.640
Concio 4 H=var	115	I[733]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	115	J[734]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2570.83	-4778.59	13376.13	10698.99	10698.99	0.687
Concio 4 H=var	115	J[734]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	116	I[734]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2574.42	-4768.78	10249.33	8556.43	10249.33	0.716
Concio 5	116	I[734]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	116	J[735]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2276.35	-4472.84	10249.33	8783.25	10249.33	0.659
Concio 5	116	J[735]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	117	I[735]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2281.02	-4549.90	10249.33	8779.70	10249.33	0.666
Concio 5	117	I[735]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	117	J[736]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.26	-2336.76	10249.33	7724.40	10249.33	0.207
Concio 5	117	J[736]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.26	1672.85	13386.70	11661.74	13386.70	0.141
Concio 5	118	I[736]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	187.77	-2357.73	10249.33	7747.89	10249.33	0.212
Concio 5	118	I[736]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	187.77	1425.22	13386.70	11672.86	13386.70	0.120
Concio 5	118	J[737]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	888.92	-1932.90	10249.33	7046.74	10249.33	0.102
Concio 5	118	J[737]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	888.92	2727.66	13386.70	11340.99	13386.70	0.270
Concio 6	119	I[737]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	889.33	-1933.97	8775.51	5644.61	8775.51	0.119
Concio 6	119	I[737]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	889.33	2727.36	12181.58	10480.17	12181.58	0.297
Concio 6	119	J[738]	Neg	ST SLU Mobili	MY-MIN	2	1	2	2	1444.28	-37.50	8775.51	5089.66	8775.51	0.160
Concio 6	119	J[738]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1444.28	3158.55	12181.58	10256.59	12181.58	0.378
Concio 6	120	I[738]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1436.50	-133.37	8775.51	5097.44	8775.51	0.148
Concio 6	120	I[738]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1436.50	3069.75	12181.58	10259.73	12181.58	0.370
Concio 6	120	J[739]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1582.69	-43.54	8775.51	4951.25	8775.51	0.175
Concio 6	120	J[739]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1582.69	3602.92	12181.58	10200.83	12181.58	0.426
Concio 6	121	I[739]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1582.69	-43.54	8775.51	4951.25	8775.51	0.175
Concio 6	121	I[739]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1582.69	3602.92	12181.58	10200.83	12181.58	0.426
Concio 6	121	J[740]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1436.48	-98.81	8775.51	5097.46	8775.51	0.152
Concio 6	121	J[740]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1436.48	3069.90	12181.58	10259.73	12181.58	0.370
Concio 6	122	I[740]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1444.26	-203.32	8775.51	5089.68	8775.51	0.141
Concio 6	122	I[740]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1444.26	3158.24	12181.58	10256.60	12181.58	0.378
Concio 6	122	J[741]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	889.28	-1933.87	8775.51	5644.65	8775.51	0.119
Concio 6	122	J[741]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	889.28	2726.62	12181.58	10480.19	12181.58	0.297
Concio 5	123	I[741]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	888.87	-1932.80	10249.33	7046.79	10249.33	0.102
Concio 5	123	I[741]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	888.87	2726.94	13386.70	11341.02	13386.70	0.270
Concio 5	123	J[742]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	187.70	-2357.53	10249.33	7747.96	10249.33	0.212
Concio 5	123	J[742]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	187.70	1425.42	13386.70	11672.89	13386.70	0.121
Concio 5	124	I[742]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.19	-2336.60	10249.33	7724.47	10249.33	0.207
Concio 5	124	I[742]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.19	1672.31	13386.70	11661.77	13386.70	0.141
Concio 5	124	J[743]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2281.12	-4551.05	10249.33	8779.62	10249.33	0.667
Concio 5	124	J[743]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	125	I[743]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2276.46	-4473.85	10249.33	8783.17	10249.33	0.659
Concio 5	125	I[743]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	125	J[744]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2574.53	-4769.72	10249.33	8556.34	10249.33	0.717
Concio 5	125	J[744]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	126	I[744]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2570.94	-4779.66	13376.13	10698.97	10698.97	0.687
Concio 4 H=var	126	I[744]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	126	J[745]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4254.59	-6544.69	21530.83	16879.19	16879.19	0.640
Concio 4 H=var	126	J[745]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	127	I[745]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4258.72	-6518.32	21530.83	16878.62	16878.62	0.639
Concio 4 H=var	127	I[745]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	127	J[746]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.52	-8833.55	30530.77	23171.55	23171.55	0.649
Concio 4 H=var	127	J[746]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	128	I[746]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.52	-8833.55	30530.77	23171.50	23171.50	0.649
Concio 4 H=200	128	I[746]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	128	J[747]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6638.61	-9456.01	30530.77	23133.13	23133.13	0.696
Concio 4 H=200	128	J[747]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	129	I[747]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6646.21	-9403.83	30530.77	23132.44	23132.44	0.694
Concio 4 H=200	129	I[747]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	129	J[748]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.95	-8770.48	30530.77	23173.27	23173.27	0.646
Concio 4 H=200	129	J[748]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	130	I[748]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.95	-8770.48	30530.77	23173.53	23173.53	0.646
Concio 4 H=var	130	I[748]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	130	J[749]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.45	-6423.08	21530.83	16899.88	16899.88	0.623
Concio 4 H=var	130	J[749]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	131	I[749]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.99	-6450.716	21530.83	16899.8	16899.8	0.625
Concio 4 H=var	131	I[749]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	131	J[750]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2284.02	-4447.55	13376.13	10750.36	10750.36	0.626
Concio 4 H=var	131	J[750]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	132	I[750]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2286.85	-4464.81	10249.33	8775.26	10249.33	0.659
Concio 3	132	I[750]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000

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Concio 3	132	J[751]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1960.24	-4113.88	10249.33	8952.84	10249.33	0.593
Concio 3	132	J[751]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	133	J[751]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1970.14	-4227.50	10249.33	8951.25	10249.33	0.605
Concio 3	133	J[751]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	133	J[752]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	736.66	-1558.53	10249.33	7199.00	10249.33	0.080
Concio 3	133	J[752]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.66	2432.91	13386.70	11413.06	13386.70	0.237
Concio 3	134	J[752]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	710.42	-1609.51	10249.33	7225.24	10249.33	0.088
Concio 3	134	J[752]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	710.42	2174.62	13386.70	11425.48	13386.70	0.216
Concio 3	134	J[753]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1086.72	-97.76	10249.33	6848.94	10249.33	0.096
Concio 3	134	J[753]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.07	3525.99	13386.70	11067.35	13386.70	0.373
Concio 2	135	J[753]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1087.13	-95.80	9000.58	5554.78	9000.58	0.110
Concio 2	135	J[753]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.63	3525.52	12635.70	10555.37	12635.70	0.395
Concio 2	135	J[754]	Neg	ST SLU Frenamento	MY-MIN	2	1	1	2	2274.02	-287.18	9000.58	4367.90	9000.58	0.221
Concio 2	135	J[754]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2274.02	4300.66	12635.70	10207.83	12635.70	0.520
Concio 2	136	J[754]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2261.79	-292.30	9000.58	4380.13	9000.58	0.219
Concio 2	136	J[754]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2261.79	4191.27	12635.70	10213.11	12635.70	0.511
Concio 2	136	J[755]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	136	J[755]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2655.93	4610.87	12635.70	10043.23	12635.70	0.575
Concio 2	137	J[755]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	137	J[755]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2653.95	4677.41	12635.70	10044.09	12635.70	0.580
Concio 2	137	J[756]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	137	J[756]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2471.56	4594.32	12635.70	10122.70	12635.70	0.559
Concio 1	138	J[756]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	138	J[756]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2464.85	4593.41	10750.90	8003.27	10750.90	0.657
Concio 1	138	J[757]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	138	J[757]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1867.48	3235.64	10750.90	8265.84	10750.90	0.475
Concio 1	139	J[757]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	139	J[757]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1888.97	3524.39	10750.90	8256.40	10750.90	0.504
Concio 1	139	J[758]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-34.14	-865.29	8011.39	6384.18	6384.18	0.141
Concio 1	139	J[758]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-34.14	232.09	10750.90	9101.69	10750.90	0.018
Concio 1	140	J[758]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-13.72	-343.82	8011.39	6363.77	6363.77	0.056
Concio 1	140	J[758]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	140	J[759]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.60	8011.39	6350.05	6350.05	0.000
Concio 1	140	J[759]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.60	10750.90	9086.69	10750.90	0.000
Concio 1	201	J[760]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	201	J[760]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	201	J[761]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-14.93	-339.51	8093.91	6494.02	6494.02	0.055
Concio 1	201	J[761]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	202	J[761]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-41.87	-511.14	8093.91	6520.95	6520.95	0.085
Concio 1	202	J[761]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-41.87	235.20	10954.92	9148.03	10954.92	0.018
Concio 1	202	J[762]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	202	J[762]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1882.97	3446.75	10954.92	8288.97	10954.92	0.487
Concio 1	203	J[762]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	203	J[762]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1855.06	3212.42	10954.92	8301.42	10954.92	0.463
Concio 1	203	J[763]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	203	J[763]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2459.13	4167.93	10954.92	8031.82	10954.92	0.605
Concio 2	204	J[763]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	204	J[763]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2468.10	4173.87	12846.08	10158.17	12846.08	0.517
Concio 2	204	J[764]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1952.64	-45.11	9113.78	4824.04	9113.78	0.209
Concio 2	204	J[764]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2636.06	4415.66	12846.08	10084.71	12846.08	0.549
Concio 2	205	J[764]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1956.15	-19.10	9113.78	4820.53	9113.78	0.213
Concio 2	205	J[764]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2640.81	4472.04	12846.08	10082.64	12846.08	0.554
Concio 2	205	J[765]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2263.92	-160.67	9113.78	4512.76	9113.78	0.231
Concio 2	205	J[765]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2263.92	4123.20	12846.08	10247.47	12846.08	0.497
Concio 2	206	J[765]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2276.14	-149.87	9113.78	4500.54	9113.78	0.233
Concio 2	206	J[765]	Pos	ST SLU Mobili	FZ-MAX	1	1	1	1	2276.14	4117.19	12846.08	10242.12	12846.08	0.498
Concio 2	206	J[766]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.75	-129.26	9113.78	5668.93	9113.78	0.107
Concio 2	206	J[766]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1495.47	3215.24	12846.08	10583.54	12846.08	0.367
Concio 3	207	J[766]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.16	-131.17	10397.59	7033.59	10397.59	0.094
Concio 3	207	J[766]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1494.67	3216.07	13602.15	11101.44	13602.15	0.346
Concio 3	207	J[767]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	736.26	-1547.73	10397.59	7404.49	10397.59	0.078
Concio 3	207	J[767]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.26	2363.29	13602.15	11465.91	13602.15	0.228
Concio 3	208	J[767]	Neg	ST SLU Mobili	MY-MIN	3	1	1	3	770.24	-1405.91	10397.59	7370.51	7370.51	0.086
Concio 3	208	J[767]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	770.24	2538.73	13602.15	11449.58	13602.15	0.243
Concio 3	208	J[768]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1976.97	-3826.71	10397.59	8994.18	10397.59	0.558
Concio 3	208	J[768]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	209	J[768]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1967.65	-3861.22	10397.59	8995.75	10397.59	0.561
Concio 3	209	J[768]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	209	J[769]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2306.60	-4164.77	10397.59	8938.69	10397.59	0.622
Concio 3	209	J[769]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	210	J[769]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2303.85	-4226.25	13534.54	10825.92	10825.92	0.603
Concio 4 H=var	210	J[769]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	210	J[770]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4201.30	-5945.46	21870.53	16988.30	16988.30	0.597
Concio 4 H=var	210	J[770]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	211	J[770]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4200.62	-5961.92	21870.53	16988.40	16988.40	0.598
Concio 4 H=var	211	J[770]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	211	J[771]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.65	-8168.04	31054.78	23218.83	23218.83	0.630
Concio 4 H=var	211	J[771]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	212	J[771]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.65	-8168.04	31054.78	23218.46	23218.46	0.630
Concio 4 H=200	212	J[771]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000

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Concio 4 H=200	212	J[772]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6954.85	-8673.02	31054.78	23171.96	23171.96	0.674
Concio 4 H=200	212	J[772]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	213	I[772]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6952.54	-8743.33	31054.78	23172.18	23172.18	0.677
Concio 4 H=200	213	I[772]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	213	J[773]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.83	-8153.85	31054.78	23215.97	23215.97	0.631
Concio 4 H=200	213	J[773]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	214	I[773]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.83	-8153.85	31054.78	23216.22	23216.22	0.631
Concio 4 H=var	214	I[773]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	214	J[774]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4368.80	-6100.40	21870.53	16963.53	16963.53	0.617
Concio 4 H=var	214	J[774]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	215	I[774]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4370.81	-6091.47	21870.53	16963.24	16963.24	0.617
Concio 4 H=var	215	I[774]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	215	J[775]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2611.59	-4568.36	13534.54	10767.41	10767.41	0.667
Concio 4 H=var	215	J[775]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	216	I[775]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2614.32	-4526.73	10397.59	8705.68	10397.59	0.687
Concio 5	216	I[775]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	216	J[776]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2302.99	-4246.77	10397.59	8939.30	10397.59	0.630
Concio 5	216	J[776]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	217	I[776]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2306.95	-4217.80	10397.59	8938.63	10397.59	0.628
Concio 5	217	I[776]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	217	J[777]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	245.14	-2286.49	10397.59	7895.61	10397.59	0.196
Concio 5	217	J[777]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	245.14	1768.76	13602.15	11701.94	13602.15	0.148
Concio 5	218	I[777]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.50	-2291.13	10397.59	7929.25	10397.59	0.200
Concio 5	218	I[777]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.50	1585.71	13602.15	11718.10	13602.15	0.132
Concio 5	218	J[778]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	921.63	-1898.55	10397.59	7219.13	10397.59	0.094
Concio 5	218	J[778]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	921.63	2361.89	13602.15	11376.83	13602.15	0.241
Concio 6	219	I[778]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	922.73	-1896.31	8891.06	5747.34	8891.06	0.110
Concio 6	219	I[778]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	922.73	2361.56	12388.93	10506.24	12388.93	0.265
Concio 6	219	J[779]	Neg	ST SLU Termico	FY-MIN	2	1	2	2	1460.50	-40.00	8891.06	5209.57	8891.06	0.160
Concio 6	219	J[779]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1460.50	3018.13	12388.93	10286.45	12388.93	0.362
Concio 6	220	I[779]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1074.88	-525.42	8891.06	5595.19	8891.06	0.062
Concio 6	220	I[779]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1451.08	2985.63	12388.93	10290.30	12388.93	0.358
Concio 6	220	J[780]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1192.63	-493.87	8891.06	5477.44	8891.06	0.079
Concio 6	220	J[780]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1610.05	3096.51	12388.93	10225.33	12388.93	0.380
Concio 6	221	I[780]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1192.63	-493.93	8891.06	5477.44	8891.06	0.079
Concio 6	221	I[780]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1610.05	3096.51	12388.93	10225.33	12388.93	0.380
Concio 6	221	J[781]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1074.86	-525.45	8891.06	5595.21	8891.06	0.062
Concio 6	221	J[781]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1451.06	2985.87	12388.93	10290.31	12388.93	0.358
Concio 6	222	I[781]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1081.84	-521.12	8891.06	5588.23	8891.06	0.063
Concio 6	222	I[781]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1460.48	3017.83	12388.93	10286.46	12388.93	0.361
Concio 6	222	J[782]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	922.69	-1896.26	8891.06	5747.38	8891.06	0.110
Concio 6	222	J[782]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	922.69	2360.99	12388.93	10506.26	12388.93	0.265
Concio 5	223	I[782]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	921.58	-1898.50	10397.59	7219.17	10397.59	0.094
Concio 5	223	I[782]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	921.58	2361.32	13602.15	11376.85	13602.15	0.241
Concio 5	223	J[783]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.44	-2291.05	10397.59	7929.32	10397.59	0.200
Concio 5	223	J[783]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.44	1586.29	13602.15	11718.13	13602.15	0.132
Concio 5	224	I[783]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	245.08	-2286.66	10397.59	7895.67	10397.59	0.196
Concio 5	224	I[783]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	245.08	1768.55	13602.15	11701.97	13602.15	0.148
Concio 5	224	J[784]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2307.05	-4218.30	10397.59	8938.61	10397.59	0.628
Concio 5	224	J[784]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	225	I[784]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2303.10	-4247.17	10397.59	8939.28	10397.59	0.630
Concio 5	225	I[784]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	225	J[785]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2614.43	-4527.20	10397.59	8705.59	10397.59	0.687
Concio 5	225	J[785]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	226	I[785]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2611.69	-4568.68	13534.54	10767.39	10767.39	0.667
Concio 4 H=var	226	I[785]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	226	J[786]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4370.93	-6091.84	21870.53	16963.22	16963.22	0.617
Concio 4 H=var	226	J[786]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	227	I[786]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4368.93	-6100.87	21870.53	16963.51	16963.51	0.617
Concio 4 H=var	227	I[786]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	227	J[787]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.99	-8154.46	31054.78	23216.21	23216.21	0.631
Concio 4 H=var	227	J[787]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	228	I[787]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.99	-8154.46	31054.78	23215.95	23215.95	0.631
Concio 4 H=200	228	I[787]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	228	J[788]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6952.70	-8743.74	31054.78	23172.17	23172.17	0.677
Concio 4 H=200	228	J[788]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	229	I[788]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6955.01	-8673.35	31054.78	23171.95	23171.95	0.674
Concio 4 H=200	229	I[788]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	229	J[789]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.81	-8168.20	31054.78	23218.45	23218.45	0.630
Concio 4 H=200	229	J[789]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	230	I[789]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.81	-8168.20	31054.78	23218.82	23218.82	0.630
Concio 4 H=var	230	I[789]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	230	J[790]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4200.77	-5962.43	21870.53	16988.38	16988.38	0.598
Concio 4 H=var	230	J[790]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	231	I[790]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4201.45	-5945.75	21870.53	16988.28	16988.28	0.597
Concio 4 H=var	231	I[790]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	231	J[791]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2303.98	-4226.41	13534.54	10825.90	10825.90	0.603
Concio 4 H=var	231	J[791]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	232	I[791]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2306.73	-4164.86	10397.59	8938.67	10397.59	0.622
Concio 3	232	I[791]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000

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Concio 3	232	J[792]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1967.78	-3861.30	10397.59	8995.72	10397.59	0.561
Concio 3	232	J[792]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	233	I[792]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1977.10	-3826.42	10397.59	8994.15	10397.59	0.558
Concio 3	233	I[792]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	233	J[793]	Neg	ST SLU Mobili	MY-MIN	3	1	1	3	770.14	-1405.88	10397.59	7370.61	7370.61	0.086
Concio 3	233	J[793]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	770.14	2538.74	13602.15	11449.63	13602.15	0.243
Concio 3	234	I[793]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	736.16	-1547.90	10397.59	7404.59	10397.59	0.078
Concio 3	234	I[793]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.16	2362.56	13602.15	11465.96	13602.15	0.228
Concio 3	234	J[794]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.09	-130.90	10397.59	7033.66	10397.59	0.094
Concio 3	234	J[794]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1494.58	3216.58	13602.15	11101.48	13602.15	0.346
Concio 2	235	I[794]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.69	-128.98	9113.78	5669.00	9113.78	0.107
Concio 2	235	I[794]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1495.38	3215.75	12846.08	10583.58	12846.08	0.367
Concio 2	235	J[795]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2276.07	-149.97	9113.78	4500.61	9113.78	0.233
Concio 2	235	J[795]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2276.07	4103.68	12846.08	10242.15	12846.08	0.497
Concio 2	236	I[795]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2263.84	-160.75	9113.78	4512.84	9113.78	0.231
Concio 2	236	I[795]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2263.84	4123.27	12846.08	10247.50	12846.08	0.497
Concio 2	236	J[796]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1956.12	-19.13	9113.78	4820.57	9113.78	0.213
Concio 2	236	J[796]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2640.76	4472.26	12846.08	10082.66	12846.08	0.554
Concio 2	237	I[796]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1952.60	-45.15	9113.78	4824.08	9113.78	0.209
Concio 2	237	I[796]	Pos	ST SLU Mobili	FZ-MAX	1	1	1	1	2636.01	4454.72	12846.08	10084.74	12846.08	0.552
Concio 2	237	J[797]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	237	J[797]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2468.06	4173.90	12846.08	10158.19	12846.08	0.517
Concio 1	238	I[797]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	238	I[797]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2459.09	4167.96	10954.92	8031.84	10954.92	0.605
Concio 1	238	J[798]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	238	J[798]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1855.04	3212.08	10954.92	8301.43	10954.92	0.463
Concio 1	239	I[798]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	239	I[798]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1882.94	3446.60	10954.92	8288.98	10954.92	0.486
Concio 1	239	J[799]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-41.87	-511.28	8093.91	6520.95	6520.95	0.085
Concio 1	239	J[799]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-41.87	235.11	10954.92	9148.03	10954.92	0.018
Concio 1	240	I[799]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-14.93	-339.49	8093.91	6494.02	6494.02	0.055
Concio 1	240	I[799]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	240	J[800]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	240	J[800]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	301	I[801]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	301	I[801]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	301	J[802]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-14.93	-339.51	8093.91	6494.02	6494.02	0.055
Concio 1	301	J[802]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	302	I[802]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-41.87	-510.22	8093.91	6520.95	6520.95	0.085
Concio 1	302	I[802]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-41.87	234.31	10954.92	9148.03	10954.92	0.018
Concio 1	302	J[803]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	302	J[803]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1882.97	3452.34	10954.92	8288.97	10954.92	0.487
Concio 1	303	I[803]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	303	I[803]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1855.06	3214.51	10954.92	8301.42	10954.92	0.463
Concio 1	303	J[804]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	303	J[804]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2459.13	4171.67	10954.92	8031.82	10954.92	0.605
Concio 2	304	I[804]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	304	I[804]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2468.10	4177.65	12846.08	10158.17	12846.08	0.517
Concio 2	304	J[805]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1952.64	-45.11	9113.78	4824.04	9113.78	0.209
Concio 2	304	J[805]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2636.06	4420.54	12846.08	10084.71	12846.08	0.549
Concio 2	305	I[805]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1956.15	-19.10	9113.78	4820.53	9113.78	0.213
Concio 2	305	I[805]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2640.81	4474.94	12846.08	10082.64	12846.08	0.554
Concio 2	305	J[806]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2263.92	-163.17	9113.78	4512.76	9113.78	0.231
Concio 2	305	J[806]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2263.92	4125.71	12846.08	10247.47	12846.08	0.497
Concio 2	306	I[806]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2276.14	-154.56	9113.78	4500.54	9113.78	0.233
Concio 2	306	I[806]	Pos	ST SLU Mobili	FZ-MAX	1	1	1	1	2276.14	4121.88	12846.08	10242.12	12846.08	0.498
Concio 2	306	J[807]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.75	-129.28	9113.78	5668.93	9113.78	0.107
Concio 2	306	J[807]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1495.47	3217.67	12846.08	10583.54	12846.08	0.367
Concio 3	307	I[807]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.16	-131.18	10397.59	7033.59	10397.59	0.094
Concio 3	307	I[807]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1494.67	3218.52	13602.15	11101.44	13602.15	0.347
Concio 3	307	J[808]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	736.26	-1548.67	10397.59	7404.49	10397.59	0.078
Concio 3	307	J[808]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.26	2364.22	13602.15	11465.91	13602.15	0.228
Concio 3	308	I[808]	Neg	ST SLU Vento	MY-MIN	3	1	1	3	770.24	-1388.91	10397.59	7370.51	7370.51	0.084
Concio 3	308	I[808]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	770.24	2541.45	13602.15	11449.58	13602.15	0.243
Concio 3	308	J[809]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1976.97	-3830.21	10397.59	8994.18	10397.59	0.559
Concio 3	308	J[809]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	309	I[809]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1967.65	-3864.27	10397.59	8995.75	10397.59	0.561
Concio 3	309	I[809]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	309	J[810]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2306.60	-4169.81	10397.59	8938.69	10397.59	0.623
Concio 3	309	J[810]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	310	I[810]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2303.85	-4227.44	13534.54	10825.92	10825.92	0.603
Concio 4 H=var	310	I[810]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	310	J[811]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4201.30	-5954.13	21870.53	16988.30	16988.30	0.598
Concio 4 H=var	310	J[811]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	311	I[811]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4200.62	-5968.78	21870.53	16988.40	16988.40	0.599
Concio 4 H=var	311	I[811]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	311	J[812]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.65	-8182.83	31054.78	23218.83	23218.83	0.631
Concio 4 H=var	311	J[812]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	312	I[812]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.65	-8182.83	31054.78	23218.46	23218.46	0.631
Concio 4 H=200	312	I[812]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000



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Concio 4 H=200	312	J[813]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6954.85	-8689.66	31054.78	23171.96	23171.96	0.675
Concio 4 H=200	312	J[813]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	313	I[813]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6952.54	-8760.11	31054.78	23172.18	23172.18	0.678
Concio 4 H=200	313	I[813]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	313	J[814]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.83	-8168.64	31054.78	23215.97	23215.97	0.632
Concio 4 H=200	313	J[814]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	314	I[814]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.83	-8168.64	31054.78	23216.22	23216.22	0.632
Concio 4 H=var	314	I[814]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	314	J[815]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4368.80	-6106.73	21870.53	16963.53	16963.53	0.618
Concio 4 H=var	314	J[815]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	315	I[815]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4370.81	-6099.76	21870.53	16963.24	16963.24	0.617
Concio 4 H=var	315	I[815]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	315	J[816]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2611.59	-4569.02	13534.54	10767.41	10767.41	0.667
Concio 4 H=var	315	J[816]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	316	I[816]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2614.32	-4532.17	10397.59	8705.68	10397.59	0.687
Concio 5	316	I[816]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	316	J[817]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2302.99	-4249.22	10397.59	8939.30	10397.59	0.630
Concio 5	316	J[817]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	317	I[817]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2306.95	-4222.06	10397.59	8938.63	10397.59	0.628
Concio 5	317	I[817]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	317	J[818]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	245.14	-2289.72	10397.59	7895.61	10397.59	0.197
Concio 5	317	J[818]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	245.14	1771.99	13602.15	11701.94	13602.15	0.148
Concio 5	318	I[818]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.50	-2293.01	10397.59	7929.25	10397.59	0.200
Concio 5	318	I[818]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.50	1587.61	13602.15	11718.10	13602.15	0.132
Concio 5	318	J[819]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	921.63	-1901.97	10397.59	7219.13	10397.59	0.094
Concio 5	318	J[819]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	921.63	2365.31	13602.15	11376.83	13602.15	0.242
Concio 6	319	I[819]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	922.73	-1899.66	8891.06	5747.34	8891.06	0.110
Concio 6	319	I[819]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	922.73	2364.91	12388.93	10506.24	12388.93	0.265
Concio 6	319	J[820]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1081.85	-521.08	8891.06	5588.22	8891.06	0.063
Concio 6	319	J[820]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1460.50	3023.42	12388.93	10286.45	12388.93	0.362
Concio 6	320	I[820]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1074.88	-525.39	8891.06	5595.19	8891.06	0.062
Concio 6	320	I[820]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1451.08	2989.13	12388.93	10290.30	12388.93	0.358
Concio 6	320	J[821]	Neg	ST SLU Termico	MZ-MIN	2	1	2	2	1610.05	-119.42	8891.06	5060.02	8891.06	0.168
Concio 6	320	J[821]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1610.05	3100.00	12388.93	10225.33	12388.93	0.380
Concio 6	321	I[821]	Neg	ST SLU Termico	MZ-MIN	2	1	2	2	1610.05	-119.42	8891.06	5060.02	8891.06	0.168
Concio 6	321	I[821]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1610.05	3100.00	12388.93	10225.33	12388.93	0.380
Concio 6	321	J[822]	Neg	ST SLV Vert	FX-MAX	2	1	2	2	1074.86	-525.42	8891.06	5595.21	8891.06	0.062
Concio 6	321	J[822]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1451.06	2989.35	12388.93	10290.31	12388.93	0.358
Concio 6	322	I[822]	Neg	ST SLU Termico	FY-MIN	2	1	2	2	1460.48	-44.85	8891.06	5209.59	8891.06	0.159
Concio 6	322	I[822]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1460.48	3023.18	12388.93	10286.46	12388.93	0.362
Concio 6	322	J[823]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	922.69	-1899.63	8891.06	5747.38	8891.06	0.110
Concio 6	322	J[823]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	922.69	2364.36	12388.93	10506.26	12388.93	0.265
Concio 5	323	I[823]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	921.58	-1901.94	10397.59	7219.17	10397.59	0.094
Concio 5	323	I[823]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	921.58	2364.76	13602.15	11376.85	13602.15	0.242
Concio 5	323	J[824]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.44	-2292.92	10397.59	7929.32	10397.59	0.200
Concio 5	323	J[824]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.44	1588.16	13602.15	11718.13	13602.15	0.132
Concio 5	324	I[824]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	245.08	-2289.66	10397.59	7895.67	10397.59	0.197
Concio 5	324	I[824]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	245.08	1771.55	13602.15	11701.97	13602.15	0.148
Concio 5	324	J[825]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2307.05	-4222.68	10397.59	8938.61	10397.59	0.628
Concio 5	324	J[825]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	325	I[825]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2303.10	-4249.72	10397.59	8939.28	10397.59	0.630
Concio 5	325	I[825]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	325	J[826]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2614.43	-4532.72	10397.59	8705.59	10397.59	0.687
Concio 5	325	J[826]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	326	I[826]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2611.69	-4569.58	13534.54	10767.39	10767.39	0.667
Concio 4 H=var	326	I[826]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	326	J[827]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4370.93	-6100.41	21870.53	16963.22	16963.22	0.617
Concio 4 H=var	326	J[827]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	327	I[827]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4368.93	-6107.56	21870.53	16963.51	16963.51	0.618
Concio 4 H=var	327	I[827]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	327	J[828]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.99	-8169.25	31054.78	23216.21	23216.21	0.632
Concio 4 H=var	327	J[828]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	328	I[828]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6492.99	-8169.25	31054.78	23215.95	23215.95	0.632
Concio 4 H=200	328	I[828]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	328	J[829]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6952.70	-8760.42	31054.78	23172.17	23172.17	0.678
Concio 4 H=200	328	J[829]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	329	I[829]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6955.01	-8690.07	31054.78	23171.95	23171.95	0.675
Concio 4 H=200	329	I[829]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	329	J[830]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.81	-8182.97	31054.78	23218.45	23218.45	0.631
Concio 4 H=200	329	J[830]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	330	I[830]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6466.81	-8182.97	31054.78	23218.82	23218.82	0.631
Concio 4 H=var	330	I[830]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	330	J[831]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4200.77	-5968.92	21870.53	16988.38	16988.38	0.599
Concio 4 H=var	330	J[831]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	331	I[831]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4201.45	-5954.13	21870.53	16988.28	16988.28	0.598
Concio 4 H=var	331	I[831]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	331	J[832]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2303.98	-4227.36	13534.54	10825.90	10825.90	0.603
Concio 4 H=var	331	J[832]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	332	I[832]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2306.73	-4169.81	10397.59	8938.67	10397.59	0.623
Concio 3	332	I[832]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000



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Concio 3	332	J[833]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1967.78	-3864.25	10397.59	8995.72	10397.59	0.561
Concio 3	332	J[833]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	333	I[833]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1977.10	-3829.87	10397.59	8994.15	10397.59	0.558
Concio 3	333	I[833]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	333	J[834]	Neg	ST SLU Vento	MY-MIN	3	1	1	3	770.14	-1389.17	10397.59	7370.61	7370.61	0.084
Concio 3	333	J[834]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	770.14	2541.69	13602.15	11449.63	13602.15	0.243
Concio 3	334	I[834]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	736.16	-1548.85	10397.59	7404.59	10397.59	0.078
Concio 3	334	I[834]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.16	2363.50	13602.15	11465.96	13602.15	0.228
Concio 3	334	J[835]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.09	-130.94	10397.59	7033.66	10397.59	0.094
Concio 3	334	J[835]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1494.58	3219.01	13602.15	11101.48	13602.15	0.347
Concio 2	335	I[835]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1107.69	-129.02	9113.78	5669.00	9113.78	0.107
Concio 2	335	I[835]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1495.38	3218.14	12846.08	10583.58	12846.08	0.367
Concio 2	335	J[836]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2276.07	-154.58	9113.78	4500.61	9113.78	0.233
Concio 2	335	J[836]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2276.07	4108.29	12846.08	10242.15	12846.08	0.497
Concio 2	336	I[836]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2263.84	-163.21	9113.78	4512.84	9113.78	0.230
Concio 2	336	I[836]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2263.84	4125.73	12846.08	10247.50	12846.08	0.497
Concio 2	336	J[837]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1956.12	-19.12	9113.78	4820.57	9113.78	0.213
Concio 2	336	J[837]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2640.76	4475.23	12846.08	10082.66	12846.08	0.554
Concio 2	337	I[837]	Neg	ST SLV Vert	FX-MIN	2	1	1	2	1952.60	-45.15	9113.78	4824.08	9113.78	0.209
Concio 2	337	I[837]	Pos	ST SLU Mobili	FZ-MAX	1	1	1	1	2636.01	4459.58	12846.08	10084.74	12846.08	0.552
Concio 2	337	J[838]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	337	J[838]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2468.06	4177.66	12846.08	10158.19	12846.08	0.517
Concio 1	338	I[838]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	338	I[838]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2459.09	4171.68	10954.92	8031.84	10954.92	0.605
Concio 1	338	J[839]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	338	J[839]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1855.04	3214.16	10954.92	8301.43	10954.92	0.463
Concio 1	339	I[839]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	339	I[839]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1882.94	3452.21	10954.92	8288.98	10954.92	0.487
Concio 1	339	J[840]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-41.87	-510.37	8093.91	6520.95	6520.95	0.085
Concio 1	339	J[840]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-41.87	234.20	10954.92	9148.03	10954.92	0.018
Concio 1	340	I[840]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-14.93	-339.49	8093.91	6494.02	6494.02	0.055
Concio 1	340	I[840]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	340	J[841]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.53	8093.91	6479.08	6479.08	0.000
Concio 1	340	J[841]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.53	10954.92	9129.34	10954.92	0.000
Concio 1	401	I[842]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.60	8011.39	6350.05	6350.05	0.000
Concio 1	401	I[842]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.60	10750.90	9086.69	10750.90	0.000
Concio 1	401	J[843]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-13.72	-343.84	8011.39	6363.77	6363.77	0.056
Concio 1	401	J[843]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	402	I[843]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-34.14	-864.76	8011.39	6384.18	6384.18	0.141
Concio 1	402	I[843]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-34.14	231.43	10750.90	9101.69	10750.90	0.018
Concio 1	402	J[844]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	402	J[844]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1889.00	3513.41	10750.90	8256.38	10750.90	0.503
Concio 1	403	I[844]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	403	I[844]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1867.51	3229.65	10750.90	8265.83	10750.90	0.474
Concio 1	403	J[845]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	403	J[845]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2464.89	4586.27	10750.90	8003.25	10750.90	0.656
Concio 2	404	I[845]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	404	I[845]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2471.60	4587.03	12635.70	10122.68	12635.70	0.559
Concio 2	404	J[846]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	404	J[846]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2654.01	4669.62	12635.70	10044.06	12635.70	0.580
Concio 2	405	I[846]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	405	I[846]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2655.98	4604.20	12635.70	10043.21	12635.70	0.575
Concio 2	405	J[847]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2261.86	-286.76	9000.58	4380.05	9000.58	0.219
Concio 2	405	J[847]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2261.86	4186.12	12635.70	10213.07	12635.70	0.510
Concio 2	406	I[847]	Neg	ST SLU Frenamento	MY-MIN	2	1	1	2	2274.09	-286.90	9000.58	4367.82	9000.58	0.221
Concio 2	406	I[847]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2274.09	4292.59	12635.70	10207.80	12635.70	0.520
Concio 2	406	J[848]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1087.20	-95.97	9000.58	5554.71	9000.58	0.110
Concio 2	406	J[848]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.72	3520.36	12635.70	10555.33	12635.70	0.395
Concio 3	407	I[848]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1086.79	-97.92	10249.33	6848.87	10249.33	0.096
Concio 3	407	I[848]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.16	3520.84	13386.70	11067.30	13386.70	0.373
Concio 3	407	J[849]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	710.53	-1606.43	10249.33	7225.13	10249.33	0.087
Concio 3	407	J[849]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	710.53	2171.63	13386.70	11425.43	13386.70	0.215
Concio 3	408	I[849]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	736.76	-1550.89	10249.33	7198.90	10249.33	0.079
Concio 3	408	I[849]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.76	2425.20	13386.70	11413.01	13386.70	0.236
Concio 3	408	J[850]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1970.02	-4219.45	10249.33	8951.27	10249.33	0.604
Concio 3	408	J[850]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	409	I[850]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1960.11	-4110.39	10249.33	8952.86	10249.33	0.592
Concio 3	409	I[850]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	409	J[851]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2286.73	-4458.69	10249.33	8775.36	10249.33	0.658
Concio 3	409	J[851]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	410	I[851]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2283.89	-4444.71	13376.13	10750.38	10750.38	0.626
Concio 4 H=var	410	I[851]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	410	J[852]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.85	-6440.43	21530.83	16899.82	16899.82	0.624
Concio 4 H=var	410	J[852]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	411	I[852]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.32	-6413.82	21530.83	16899.90	16899.90	0.622
Concio 4 H=var	411	I[852]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	411	J[853]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.80	-8752.34	30530.77	23173.54	23173.54	0.645
Concio 4 H=var	411	J[853]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	412	I[853]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.80	-8752.34	30530.77	23173.28	23173.28	0.645
Concio 4 H=200	412	I[853]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000

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Concio 4 H=200	412	J[854]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6646.06	-9383.65	30530.77	23132.45	23132.45	0.693
Concio 4 H=200	412	J[854]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	413	I[854]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6638.47	-9436.20	30530.77	23133.14	23133.14	0.695
Concio 4 H=200	413	I[854]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	413	J[855]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.38	-8815.26	30530.77	23171.51	23171.51	0.649
Concio 4 H=200	413	J[855]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	414	I[855]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.38	-8815.26	30530.77	23171.56	23171.56	0.649
Concio 4 H=var	414	I[855]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	414	J[856]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4258.59	-6507.79	21530.83	16878.64	16878.64	0.638
Concio 4 H=var	414	J[856]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	415	I[856]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4254.46	-6533.22	21530.83	16879.21	16879.21	0.639
Concio 4 H=var	415	I[856]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	415	J[857]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2570.83	-4775.47	13376.13	10698.99	10698.99	0.687
Concio 4 H=var	415	J[857]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	416	I[857]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2574.42	-4761.97	10249.33	8556.43	10249.33	0.716
Concio 5	416	I[857]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	416	J[858]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2276.35	-4468.52	10249.33	8783.25	10249.33	0.658
Concio 5	416	J[858]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	417	I[858]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2281.02	-4540.89	10249.33	8779.70	10249.33	0.666
Concio 5	417	I[858]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	417	J[859]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.26	-2329.74	10249.33	7724.40	10249.33	0.207
Concio 5	417	J[859]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.26	1665.84	13386.70	11661.74	13386.70	0.140
Concio 5	418	I[859]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	187.77	-2355.39	10249.33	7747.89	10249.33	0.211
Concio 5	418	I[859]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	187.77	1422.88	13386.70	11672.86	13386.70	0.120
Concio 5	418	J[860]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	888.92	-1929.02	10249.33	7046.74	10249.33	0.101
Concio 5	418	J[860]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	888.92	2723.78	13386.70	11340.99	13386.70	0.270
Concio 6	419	I[860]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	889.33	-1930.08	8775.51	5644.61	8775.51	0.119
Concio 6	419	I[860]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	889.33	2723.46	12181.58	10480.17	12181.58	0.297
Concio 6	419	J[861]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1444.28	-209.14	8775.51	5089.66	8775.51	0.141
Concio 6	419	J[861]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1444.28	3152.72	12181.58	10256.59	12181.58	0.377
Concio 6	420	I[861]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1436.50	-137.74	8775.51	5097.44	8775.51	0.148
Concio 6	420	I[861]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1436.50	3065.38	12181.58	10259.73	12181.58	0.370
Concio 6	420	J[862]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1582.69	-47.91	8775.51	4951.25	8775.51	0.175
Concio 6	420	J[862]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1582.69	3598.55	12181.58	10200.83	12181.58	0.425
Concio 6	421	I[862]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1582.69	-47.91	8775.51	4951.25	8775.51	0.175
Concio 6	421	I[862]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1582.69	3598.55	12181.58	10200.83	12181.58	0.425
Concio 6	421	J[863]	Neg	ST SLU Termico	FX-MAX	2	1	2	2	1436.48	-103.17	8775.51	5097.46	8775.51	0.152
Concio 6	421	J[863]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1436.48	3065.54	12181.58	10259.73	12181.58	0.370
Concio 6	422	I[863]	Neg	ST SLU Mobili	MX-MIN	2	1	2	2	1444.26	-31.72	8775.51	5089.68	8775.51	0.161
Concio 6	422	I[863]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1444.26	3152.38	12181.58	10256.60	12181.58	0.377
Concio 6	422	J[864]	Neg	ST SLU Termico	MY-MIN	1	2	2	2	889.28	-1929.99	8775.51	5644.65	8775.51	0.119
Concio 6	422	J[864]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	889.28	2722.73	12181.58	10480.19	12181.58	0.297
Concio 5	423	I[864]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	888.87	-1928.93	10249.33	7046.79	10249.33	0.101
Concio 5	423	I[864]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	888.87	2723.07	13386.70	11341.02	13386.70	0.270
Concio 5	423	J[865]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	187.70	-2355.23	10249.33	7747.96	10249.33	0.211
Concio 5	423	J[865]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	187.70	1423.12	13386.70	11672.89	13386.70	0.120
Concio 5	424	I[865]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	211.19	-2329.61	10249.33	7724.47	10249.33	0.207
Concio 5	424	I[865]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	211.19	1665.32	13386.70	11661.77	13386.70	0.140
Concio 5	424	J[866]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2281.12	-4541.81	10249.33	8779.62	10249.33	0.666
Concio 5	424	J[866]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	425	I[866]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2276.46	-4469.26	10249.33	8783.17	10249.33	0.658
Concio 5	425	I[866]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 5	425	J[867]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2574.53	-4762.66	10249.33	8556.34	10249.33	0.716
Concio 5	425	J[867]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	426	I[867]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2570.94	-4776.18	13376.13	10698.97	10698.97	0.687
Concio 4 H=var	426	I[867]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	426	J[868]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4254.59	-6534.10	21530.83	16879.19	16879.19	0.639
Concio 4 H=var	426	J[868]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	427	I[868]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4258.72	-6508.67	21530.83	16878.62	16878.62	0.638
Concio 4 H=var	427	I[868]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	427	J[869]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.52	-8816.10	30530.77	23171.55	23171.55	0.649
Concio 4 H=var	427	J[869]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	428	I[869]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6215.52	-8816.10	30530.77	23171.50	23171.50	0.649
Concio 4 H=200	428	I[869]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	428	J[870]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6638.61	-9436.80	30530.77	23133.13	23133.13	0.695
Concio 4 H=200	428	J[870]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	429	I[870]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6646.21	-9384.70	30530.77	23132.44	23132.44	0.693
Concio 4 H=200	429	I[870]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=200	429	J[871]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.95	-8753.23	30530.77	23173.27	23173.27	0.645
Concio 4 H=200	429	J[871]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	430	I[871]	Neg	ST SLU Mobili	MY-MIN	1	3	4	4	-6195.95	-8753.23	30530.77	23173.53	23173.53	0.645
Concio 4 H=var	430	I[871]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	430	J[872]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.45	-6414.14	21530.83	16899.88	16899.88	0.622
Concio 4 H=var	430	J[872]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	431	I[872]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-4105.99	-6440.79	21530.83	16899.80	16899.80	0.624
Concio 4 H=var	431	I[872]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 4 H=var	431	J[873]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-2284.02	-4444.82	13376.13	10750.36	10750.36	0.626
Concio 4 H=var	431	J[873]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	432	I[873]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-2286.85	-4458.71	10249.33	8775.26	10249.33	0.658
Concio 3	432	I[873]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000

Concio 3	432	J[874]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1960.24	-4110.29	10249.33	8952.84	10249.33	0.592
Concio 3	432	J[874]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	433	I[874]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	-1970.14	-4219.38	10249.33	8951.25	10249.33	0.604
Concio 3	433	I[874]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 3	433	J[875]	Neg	ST SLU Mobili	MY-MIN	1	1	1	1	736.66	-1551.09	10249.33	7199.00	10249.33	0.079
Concio 3	433	J[875]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	736.66	2425.47	13386.70	11413.06	13386.70	0.236
Concio 3	434	I[875]	Neg	ST SLU Termico	MY-MIN	1	1	1	1	710.42	-1606.72	10249.33	7225.24	10249.33	0.087
Concio 3	434	I[875]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	710.42	2171.82	13386.70	11425.48	13386.70	0.215
Concio 3	434	J[876]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1086.72	-97.79	10249.33	6848.94	10249.33	0.096
Concio 3	434	J[876]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.07	3521.45	13386.70	11067.35	13386.70	0.373
Concio 2	435	I[876]	Neg	ST SLV Vert	FX-MAX	2	1	1	2	1087.13	-95.84	9000.58	5554.78	9000.58	0.110
Concio 2	435	I[876]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1467.63	3520.97	12635.70	10555.37	12635.70	0.395
Concio 2	435	J[877]	Neg	ST SLU Frenamento	MY-MIN	2	1	1	2	2274.02	-287.18	9000.58	4367.90	9000.58	0.221
Concio 2	435	J[877]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2274.02	4293.48	12635.70	10207.83	12635.70	0.520
Concio 2	436	I[877]	Neg	ST SLU Mobili	MY-MIN	2	1	1	2	2261.79	-286.92	9000.58	4380.13	9000.58	0.219
Concio 2	436	I[877]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2261.79	4185.89	12635.70	10213.11	12635.70	0.510
Concio 2	436	J[878]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	436	J[878]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2655.93	4604.33	12635.70	10043.23	12635.70	0.575
Concio 2	437	I[878]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	437	I[878]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2653.95	4669.42	12635.70	10044.09	12635.70	0.580
Concio 2	437	J[879]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 2	437	J[879]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2471.56	4587.17	12635.70	10122.70	12635.70	0.559
Concio 1	438	I[879]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	438	I[879]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	2464.85	4586.11	10750.90	8003.27	10750.90	0.656
Concio 1	438	J[880]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	438	J[880]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1867.48	3229.59	10750.90	8265.84	10750.90	0.474
Concio 1	439	I[880]	Neg	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	439	I[880]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	1888.97	3513.38	10750.90	8256.40	10750.90	0.503
Concio 1	439	J[881]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-34.14	-864.67	8011.39	6384.18	6384.18	0.141
Concio 1	439	J[881]	Pos	ST SLU Mobili	MY-MAX	1	1	1	1	-34.14	231.47	10750.90	9101.69	10750.90	0.018
Concio 1	440	I[881]	Neg	ST SLU Mobili	MY-MIN	1	3	3	3	-13.72	-343.82	8011.39	6363.77	6363.77	0.056
Concio 1	440	I[881]	Pos	-	-	-	-	-	-	-	-	-	-	-	0.000
Concio 1	440	J[882]	Neg	ST SLV Long	FX-MIN	2	3	4	4	0.00	-0.60	8011.39	6350.05	6350.05	0.000
Concio 1	440	J[882]	Pos	ST SLV Long	FX-MAX	1	1	1	1	0.00	0.60	10750.90	9086.69	10750.90	0.000

## 8.2 VERIFICHE DI RESISTENZA AL TAGLIO VERTICALE ALLO SLU

Si effettuano le verifiche allo stato limite ultimo per taglio verticale delle sezioni composte travi-soletta.

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem Property:	nome delle caratteristiche geometriche dell'elemento
Elem:	numero dell'elemento
Position:	nodo iniziale (I) o finale (J) dell'elemento
Lcom:	combinazione di carico My positivo o negativo più gravosa
Type:	sollecitazione (massima o minima)
Top class:	classificazione flangia superiore
Bot class:	classificazione flangia inferiore
Web class:	classificazione anima
Sect. class:	classificazione complessiva sezione
N_Ed:	sforzo assiale di calcolo
M_Ed:	momento di calcolo
V_Ed:	sforzo tagliante di calcolo
Vpl,Rd:	resistenza al taglio della sezione di acciaio ( $\geq V_{Ed}$ )
Vb,Rd:	resistenza all'instabilità per taglio dell'anima di acciaio ( $\geq V_{Ed}$ )

La condizione di verifica della sezione è la seguente:

$$\text{Verification ratio: } \frac{V_{Ed}}{\min(V_{pl,Rd}, V_{b,Rd})} \leq 1$$

CAVALCAVIA ASSE 80 – IMPALCATO – RELAZIONE DI CALCOLO

Elem property	Elem number	Position	Lcom	Type	Top Class	Bot Class	Web Class	Sect. Class	N_Ed (kN)	M_Ed (kN*m)	V_Ed (kN)	Vpl,Rd (kN)	Vb,Rd (kN)	Verif. Ratio
Concio 1	101	I[719]	ST SLV Vert	FX-MAX	1	1	1	1	0.91	4.70	1.95	3302.77	2574.42	0.001
Concio 1	101	J[720]	ST SLU Mobili	FZ-MAX	1	3	3	3	0.00	364.81	488.88	3302.77	2574.10	0.190
Concio 1	102	I[720]	ST SLU Mobili	FZ-MIN	1	3	3	3	-176.76	545.44	-1851.81	3302.77	2573.69	0.720
Concio 1	102	J[721]	ST SLU Mobili	FZ-MIN	1	1	1	1	-112.46	104944.97	-888.31	3302.77	2478.97	0.358
Concio 1	103	I[721]	ST SLU Mobili	FZ-MIN	1	1	1	1	-168.15	105089.54	-1239.71	3302.77	2478.97	0.500
Concio 1	103	J[722]	ST SLU Mobili	FZ-MIN	1	1	1	1	-154.93	137952.75	-653.94	3302.77	2478.97	0.264
Concio 2	104	I[722]	ST SLU Mobili	FZ-MIN	1	1	1	1	-154.93	78315.91	-653.94	3267.64	2478.30	0.264
Concio 2	104	J[723]	ST SLU Mobili	FZ-MAX	1	1	1	1	48.06	82307.58	533.96	3267.64	2478.30	0.215
Concio 2	105	I[723]	ST SLU Mobili	FZ-MIN	1	1	1	1	-158.33	83443.88	-685.68	3267.64	2478.30	0.277
Concio 2	105	J[724]	ST SLU Mobili	FZ-MAX	1	1	1	1	46.70	70363.41	1047.83	3267.64	2478.30	0.423
Concio 2	106	I[724]	ST SLU Mobili	FZ-MAX	1	1	1	1	7.97	71281.84	672.79	3267.64	2478.30	0.271
Concio 2	106	J[725]	ST SLU Mobili	FZ-MAX	1	1	1	1	51.06	46711.40	1209.65	3267.64	2478.30	0.488
Concio 3	107	I[725]	ST SLU Mobili	FZ-MAX	1	1	1	1	51.06	39378.38	1209.65	4356.85	4158.81	0.291
Concio 3	107	J[726]	ST SLU Mobili	FZ-MAX	1	1	1	1	31.10	19590.25	1591.06	4356.85	4158.81	0.383
Concio 3	108	I[726]	ST SLU Mobili	FZ-MAX	1	1	1	1	8.41	20132.37	1222.11	4356.85	4158.81	0.294
Concio 3	108	J[727]	ST SLU Mobili	FZ-MAX	1	1	1	1	65.51	4955.86	2127.92	4356.85	4158.81	0.512
Concio 3	109	I[727]	ST SLU Mobili	FZ-MAX	1	1	1	1	80.54	4576.03	1793.51	4356.85	4158.81	0.431
Concio 3	109	J[728]	ST SLU Mobili	FZ-MAX	1	1	1	1	67.20	5504.77	1898.05	4356.85	4158.81	0.456
Concio 4 H=var	110	I[728]	ST SLU Mobili	FZ-MAX	1	3	3	3	-3.38	6217.04	1889.58	4310.00	4114.09	0.459
Concio 4 H=var	110	J[729]	ST SLU Mobili	FZ-MAX	1	3	3	3	86.93	11411.42	2393.14	6652.39	4857.52	0.493
Concio 4 H=var	111	I[729]	ST SLU Mobili	FZ-MAX	1	3	3	3	-3.77	11084.47	2140.77	6652.39	4868.83	0.440
Concio 4 H=var	111	J[730]	ST SLU Mobili	FZ-MAX	1	3	4	4	118.04	14680.02	2631.24	8994.79	5349.50	0.492
Concio 4 H=200	112	I[730]	ST SLU Mobili	FZ-MAX	1	3	4	4	233.81	14825.48	2630.64	8994.79	5345.38	0.492
Concio 4 H=200	112	J[731]	ST SLU Mobili	FZ-MAX	1	3	4	4	186.19	16016.71	2729.74	8994.79	5310.17	0.514
Concio 4 H=200	113	I[731]	ST SLU Mobili	FZ-MIN	1	3	4	4	8.51	17127.13	-2591.14	8994.79	5274.91	0.491
Concio 4 H=200	113	J[732]	ST SLU Mobili	FZ-MIN	1	3	4	4	44.44	15920.96	-2492.01	8994.79	5313.11	0.469
Concio 4 H=var	114	I[732]	ST SLU Mobili	FZ-MIN	1	3	4	4	-94.08	15680.40	-2495.34	8994.79	5317.85	0.469
Concio 4 H=var	114	J[733]	ST SLU Mobili	FZ-MIN	1	3	3	3	-215.31	12229.86	-2004.30	6652.39	4821.77	0.416
Concio 4 H=var	115	I[733]	ST SLU Mobili	FZ-MIN	1	3	3	3	-125.43	12676.45	-2251.32	6652.39	4806.74	0.468
Concio 4 H=var	115	J[734]	ST SLU Mobili	FZ-MIN	1	3	3	3	-219.20	7839.05	-1747.90	4310.00	4114.09	0.425
Concio 5	116	I[734]	ST SLU Mobili	FZ-MIN	1	1	1	1	-154.00	7028.10	-1752.57	4356.85	4158.81	0.421
Concio 5	116	J[735]	ST SLU Mobili	FZ-MIN	1	1	1	1	-130.82	6162.26	-1648.03	4356.85	4158.81	0.396
Concio 5	117	I[735]	ST SLU Mobili	FZ-MIN	1	1	1	1	-40.67	6596.51	-1983.83	4356.85	4158.81	0.477
Concio 5	117	J[736]	ST SLU Mobili	FZ-MIN	1	1	1	1	-106.02	5903.77	-1051.28	4356.85	4158.81	0.253
Concio 5	118	I[736]	ST SLU Mobili	FZ-MIN	1	1	1	1	-153.48	5628.67	-1431.92	4356.85	4158.81	0.344
Concio 5	118	J[737]	ST SLU Mobili	FZ-MIN	1	1	1	1	-135.09	24018.65	-999.49	4356.85	4158.81	0.240
Concio 6	119	I[737]	ST SLU Mobili	FZ-MIN	1	1	1	1	-135.09	32301.86	-999.49	2614.11	1592.23	0.628
Concio 6	119	J[738]	ST SLU Mobili	FZ-MIN	1	1	1	1	-181.35	51480.53	-489.09	2614.11	1592.23	0.307
Concio 6	120	I[738]	ST SLU Mobili	FZ-MIN	1	1	1	1	-177.42	51224.83	-871.63	2614.11	1592.23	0.547
Concio 6	120	J[739]	ST SLU Mobili	FZ-MIN	1	1	1	1	-157.97	56668.03	-380.56	2614.11	1592.23	0.239
Concio 6	121	I[739]	ST SLU Mobili	FZ-MIN	1	1	1	1	-157.97	56668.03	-380.56	2614.11	1592.23	0.239
Concio 6	121	J[740]	ST SLU Mobili	FZ-MAX	1	1	1	1	64.14	51073.72	871.45	2614.11	1592.23	0.547
Concio 6	122	I[740]	ST SLU Mobili	FZ-MAX	1	1	1	1	40.59	51392.00	489.05	2614.11	1592.23	0.307
Concio 6	122	J[741]	ST SLU Mobili	FZ-MAX	1	1	1	1	86.84	32183.33	999.36	2614.11	1592.23	0.628
Concio 5	123	I[741]	ST SLU Mobili	FZ-MAX	1	1	1	1	86.84	24079.37	999.36	4356.85	4158.81	0.240
Concio 5	123	J[742]	ST SLU Mobili	FZ-MAX	1	1	1	1	68.48	5666.04	1431.82	4356.85	4158.81	0.344
Concio 5	124	I[742]	ST SLU Mobili	FZ-MAX	1	1	1	1	62.26	6146.95	1051.25	4356.85	4158.81	0.253
Concio 5	124	J[743]	ST SLU Mobili	FZ-MAX	1	1	1	1	127.63	6041.76	1983.62	4356.85	4158.81	0.477
Concio 5	125	I[743]	ST SLU Mobili	FZ-MAX	1	1	1	1	74.71	5634.31	1647.91	4356.85	4158.81	0.396
Concio 5	125	J[744]	ST SLU Mobili	FZ-MAX	1	1	1	1	62.87	6480.81	1752.44	4356.85	4158.81	0.421
Concio 4 H=var	126	I[744]	ST SLU Mobili	FZ-MAX	1	3	3	3	13.70	7312.27	1747.76	4310.00	4114.09	0.425
Concio 4 H=var	126	J[745]	ST SLU Mobili	FZ-MAX	1	3	3	3	103.83	11997.65	2251.15	6652.39	4836.26	0.465
Concio 4 H=var	127	I[745]	ST SLU Mobili	FZ-MAX	1	3	3	3	-4.59	11636.35	2003.98	6652.39	4849.38	0.413
Concio 4 H=var	127	J[746]	ST SLU Mobili	FZ-MAX	1	3	4	4	122.95	14795.11	2494.95	8994.79	5346.25	0.467
Concio 4 H=200	128	I[746]	ST SLU Mobili	FZ-MAX	1	3	4	4	229.59	14927.83	2491.61	8994.79	5342.47	0.466
Concio 4 H=200	128	J[747]	ST SLU Mobili	FZ-MAX	1	3	4	4	185.47	16046.23	2590.74	8994.79	5309.27	0.488
Concio 4 H=200	129	I[747]	ST SLU Mobili	FZ-MIN	1	3	4	4	-2.95	17100.23	-2730.30	8994.79	5275.69	0.518
Concio 4 H=200	129	J[748]	ST SLU Mobili	FZ-MIN	1	3	4	4	33.30	15817.08	-2631.20	8994.79	5316.26	0.495
Concio 4 H=var	130	I[748]	ST SLU Mobili	FZ-MIN	1	3	4	4	-110.34	15551.10	-2631.76	8994.79	5321.32	0.495
Concio 4 H=var	130	J[749]	ST SLU Mobili	FZ-MIN	1	3	3	3	-228.73	11645.06	-2141.23	6652.39	4843.60	0.442
Concio 4 H=var	131	I[749]	ST SLU Mobili	FZ-MIN	1	3	3	3	-137.74	12057.33	-2393.41	6652.39	4830.47	0.495
Concio 4 H=var	131	J[750]	ST SLU Mobili	FZ-MIN	1	3	3	3	-235.31	6636.02	-1889.82	4310.00	4114.09	0.459
Concio 3	132	I[750]	ST SLU Mobili	FZ-MIN	1	1	1	1	-140.19	5983.16	-1898.35	4356.85	4158.81	0.456
Concio 3	132	J[751]	ST SLU Mobili	FZ-MIN	1	1	1	1	-118.87	5009.30	-1793.79	4356.85	4158.81	0.431
Concio 3	133	I[751]	ST SLU Mobili	FZ-MIN	1	1	1	1	-82.78	5443.60	-2128.28	4356.85	4158.81	0.512
Concio 3	133	J[752]	ST SLU Mobili	FZ-MIN	1	1	1	1	-139.87	20043.82	-1222.28	4356.85	4158.81	0.294
Concio 3	134	I[752]	ST SLU Mobili	FZ-MIN	1	1	1	1	-128.19	19586.18	-1591.32	4356.85	4158.81	0.383
Concio 3	134	J[753]	ST SLU Mobili	FZ-MIN	1	1	1	1	-108.20	39387.24	-1209.96	4356.85	4158.81	0.291
Concio 2	135	I[753]	ST SLU Mobili	FZ-MIN	1	1	1	1	-108.20	46789.61	-1209.96	3267.64	2478.30	0.488
Concio 2	135	J[754]	ST SLU Mobili	FZ-MIN	1	1	1	1	-151.28	71379.14	-673.02	3267.64	2478.30	0.272
Concio 2	136	I[754]	ST SLU Mobili	FZ-MIN	1	1	1	1	-133.47	70534.90	-1048.23	3267.64	2478.30	0.423
Concio 2	136	J[755]	ST SLU Mobili	FZ-MAX	1	1	1	1	21.83	83189.79	685.24	3267.64	2478.30	0.276
Concio 2	137	I[755]	ST SLU Mobili	FZ-MIN	1	1	1	1	-120.88	82491.16	-534.26	3267.64	2478.30	0.216
Concio 2	137	J[756]	ST SLU Mobili	FZ-MAX	1	1	1	1	14.00	78078.87	653.66	3267.64	2478.30	0.264



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Concio 1	138	I[756]	ST SLU Mobili	FZ-MAX	1	1	1	1	14.00	137299.45	653.66	3302.77	2478.97	0.264
Concio 1	138	J[757]	ST SLU Mobili	FZ-MAX	1	1	1	1	0.82	104361.04	1239.37	3302.77	2478.97	0.500
Concio 1	139	I[757]	ST SLU Mobili	FZ-MAX	1	1	1	1	4.47	104588.72	887.94	3302.77	2478.97	0.358
Concio 1	139	J[758]	ST SLU Mobili	FZ-MAX	1	3	3	3	-59.75	490.02	1851.39	3302.77	2573.84	0.719
Concio 1	140	I[758]	ST SLU Mobili	MY-MIN	1	3	3	3	0.00	364.79	-488.88	3302.77	2574.10	0.190
Concio 1	140	J[759]	ST SLV Vert	FX-MAX	1	1	1	1	0.93	4.81	1.95	3302.77	2574.42	0.001
Concio 1	201	I[760]	ST SLU Vert	FX-MAX	1	1	1	1	0.83	6.99	1.66	3302.77	2574.42	0.001
Concio 1	201	J[761]	ST SLU Mobili	FZ-MAX	1	3	3	3	0.00	362.88	481.10	3302.77	2574.11	0.187
Concio 1	202	I[761]	ST SLU Mobili	FZ-MIN	1	3	4	4	-209.01	432.40	-1809.45	3302.77	2573.96	0.703
Concio 1	202	J[762]	ST SLU Mobili	FZ-MIN	1	1	1	1	-212.98	175385.90	-910.77	3302.77	2478.97	0.367
Concio 1	203	I[762]	ST SLU Mobili	FZ-MIN	1	1	1	1	-270.02	175882.34	-1180.90	3302.77	2478.97	0.476
Concio 1	203	J[763]	ST SLU Mobili	FZ-MIN	1	1	1	1	-239.61	230515.39	-688.84	3302.77	2478.97	0.278
Concio 2	204	I[763]	ST SLU Mobili	FZ-MIN	1	1	1	1	-239.61	100137.83	-688.84	3267.64	2478.30	0.278
Concio 2	204	J[764]	ST SLU Mobili	FZ-MAX	1	1	1	1	-95.80	105306.01	587.12	3267.64	2478.30	0.237
Concio 2	205	I[764]	ST SLU Mobili	FZ-MIN	1	1	1	1	-241.62	106026.67	-708.83	3267.64	2478.30	0.286
Concio 2	205	J[765]	ST SLU Mobili	FZ-MAX	1	1	1	1	-138.28	91358.72	1041.51	3267.64	2478.30	0.420
Concio 2	206	I[765]	ST SLU Mobili	FZ-MAX	1	1	1	1	-171.61	92061.27	768.29	3267.64	2478.30	0.310
Concio 2	206	J[766]	ST SLU Mobili	FZ-MAX	1	1	1	1	-37.40	61040.57	1196.25	3267.64	2478.30	0.483
Concio 3	207	I[766]	ST SLU Mobili	FZ-MAX	1	1	1	1	-37.40	49583.71	1196.25	4356.85	4158.81	0.288
Concio 3	207	J[767]	ST SLU Mobili	FZ-MAX	1	1	1	1	-83.84	25233.43	1519.24	4356.85	4158.81	0.365
Concio 3	208	I[767]	ST SLU Mobili	FZ-MAX	1	1	1	1	-31.00	25819.16	1253.83	4356.85	4158.81	0.301
Concio 3	208	J[768]	ST SLU Mobili	FZ-MAX	1	1	1	1	49.66	4777.25	2051.43	4356.85	4158.81	0.493
Concio 3	209	I[768]	ST SLU Mobili	FZ-MAX	1	1	1	1	-13.91	4216.90	1844.09	4356.85	4158.81	0.443
Concio 3	209	J[769]	ST SLU Mobili	FZ-MAX	1	1	1	1	26.88	5262.92	1921.44	4356.85	4158.81	0.462
Concio 4 H=var	210	I[769]	ST SLU Mobili	FZ-MAX	1	3	3	3	-16.68	6153.80	1911.52	4310.00	4114.09	0.465
Concio 4 H=var	210	J[770]	ST SLU Mobili	FZ-MAX	1	3	3	3	169.23	11616.81	2351.29	6652.39	4853.99	0.484
Concio 4 H=var	211	I[770]	ST SLU Mobili	FZ-MAX	1	3	3	3	91.83	11246.29	2256.18	6652.39	4866.87	0.464
Concio 4 H=var	211	J[771]	ST SLU Mobili	FZ-MAX	1	3	4	4	189.44	14990.67	2707.25	8994.79	5343.65	0.507
Concio 4 H=200	212	I[771]	ST SLU Mobili	FZ-MAX	1	3	3	3	309.05	17599.64	2701.29	8994.79	5263.30	0.513
Concio 4 H=200	212	J[772]	ST SLU Mobili	FZ-MAX	1	3	4	4	285.70	16425.70	2788.41	8994.79	5301.05	0.526
Concio 4 H=200	213	I[772]	ST SLU Mobili	FZ-MIN	1	3	4	4	77.71	17335.81	-2644.30	8994.79	5272.01	0.502
Concio 4 H=200	213	J[773]	ST SLU Mobili	FZ-MIN	1	3	4	4	90.94	16055.40	-2557.22	8994.79	5312.42	0.481
Concio 4 H=var	214	I[773]	ST SLU Mobili	FZ-MIN	1	3	4	4	-49.43	15854.32	-2563.54	8994.79	5317.14	0.482
Concio 4 H=var	214	J[774]	ST SLU Mobili	FZ-MIN	1	3	3	3	-143.40	12356.55	-2112.61	6652.39	4823.18	0.438
Concio 4 H=var	215	I[774]	ST SLU Mobili	FZ-MIN	1	3	3	3	-6.22	12797.51	-2211.34	6652.39	4809.98	0.460
Concio 4 H=var	215	J[775]	ST SLU Mobili	FZ-MIN	1	3	3	3	-196.96	7828.08	-1772.55	4310.00	4114.09	0.431
Concio 5	216	I[775]	ST SLU Mobili	FZ-MIN	1	1	1	1	-141.54	6802.73	-1782.46	4356.85	4158.81	0.429
Concio 5	216	J[776]	ST SLU Mobili	FZ-MIN	1	1	1	1	-172.29	5841.65	-1705.20	4356.85	4158.81	0.410
Concio 5	217	I[776]	ST SLU Mobili	FZ-MIN	1	1	1	1	11.03	6471.43	-1920.04	4356.85	4158.81	0.462
Concio 5	217	J[777]	ST SLU Mobili	FZ-MIN	1	1	1	1	-41.09	8337.48	-1131.80	4356.85	4158.81	0.272
Concio 5	218	I[777]	ST SLU Mobili	FZ-MIN	1	1	1	1	-66.69	7984.75	-1371.77	4356.85	4158.81	0.330
Concio 5	218	J[778]	ST SLU Mobili	FZ-MIN	1	1	1	1	-22.91	30558.75	-1001.01	4356.85	4158.81	0.241
Concio 6	219	I[778]	ST SLU Mobili	FZ-MIN	1	1	1	1	-22.91	44142.09	-1001.01	2614.11	1592.23	0.629
Concio 6	219	J[779]	ST SLU Mobili	FZ-MIN	1	1	1	1	-161.81	69382.37	-597.57	2614.11	1592.23	0.375
Concio 6	220	I[779]	ST SLU Mobili	FZ-MIN	1	1	1	1	-128.82	68606.78	-882.68	2614.11	1592.23	0.554
Concio 6	220	J[780]	ST SLU Mobili	FZ-MIN	1	1	1	1	-83.75	76114.53	-464.45	2614.11	1592.23	0.292
Concio 6	221	I[780]	ST SLU Mobili	FZ-MIN	1	1	1	1	-83.75	76114.53	-464.45	2614.11	1592.23	0.292
Concio 6	221	J[781]	ST SLU Mobili	FZ-MAX	1	1	1	1	-41.34	68896.90	882.52	2614.11	1592.23	0.554
Concio 6	222	I[781]	ST SLU Mobili	FZ-MAX	1	1	1	1	-75.44	69599.66	597.54	2614.11	1592.23	0.375
Concio 6	222	J[782]	ST SLU Mobili	FZ-MAX	1	1	1	1	63.45	44345.19	1000.84	2614.11	1592.23	0.629
Concio 5	223	I[782]	ST SLU Mobili	FZ-MAX	1	1	1	1	63.45	30867.76	1000.84	4356.85	4158.81	0.241
Concio 5	223	J[783]	ST SLU Mobili	FZ-MAX	1	1	1	1	19.75	8282.80	1371.63	4356.85	4158.81	0.330
Concio 5	224	I[783]	ST SLU Mobili	FZ-MAX	1	1	1	1	67.23	8625.16	1131.89	4356.85	4158.81	0.272
Concio 5	224	J[784]	ST SLU Mobili	FZ-MAX	1	1	1	1	119.40	5902.19	1919.84	4356.85	4158.81	0.462
Concio 5	225	I[784]	ST SLU Mobili	FZ-MAX	1	1	1	1	-4.84	5323.78	1704.98	4356.85	4158.81	0.410
Concio 5	225	J[785]	ST SLU Mobili	FZ-MAX	1	1	1	1	36.66	6263.05	1782.23	4356.85	4158.81	0.429
Concio 4 H=var	226	I[785]	ST SLU Mobili	FZ-MAX	1	3	3	3	-0.44	7314.71	1772.34	4310.00	4114.09	0.431
Concio 4 H=var	226	J[786]	ST SLU Mobili	FZ-MAX	1	3	3	3	185.88	12219.33	2211.19	6652.39	4832.15	0.458
Concio 4 H=var	227	I[786]	ST SLU Mobili	FZ-MAX	1	3	3	3	105.22	11829.10	2112.35	6652.39	4846.42	0.436
Concio 4 H=var	227	J[787]	ST SLU Mobili	FZ-MAX	1	3	4	4	205.71	15103.96	2563.26	8994.79	5340.43	0.480
Concio 4 H=200	228	I[787]	ST SLU Mobili	FZ-MAX	1	3	3	3	316.93	17718.63	2556.92	8994.79	5259.33	0.486
Concio 4 H=200	228	J[788]	ST SLU Mobili	FZ-MAX	1	3	4	4	295.11	16457.19	2644.01	8994.79	5300.07	0.499
Concio 4 H=200	229	I[788]	ST SLU Mobili	FZ-MIN	1	3	4	4	82.89	17294.85	-2788.77	8994.79	5273.35	0.529
Concio 4 H=200	229	J[789]	ST SLU Mobili	FZ-MIN	1	3	4	4	96.32	15929.54	-2701.65	8994.79	5316.22	0.508
Concio 4 H=var	230	I[789]	ST SLU Mobili	FZ-MIN	1	3	4	4	-51.46	15721.34	-2707.58	8994.79	5321.07	0.509
Concio 4 H=var	230	J[790]	ST SLU Mobili	FZ-MIN	1	3	3	3	-143.77	11710.53	-2256.50	6652.39	4847.21	0.466
Concio 4 H=var	231	I[790]	ST SLU Mobili	FZ-MIN	1	3	3	3	-3.17	12133.13	-2351.50	6652.39	4835.26	0.486
Concio 4 H=var	231	J[791]	ST SLU Mobili	FZ-MIN	1	3	3	3	-195.03	6596.74	-1911.79	4310.00	4114.09	0.465
Concio 3	232	I[791]	ST SLU Mobili	FZ-MIN	1	1	1	1	-131.68	5687.76	-1921.72	4356.85	4158.81	0.462
Concio 3	232	J[792]	ST SLU Mobili	FZ-MIN	1	1	1	1	-163.15	4651.69	-1844.36	4356.85	4158.81	0.443
Concio 3	233	I[792]	ST SLU Mobili	FZ-MIN	1	1	1	1	-34.68	5229.37	-2051.69	4356.85	4158.81	0.493
Concio 3	233	J[793]	ST SLU Mobili	FZ-MIN	1	1	1	1	-115.27	25616.46	-1253.97	4356.85	4158.81	0.302
Concio 3	234	I[793]	ST SLU Mobili	FZ-MIN	1	1	1	1	-155.12	25333.36	-1519.48	4356.85	4158.81	0.365
Concio 3	234	J[794]	ST SLU Mobili	FZ-MIN	1	1	1	1	-108.62	49416.43	-1196.53	4356.85	4158.81	0.288
Concio 2	235	I[794]	ST SLU Mobili	FZ-MIN	1	1	1	1	-108.62	60918.18	-1196.53	3267.64	2478.30	0.483
Concio 2	235	J[795]	ST SLU Mobili	FZ-MIN	1	1	1	1	-242.83	91973.20	-768.46	3267.64	2478.30	0.310
Concio 2	236	I[795]	ST SLU Mobili	FZ-MIN	1	1	1	1	-229.05	91351.04	-1041.81	3267.64	2478.30	0.420
Concio 2	236	J[796]	ST SLU Mobili	FZ-MAX	1	1	1	1	-150.89	105950.25	708.53	3267.64	2478.30	0.286
Concio 2	237	I[796]	ST SLU Mobili	FZ-MIN	1	1	1	1	-205.86	105420.07	-587.35	3267.64	2478.30	0.237
Concio 2	237	J[797]	ST SLU Mobili	FZ-MAX	1	1	1	1	-129.66	99976.76	688.63	3267.64	2478.30	0.278

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Concio 1	238	I[797]	ST SLU Mobili	FZ-MAX	1	1	1	1	-129.66	229800.28	688.63	3302.77	2478.97	0.278
Concio 1	238	J[798]	ST SLU Mobili	FZ-MAX	1	1	1	1	-159.98	175101.61	1180.65	3302.77	2478.97	0.476
Concio 1	239	I[798]	ST SLU Mobili	FZ-MAX	1	1	1	1	-90.45	174583.44	910.56	3302.77	2478.97	0.367
Concio 1	239	J[799]	ST SLU Mobili	FZ-MAX	1	3	3	3	-86.32	312.69	1809.07	3302.77	2574.18	0.703
Concio 1	240	I[799]	ST SLU Mobili	MY-MIN	1	3	3	3	0.00	362.86	-481.10	3302.77	2574.11	0.187
Concio 1	240	J[800]	ST SLV Vert	FX-MAX	1	1	1	1	0.86	7.17	1.66	3302.77	2574.42	0.001
Concio 1	301	I[801]	ST SLU Vert	FX-MAX	1	1	1	1	0.83	6.98	1.66	3302.77	2574.42	0.001
Concio 1	301	J[802]	ST SLU Mobili	FZ-MAX	1	3	3	3	0.00	362.88	481.10	3302.77	2574.11	0.187
Concio 1	302	I[802]	ST SLU Mobili	FZ-MIN	3	3	4	4	-206.44	430.42	-1810.38	3302.77	2573.96	0.703
Concio 1	302	J[803]	ST SLU Mobili	FZ-MIN	1	1	1	1	-210.40	175357.95	-911.70	3302.77	2478.97	0.368
Concio 1	303	I[803]	ST SLU Mobili	FZ-MIN	1	1	1	1	-268.74	175869.16	-1181.45	3302.77	2478.97	0.477
Concio 1	303	J[804]	ST SLU Mobili	FZ-MIN	1	1	1	1	-238.33	230500.57	-689.39	3302.77	2478.97	0.278
Concio 2	304	I[804]	ST SLU Mobili	FZ-MIN	1	1	1	1	-238.33	100129.41	-689.39	3267.64	2478.30	0.278
Concio 2	304	J[805]	ST SLU Mobili	FZ-MAX	1	1	1	1	-97.08	105315.52	587.67	3267.64	2478.30	0.237
Concio 2	305	I[805]	ST SLU Mobili	FZ-MIN	1	1	1	1	-239.94	106017.54	-708.91	3267.64	2478.30	0.286
Concio 2	305	J[806]	ST SLU Mobili	FZ-MAX	1	1	1	1	-139.97	91367.33	1041.62	3267.64	2478.30	0.420
Concio 2	306	I[806]	ST SLU Mobili	FZ-MAX	1	1	1	1	-175.94	92081.68	769.05	3267.64	2478.30	0.310
Concio 2	306	J[807]	ST SLU Mobili	FZ-MAX	1	1	1	1	-41.73	61058.71	1197.01	3267.64	2478.30	0.483
Concio 3	307	I[807]	ST SLU Mobili	FZ-MAX	1	1	1	1	-41.73	49599.08	1197.01	4356.85	4158.81	0.288
Concio 3	307	J[808]	ST SLU Mobili	FZ-MAX	1	1	1	1	-88.17	25537.29	1520.00	4356.85	4158.81	0.365
Concio 3	308	I[808]	ST SLU Mobili	FZ-MAX	1	1	1	1	-40.72	25850.90	1255.01	4356.85	4158.81	0.302
Concio 3	308	J[809]	ST SLU Mobili	FZ-MAX	1	1	1	1	39.93	4772.01	2052.61	4356.85	4158.81	0.494
Concio 3	309	I[809]	ST SLU Mobili	FZ-MAX	1	1	1	1	-19.24	4212.90	1845.48	4356.85	4158.81	0.444
Concio 3	309	J[810]	ST SLU Mobili	FZ-MAX	1	1	1	1	21.55	5256.92	1922.83	4356.85	4158.81	0.462
Concio 4 H=var	310	I[810]	ST SLU Mobili	FZ-MAX	1	3	3	3	-21.69	6151.16	1913.87	4310.00	4114.09	0.465
Concio 4 H=var	310	J[811]	ST SLU Mobili	FZ-MAX	1	3	3	3	164.22	11606.19	2353.64	6652.39	4854.36	0.485
Concio 4 H=var	311	I[811]	ST SLU Mobili	FZ-MAX	1	3	3	3	87.74	11237.84	2259.11	6652.39	4867.16	0.464
Concio 4 H=var	311	J[812]	ST SLU Mobili	FZ-MAX	1	3	4	4	185.35	14974.30	2710.18	8994.79	5344.12	0.507
Concio 4 H=200	312	I[812]	ST SLU Mobili	FZ-MAX	1	3	3	3	304.40	17582.58	2703.17	8994.79	5263.87	0.514
Concio 4 H=200	312	J[813]	ST SLU Mobili	FZ-MAX	1	3	4	4	281.05	16407.26	2790.29	8994.79	5301.62	0.526
Concio 4 H=200	313	I[813]	ST SLU Mobili	FZ-MIN	1	3	4	4	83.00	17354.63	-2646.17	8994.79	5271.40	0.502
Concio 4 H=200	313	J[814]	ST SLU Mobili	FZ-MIN	1	3	4	4	96.22	16072.23	-2559.09	8994.79	5311.91	0.482
Concio 4 H=var	314	I[814]	ST SLU Mobili	FZ-MIN	1	3	4	4	-44.71	15875.87	-2566.52	8994.79	5316.62	0.483
Concio 4 H=var	314	J[815]	ST SLU Mobili	FZ-MIN	1	3	3	3	-138.68	12364.72	-2115.59	6652.39	4823.00	0.439
Concio 4 H=var	315	I[815]	ST SLU Mobili	FZ-MIN	1	3	3	3	0.32	12808.36	-2213.83	6652.39	4809.73	0.460
Concio 4 H=var	315	J[816]	ST SLU Mobili	FZ-MIN	1	3	3	3	-190.42	7830.63	-1775.04	4310.00	4114.09	0.431
Concio 5	316	I[816]	ST SLU Mobili	FZ-MIN	1	1	1	1	-134.66	6809.40	-1783.85	4356.85	4158.81	0.429
Concio 5	316	J[817]	ST SLU Mobili	FZ-MIN	1	1	1	1	-165.41	5845.33	-1706.59	4356.85	4158.81	0.410
Concio 5	317	I[817]	ST SLU Mobili	FZ-MIN	1	1	1	1	21.24	6477.51	-1921.24	4356.85	4158.81	0.462
Concio 5	317	J[818]	ST SLU Mobili	FZ-MIN	1	1	1	1	-30.88	8303.77	-1133.00	4356.85	4158.81	0.272
Concio 5	318	I[818]	ST SLU Mobili	FZ-MIN	1	1	1	1	-61.84	7968.40	-1372.44	4356.85	4158.81	0.330
Concio 5	318	J[819]	ST SLU Mobili	FZ-MIN	1	1	1	1	-18.07	30540.86	-1001.67	4356.85	4158.81	0.241
Concio 6	319	I[819]	ST SLU Mobili	FZ-MIN	1	1	1	1	-18.07	44118.32	-1001.67	2614.11	1592.23	0.629
Concio 6	319	J[820]	ST SLU Mobili	FZ-MIN	1	1	1	1	-156.96	69356.67	-598.23	2614.11	1592.23	0.376
Concio 6	320	I[820]	ST SLU Mobili	FZ-MIN	1	1	1	1	-125.05	68587.39	-882.69	2614.11	1592.23	0.554
Concio 6	320	J[821]	ST SLU Mobili	FZ-MIN	1	1	1	1	-79.97	76095.14	-464.45	2614.11	1592.23	0.292
Concio 6	321	I[821]	ST SLU Mobili	FZ-MIN	1	1	1	1	-79.97	76095.14	-464.45	2614.11	1592.23	0.292
Concio 6	321	J[822]	ST SLU Mobili	FZ-MAX	1	1	1	1	-45.11	68916.28	882.52	2614.11	1592.23	0.554
Concio 6	322	I[822]	ST SLU Mobili	FZ-MAX	1	1	1	1	-80.20	69625.05	598.22	2614.11	1592.23	0.376
Concio 6	322	J[823]	ST SLU Mobili	FZ-MAX	1	1	1	1	58.69	44368.61	1001.52	2614.11	1592.23	0.629
Concio 5	323	I[823]	ST SLU Mobili	FZ-MAX	1	1	1	1	58.69	30885.40	1001.52	4356.85	4158.81	0.241
Concio 5	323	J[824]	ST SLU Mobili	FZ-MAX	1	1	1	1	14.99	8298.87	1372.31	4356.85	4158.81	0.330
Concio 5	324	I[824]	ST SLU Mobili	FZ-MAX	1	1	1	1	57.34	8657.65	1133.07	4356.85	4158.81	0.272
Concio 5	324	J[825]	ST SLU Mobili	FZ-MAX	1	1	1	1	109.51	5896.05	1921.02	4356.85	4158.81	0.462
Concio 5	325	I[825]	ST SLU Mobili	FZ-MAX	1	1	1	1	-11.61	5320.03	1706.35	4356.85	4158.81	0.410
Concio 5	325	J[826]	ST SLU Mobili	FZ-MAX	1	1	1	1	29.89	6256.32	1783.61	4356.85	4158.81	0.429
Concio 4 H=var	326	I[826]	ST SLU Mobili	FZ-MAX	1	3	3	3	-6.88	7311.94	1774.81	4310.00	4114.09	0.431
Concio 4 H=var	326	J[827]	ST SLU Mobili	FZ-MAX	1	3	3	3	179.44	12208.24	2213.66	6652.39	4832.56	0.458
Concio 4 H=var	327	I[827]	ST SLU Mobili	FZ-MAX	1	3	3	3	100.65	11820.62	2115.40	6652.39	4846.72	0.436
Concio 4 H=var	327	J[828]	ST SLU Mobili	FZ-MAX	1	3	4	4	201.14	15087.41	2566.32	8994.79	5340.90	0.481
Concio 4 H=200	328	I[828]	ST SLU Mobili	FZ-MAX	1	3	3	3	311.79	17701.32	2558.88	8994.79	5259.91	0.486
Concio 4 H=200	328	J[829]	ST SLU Mobili	FZ-MAX	1	3	4	4	289.97	16438.52	2645.96	8994.79	5300.65	0.499
Concio 4 H=200	329	I[829]	ST SLU Mobili	FZ-MIN	1	3	4	4	87.67	17313.42	-2790.58	8994.79	5272.75	0.529
Concio 4 H=200	329	J[830]	ST SLU Mobili	FZ-MIN	1	3	4	4	101.10	15946.16	-2703.47	8994.79	5315.72	0.509
Concio 4 H=var	330	I[830]	ST SLU Mobili	FZ-MIN	1	3	4	4	-47.23	15742.17	-2710.46	8994.79	5320.57	0.509
Concio 4 H=var	330	J[831]	ST SLU Mobili	FZ-MIN	1	3	3	3	-139.54	11718.67	-2259.38	6652.39	4847.02	0.466
Concio 4 H=var	331	I[831]	ST SLU Mobili	FZ-MIN	1	3	3	3	1.93	12143.49	-2353.89	6652.39	4834.96	0.487
Concio 4 H=var	331	J[832]	ST SLU Mobili	FZ-MIN	1	3	3	3	-189.93	6599.17	-1914.18	4310.00	4114.09	0.465
Concio 3	332	I[832]	ST SLU Mobili	FZ-MIN	1	1	1	1	-126.25	5693.68	-1923.14	4356.85	4158.81	0.462
Concio 3	332	J[833]	ST SLU Mobili	FZ-MIN	1	1	1	1	-157.73	4655.61	-1845.78	4356.85	4158.81	0.444
Concio 3	333	I[833]	ST SLU Mobili	FZ-MIN	1	1	1	1	-24.89	5234.57	-2052.89	4356.85	4158.81	0.494
Concio 3	333	J[834]	ST SLU Mobili	FZ-MIN	1	1	1	1	-105.48	25584.31	-1255.17	4356.85	4158.81	0.302
Concio 3	334	I[834]	ST SLU Mobili	FZ-MIN	1	1	1	1	-150.68	25319.18	-1520.22	4356.85	4158.81	0.366
Concio 3	334	J[835]	ST SLU Mobili	FZ-MIN	1	1	1	1	-104.18	49400.77	-1197.27	4356.85	4158.81	0.288
Concio 2	335	I[835]	ST SLU Mobili	FZ-MIN	1	1	1	1	-104.18	60899.68	-1197.27	3267.64	2478.30	0.483
Concio 2	335	J[836]	ST SLU Mobili	FZ-MIN	1	1	1	1	-238.39	91952.48	-769.20	3267.64	2478.30	0.310
Concio 2	336	I[836]	ST SLU Mobili	FZ-MIN	1	1	1	1	-227.34	91342.34	-1041.89	3267.64	2478.30	0.420
Concio 2	336	J[837]	ST SLU Mobili	FZ-MAX	1	1	1	1	-152.61	105959.46	708.62	3267.64	2478.30	0.286
Concio 2	337	I[837]	ST SLU Mobili	FZ-MIN	1	1	1	1	-204.57	105410.54	-587.90	3267.64	2478.30	0.237
Concio 2	337	J[838]	ST SLU Mobili	FZ-MAX	1	1	1	1	-130.94	99985.20	689.18	3267.64	2478.30	0.278



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Concio 1	338	I[838]	ST SLU Mobili	FZ-MAX	1	1	1	1	-130.94	229815.18	689.18	3302.77	2478.97	0.278
Concio 1	338	J[839]	ST SLU Mobili	FZ-MAX	1	1	1	1	-161.26	175114.86	1181.19	3302.77	2478.97	0.476
Concio 1	339	I[839]	ST SLU Mobili	FZ-MAX	1	1	1	1	-93.03	174611.44	911.50	3302.77	2478.97	0.368
Concio 1	339	J[840]	ST SLU Mobili	FZ-MAX	3	3	3	3	-88.90	313.92	1810.01	3302.77	2574.18	0.703
Concio 1	340	I[840]	ST SLU Mobili	MY-MIN	1	3	3	3	0.00	362.86	-481.10	3302.77	2574.11	0.187
Concio 1	340	J[841]	ST SLV Vert	FX-MAX	1	1	1	1	0.85	7.16	1.66	3302.77	2574.42	0.001
Concio 1	401	I[842]	ST SLU Vert	FX-MAX	1	1	1	1	0.91	4.69	1.96	3302.77	2574.42	0.001
Concio 1	401	J[843]	ST SLU Mobili	FZ-MAX	1	3	3	3	0.00	364.81	488.88	3302.77	2574.10	0.190
Concio 1	402	I[843]	ST SLU Mobili	FZ-MIN	3	3	3	3	-175.41	544.65	-1849.52	3302.77	2573.69	0.719
Concio 1	402	J[844]	ST SLU Mobili	FZ-MIN	1	1	1	1	-111.11	104948.56	-886.19	3302.77	2478.97	0.357
Concio 1	403	I[844]	ST SLU Mobili	FZ-MIN	1	1	1	1	-163.77	105071.70	-1239.29	3302.77	2478.97	0.500
Concio 1	403	J[845]	ST SLU Mobili	FZ-MIN	1	1	1	1	-150.55	137936.16	-653.53	3302.77	2478.97	0.264
Concio 2	404	I[845]	ST SLU Mobili	FZ-MIN	1	1	1	1	-150.55	78310.07	-653.53	3267.64	2478.30	0.264
Concio 2	404	J[846]	ST SLU Mobili	FZ-MAX	1	1	1	1	43.69	82312.58	533.55	3267.64	2478.30	0.215
Concio 2	405	I[846]	ST SLU Mobili	FZ-MIN	1	1	1	1	-153.81	83436.99	-685.32	3267.64	2478.30	0.277
Concio 2	405	J[847]	ST SLU Mobili	FZ-MAX	1	1	1	1	42.18	70371.48	1047.50	3267.64	2478.30	0.423
Concio 2	406	I[847]	ST SLU Mobili	FZ-MAX	1	1	1	1	6.60	71278.67	671.93	3267.64	2478.30	0.271
Concio 2	406	J[848]	ST SLU Mobili	FZ-MAX	1	1	1	1	49.69	46710.91	1208.79	3267.64	2478.30	0.488
Concio 3	407	I[848]	ST SLU Mobili	FZ-MAX	1	1	1	1	49.69	39377.29	1208.79	4356.85	4158.81	0.291
Concio 3	407	J[849]	ST SLU Mobili	FZ-MAX	1	1	1	1	29.73	19590.94	1590.20	4356.85	4158.81	0.382
Concio 3	408	I[849]	ST SLU Mobili	FZ-MAX	1	1	1	1	14.38	20109.83	1219.32	4356.85	4158.81	0.293
Concio 3	408	J[850]	ST SLU Mobili	FZ-MAX	1	1	1	1	71.48	4965.33	2125.13	4356.85	4158.81	0.511
Concio 3	409	I[850]	ST SLU Mobili	FZ-MAX	1	1	1	1	86.75	4581.00	1790.03	4356.85	4158.81	0.430
Concio 3	409	J[851]	ST SLU Mobili	FZ-MAX	1	1	1	1	73.41	5512.25	1894.58	4356.85	4158.81	0.456
Concio 4 H=var	410	I[851]	ST SLU Mobili	FZ-MAX	1	3	3	3	3.12	6222.00	1886.68	4310.00	4114.09	0.459
Concio 4 H=var	410	J[852]	ST SLU Mobili	FZ-MAX	1	3	3	3	93.43	11424.31	2390.24	6652.39	4857.06	0.492
Concio 4 H=var	411	I[852]	ST SLU Mobili	FZ-MAX	1	3	3	3	-9.52	11091.63	2137.60	6652.39	4868.46	0.439
Concio 4 H=var	411	J[853]	ST SLU Mobili	FZ-MAX	1	3	4	4	112.28	14695.12	2628.07	8994.79	5349.07	0.491
Concio 4 H=200	412	I[853]	ST SLU Mobili	FZ-MAX	1	3	4	4	230.18	14841.42	2627.05	8994.79	5344.93	0.492
Concio 4 H=200	412	J[854]	ST SLU Mobili	FZ-MAX	1	3	4	4	182.56	16034.45	2726.14	8994.79	5309.63	0.513
Concio 4 H=200	413	I[854]	ST SLU Mobili	FZ-MIN	1	3	4	4	10.56	17108.79	-2587.46	8994.79	5275.51	0.490
Concio 4 H=200	413	J[855]	ST SLU Mobili	FZ-MIN	1	3	4	4	46.49	15904.47	-2488.32	8994.79	5313.61	0.468
Concio 4 H=var	414	I[855]	ST SLU Mobili	FZ-MIN	1	3	4	4	-89.79	15669.05	-2492.09	8994.79	5318.31	0.469
Concio 4 H=var	414	J[856]	ST SLU Mobili	FZ-MIN	1	3	3	3	-211.03	12222.37	-2001.06	6652.39	4822.18	0.415
Concio 4 H=var	415	I[856]	ST SLU Mobili	FZ-MIN	1	3	3	3	-132.25	12663.61	-2248.51	6652.39	4807.05	0.468
Concio 4 H=var	415	J[857]	ST SLU Mobili	FZ-MIN	1	3	3	3	-226.02	7833.97	-1745.09	4310.00	4114.09	0.424
Concio 5	416	I[857]	ST SLU Mobili	FZ-MIN	1	1	1	1	-160.53	7020.12	-1749.16	4356.85	4158.81	0.421
Concio 5	416	J[858]	ST SLU Mobili	FZ-MIN	1	1	1	1	-137.36	6156.75	-1644.61	4356.85	4158.81	0.395
Concio 5	417	I[858]	ST SLU Mobili	FZ-MIN	1	1	1	1	-47.02	6586.39	-1981.18	4356.85	4158.81	0.476
Concio 5	417	J[859]	ST SLU Mobili	FZ-MIN	1	1	1	1	-112.36	5926.86	-1048.64	4356.85	4158.81	0.252
Concio 5	418	I[859]	ST SLU Mobili	FZ-MIN	1	1	1	1	-152.53	5628.61	-1431.25	4356.85	4158.81	0.344
Concio 5	418	J[860]	ST SLU Mobili	FZ-MIN	1	1	1	1	-134.15	24020.13	-998.82	4356.85	4158.81	0.240
Concio 6	419	I[860]	ST SLU Mobili	FZ-MIN	1	1	1	1	-134.15	32302.58	-998.82	2614.11	1592.23	0.627
Concio 6	419	J[861]	ST SLU Mobili	FZ-MIN	1	1	1	1	-180.40	51483.18	-488.42	2614.11	1592.23	0.307
Concio 6	420	I[861]	ST SLU Mobili	FZ-MIN	1	1	1	1	-174.95	51220.94	-871.62	2614.11	1592.23	0.547
Concio 6	420	J[862]	ST SLU Mobili	FZ-MIN	1	1	1	1	-155.51	56664.13	-380.56	2614.11	1592.23	0.239
Concio 6	421	I[862]	ST SLU Mobili	FZ-MIN	1	1	1	1	-155.51	56664.13	-380.56	2614.11	1592.23	0.239
Concio 6	421	J[863]	ST SLU Mobili	FZ-MAX	1	1	1	1	61.67	51077.62	871.45	2614.11	1592.23	0.547
Concio 6	422	I[863]	ST SLU Mobili	FZ-MAX	1	1	1	1	39.58	51389.52	488.37	2614.11	1592.23	0.307
Concio 6	422	J[864]	ST SLU Mobili	FZ-MAX	1	1	1	1	85.82	32182.84	998.68	2614.11	1592.23	0.627
Concio 5	423	I[864]	ST SLU Mobili	FZ-MAX	1	1	1	1	85.82	24078.05	998.68	4356.85	4158.81	0.240
Concio 5	423	J[865]	ST SLU Mobili	FZ-MAX	1	1	1	1	67.47	5666.30	1431.14	4356.85	4158.81	0.344
Concio 5	424	I[865]	ST SLU Mobili	FZ-MAX	1	1	1	1	68.34	6124.56	1048.55	4356.85	4158.81	0.252
Concio 5	424	J[866]	ST SLU Mobili	FZ-MAX	1	1	1	1	133.71	6052.11	1980.92	4356.85	4158.81	0.476
Concio 5	425	I[866]	ST SLU Mobili	FZ-MAX	1	1	1	1	80.98	5640.04	1644.48	4356.85	4158.81	0.395
Concio 5	425	J[867]	ST SLU Mobili	FZ-MAX	1	1	1	1	69.14	6489.01	1749.02	4356.85	4158.81	0.421
Concio 4 H=var	426	I[867]	ST SLU Mobili	FZ-MAX	1	3	3	3	20.25	7317.64	1744.92	4310.00	4114.09	0.424
Concio 4 H=var	426	J[868]	ST SLU Mobili	FZ-MAX	1	3	3	3	110.38	12010.83	2248.31	6652.39	4835.77	0.465
Concio 4 H=var	427	I[868]	ST SLU Mobili	FZ-MAX	1	3	3	3	-8.90	11644.31	2000.87	6652.39	4848.99	0.413
Concio 4 H=var	427	J[869]	ST SLU Mobili	FZ-MAX	1	3	4	4	118.64	14810.84	2491.85	8994.79	5345.80	0.466
Concio 4 H=200	428	I[869]	ST SLU Mobili	FZ-MAX	1	3	4	4	227.50	14944.44	2488.08	8994.79	5341.99	0.466
Concio 4 H=200	428	J[870]	ST SLU Mobili	FZ-MAX	1	3	4	4	183.37	16064.61	2587.21	8994.79	5308.70	0.487
Concio 4 H=200	429	I[870]	ST SLU Mobili	FZ-MIN	1	3	4	4	0.67	17085.47	-2726.55	8994.79	5276.27	0.517
Concio 4 H=200	429	J[871]	ST SLU Mobili	FZ-MIN	1	3	4	4	36.92	15801.26	-2627.45	8994.79	5316.74	0.494
Concio 4 H=var	430	I[871]	ST SLU Mobili	FZ-MIN	1	3	4	4	-104.56	15541.87	-2628.46	8994.79	5321.76	0.494
Concio 4 H=var	430	J[872]	ST SLU Mobili	FZ-MIN	1	3	3	3	-222.95	11638.36	-2137.92	6652.39	4843.99	0.441
Concio 4 H=var	431	I[872]	ST SLU Mobili	FZ-MIN	1	3	3	3	-144.50	12044.77	-2390.55	6652.39	4830.77	0.495
Concio 4 H=var	431	J[873]	ST SLU Mobili	FZ-MIN	1	3	3	3	-242.07	6631.34	-1886.96	4310.00	4114.09	0.459
Concio 3	432	I[873]	ST SLU Mobili	FZ-MIN	1	1	1	1	-146.66	5975.89	-1894.89	4356.85	4158.81	0.456
Concio 3	432	J[874]	ST SLU Mobili	FZ-MIN	1	1	1	1	-125.35	5004.53	-1790.33	4356.85	4158.81	0.430
Concio 3	433	I[874]	ST SLU Mobili	FZ-MIN	1	1	1	1	-89.03	5434.34	-1215.54	4356.85	4158.81	0.511
Concio 3	433	J[875]	ST SLU Mobili	FZ-MIN	1	1	1	1	-146.11	20067.06	-1219.55	4356.85	4158.81	0.293
Concio 3	434	I[875]	ST SLU Mobili	FZ-MIN	1	1	1	1	-126.90	19585.71	-1590.45	4356.85	4158.81	0.382
Concio 3	434	J[876]	ST SLU Mobili	FZ-MIN	1	1	1	1	-106.91	39388.52	-1209.09	4356.85	4158.81	0.291
Concio 2	435	I[876]	ST SLU Mobili	FZ-MIN	1	1	1	1	-106.91	46790.32	-1209.09	3267.64	2478.30	0.488
Concio 2	435	J[877]	ST SLU Mobili	FZ-MIN	1	1	1	1	-149.99	71382.48	-672.15	3267.64	2478.30	0.271
Concio 2	436	I[877]	ST SLU Mobili	FZ-MIN	1	1	1	1	-128.97	70526.87	-1047.89	3267.64	2478.30	0.423
Concio 2	436	J[878]	ST SLU Mobili	FZ-MAX	1	1	1	1	17.33	83196.65	684.90	3267.64	2478.30	0.276
Concio 2	437	I[878]	ST SLU Mobili	FZ-MIN	1	1	1	1	-116.52	82486.16	-533.84	3267.64	2478.30	0.215
Concio 2	437	J[879]	ST SLU Mobili	FZ-MAX	1	1	1	1	9.64	78084.71	653.24	3267.64	2478.30	0.264

Concio 1	438	I[879]	ST SLU Mobili	FZ-MAX	1	1	1	1	9.64	137316.01	653.24	3302.77	2478.97	0.264
Concio 1	438	J[880]	ST SLU Mobili	FZ-MAX	1	1	1	1	-3.55	104378.85	1238.95	3302.77	2478.97	0.500
Concio 1	439	I[880]	ST SLU Mobili	FZ-MAX	1	1	1	1	3.11	104585.12	885.66	3302.77	2478.97	0.357
Concio 1	439	J[881]	ST SLU Mobili	FZ-MAX	1	3	3	3	-61.11	490.81	1849.11	3302.77	2573.84	0.718
Concio 1	440	I[881]	ST SLU Mobili	MY-MIN	1	3	3	3	0.00	364.79	-488.88	3302.77	2574.10	0.190
Concio 1	440	J[882]	ST SLV Vert	FX-MAX	1	1	1	1	0.93	4.79	1.96	3302.77	2574.42	0.001

### 8.3 VERIFICHE ALL'INSTABILITÀ FLESSO-TORSIONALE DELL'ANIMA

Si effettuano le verifiche allo stato limite ultimo per instabilità flesso-torsionale delle sezioni composte travi-soletta secondo lo schema ad U invertita (Circ. 02/02/2009, C4.3.4.4).

I coefficienti parziali per SLU assunti sono i seguenti:

$$\gamma_{M1} = 1.10 \quad \text{acciaio strutturale: resistenza all'instabilità (ponti)}$$

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem Property:	nome delle caratteristiche geometriche dell'elemento
Elem:	numero dell'elemento
Position:	nodo iniziale (I) o finale (J) dell'elemento
Lcom:	combinazione di carico positivo o negativo più gravosa
Type:	sollecitazione (massima o minima)
Sect. class:	classificazione complessiva sezione
N_Ed:	sforzamento assiale di calcolo
M_Ed:	momento di calcolo
Nb,Rd:	sforzamento assiale resistente all'instabilità del corrente compresso
Mb,Rd:	momento resistente all'instabilità
Mcr:	momento critico

Interaction ratio: condizione di verifica: 
$$\frac{N_{Ed}}{N_{b,Rd}} + \frac{M_{Ed}}{M_{b,Rd}} \leq 1$$

Elem property	Elem number	Position	Lcom	Type	Sect. Class	N_Ed (kN)	M_Ed (kN*m)	Nb,Rd (kN)	Mb,Rd (kN*m)	Mcr (kN*m)	Interaction Ratio
Concio 1	101	I[719]	ST SLV Long	FX-MIN	4	-1.90	-0.60	17963.00	6350.05	109.96	0.000
Concio 1	101	J[720]	ST SLU Mobili	MY-MIN	3	0.00	-343.84	17963.00	6363.77	112.41	0.054
Concio 1	102	I[720]	ST SLU Mobili	MY-MIN	3	-231.67	-865.38	15885.38	6132.56	25.55	0.156
Concio 1	102	J[721]	ST SLU Mobili	MY-MAX	1	-65.22	3524.43	46848.01	10054.75	45.54	0.352
Concio 1	103	I[721]	ST SLU Mobili	MY-MAX	1	-221.28	3235.71	51956.43	10463.93	46.70	0.314
Concio 1	103	J[722]	ST SLU Mobili	MY-MAX	1	-162.48	4593.24	51956.43	10463.93	46.70	0.442
Concio 2	104	I[722]	ST SLU Mobili	MY-MAX	1	-162.48	4594.21	55693.51	12635.70	86.73	0.367
Concio 2	104	J[723]	ST SLU Mobili	MY-MAX	1	-173.87	4677.64	55693.51	12635.70	86.73	0.373
Concio 2	105	I[723]	ST SLU Mobili	MY-MAX	1	-216.89	4610.79	46533.18	11019.82	38.64	0.423
Concio 2	105	J[724]	ST SLU Mobili	MY-MAX	1	-237.77	4191.53	46533.18	11019.82	38.64	0.386
Concio 2	106	I[724]	ST SLU Mobili	MY-MAX	1	-179.58	4299.66	52741.05	12258.24	65.60	0.354
Concio 2	106	J[725]	ST SLU Mobili	MY-MAX	1	-134.81	3524.92	52741.05	12258.24	65.60	0.290
Concio 3	107	I[725]	ST SLU Mobili	MY-MAX	1	-134.81	3525.39	57024.39	13386.70	115.43	0.266
Concio 3	107	J[726]	ST SLU Mobili	MY-MAX	1	-146.73	2174.40	57024.39	13386.70	115.43	0.165
Concio 3	108	I[726]	ST SLU Mobili	MY-MAX	1	57.11	2432.61	47184.76	12485.79	116.45	0.196
Concio 3	108	J[727]	ST SLU Mobili	MY-MIN	1	-60.10	-4227.83	20044.03	9733.87	31.08	0.437
Concio 3	109	I[727]	ST SLU Mobili	MY-MIN	1	-48.75	-4114.23	22659.14	10249.33	169.81	0.404
Concio 3	109	J[728]	ST SLU Mobili	MY-MIN	1	-48.65	-4465.04	22659.14	10249.33	169.81	0.438
Concio 4 H=var	110	I[728]	ST SLU Mobili	MY-MIN	3	-178.80	-4447.79	29959.30	10750.38	78.45	0.420
Concio 4 H=var	110	J[729]	ST SLU Mobili	MY-MIN	3	-196.18	-6450.78	33340.25	16899.82	78.34	0.388
Concio 4 H=var	111	I[729]	ST SLU Mobili	MY-MIN	3	-196.83	-6423.14	33340.25	16899.90	75.35	0.386
Concio 4 H=var	111	J[730]	ST SLU Mobili	MY-MIN	4	-234.69	-8769.70	36721.20	23173.54	76.04	0.385
Concio 4 H=200	112	I[730]	ST SLU Mobili	MY-MIN	4	-30.87	-8769.70	36721.20	23173.28	330.95	0.379
Concio 4 H=200	112	J[731]	ST SLU Mobili	MY-MIN	4	-32.89	-9402.83	36721.20	23132.45	330.95	0.407
Concio 4 H=200	113	I[731]	ST SLU Mobili	MY-MIN	4	-14.70	-9455.35	36721.20	23133.14	330.49	0.409
Concio 4 H=200	113	J[732]	ST SLU Mobili	MY-MIN	4	-17.06	-8832.54	36721.20	23171.51	330.49	0.382
Concio 4 H=var	114	I[732]	ST SLU Mobili	MY-MIN	4	-213.03	-8832.54	36721.20	23171.56	75.47	0.387
Concio 4 H=var	114	J[733]	ST SLU Mobili	MY-MIN	3	-146.09	-6516.98	33340.25	16878.64	74.78	0.391
Concio 4 H=var	115	I[733]	ST SLU Mobili	MY-MIN	3	-141.57	-6543.31	33340.25	16879.21	76.39	0.392

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Concio 4 H=var	115	J[734]	ST SLU Mobili	MY-MIN	3	-124.02	-4778.59	29959.30	10698.99	76.50	0.451
Concio 5	116	I[734]	ST SLU Mobili	MY-MIN	1	-8.36	-4768.78	22659.14	10249.33	168.00	0.466
Concio 5	116	J[735]	ST SLU Mobili	MY-MIN	1	-7.84	-4472.84	22659.14	10249.33	168.00	0.437
Concio 5	117	I[735]	ST SLU Mobili	MY-MIN	1	1.98	-4549.90	19837.42	9560.39	27.13	0.476
Concio 5	117	J[736]	ST SLU Termico	MY-MIN	1	38.23	-2336.76	19837.42	9525.08	26.59	0.247
Concio 5	118	I[736]	ST SLU Termico	MY-MIN	1	-131.03	-2357.73	22659.14	10249.33	48.43	0.236
Concio 5	118	J[737]	ST SLU Mobili	MY-MAX	1	-55.78	2727.66	56079.69	13386.70	109.32	0.205
Concio 6	119	I[737]	ST SLU Mobili	MY-MAX	1	-55.78	2727.36	52344.97	11874.62	64.35	0.231
Concio 6	119	J[738]	ST SLU Mobili	MY-MAX	1	-103.84	3158.55	52344.97	11874.62	64.35	0.268
Concio 6	120	I[738]	ST SLU Mobili	MY-MAX	1	-135.66	3069.75	53194.79	11988.26	68.49	0.259
Concio 6	120	J[739]	ST SLU Mobili	MY-MAX	1	-102.57	3602.92	53194.79	11988.26	68.49	0.303
Concio 6	121	I[739]	ST SLU Mobili	MY-MAX	1	-102.57	3602.92	53194.79	11994.06	68.49	0.302
Concio 6	121	J[740]	ST SLU Mobili	MY-MAX	1	-135.66	3069.90	53194.79	11994.06	68.49	0.259
Concio 6	122	I[740]	ST SLU Mobili	MY-MAX	1	-103.80	3158.24	52344.97	11862.87	64.35	0.268
Concio 6	122	J[741]	ST SLU Mobili	MY-MAX	1	-55.75	2726.62	52344.97	11862.87	64.35	0.231
Concio 5	123	I[741]	ST SLU Mobili	MY-MAX	1	-55.75	2726.94	56079.69	13386.70	109.31	0.205
Concio 5	123	J[742]	ST SLU Termico	MY-MIN	1	-131.06	-2357.53	22659.14	10249.33	48.43	0.236
Concio 5	124	I[742]	ST SLU Termico	MY-MIN	1	38.14	-2336.60	19837.42	9527.28	26.59	0.247
Concio 5	124	J[743]	ST SLU Mobili	MY-MIN	1	1.86	-4551.05	19837.42	9562.14	27.13	0.476
Concio 5	125	I[743]	ST SLU Mobili	MY-MIN	1	-7.91	-4473.85	22659.14	10249.33	168.00	0.437
Concio 5	125	J[744]	ST SLU Mobili	MY-MIN	1	-8.43	-4769.72	22659.14	10249.33	168.00	0.466
Concio 4 H=var	126	I[744]	ST SLU Mobili	MY-MIN	3	-124.08	-4779.66	29959.30	10698.97	76.50	0.451
Concio 4 H=var	126	J[745]	ST SLU Mobili	MY-MIN	3	-141.62	-6544.69	33340.25	16879.19	76.39	0.392
Concio 4 H=var	127	I[745]	ST SLU Mobili	MY-MIN	3	-146.01	-6518.32	33340.25	16878.62	74.78	0.391
Concio 4 H=var	127	J[746]	ST SLU Mobili	MY-MIN	4	-212.97	-8833.55	36721.20	23171.55	75.46	0.387
Concio 4 H=200	128	I[746]	ST SLU Mobili	MY-MIN	4	-17.03	-8833.55	36721.20	23171.50	330.48	0.382
Concio 4 H=200	128	J[747]	ST SLU Mobili	MY-MIN	4	-14.68	-9456.01	36721.20	23133.13	330.48	0.409
Concio 4 H=200	129	I[747]	ST SLU Mobili	MY-MIN	4	-33.00	-9403.83	36721.20	23132.44	330.95	0.407
Concio 4 H=200	129	J[748]	ST SLU Mobili	MY-MIN	4	-30.98	-8770.48	36721.20	23173.27	330.95	0.379
Concio 4 H=var	130	I[748]	ST SLU Mobili	MY-MIN	4	-234.84	-8770.48	36721.20	23173.53	76.04	0.385
Concio 4 H=var	130	J[749]	ST SLU Mobili	MY-MIN	3	-196.97	-6423.08	33340.25	16899.88	75.35	0.386
Concio 4 H=var	131	I[749]	ST SLU Mobili	MY-MIN	3	-196.14	-6450.72	33340.25	16899.80	78.34	0.388
Concio 4 H=var	131	J[750]	ST SLU Mobili	MY-MIN	3	-178.76	-4447.55	29959.30	10750.36	78.45	0.420
Concio 3	132	I[750]	ST SLU Mobili	MY-MIN	1	-48.59	-4464.81	22659.14	10249.33	169.81	0.438
Concio 3	132	J[751]	ST SLU Mobili	MY-MIN	1	-48.69	-4113.88	22659.14	10249.33	169.81	0.404
Concio 3	133	I[751]	ST SLU Mobili	MY-MIN	1	-59.99	-4227.50	20044.03	9731.69	31.08	0.437
Concio 3	133	J[752]	ST SLU Mobili	MY-MAX	1	56.94	2432.91	47184.76	12510.32	116.46	0.196
Concio 3	134	I[752]	ST SLU Mobili	MY-MAX	1	-146.88	2174.62	57024.39	13386.70	115.43	0.165
Concio 3	134	J[753]	ST SLU Mobili	MY-MAX	1	-134.91	3525.99	57024.39	13386.70	115.43	0.266
Concio 2	135	I[753]	ST SLU Mobili	MY-MAX	1	-134.91	3525.52	52741.05	12266.66	65.61	0.290
Concio 2	135	J[754]	ST SLU Mobili	MY-MAX	1	-179.65	4300.66	52741.05	12266.66	65.61	0.354
Concio 2	136	I[754]	ST SLU Mobili	MY-MAX	1	-237.84	4191.27	46533.18	11018.84	38.65	0.386
Concio 2	136	J[755]	ST SLU Mobili	MY-MAX	1	-216.84	4610.87	46533.18	11018.84	38.65	0.423
Concio 2	137	I[755]	ST SLU Mobili	MY-MAX	1	-173.85	4677.41	55693.51	12635.70	86.72	0.373
Concio 2	137	J[756]	ST SLU Mobili	MY-MAX	1	-162.43	4594.32	55693.51	12635.70	86.72	0.367
Concio 1	138	I[756]	ST SLU Mobili	MY-MAX	1	-162.43	4593.41	51956.43	10455.29	46.70	0.443
Concio 1	138	J[757]	ST SLU Mobili	MY-MAX	1	-221.26	3235.64	51956.43	10455.29	46.70	0.314
Concio 1	139	I[757]	ST SLU Mobili	MY-MAX	1	-65.21	3524.39	46848.01	10030.53	45.54	0.353
Concio 1	139	J[758]	ST SLU Mobili	MY-MIN	3	-231.65	-865.29	15885.38	6135.56	25.55	0.156
Concio 1	140	I[758]	ST SLU Mobili	MY-MIN	3	0.00	-343.82	17963.00	6363.77	112.41	0.054
Concio 1	140	J[759]	ST SLV Long	FX-MIN	4	-1.91	-0.60	17963.00	6350.05	109.95	0.000
Concio 1	201	I[760]	ST SLV Long	FX-MIN	4	-1.75	-0.53	18098.91	6479.08	110.14	0.000
Concio 1	201	J[761]	ST SLU Mobili	MY-MIN	3	0.00	-339.51	18098.91	6494.02	112.51	0.052
Concio 1	202	I[761]	ST SLU Mobili	MY-MIN	3	-336.52	-511.14	16005.57	6263.29	23.59	0.103
Concio 1	202	J[762]	ST SLU Mobili	MY-MAX	1	-180.23	3446.75	49645.95	10249.23	45.74	0.340
Concio 1	203	I[762]	ST SLU Mobili	MY-MAX	1	-401.19	3212.42	55059.46	10637.44	45.78	0.309
Concio 1	203	J[763]	ST SLU Mobili	MY-MAX	1	-366.25	4167.93	55059.46	10637.44	45.78	0.399
Concio 2	204	I[763]	ST SLU Mobili	MY-MAX	1	-366.25	4173.87	58902.07	12846.08	88.15	0.331
Concio 2	204	J[764]	ST SLU Mobili	MY-MAX	1	-416.77	4415.66	58902.07	12846.08	88.15	0.351
Concio 2	205	I[764]	ST SLU Mobili	MY-MAX	1	-511.06	4472.04	49214.00	11196.81	38.59	0.410
Concio 2	205	J[765]	ST SLU Mobili	MY-MAX	1	-507.40	4123.20	49214.00	11196.81	38.59	0.379
Concio 2	206	I[765]	ST SLU Mobili	FZ-MAX	1	-289.80	4117.19	55779.51	12490.55	67.80	0.335
Concio 2	206	J[766]	ST SLU Mobili	MY-MAX	1	-221.71	3215.24	55779.51	12474.68	66.66	0.262
Concio 3	207	I[766]	ST SLU Mobili	MY-MAX	1	-221.71	3216.07	60219.77	13602.15	110.72	0.240
Concio 3	207	J[767]	ST SLU Mobili	MY-MAX	1	-338.41	2363.29	60219.77	13602.15	110.72	0.179
Concio 3	208	I[767]	ST SLU Mobili	MY-MIN	3	207.48	-1405.91	20231.85	7001.85	31.59	0.211
Concio 3	208	J[768]	ST SLU Mobili	MY-MIN	1	184.51	-3826.71	20231.85	9877.53	31.59	0.397
Concio 3	209	I[768]	ST SLU Mobili	MY-MIN	1	130.85	-3861.22	22871.47	10397.59	170.08	0.377
Concio 3	209	J[769]	ST SLU Mobili	MY-MIN	1	129.37	-4164.77	22871.47	10397.59	170.08	0.406
Concio 4 H=var	210	I[769]	ST SLU Mobili	MY-MIN	3	3.78	-4226.25	30596.27	10825.92	79.16	0.391
Concio 4 H=var	210	J[770]	ST SLU Mobili	MY-MIN	3	-68.02	-5945.46	33977.22	16988.30	78.75	0.352
Concio 4 H=var	211	I[770]	ST SLU Mobili	MY-MIN	3	-88.04	-5961.92	33977.22	16988.40	76.42	0.354
Concio 4 H=var	211	J[771]	ST SLU Mobili	MY-MIN	4	-209.20	-8168.04	37358.17	23218.83	76.93	0.357
Concio 4 H=200	212	I[771]	ST SLU Mobili	MY-MIN	4	-22.87	-8168.04	37358.17	23218.46	332.27	0.352
Concio 4 H=200	212	J[772]	ST SLU Mobili	MY-MIN	4	-96.24	-8673.02	37358.17	23171.96	332.27	0.377
Concio 4 H=200	213	I[772]	ST SLU Mobili	MY-MIN	4	-110.79	-8743.33	37358.17	23172.18	332.88	0.380
Concio 4 H=200	213	J[773]	ST SLU Mobili	MY-MIN	4	-85.45	-8153.85	37358.17	23215.97	332.88	0.354
Concio 4 H=var	214	I[773]	ST SLU Mobili	MY-MIN	4	-280.96	-8153.85	37358.17	23216.22	75.99	0.359
Concio 4 H=var	214	J[774]	ST SLU Mobili	MY-MIN	3	-78.89	-6100.40	33977.22	16963.53	75.48	0.362
Concio 4 H=var	215	I[774]	ST SLU Mobili	MY-MIN	3	-64.06	-6091.47	33977.22	16963.24	76.79	0.361

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Concio 4 H=var	215	J[775]	ST SLU Mobili	MY-MIN	3	10.95	-4568.36	30596.27	10767.41	77.19	0.425
Concio 5	216	I[775]	ST SLU Mobili	MY-MIN	1	124.14	-4526.73	22871.47	10397.59	168.54	0.441
Concio 5	216	J[776]	ST SLU Mobili	MY-MIN	1	126.09	-4246.77	22871.47	10397.59	168.54	0.414
Concio 5	217	I[776]	ST SLU Mobili	MY-MIN	1	240.13	-4217.80	20023.30	9700.76	27.49	0.447
Concio 5	217	J[777]	ST SLU Termico	MY-MIN	1	238.20	-2286.49	20023.30	9660.62	26.71	0.249
Concio 5	218	I[777]	ST SLU Termico	MY-MIN	1	21.70	-2291.13	22871.47	10397.59	48.87	0.221
Concio 5	218	J[778]	ST SLU Termico	MY-MIN	1	30.38	-1898.55	22871.47	10397.59	48.87	0.184
Concio 6	219	I[778]	ST SLU Termico	MY-MIN	2	30.38	-1896.31	18787.18	8891.06	40.26	0.215
Concio 6	219	J[779]	ST SLU Mobili	MY-MAX	1	-231.34	3018.13	55412.36	12090.64	66.47	0.254
Concio 6	220	I[779]	ST SLU Mobili	MY-MAX	1	-351.80	2985.63	56311.98	12170.78	66.02	0.252
Concio 6	220	J[780]	ST SLU Mobili	MY-MAX	1	-206.65	3096.51	56311.98	12170.78	66.02	0.258
Concio 6	221	I[780]	ST SLU Mobili	MY-MAX	1	-206.65	3096.51	56311.98	12172.05	66.02	0.252
Concio 6	221	J[781]	ST SLU Mobili	MY-MAX	1	-351.88	2985.87	56311.98	12172.05	66.02	0.252
Concio 6	222	I[781]	ST SLU Mobili	MY-MAX	1	-231.31	3017.83	55412.36	12090.03	66.47	0.254
Concio 6	222	J[782]	ST SLU Termico	MY-MIN	2	30.44	-1896.26	18787.18	8891.06	40.26	0.215
Concio 5	223	I[782]	ST SLU Termico	MY-MIN	1	30.44	-1898.50	22871.47	10397.59	48.87	0.184
Concio 5	223	J[783]	ST SLU Termico	MY-MIN	1	21.76	-2291.05	22871.47	10397.59	48.87	0.221
Concio 5	224	I[783]	ST SLU Termico	MY-MIN	1	238.38	-2286.66	20023.30	9662.14	26.71	0.249
Concio 5	224	J[784]	ST SLU Mobili	MY-MIN	1	240.33	-4218.30	20023.30	9701.72	27.49	0.447
Concio 5	225	I[784]	ST SLU Mobili	MY-MIN	1	126.39	-4247.17	22871.47	10397.59	168.54	0.414
Concio 5	225	J[785]	ST SLU Mobili	MY-MIN	1	124.97	-4527.20	22871.47	10397.59	168.54	0.441
Concio 4 H=var	226	I[785]	ST SLU Mobili	MY-MIN	3	11.25	-4568.68	30596.27	10767.39	77.19	0.425
Concio 4 H=var	226	J[786]	ST SLU Mobili	MY-MIN	3	-63.78	-6091.84	33977.22	16963.22	76.79	0.361
Concio 4 H=var	227	I[786]	ST SLU Mobili	MY-MIN	3	-78.67	-6100.87	33977.22	16963.51	75.48	0.362
Concio 4 H=var	227	J[787]	ST SLU Mobili	MY-MIN	4	-280.67	-8154.46	37358.17	23216.21	75.99	0.359
Concio 4 H=200	228	I[787]	ST SLU Mobili	MY-MIN	4	-85.17	-8154.46	37358.17	23215.95	332.88	0.354
Concio 4 H=200	228	J[788]	ST SLU Mobili	MY-MIN	4	-110.53	-8743.74	37358.17	23172.17	332.88	0.380
Concio 4 H=200	229	I[788]	ST SLU Mobili	MY-MIN	4	-96.47	-8673.35	37358.17	23171.95	332.27	0.377
Concio 4 H=200	229	J[789]	ST SLU Mobili	MY-MIN	4	-23.06	-8168.20	37358.17	23218.45	332.27	0.352
Concio 4 H=var	230	I[789]	ST SLU Mobili	MY-MIN	4	-209.41	-8168.20	37358.17	23218.82	76.93	0.357
Concio 4 H=var	230	J[790]	ST SLU Mobili	MY-MIN	3	-88.26	-5962.43	33977.22	16988.38	76.42	0.354
Concio 4 H=var	231	I[790]	ST SLU Mobili	MY-MIN	3	-68.29	-5945.75	33977.22	16988.28	78.75	0.352
Concio 4 H=var	231	J[791]	ST SLU Mobili	MY-MIN	3	3.50	-4226.41	30596.27	10825.90	79.16	0.391
Concio 3	232	I[791]	ST SLU Mobili	MY-MIN	1	129.09	-4164.86	22871.47	10397.59	170.08	0.406
Concio 3	232	J[792]	ST SLU Mobili	MY-MIN	1	130.57	-3861.30	22871.47	10397.59	170.08	0.377
Concio 3	233	I[792]	ST SLU Mobili	MY-MIN	1	184.33	-3826.42	20231.85	9876.14	31.59	0.397
Concio 3	233	J[793]	ST SLU Mobili	MY-MIN	3	207.31	-1405.88	20231.85	7000.96	31.59	0.211
Concio 3	234	I[793]	ST SLU Mobili	MY-MAX	1	-338.41	2362.56	60219.77	13602.15	110.73	0.179
Concio 3	234	J[794]	ST SLU Mobili	MY-MAX	1	-221.61	3216.58	60219.77	13602.15	110.73	0.240
Concio 2	235	I[794]	ST SLU Mobili	MY-MAX	1	-221.61	3215.75	55779.51	12481.93	66.66	0.262
Concio 2	235	J[795]	ST SLU Mobili	MY-MAX	1	-319.74	4103.68	55779.51	12481.93	66.66	0.335
Concio 2	236	I[795]	ST SLU Mobili	MY-MAX	1	-507.26	4123.27	49214.00	11199.36	38.59	0.379
Concio 2	236	J[796]	ST SLU Mobili	MY-MAX	1	-511.10	4472.26	49214.00	11199.36	38.59	0.410
Concio 2	237	I[796]	ST SLU Mobili	FZ-MAX	1	-461.91	4454.72	58902.07	12846.08	88.81	0.355
Concio 2	237	J[797]	ST SLU Mobili	MY-MAX	1	-366.27	4173.90	58902.07	12846.08	88.15	0.331
Concio 1	238	I[797]	ST SLU Mobili	MY-MAX	1	-366.27	4167.96	55059.46	10631.08	45.78	0.399
Concio 1	238	J[798]	ST SLU Mobili	MY-MAX	1	-401.15	3212.08	55059.46	10631.08	45.78	0.309
Concio 1	239	I[798]	ST SLU Mobili	MY-MAX	1	-180.28	3446.60	49645.95	10214.57	45.74	0.341
Concio 1	239	J[799]	ST SLU Mobili	MY-MIN	3	-336.42	-511.28	16005.57	6266.12	23.59	0.103
Concio 1	240	I[799]	ST SLU Mobili	MY-MIN	3	0.00	-339.49	18098.91	6494.02	112.51	0.052
Concio 1	240	J[800]	ST SLV Long	FX-MIN	4	-1.76	-0.53	18098.91	6479.08	110.13	0.000
Concio 1	301	I[801]	ST SLV Long	FX-MIN	4	-1.75	-0.53	18098.91	6479.08	110.14	0.000
Concio 1	301	J[802]	ST SLU Mobili	MY-MIN	3	0.00	-339.51	18098.91	6494.02	112.51	0.052
Concio 1	302	I[802]	ST SLU Mobili	MY-MIN	3	-333.94	-510.22	16005.57	6263.24	23.60	0.102
Concio 1	302	J[803]	ST SLU Mobili	MY-MAX	1	-182.81	3452.34	49645.95	10249.44	45.74	0.341
Concio 1	303	I[803]	ST SLU Mobili	MY-MAX	1	-402.46	3214.51	55059.46	10637.52	45.78	0.310
Concio 1	303	J[804]	ST SLU Mobili	MY-MAX	1	-367.53	4171.67	55059.46	10637.52	45.78	0.399
Concio 2	304	I[804]	ST SLU Mobili	MY-MAX	1	-367.53	4177.65	58902.07	12846.08	88.16	0.331
Concio 2	304	J[805]	ST SLU Mobili	MY-MAX	1	-418.04	4420.54	58902.07	12846.08	88.16	0.351
Concio 2	305	I[805]	ST SLU Mobili	MY-MAX	1	-512.74	4474.94	49214.00	11196.82	38.59	0.410
Concio 2	305	J[806]	ST SLU Mobili	MY-MAX	1	-509.09	4125.71	49214.00	11196.82	38.59	0.379
Concio 2	306	I[806]	ST SLU Mobili	FZ-MAX	1	-294.14	4121.88	55779.51	12490.63	67.81	0.335
Concio 2	306	J[807]	ST SLU Mobili	MY-MAX	1	-226.04	3217.67	55779.51	12474.77	66.66	0.262
Concio 3	307	I[807]	ST SLU Mobili	MY-MAX	1	-226.04	3218.52	60219.77	13602.15	110.73	0.240
Concio 3	307	J[808]	ST SLU Mobili	MY-MAX	1	-342.74	2364.22	60219.77	13602.15	110.73	0.180
Concio 3	308	I[808]	ST SLU Vento	MY-MIN	3	180.45	-1388.91	20231.85	6994.11	31.39	0.208
Concio 3	308	J[809]	ST SLU Mobili	MY-MIN	1	194.24	-3830.21	20231.85	9877.32	31.59	0.397
Concio 3	309	I[809]	ST SLU Mobili	MY-MIN	1	136.18	-3864.27	22871.47	10397.59	170.10	0.378
Concio 3	309	J[810]	ST SLU Mobili	MY-MIN	1	134.70	-4169.81	22871.47	10397.59	170.10	0.407
Concio 4 H=var	310	I[810]	ST SLU Mobili	MY-MIN	3	8.79	-4227.44	30596.27	10825.92	79.18	0.391
Concio 4 H=var	310	J[811]	ST SLU Mobili	MY-MIN	3	-63.01	-5954.13	33977.22	16988.30	78.77	0.352
Concio 4 H=var	311	I[811]	ST SLU Mobili	MY-MIN	3	-83.95	-5968.78	33977.22	16988.40	76.43	0.354
Concio 4 H=var	311	J[812]	ST SLU Mobili	MY-MIN	4	-205.11	-8182.83	37358.17	23218.83	76.94	0.358
Concio 4 H=200	312	I[812]	ST SLU Mobili	MY-MIN	4	-18.22	-8182.83	37358.17	23218.46	332.28	0.353
Concio 4 H=200	312	J[813]	ST SLU Mobili	MY-MIN	4	-91.59	-8689.66	37358.17	23171.96	332.28	0.378
Concio 4 H=200	313	I[813]	ST SLU Mobili	MY-MIN	4	-105.50	-8760.11	37358.17	23172.18	332.89	0.381
Concio 4 H=200	313	J[814]	ST SLU Mobili	MY-MIN	4	-80.16	-8168.64	37358.17	23215.97	332.89	0.354
Concio 4 H=var	314	I[814]	ST SLU Mobili	MY-MIN	4	-276.24	-8168.64	37358.17	23216.22	76.01	0.359
Concio 4 H=var	314	J[815]	ST SLU Mobili	MY-MIN	3	-74.17	-6106.73	33977.22	16963.53	75.50	0.362
Concio 4 H=var	315	I[815]	ST SLU Mobili	MY-MIN	3	-57.52	-6099.76	33977.22	16963.24	76.81	0.361



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Concio 4 H=var	315	J[816]	ST SLU Mobili	MY-MIN	3	17.50	-4569.02	30596.27	10767.41	77.21	0.425
Concio 5	316	I[816]	ST SLU Mobili	MY-MIN	1	131.02	-4532.17	22871.47	10397.59	168.57	0.442
Concio 5	316	J[817]	ST SLU Mobili	MY-MIN	1	132.97	-4249.22	22871.47	10397.59	168.57	0.415
Concio 5	317	I[817]	ST SLU Mobili	MY-MIN	1	250.35	-4222.06	20023.30	9700.50	27.48	0.448
Concio 5	317	J[818]	ST SLU Termico	MY-MIN	1	248.41	-2289.72	20023.30	9660.36	26.71	0.249
Concio 5	318	I[818]	ST SLU Termico	MY-MIN	1	26.55	-2293.01	22871.47	10397.59	48.84	0.222
Concio 5	318	J[819]	ST SLU Termico	MY-MIN	1	35.22	-1901.97	22871.47	10397.59	48.84	0.185
Concio 6	319	I[819]	ST SLU Termico	MY-MIN	2	35.22	-1899.66	18787.18	8891.06	40.13	0.216
Concio 6	319	J[820]	ST SLU Mobili	MY-MAX	1	-236.19	3023.42	55412.36	12090.72	66.47	0.254
Concio 6	320	I[820]	ST SLU Mobili	MY-MAX	1	-355.58	2989.13	56311.98	12170.77	66.02	0.252
Concio 6	320	J[821]	ST SLU Mobili	MY-MAX	1	-210.42	3100.00	56311.98	12170.77	66.02	0.258
Concio 6	321	I[821]	ST SLU Mobili	MY-MAX	1	-210.42	3100.00	56311.98	12172.04	66.01	0.258
Concio 6	321	J[822]	ST SLU Mobili	MY-MAX	1	-355.65	2989.35	56311.98	12172.04	66.01	0.252
Concio 6	322	I[822]	ST SLU Mobili	MY-MAX	1	-236.07	3023.18	55412.36	12090.09	66.48	0.254
Concio 6	322	J[823]	ST SLU Termico	MY-MIN	2	35.20	-1899.63	18787.18	8891.06	40.13	0.216
Concio 5	323	I[823]	ST SLU Termico	MY-MIN	1	35.20	-1901.94	22871.47	10397.59	48.84	0.185
Concio 5	323	J[824]	ST SLU Termico	MY-MIN	1	26.52	-2292.92	22871.47	10397.59	48.84	0.222
Concio 5	324	I[824]	ST SLU Termico	MY-MIN	1	248.27	-2289.66	20023.30	9662.01	26.71	0.249
Concio 5	324	J[825]	ST SLU Mobili	MY-MIN	1	250.21	-4222.68	20023.30	9701.59	27.48	0.448
Concio 5	325	I[825]	ST SLU Mobili	MY-MIN	1	133.16	-4249.72	22871.47	10397.59	168.57	0.415
Concio 5	325	J[826]	ST SLU Mobili	MY-MIN	1	131.74	-4532.72	22871.47	10397.59	168.57	0.442
Concio 4 H=var	326	I[826]	ST SLU Mobili	MY-MIN	3	17.69	-4569.58	30596.27	10767.39	77.21	0.425
Concio 4 H=var	326	J[827]	ST SLU Mobili	MY-MIN	3	-57.34	-6100.41	33977.22	16963.22	76.81	0.361
Concio 4 H=var	327	I[827]	ST SLU Mobili	MY-MIN	3	-74.10	-6107.56	33977.22	16963.51	75.50	0.362
Concio 4 H=var	327	J[828]	ST SLU Mobili	MY-MIN	4	-276.09	-8169.25	37358.17	23216.21	76.01	0.359
Concio 4 H=200	328	I[828]	ST SLU Mobili	MY-MIN	4	-80.03	-8169.25	37358.17	23215.95	332.89	0.354
Concio 4 H=200	328	J[829]	ST SLU Mobili	MY-MIN	4	-105.38	-8760.42	37358.17	23172.17	332.89	0.381
Concio 4 H=200	329	I[829]	ST SLU Mobili	MY-MIN	4	-91.69	-8690.07	37358.17	23171.95	332.28	0.378
Concio 4 H=200	329	J[830]	ST SLU Mobili	MY-MIN	4	-18.28	-8182.97	37358.17	23218.45	332.28	0.353
Concio 4 H=var	330	I[830]	ST SLU Mobili	MY-MIN	4	-205.18	-8182.97	37358.17	23218.82	76.94	0.358
Concio 4 H=var	330	J[831]	ST SLU Mobili	MY-MIN	3	-84.03	-5968.92	33977.22	16988.38	76.43	0.354
Concio 4 H=var	331	I[831]	ST SLU Mobili	MY-MIN	3	-63.19	-5954.13	33977.22	16988.28	78.77	0.352
Concio 4 H=var	331	J[832]	ST SLU Mobili	MY-MIN	3	8.60	-4227.36	30596.27	10825.90	79.18	0.391
Concio 3	332	I[832]	ST SLU Mobili	MY-MIN	1	134.52	-4169.81	22871.47	10397.59	170.10	0.407
Concio 3	332	J[833]	ST SLU Mobili	MY-MIN	1	136.00	-3864.25	22871.47	10397.59	170.10	0.378
Concio 3	333	I[833]	ST SLU Mobili	MY-MIN	1	194.12	-3829.87	20231.85	9875.83	31.59	0.397
Concio 3	333	J[834]	ST SLU Vento	MY-MIN	3	180.24	-1389.17	20231.85	6993.03	31.39	0.208
Concio 3	334	I[834]	ST SLU Mobili	MY-MAX	1	-342.85	2363.50	60219.77	13602.15	110.74	0.180
Concio 3	334	J[835]	ST SLU Mobili	MY-MAX	1	-226.05	3219.01	60219.77	13602.15	110.74	0.240
Concio 2	335	I[835]	ST SLU Mobili	MY-MAX	1	-226.05	3218.14	55779.51	12482.05	66.67	0.262
Concio 2	335	J[836]	ST SLU Mobili	MY-MAX	1	-324.18	4108.29	55779.51	12482.05	66.67	0.335
Concio 2	336	I[836]	ST SLU Mobili	MY-MAX	1	-508.98	4125.73	49214.00	11199.35	38.59	0.379
Concio 2	336	J[837]	ST SLU Mobili	MY-MAX	1	-512.81	4475.23	49214.00	11199.35	38.59	0.410
Concio 2	337	I[837]	ST SLU Mobili	FZ-MAX	1	-463.20	4459.58	58902.07	12846.08	88.82	0.355
Concio 2	337	J[838]	ST SLU Mobili	MY-MAX	1	-367.55	4177.66	58902.07	12846.08	88.16	0.331
Concio 1	338	I[838]	ST SLU Mobili	MY-MAX	1	-367.55	4171.68	55059.46	10631.13	45.78	0.399
Concio 1	338	J[839]	ST SLU Mobili	MY-MAX	1	-402.44	3214.16	55059.46	10631.13	45.78	0.310
Concio 1	339	I[839]	ST SLU Mobili	MY-MAX	1	-182.85	3452.21	49645.95	10214.70	45.75	0.342
Concio 1	339	J[840]	ST SLU Mobili	MY-MIN	3	-333.84	-510.37	16005.57	6266.17	23.60	0.102
Concio 1	340	I[840]	ST SLU Mobili	MY-MIN	3	0.00	-339.49	18098.91	6494.02	112.51	0.052
Concio 1	340	J[841]	ST SLV Long	FX-MIN	4	-1.76	-0.53	18098.91	6479.08	110.13	0.000
Concio 1	401	I[842]	ST SLV Long	FX-MIN	4	-1.90	-0.60	17963.00	6350.05	109.96	0.000
Concio 1	401	J[843]	ST SLU Mobili	MY-MIN	3	0.00	-343.84	17963.00	6363.77	112.41	0.054
Concio 1	402	I[843]	ST SLU Mobili	MY-MIN	3	-230.31	-864.76	15885.38	6132.68	25.51	0.156
Concio 1	402	J[844]	ST SLU Mobili	MY-MAX	1	-66.57	3513.41	46848.01	10054.61	45.54	0.351
Concio 1	403	I[844]	ST SLU Mobili	MY-MAX	1	-225.65	3229.65	51956.43	10463.99	46.70	0.313
Concio 1	403	J[845]	ST SLU Mobili	MY-MAX	1	-166.85	4586.27	51956.43	10463.99	46.70	0.442
Concio 2	404	I[845]	ST SLU Mobili	MY-MAX	1	-166.85	4587.03	55693.51	12635.70	86.72	0.366
Concio 2	404	J[846]	ST SLU Mobili	MY-MAX	1	-178.24	4669.62	55693.51	12635.70	86.72	0.373
Concio 2	405	I[846]	ST SLU Mobili	MY-MAX	1	-221.42	4604.20	46533.18	11019.76	38.64	0.423
Concio 2	405	J[847]	ST SLU Mobili	MY-MAX	1	-242.30	4186.12	46533.18	11019.76	38.64	0.385
Concio 2	406	I[847]	ST SLU Mobili	MY-MAX	1	-180.94	4292.59	52741.05	12258.18	65.60	0.354
Concio 2	406	J[848]	ST SLU Mobili	MY-MAX	1	-136.18	3520.36	52741.05	12258.18	65.60	0.290
Concio 3	407	I[848]	ST SLU Mobili	MY-MAX	1	-136.18	3520.84	57024.39	13386.70	115.43	0.265
Concio 3	407	J[849]	ST SLU Mobili	MY-MAX	1	-148.10	2171.63	57024.39	13386.70	115.43	0.165
Concio 3	408	I[849]	ST SLU Mobili	MY-MAX	1	63.08	2425.20	47184.76	12485.82	116.68	0.196
Concio 3	408	J[850]	ST SLU Mobili	MY-MIN	1	-66.06	-4219.45	20044.03	9734.38	31.10	0.437
Concio 3	409	I[850]	ST SLU Mobili	MY-MIN	1	-54.96	-4110.39	22659.14	10249.33	169.78	0.404
Concio 3	409	J[851]	ST SLU Mobili	MY-MIN	1	-54.86	-4458.69	22659.14	10249.33	169.78	0.437
Concio 4 H=var	410	I[851]	ST SLU Mobili	MY-MIN	3	-185.30	-4444.71	29959.30	10750.38	78.44	0.420
Concio 4 H=var	410	J[852]	ST SLU Mobili	MY-MIN	3	-202.68	-6440.43	33340.25	16899.82	78.33	0.387
Concio 4 H=var	411	I[852]	ST SLU Mobili	MY-MIN	3	-191.08	-6413.82	33340.25	16899.90	75.34	0.385
Concio 4 H=var	411	J[853]	ST SLU Mobili	MY-MIN	4	-228.94	-8752.34	36721.20	23173.54	76.03	0.384
Concio 4 H=200	412	I[853]	ST SLU Mobili	MY-MIN	4	-27.24	-8752.34	36721.20	23173.28	330.94	0.378
Concio 4 H=200	412	J[854]	ST SLU Mobili	MY-MIN	4	-29.26	-9383.65	36721.20	23132.45	330.94	0.406
Concio 4 H=200	413	I[854]	ST SLU Mobili	MY-MIN	4	-12.65	-9436.20	36721.20	23133.14	330.48	0.408
Concio 4 H=200	413	J[855]	ST SLU Mobili	MY-MIN	4	-15.01	-8815.26	36721.20	23171.51	330.48	0.381
Concio 4 H=var	414	I[855]	ST SLU Mobili	MY-MIN	4	-208.74	-8815.26	36721.20	23171.56	75.46	0.386
Concio 4 H=var	414	J[856]	ST SLU Mobili	MY-MIN	3	-141.81	-6507.79	33340.25	16878.64	74.77	0.390
Concio 4 H=var	415	I[856]	ST SLU Mobili	MY-MIN	3	-148.38	-6533.22	33340.25	16879.21	76.37	0.392

Concio 4 H=var	415	J[857]	ST SLU Mobili	MY-MIN	3	-130.83	-4775.47	29959.30	10698.99	76.48	0.451
Concio 5	416	I[857]	ST SLU Mobili	MY-MIN	1	-14.90	-4761.97	22659.14	10249.33	167.97	0.465
Concio 5	416	J[858]	ST SLU Mobili	MY-MIN	1	-14.38	-4468.52	22659.14	10249.33	167.97	0.437
Concio 5	417	I[858]	ST SLU Mobili	MY-MIN	1	-4.37	-4540.89	19837.42	9560.96	27.14	0.475
Concio 5	417	J[859]	ST SLU Termico	MY-MIN	1	31.88	-2329.74	19837.42	9525.65	26.60	0.246
Concio 5	418	I[859]	ST SLU Termico	MY-MIN	1	-130.08	-2355.39	22659.14	10249.33	48.46	0.236
Concio 5	418	J[860]	ST SLU Mobili	MY-MAX	1	-56.73	2723.78	56079.69	13386.70	109.33	0.205
Concio 6	419	I[860]	ST SLU Mobili	MY-MAX	1	-56.73	2723.46	52344.97	11874.49	64.34	0.230
Concio 6	419	J[861]	ST SLU Mobili	MY-MAX	1	-104.78	3152.72	52344.97	11874.49	64.34	0.268
Concio 6	420	I[861]	ST SLU Mobili	MY-MAX	1	-138.13	3065.38	53194.79	11988.30	68.49	0.258
Concio 6	420	J[862]	ST SLU Mobili	MY-MAX	1	-105.04	3598.55	53194.79	11988.30	68.49	0.302
Concio 6	421	I[862]	ST SLU Mobili	MY-MAX	1	-105.04	3598.55	53194.79	11994.11	68.49	0.302
Concio 6	421	J[863]	ST SLU Mobili	MY-MAX	1	-138.12	3065.54	53194.79	11994.11	68.49	0.258
Concio 6	422	I[863]	ST SLU Mobili	MY-MAX	1	-104.82	3152.38	52344.97	11862.80	64.35	0.268
Concio 6	422	J[864]	ST SLU Mobili	MY-MAX	1	-56.76	2722.73	52344.97	11862.80	64.35	0.231
Concio 5	423	I[864]	ST SLU Mobili	MY-MAX	1	-56.76	2723.07	56079.69	13386.70	109.32	0.204
Concio 5	423	J[865]	ST SLU Termico	MY-MIN	1	-130.04	-2355.23	22659.14	10249.33	48.46	0.236
Concio 5	424	I[865]	ST SLU Termico	MY-MIN	1	32.06	-2329.61	19837.42	9527.59	26.60	0.246
Concio 5	424	J[866]	ST SLU Mobili	MY-MIN	1	-4.22	-4541.81	19837.42	9562.43	27.14	0.475
Concio 5	425	I[866]	ST SLU Mobili	MY-MIN	1	-14.18	-4469.26	22659.14	10249.33	167.97	0.437
Concio 5	425	J[867]	ST SLU Mobili	MY-MIN	1	-14.70	-4762.66	22659.14	10249.33	167.97	0.465
Concio 4 H=var	426	I[867]	ST SLU Mobili	MY-MIN	3	-130.63	-4776.18	29959.30	10698.97	76.48	0.451
Concio 4 H=var	426	J[868]	ST SLU Mobili	MY-MIN	3	-148.18	-6534.10	33340.25	16879.19	76.37	0.392
Concio 4 H=var	427	I[868]	ST SLU Mobili	MY-MIN	3	-141.71	-6508.67	33340.25	16878.62	74.77	0.390
Concio 4 H=var	427	J[869]	ST SLU Mobili	MY-MIN	4	-208.67	-8816.10	36721.20	23171.55	75.46	0.386
Concio 4 H=200	428	I[869]	ST SLU Mobili	MY-MIN	4	-14.94	-8816.10	36721.20	23171.50	330.47	0.381
Concio 4 H=200	428	J[870]	ST SLU Mobili	MY-MIN	4	-12.58	-9436.80	36721.20	23133.13	330.47	0.408
Concio 4 H=200	429	I[870]	ST SLU Mobili	MY-MIN	4	-29.39	-9384.70	36721.20	23132.44	330.95	0.407
Concio 4 H=200	429	J[871]	ST SLU Mobili	MY-MIN	4	-27.36	-8753.23	36721.20	23173.27	330.95	0.379
Concio 4 H=var	430	I[871]	ST SLU Mobili	MY-MIN	4	-229.07	-8753.23	36721.20	23173.53	76.03	0.384
Concio 4 H=var	430	J[872]	ST SLU Mobili	MY-MIN	3	-191.19	-6414.14	33340.25	16899.88	75.34	0.385
Concio 4 H=var	431	I[872]	ST SLU Mobili	MY-MIN	3	-202.90	-6440.79	33340.25	16899.80	78.33	0.387
Concio 4 H=var	431	J[873]	ST SLU Mobili	MY-MIN	3	-185.52	-4444.82	29959.30	10750.36	78.44	0.420
Concio 3	432	I[873]	ST SLU Mobili	MY-MIN	1	-55.07	-4458.71	22659.14	10249.33	169.78	0.438
Concio 3	432	J[874]	ST SLU Mobili	MY-MIN	1	-55.17	-4110.29	22659.14	10249.33	169.78	0.404
Concio 3	433	I[874]	ST SLU Mobili	MY-MIN	1	-66.24	-4219.38	20044.03	9732.42	31.10	0.437
Concio 3	433	J[875]	ST SLU Mobili	MY-MAX	1	63.18	2425.47	47184.76	12510.21	116.69	0.195
Concio 3	434	I[875]	ST SLU Mobili	MY-MAX	1	-148.17	2171.82	57024.39	13386.70	115.43	0.165
Concio 3	434	J[876]	ST SLU Mobili	MY-MAX	1	-136.20	3521.45	57024.39	13386.70	115.43	0.265
Concio 2	435	I[876]	ST SLU Mobili	MY-MAX	1	-136.20	3520.97	52741.05	12266.55	65.60	0.290
Concio 2	435	J[877]	ST SLU Mobili	MY-MAX	1	-180.94	4293.48	52741.05	12266.55	65.60	0.353
Concio 2	436	I[877]	ST SLU Mobili	MY-MAX	1	-242.34	4185.89	46533.18	11018.78	38.64	0.385
Concio 2	436	J[878]	ST SLU Mobili	MY-MAX	1	-221.35	4604.33	46533.18	11018.78	38.64	0.423
Concio 2	437	I[878]	ST SLU Mobili	MY-MAX	1	-178.22	4669.42	55693.51	12635.70	86.72	0.373
Concio 2	437	J[879]	ST SLU Mobili	MY-MAX	1	-166.80	4587.17	55693.51	12635.70	86.72	0.366
Concio 1	438	I[879]	ST SLU Mobili	MY-MAX	1	-166.80	4586.11	51956.43	10455.35	46.70	0.442
Concio 1	438	J[880]	ST SLU Mobili	MY-MAX	1	-225.62	3229.59	51956.43	10455.35	46.70	0.313
Concio 1	439	I[880]	ST SLU Mobili	MY-MAX	1	-66.57	3513.38	46848.01	10030.61	45.54	0.352
Concio 1	439	J[881]	ST SLU Mobili	MY-MIN	3	-230.30	-864.67	15885.38	6135.43	25.51	0.155
Concio 1	440	I[881]	ST SLU Mobili	MY-MIN	3	0.00	-343.82	17963.00	6363.77	112.41	0.054
Concio 1	440	J[882]	ST SLV Long	FX-MIN	4	-1.91	-0.60	17963.00	6350.05	109.95	0.000

## 8.4 VERIFICA DELLE TENSIONI IN ESERCIZIO

Si effettuano le verifiche allo stato limite di esercizio (SLE) delle tensioni massime nell'acciaio delle travi, nel calcestruzzo della soletta e nelle armature della soletta.

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

- Elem: numero dell'elemento
- Position: nodo iniziale (I) o finale (J) dell'elemento
- Lcom: combinazione di carico più gravosa
- Type: tipo combinazione (caratteristica, frequente, quasi permanente)

Flange travi in acciaio:

- Sigma\_Ed\_ser: tensione assiale massima
- Tau\_Ed\_ser: tensione tangenziale massima
- SQRT(sigma^2+3 tau^2): tensione ideale
- ALW =  $f_{yk} / \gamma_{M,ser}$ : tensione limite



Soletta in calcestruzzo:

Sigma\_c: tensione assiale massima

k\*fck: tensione limite (comb. caratteristica:  $k_1 = 0.6$ ; quasi perm.:  $k_2 = 0.45$ )

Armatura soletta:

Sigma\_s: tensione assiale massima

k\*fsk: tensione limite (comb. caratteristica:  $k_3 = 0.8$ )

Elem property	Elem number	Position	Top and Bottom Flange of Structural Steel							Concrete Deck			Reinforcement in Deck				VERIFICATIO		
			Loom	Type	Sigma_Ed.ser (kN/m²)	ALW (kN/m²)	Tau_Ed.ser (kN/m²)	ALW (kN/m²)	SQRT(sigma <sup>2</sup> +3tau <sup>2</sup> ) (kN/m²)	ALW (kN/m²)	Loom	Type	Sigma_c (kN/m²)	k'fck (kN/m²)	Loom	Type		Sigma_s (kN/m²)	k'fsk (kN/m²)
Concio 1	101	J[19]	ST RARA Vento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Vento	Characteristic	0.00	19200	ST RARA Vento	Characteristic	0.00	360000	Verified
Concio 1	101	J[20]	ST RARA Mobili	Characteristic	28333.04	355000	25593.18	204959.35	46148.18	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	15932.16	360000	Verified
Concio 1	102	J[20]	ST RARA Mobili	Characteristic	54608.72	355000	69137.32	204959.35	131613.34	355000	ST RARA Mobili	Characteristic	1167.98	19200	ST RARA Mobili	Characteristic	161015.00	360000	Verified
Concio 1	102	J[21]	ST RARA Mobili	Characteristic	16923.138	355000	21361.14	204959.35	173228.72	355000	ST RARA Mobili	Characteristic	5972.84	19200	ST RARA Mobili	Characteristic	32844.51	360000	Verified
Concio 1	103	J[21]	ST RARA Mobili	Characteristic	15928.91	355000	40058.42	204959.35	173414.04	355000	ST RARA Mobili	Characteristic	5460.95	19200	ST RARA Mobili	Characteristic	30039.251	360000	Verified
Concio 1	103	J[22]	ST RARA Mobili	Characteristic	-219023.50	355000	10154.99	204959.35	219728.62	355000	ST RARA Mobili	Characteristic	6857.64	19200	ST RARA Mobili	Characteristic	-3722191	360000	Verified
Concio 2	104	J[22]	ST RARA Mobili	Characteristic	177538.79	355000	10264.18	204959.35	178426.69	355000	ST RARA Mobili	Characteristic	6366.92	19200	ST RARA Mobili	Characteristic	35105.061	360000	Verified
Concio 2	104	J[23]	ST RARA Mobili	Characteristic	184371.27	355000	7301.63	204959.35	184804.51	355000	ST RARA Mobili	Characteristic	6798.49	19200	ST RARA Mobili	Characteristic	-37665.68	360000	Verified
Concio 2	105	J[23]	ST RARA Mobili	Characteristic	182754.92	355000	11816.43	204959.35	183997.73	355000	ST RARA Mobili	Characteristic	6657.61	19200	ST RARA Mobili	Characteristic	36875.27	360000	Verified
Concio 2	105	J[24]	ST RARA Mobili	Characteristic	-180733.18	355000	36271.21	204959.35	172574.49	355000	ST RARA Mobili	Characteristic	6319.03	19200	ST RARA Mobili	Characteristic	-35164.24	360000	Verified
Concio 2	106	J[24]	ST RARA Mobili	Characteristic	-164131.70	355000	34658.90	204959.35	174765.36	355000	ST RARA Mobili	Characteristic	6403.12	19200	ST RARA Mobili	Characteristic	35978.72	360000	Verified
Concio 2	106	J[25]	ST RARA Mobili	Characteristic	-121302.52	355000	62685.56	204959.35	162796.63	355000	ST RARA Mobili	Characteristic	4745.52	19200	ST RARA Mobili	Characteristic	55152.88	360000	Verified
Concio 3	107	J[25]	ST RARA Mobili	Characteristic	-116754.52	355000	4704.17	204959.35	142346.81	355000	ST RARA Mobili	Characteristic	4720.93	19200	ST RARA Mobili	Characteristic	45687.60	360000	Verified
Concio 3	107	J[26]	ST RARA Mobili	Characteristic	54856.21	355000	61923.21	204959.35	25235.29	355000	ST RARA Mobili	Characteristic	3408.36	19200	ST RARA Mobili	Characteristic	67259.93	360000	Verified
Concio 3	108	J[26]	ST RARA Mobili	Characteristic	172988.83	355000	47863.70	204959.35	10454.38	355000	ST RARA Mobili	Characteristic	4044.01	19200	ST RARA Mobili	Characteristic	7920.53	360000	Verified
Concio 3	108	J[27]	ST RARA Mobili	Characteristic	186596.47	355000	53371.20	204959.35	208239.52	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	166043.30	360000	Verified
Concio 3	109	J[27]	ST RARA Mobili	Characteristic	186290.62	355000	47939.33	204959.35	203957.68	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	243125.63	360000	Verified
Concio 3	109	J[28]	ST RARA Mobili	Characteristic	208052.97	355000	50106.23	204959.35	225428.35	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	203378.61	360000	Verified
Concio 4 H=10	110	J[28]	ST RARA Mobili	Characteristic	166946.16	355000	50845.33	204959.35	188750.53	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	16798.62	360000	Verified
Concio 4 H=10	110	J[29]	ST RARA Mobili	Characteristic	163337.23	355000	39993.83	204959.35	177419.19	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	109260.31	360000	Verified
Concio 4 H=10	111	J[29]	ST RARA Mobili	Characteristic	163610.17	355000	37063.92	204959.35	175754.56	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	90272.97	360000	Verified
Concio 4 H=10	111	J[30]	ST RARA Mobili	Characteristic	166786.02	355000	37070.00	204959.35	178685.06	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	103805.02	360000	Verified
Concio 4 H=20	112	J[30]	ST RARA Mobili	Characteristic	165330.66	355000	36678.89	204959.35	177239.79	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	107309.32	360000	Verified
Concio 4 H=20	112	J[31]	ST RARA Mobili	Characteristic	178563.38	355000	38112.01	204959.35	190374.51	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	123220.18	360000	Verified
Concio 4 H=20	113	J[31]	ST RARA Mobili	Characteristic	179478.58	355000	40334.99	204959.35	182594.30	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	124061.57	360000	Verified
Concio 4 H=20	113	J[32]	ST RARA Mobili	Characteristic	166481.30	355000	39069.56	204959.35	179708.98	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	10245.62	360000	Verified
Concio 4 H=14	114	J[32]	ST RARA Mobili	Characteristic	167871.26	355000	38896.19	204959.35	180885.32	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	106832.34	360000	Verified
Concio 4 H=14	114	J[33]	ST RARA Mobili	Characteristic	167217.89	355000	37979.83	204959.35	179692.03	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	92311.39	360000	Verified
Concio 4 H=14	115	J[33]	ST RARA Mobili	Characteristic	166493.57	355000	38860.18	204959.35	179584.10	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	113670.55	360000	Verified
Concio 4 H=14	115	J[34]	ST RARA Mobili	Characteristic	182244.02	355000	48989.21	204959.35	201029.13	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	129681.25	360000	Verified
Concio 5	116	J[34]	ST RARA Mobili	Characteristic	-228671.30	355000	48418.47	204959.35	243564.98	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	231121.48	360000	Verified
Concio 5	116	J[35]	ST RARA Mobili	Characteristic	-209661.13	355000	46342.99	204959.35	224501.24	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	269730.71	360000	Verified
Concio 5	117	J[35]	ST RARA Mobili	Characteristic	-208583.04	355000	49456.52	204959.35	225487.75	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	188432.16	360000	Verified
Concio 5	117	J[36]	ST RARA Termico	Characteristic	65747.49	355000	22886.44	204959.35	76773.01	355000	ST RARA Mobili	Characteristic	3004.83	19200	ST RARA Mobili	Characteristic	106881.67	360000	Verified
Concio 5	118	J[36]	ST RARA Termico	Characteristic	67693.70	355000	24588.31	204959.35	79976.20	355000	ST RARA Mobili	Characteristic	2524.46	19200	ST RARA Mobili	Characteristic	98630.36	360000	Verified
Concio 5	118	J[37]	ST RARA Mobili	Characteristic	-81253.41	355000	21734.20	204959.35	69550.23	355000	ST RARA Mobili	Characteristic	3562.88	19200	ST RARA Mobili	Characteristic	81938.03	360000	Verified
Concio 6	119	J[37]	ST RARA Mobili	Characteristic	-86664.79	355000	36223.66	204959.35	106991.11	355000	ST RARA Mobili	Characteristic	3992.17	19200	ST RARA Mobili	Characteristic	98956.71	360000	Verified
Concio 6	119	J[38]	ST RARA Mobili	Characteristic	-118314.63	355000	28512.00	204959.35	11938.53	355000	ST RARA Mobili	Characteristic	4730.66	19200	ST RARA Mobili	Characteristic	95766.49	360000	Verified
Concio 6	120	J[38]	ST RARA Mobili	Characteristic	-11162.80	355000	5938.27	204959.35	11637.62	355000	ST RARA Mobili	Characteristic	4621.77	19200	ST RARA Mobili	Characteristic	86840.38	360000	Verified
Concio 6	120	J[39]	ST RARA Mobili	Characteristic	-128311.77	355000	3363.98	204959.35	128443.99	355000	ST RARA Mobili	Characteristic	4733.81	19200	ST RARA Mobili	Characteristic	79938.82	360000	Verified
Concio 6	121	J[39]	ST RARA Mobili	Characteristic	-128311.77	355000	3363.98	204959.35	128443.99	355000	ST RARA Mobili	Characteristic	4733.81	19200	ST RARA Mobili	Characteristic	79938.82	360000	Verified
Concio 6	121	J[40]	ST RARA Mobili	Characteristic	-11166.22	355000	32816.00	204959.35	124854.30	355000	ST RARA Mobili	Characteristic	4622.18	19200	ST RARA Mobili	Characteristic	86838.68	360000	Verified
Concio 6	122	J[40]	ST RARA Mobili	Characteristic	-118823.77	355000	30381.63	204959.35	125399.29	355000	ST RARA Mobili	Characteristic	4730.06	19200	ST RARA Mobili	Characteristic	95769.96	360000	Verified
Concio 6	122	J[41]	ST RARA Mobili	Characteristic	-86646.78	355000	63747.52	204959.35	10352.78	355000	ST RARA Mobili	Characteristic	3591.14	19200	ST RARA Mobili	Characteristic	98848.33	360000	Verified
Concio 5	123	J[41]	ST RARA Mobili	Characteristic	-81237.18	355000	39248.91	204959.35	104825.20	355000	ST RARA Mobili	Characteristic	3561.88	19200	ST RARA Mobili	Characteristic	81931.68	360000	Verified
Concio 5	123	J[42]	ST RARA Termico	Characteristic	67889.52	355000	24076.93	204959.35	79504.02	355000	ST RARA Mobili	Characteristic	2524.67	19200	ST RARA Mobili	Characteristic	98615.68	360000	Verified
Concio 5	124	J[42]	ST RARA Termico	Characteristic	65744.77	355000	20825.25	204959.35	74989.66	355000	ST RARA Mobili	Characteristic	3004.02	19200	ST RARA Mobili	Characteristic	106871.62	360000	Verified
Concio 5	124	J[43]	ST RARA Mobili	Characteristic	-208620.15	355000	48007.59	204959.35	224580.84	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	188460.36	360000	Verified
Concio 5	125	J[43]	ST RARA Mobili	Characteristic	-209706.69	355000	42473.01	204959.35	222235.83	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	262311.14	360000	Verified
Concio 5	125	J[44]	ST RARA Mobili	Characteristic	-228711.84	355000	44549.53	204959.35	241377.48	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	220809.79	360000	Verified
Concio 4 H=14	126	J[44]	ST RARA Mobili	Characteristic	182281.10	355000	45371.55	204959.35	198499.70	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	129798.97	360000	Verified
Concio 4 H=14	126	J[45]	ST RARA Mobili	Characteristic	166514.11	355000	36516.37	204959.35	178121.55	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	11734.21	360000	Verified
Concio 4 H=14	127	J[45]	ST RARA Mobili	Characteristic	167229.37	355000	33680.06	204959.35	177121.11	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	92925.56	360000	Verified
Concio 4 H=14	127	J[46]	ST RARA Mobili	Characteristic	167875.47	355000	35754.47	204959.35	178910.48	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	104450.02	360000	Verified
Concio 4 H=20	128	J[46]	ST RARA Mobili	Characteristic	166484.49	355000	35572.56	204959.35	17759.88	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	107658.59	360000	Verified
Concio 4 H=20	128	J[47]	ST RARA Mobili	Characteristic	179494.60	355000	36637.90	204959.35	190497.78	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	124106.64	360000	Verified
Concio 4 H=20	129	J[47]	ST RARA Mobili	Characteristic	178572.16	355000	41931.09	204959.35	192776.21	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	123314.72	360000	Verified
Concio 4 H=20	129	J[48]																	

Concio 2	135	IJ753	ST RARA Mobili	Characteristic	-12 13 24,6	355000	39479,92	204959,35	139257,70	355000	ST RARA Mobili	Characteristic	4746,41	19200	ST RARA Mobili	Characteristic	55167,71	360000	Verified
Concio 2	135	IJ754	ST RARA Mobili	Characteristic	-164 151,97	355000	11449,76	204959,35	16345,58	355000	ST RARA Mobili	Characteristic	6405,08	19200	ST RARA Mobili	Characteristic	35977,44	360000	Verified
Concio 2	136	IJ754	ST RARA Mobili	Characteristic	-160724,11	355000	13535,44	204959,35	162424,94	355000	ST RARA Mobili	Characteristic	6318,35	19200	ST RARA Mobili	Characteristic	35160,36	360000	Verified
Concio 2	136	IJ755	ST RARA Mobili	Characteristic	-182754,34	355000	34541,23	204959,35	192297,78	355000	ST RARA Mobili	Characteristic	6656,85	19200	ST RARA Mobili	Characteristic	36870,63	360000	Verified
Concio 2	137	IJ755	ST RARA Mobili	Characteristic	-184364,07	355000	15646,11	204959,35	186345,15	355000	ST RARA Mobili	Characteristic	7253,17	19200	ST RARA Mobili	Characteristic	40465,61	360000	Verified
Concio 2	137	IJ756	ST RARA Mobili	Characteristic	-177541,2	355000	33211,04	204959,35	186626,41	355000	ST RARA Mobili	Characteristic	6366,94	19200	ST RARA Mobili	Characteristic	3505,05	360000	Verified
Concio 1	138	IJ756	ST RARA Mobili	Characteristic	-219026,43	355000	32857,73	204959,35	226299,51	355000	ST RARA Mobili	Characteristic	6857,74	19200	ST RARA Mobili	Characteristic	37222,33	360000	Verified
Concio 1	138	IJ757	ST RARA Mobili	Characteristic	-18926,63	355000	6276,46	204959,35	192452,90	355000	ST RARA Mobili	Characteristic	5461,39	19200	ST RARA Mobili	Characteristic	30041,97	360000	Verified
Concio 1	139	IJ757	ST RARA Mobili	Characteristic	-169229,03	355000	45754,27	204959,35	186865,79	355000	ST RARA Mobili	Characteristic	5979,52	19200	ST RARA Mobili	Characteristic	32889,31	360000	Verified
Concio 1	139	IJ758	ST RARA Mobili	Characteristic	54560,68	355000	72247,43	204959,35	13653,62	355000	ST RARA Mobili	Characteristic	188,04	19200	ST RARA Mobili	Characteristic	160812,64	360000	Verified
Concio 1	140	IJ758	ST RARA Mobili	Characteristic	12832,22	355000	25593,17	204959,35	46148,64	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	15931,11	360000	Verified
Concio 1	140	IJ759	ST RARA Frenamento	Characteristic	0,00	355000	0,00	204959,35	0,00	355000	ST RARA Frenamento	Characteristic	0,00	19200	ST RARA Frenamento	Characteristic	0,00	360000	Verified
Concio 1	201	IJ760	ST RARA Frenamento	Characteristic	0,00	355000	0,00	204959,35	0,00	355000	ST RARA Frenamento	Characteristic	0,00	19200	ST RARA Frenamento	Characteristic	0,00	360000	Verified
Concio 1	201	IJ761	ST RARA Mobili	Characteristic	12619,58	355000	29223,82	204959,35	45474,81	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	1854,70	360000	Verified
Concio 1	202	IJ761	ST RARA Mobili	Characteristic	30996,40	355000	53473,87	204959,35	97542,33	355000	ST RARA Mobili	Characteristic	735,64	19200	ST RARA Mobili	Characteristic	12382,98	360000	Verified
Concio 1	202	IJ762	ST RARA Mobili	Characteristic	-165490,14	355000	22879,77	204959,35	170168,85	355000	ST RARA Mobili	Characteristic	5013,59	19200	ST RARA Mobili	Characteristic	27099,57	360000	Verified
Concio 1	203	IJ762	ST RARA Mobili	Characteristic	-158752,24	355000	30478,34	204959,35	164455,31	355000	ST RARA Mobili	Characteristic	4722,75	19200	ST RARA Mobili	Characteristic	25600,09	360000	Verified
Concio 1	203	IJ763	ST RARA Mobili	Characteristic	-204833,14	355000	11221,51	204959,35	205755,19	355000	ST RARA Mobili	Characteristic	5866,08	19200	ST RARA Mobili	Characteristic	16338,50	360000	Verified
Concio 2	204	IJ763	ST RARA Mobili	Characteristic	-166009,48	355000	11342,17	204959,35	167167,83	355000	ST RARA Mobili	Characteristic	5434,77	19200	ST RARA Mobili	Characteristic	29794,14	360000	Verified
Concio 2	204	IJ764	ST RARA Mobili	Characteristic	-176172,98	355000	3149,79	204959,35	176257,43	355000	ST RARA Mobili	Characteristic	5724,18	19200	ST RARA Mobili	Characteristic	31625,14	360000	Verified
Concio 2	205	IJ764	ST RARA Mobili	Characteristic	-177786,15	355000	13600,08	204959,35	179339,91	355000	ST RARA Mobili	Characteristic	6119,59	19200	ST RARA Mobili	Characteristic	33740,13	360000	Verified
Concio 2	205	IJ765	ST RARA Mobili	Characteristic	-157389,97	355000	19734,21	204959,35	161038,74	355000	ST RARA Mobili	Characteristic	5566,99	19200	ST RARA Mobili	Characteristic	30625,32	360000	Verified
Concio 2	206	IJ765	ST RARA Mobili	Characteristic	-158977,17	355000	40875,00	204959,35	174028,84	355000	ST RARA Mobili	Characteristic	5357,92	19200	ST RARA Mobili	Characteristic	33975,52	360000	Verified
Concio 2	206	IJ766	ST RARA Mobili	Characteristic	-114418,09	355000	57930,20	204959,35	159785,65	355000	ST RARA Mobili	Characteristic	4014,67	19200	ST RARA Mobili	Characteristic	54621,36	360000	Verified
Concio 3	207	IJ766	ST RARA Mobili	Characteristic	-109933,27	355000	43447,65	204959,35	133223,19	355000	ST RARA Mobili	Characteristic	3991,02	19200	ST RARA Mobili	Characteristic	45402,55	360000	Verified
Concio 3	207	IJ767	ST RARA Mobili	Characteristic	67378,46	355000	40127,90	204959,35	9750,56	355000	ST RARA Mobili	Characteristic	3122,96	19200	ST RARA Mobili	Characteristic	65363,03	360000	Verified
Concio 3	208	IJ767	ST RARA Mobili	Characteristic	75319,92	355000	49566,83	204959,35	114008,27	355000	ST RARA Mobili	Characteristic	3935,59	19200	ST RARA Mobili	Characteristic	73931,50	360000	Verified
Concio 3	208	IJ768	ST RARA Mobili	Characteristic	-177932,38	355000	100182,31	204959,35	198027,08	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	150639,25	360000	Verified
Concio 3	209	IJ768	ST RARA Mobili	Characteristic	-167272,51	355000	48666,88	204959,35	195800,94	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	218561,89	360000	Verified
Concio 3	209	IJ769	ST RARA Mobili	Characteristic	-197982,37	355000	50056,62	204959,35	216134,25	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	185445,19	360000	Verified
Concio 4 H-var	210	IJ769	ST RARA Mobili	Characteristic	158364,17	355000	51919,82	204959,35	216105,02	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	105183,02	360000	Verified
Concio 4 H-var	210	IJ770	ST RARA Mobili	Characteristic	155903,87	355000	39583,31	204959,35	170310,29	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	93532,78	360000	Verified
Concio 4 H-var	211	IJ770	ST RARA Mobili	Characteristic	156112,17	355000	39621,93	204959,35	170303,65	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	84210,27	360000	Verified
Concio 4 H-var	211	IJ771	ST RARA Mobili	Characteristic	160677,53	355000	34181,66	204959,35	172338,20	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	99035,59	360000	Verified
Concio 4 H:20	212	IJ771	ST RARA Mobili	Characteristic	159334,85	355000	34024,94	204959,35	169884,33	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	101722,97	360000	Verified
Concio 4 H:20	212	IJ772	ST RARA Mobili	Characteristic	174444,09	355000	38937,62	204959,35	184232,17	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	11559,33	360000	Verified
Concio 4 H:20	213	IJ772	ST RARA Mobili	Characteristic	17248,92	355000	41065,57	204959,35	185432,84	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	119225,11	360000	Verified
Concio 4 H:20	213	IJ773	ST RARA Mobili	Characteristic	159451,63	355000	39168,95	204959,35	173284,28	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	104297,25	360000	Verified
Concio 4 H-var	214	IJ773	ST RARA Mobili	Characteristic	160951,51	355000	38842,54	204959,35	174446,11	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	101626,59	360000	Verified
Concio 4 H-var	214	IJ774	ST RARA Mobili	Characteristic	161102,75	355000	39386,47	204959,35	174951,36	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	86682,98	360000	Verified
Concio 4 H-var	215	IJ774	ST RARA Mobili	Characteristic	160824,69	355000	37574,96	204959,35	173494,14	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	99600,79	360000	Verified
Concio 4 H-var	215	IJ775	ST RARA Mobili	Characteristic	174924,36	355000	48192,54	204959,35	193612,19	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	117325,29	360000	Verified
Concio 5	216	IJ775	ST RARA Mobili	Characteristic	219911,21	355000	47434,98	204959,35	234757,69	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	213548,26	360000	Verified
Concio 5	216	IJ776	ST RARA Mobili	Characteristic	200664,54	355000	46039,63	204959,35	215928,69	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	244566,98	360000	Verified
Concio 5	217	IJ776	ST RARA Mobili	Characteristic	202552,33	355000	46243,39	204959,35	217813,68	355000	ST RARA Mobili	Characteristic	0,00	19200	ST RARA Mobili	Characteristic	167437,87	360000	Verified
Concio 5	217	IJ777	ST RARA Termico	Characteristic	61015,92	355000	23511,36	204959,35	73357,31	355000	ST RARA Mobili	Characteristic	2322,32	19200	ST RARA Mobili	Characteristic	106594,07	360000	Verified
Concio 5	218	IJ777	ST RARA Termico	Characteristic	62862,70	355000	22973,13	204959,35	74397,66	355000	ST RARA Mobili	Characteristic	2014,76	19200	ST RARA Mobili	Characteristic	96837,20	360000	Verified
Concio 5	218	IJ778	ST RARA Mobili	Characteristic	733412,20	355000	19204,47	204959,35	80531,78	355000	ST RARA Mobili	Characteristic	2743,59	19200	ST RARA Mobili	Characteristic	80595,50	360000	Verified
Concio 6	219	IJ778	ST RARA Mobili	Characteristic	78075,50	355000	32007,45	204959,35	95756,01	355000	ST RARA Mobili	Characteristic	2769,26	19200	ST RARA Mobili	Characteristic	98311,02	360000	Verified
Concio 6	219	IJ779	ST RARA Mobili	Characteristic	109796,88	355000	19862,59	204959,35	115601,51	355000	ST RARA Mobili	Characteristic	3747,28	19200	ST RARA Mobili	Characteristic	93317,05	360000	Verified
Concio 6	220	IJ779	ST RARA Mobili	Characteristic	107999,45	355000	51916,55	204959,35	108373,15	355000	ST RARA Mobili	Characteristic	3946,01	19200	ST RARA Mobili	Characteristic	84575,46	360000	Verified
Concio 6	220	IJ780	ST RARA Mobili	Characteristic	115995,85	355000	2650,21	204959,35	116086,64	355000	ST RARA Mobili	Characteristic	3785,22	19200	ST RARA Mobili	Characteristic	78206,77	360000	Verified
Concio 6	221	IJ780	ST RARA Mobili	Characteristic	115995,85	355000	2650,21	204959,35	116086,64	355000	ST RARA Mobili	Characteristic	3785,22	19200	ST RARA Mobili	Characteristic	78206,77	360000	Verified
Concio 6	221	IJ781	ST RARA Mobili	Characteristic	108004,17	355000	12730,97	204959,35	110232,18	355000	ST RARA Mobili	Characteristic	3946,51	19200	ST RARA Mobili	Characteristic	84587,69	360000	Verified
Concio 6	222	IJ781	ST RARA Mobili	Characteristic	109789,35	355000	38133,64	204959,35	128125,82	355000	ST RARA Mobili	Characteristic	3746,58	19200	ST RARA Mobili	Characteristic	93332,35	360000	Verified
Concio 6	222	IJ782	ST RARA Mobili	Characteristic	780610,11	355000	59231,13	204959,35	128912,77	355000	ST RARA Mobili	Characteristic	2768,69	19200	ST RARA Mobili	Characteristic	98315,07	360000	Verified
Concio 5	223	IJ782	ST RARA Mobili	Characteristic	73327,83	355000	35538,68	204959,35	95739,04	355000	ST RARA Mobili	Characteristic	2743,03	19200	ST RARA Mobili	Characteristic	80598,10	360000	Verified
Concio 5	223	IJ783	ST RARA Termico	Characteristic	62862,05	355000	22645,00	204959,35	74094,71	355000	ST RARA Mobili	Characteristic	2015,97	19200	ST RARA Mobili	Characteristic	96835,35	360000	Verified
Concio 5	224	IJ783	ST RARA Termico	Characteristic	61024,09	355000	2185												



Concio 4 Hvak	230	I[789]	ST RARA Mobili	Characteristic	160679.14	355000	36934.86	204959.35	172946.05	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	100237.33	360000	Verified
Concio 4 Hvak	230	J[790]	ST RARA Mobili	Characteristic	156126.88	355000	43343.30	204959.35	173238.36	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	84233.70	360000	Verified
Concio 4 Hvak	231	I[791]	ST RARA Mobili	Characteristic	155906.13	355000	41444.29	204959.35	173638.02	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	93516.66	360000	Verified
Concio 4 Hvak	231	J[792]	ST RARA Mobili	Characteristic	158372.13	355000	54189.73	204959.35	184095.93	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	100416.68	360000	Verified
Concio 3	232	I[791]	ST RARA Mobili	Characteristic	-197985.93	355000	53384.08	204959.35	218513.18	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	186670.00	360000	Verified
Concio 3	232	J[792]	ST RARA Mobili	Characteristic	-176734.16	355000	51994.41	204959.35	198356.08	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	219137.00	360000	Verified
Concio 3	233	I[792]	ST RARA Mobili	Characteristic	-177930.22	355000	52005.50	204959.35	199431.42	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	150633.82	360000	Verified
Concio 3	233	J[793]	ST RARA Mobili	Characteristic	-75316.93	355000	31627.78	204959.35	93132.11	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	73828.93	360000	Verified
Concio 3	234	I[793]	ST RARA Mobili	Characteristic	-67859.31	355000	23007.03	204959.35	78694.70	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	65577.10	360000	Verified
Concio 3	234	J[794]	ST RARA Mobili	Characteristic	-109941.64	355000	26234.52	204959.35	119021.45	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	15402.26	360000	Verified
Concio 2	235	I[794]	ST RARA Mobili	Characteristic	-114166.85	355000	35099.36	204959.35	129335.54	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	54619.78	360000	Verified
Concio 2	235	J[795]	ST RARA Mobili	Characteristic	-158353.61	355000	15024.03	204959.35	160477.50	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	33979.49	360000	Verified
Concio 2	236	I[795]	ST RARA Mobili	Characteristic	-157390.14	355000	4318.95	204959.35	15767.81	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-30627.52	360000	Verified
Concio 2	236	J[796]	ST RARA Mobili	Characteristic	-177788.50	355000	1816.45	204959.35	177816.34	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-33738.03	360000	Verified
Concio 2	237	I[796]	ST RARA Mobili	Characteristic	-176918.47	355000	22129.24	204959.35	181022.80	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-31626.83	360000	Verified
Concio 2	237	J[797]	ST RARA Mobili	Characteristic	-166008.51	355000	34140.25	204959.35	176225.69	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-29793.63	360000	Verified
Concio 1	238	I[797]	ST RARA Mobili	Characteristic	-204834.05	355000	33777.03	204959.35	213025.00	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-31638.03	360000	Verified
Concio 1	238	J[798]	ST RARA Mobili	Characteristic	-155742.75	355000	53033.29	204959.35	180813.15	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-25682.66	360000	Verified
Concio 1	239	I[798]	ST RARA Mobili	Characteristic	-165485.02	355000	47120.56	204959.35	184516.49	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-27129.11	360000	Verified
Concio 1	239	J[799]	ST RARA Mobili	Characteristic	-30598.32	355000	56442.44	204959.35	102437.81	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	112376.71	360000	Verified
Concio 1	240	I[799]	ST RARA Mobili	Characteristic	12618.14	355000	25223.81	204959.35	45474.60	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	1854.70	360000	Verified
Concio 1	240	J[800]	ST RARA Vento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	0.00	360000	Verified
Concio 1	301	I[801]	ST RARA Frenamento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Frenamento	Characteristic	0.00	19200	ST RARA Frenamento	Characteristic	0.00	360000	Verified
Concio 1	301	J[802]	ST RARA Mobili	Characteristic	12618.88	355000	25223.82	204959.35	45474.81	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	1854.70	360000	Verified
Concio 1	302	I[802]	ST RARA Mobili	Characteristic	34087.57	355000	74535.41	204959.35	133523.57	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	112360.18	360000	Verified
Concio 1	302	J[803]	ST RARA Mobili	Characteristic	-165412.53	355000	22835.53	204959.35	170075.54	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-25744.99	360000	Verified
Concio 1	303	I[803]	ST RARA Mobili	Characteristic	-156633.49	355000	30452.36	204959.35	164328.40	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-25573.15	360000	Verified
Concio 1	303	J[804]	ST RARA Mobili	Characteristic	-204965.36	355000	11195.52	204959.35	205880.59	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-31312.61	360000	Verified
Concio 2	304	I[804]	ST RARA Mobili	Characteristic	-166082.21	355000	11519.90	204959.35	167234.71	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-29467.37	360000	Verified
Concio 2	304	J[805]	ST RARA Mobili	Characteristic	-176492.18	355000	3176.05	204959.35	176577.89	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-31910.61	360000	Verified
Concio 2	305	I[805]	ST RARA Mobili	Characteristic	-177867.10	355000	13604.70	204959.35	179421.21	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-33912.85	360000	Verified
Concio 2	305	J[806]	ST RARA Mobili	Characteristic	-157545.44	355000	19738.83	204959.35	161212.38	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	36374.12	360000	Verified
Concio 2	306	I[806]	ST RARA Mobili	Characteristic	-159156.98	355000	40911.19	204959.35	174218.60	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	-29675.02	360000	Verified
Concio 2	306	J[807]	ST RARA Mobili	Characteristic	-144224.96	355000	37966.39	204959.35	152077.69	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	59680.25	360000	Verified
Concio 3	307	I[807]	ST RARA Mobili	Characteristic	-100008.41	355000	43474.79	204959.35	133311.75	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	48862.32	360000	Verified
Concio 3	307	J[808]	ST RARA Mobili	Characteristic	-67809.02	355000	40155.04	204959.35	97355.70	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	66279.12	360000	Verified
Concio 3	308	I[808]	ST RARA Mobili	Characteristic	-75155.05	355000	49608.81	204959.35	114155.09	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	77153.89	360000	Verified
Concio 3	308	J[809]	ST RARA Mobili	Characteristic	-178055.50	355000	50140.33	204959.35	191053.83	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	148511.26	360000	Verified
Concio 3	309	I[809]	ST RARA Mobili	Characteristic	-178091.55	355000	48616.90	204959.35	196995.96	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	235351.31	360000	Verified
Concio 3	309	J[810]	ST RARA Mobili	Characteristic	-199182.61	355000	50006.64	204959.35	217813.34	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	201947.90	360000	Verified
Concio 4 Hvak	310	I[810]	ST RARA Mobili	Characteristic	-159069.48	355000	51234.57	204959.35	192148.41	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	110204.18	360000	Verified
Concio 4 Hvak	310	J[811]	ST RARA Mobili	Characteristic	-157048.87	355000	39528.08	204959.35	17323.54	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	8592.50	360000	Verified
Concio 4 Hvak	311	I[811]	ST RARA Mobili	Characteristic	-156213.06	355000	39553.04	204959.35	170575.05	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	80598.14	360000	Verified
Concio 4 Hvak	311	J[812]	ST RARA Mobili	Characteristic	-161130.95	355000	34130.70	204959.35	171633.03	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	96696.49	360000	Verified
Concio 4 H20	312	I[812]	ST RARA Mobili	Characteristic	-159787.16	355000	33992.29	204959.35	170289.06	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	100699.05	360000	Verified
Concio 4 H20	312	J[813]	ST RARA Mobili	Characteristic	-171022.18	355000	38904.97	204959.35	183818.87	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	144989.78	360000	Verified
Concio 4 H20	313	I[813]	ST RARA Mobili	Characteristic	-170549.65	355000	41097.99	204959.35	184808.86	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	110809.85	360000	Verified
Concio 4 H20	313	J[814]	ST RARA Mobili	Characteristic	-159107.76	355000	39201.37	204959.35	172989.94	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	95726.83	360000	Verified
Concio 4 Hvak	314	I[814]	ST RARA Mobili	Characteristic	-160529.27	355000	38894.29	204959.35	174091.77	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	94307.26	360000	Verified
Concio 4 Hvak	314	J[815]	ST RARA Mobili	Characteristic	-161285.70	355000	39456.44	204959.35	175167.09	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	83558.68	360000	Verified
Concio 4 Hvak	315	I[815]	ST RARA Mobili	Characteristic	-162196.11	355000	37633.47	204959.35	174803.93	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	91601.93	360000	Verified
Concio 4 Hvak	315	J[816]	ST RARA Mobili	Characteristic	-175541.74	355000	48282.85	204959.35	194444.34	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	123879.33	360000	Verified
Concio 5	316	I[816]	ST RARA Mobili	Characteristic	-221348.56	355000	47484.80	204959.35	236134.71	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	224939.75	360000	Verified
Concio 5	316	J[817]	ST RARA Mobili	Characteristic	-202033.76	355000	46089.45	204959.35	217233.40	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	258844.95	360000	Verified
Concio 5	317	I[817]	ST RARA Mobili	Characteristic	-202842.00	355000	46286.39	204959.35	218110.45	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	162244.63	360000	Verified
Concio 5	317	J[818]	ST RARA Termico	Characteristic	59323.92	355000	23554.36	204959.35	7998.28	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	98410.06	360000	Verified
Concio 5	318	I[818]	ST RARA Termico	Characteristic	62776.65	355000	22997.05	204959.35	74347.16	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	96671.42	360000	Verified
Concio 5	318	J[819]	ST RARA Termico	Characteristic	73435.29	355000	19180.56	204959.35	80600.38	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	84045.21	360000	Verified
Concio 6	319	I[819]	ST RARA Mobili	Characteristic	-78172.96	355000	31967.58	204959.35	95795.56	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	103348.57	360000	Verified
Concio 6	319	J[820]	ST RARA Mobili	Characteristic	-110071.91	355000	19822.72	204959.35	115311.55	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	89824		



Concio 6	420	J[861]	ST RARA Mobili	Characteristic	-11002.82	355000	5938.40	204959.35	11478.34	355000	ST RARA Mobili	Characteristic	4572.37	19200	ST RARA Mobili	Characteristic	92163.14	360000	Verified
Concio 6	420	J[862]	ST RARA Mobili	Characteristic	-28308.50	355000	3363.85	204959.35	128440.71	355000	ST RARA Mobili	Characteristic	4805.50	19200	ST RARA Mobili	Characteristic	87042.39	360000	Verified
Concio 6	421	J[862]	ST RARA Mobili	Characteristic	-28308.50	355000	3363.85	204959.35	128440.71	355000	ST RARA Mobili	Characteristic	4805.50	19200	ST RARA Mobili	Characteristic	87042.39	360000	Verified
Concio 6	421	J[863]	ST RARA Mobili	Characteristic	-11006.52	355000	32815.86	204959.35	124712.03	355000	ST RARA Mobili	Characteristic	4572.92	19200	ST RARA Mobili	Characteristic	92149.49	360000	Verified
Concio 6	422	J[863]	ST RARA Mobili	Characteristic	-13551.90	355000	30340.83	204959.35	125122.87	355000	ST RARA Mobili	Characteristic	4610.84	19200	ST RARA Mobili	Characteristic	88251.66	360000	Verified
Concio 6	422	J[864]	ST RARA Mobili	Characteristic	-86651.58	355000	63706.72	204959.35	103000.15	355000	ST RARA Mobili	Characteristic	3656.34	19200	ST RARA Mobili	Characteristic	106678.04	360000	Verified
Concio 5	423	J[864]	ST RARA Mobili	Characteristic	-81248.80	355000	38224.02	204959.35	104807.42	355000	ST RARA Mobili	Characteristic	3627.11	19200	ST RARA Mobili	Characteristic	86858.06	360000	Verified
Concio 5	423	J[865]	ST RARA Termico	Characteristic	67411.76	355000	24100.87	204959.35	79289.99	355000	ST RARA Mobili	Characteristic	2423.59	19200	ST RARA Mobili	Characteristic	99527.21	360000	Verified
Concio 5	424	J[865]	ST RARA Termico	Characteristic	64290.11	355000	20921.80	204959.35	73799.61	355000	ST RARA Mobili	Characteristic	2716.93	19200	ST RARA Mobili	Characteristic	100026.43	360000	Verified
Concio 5	424	J[866]	ST RARA Mobili	Characteristic	-21116.14	355000	48104.14	204959.35	227004.51	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	84356.42	360000	Verified
Concio 5	425	J[866]	ST RARA Mobili	Characteristic	-211497.93	355000	42595.68	204959.35	223996.25	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	283525.56	360000	Verified
Concio 5	425	J[867]	ST RARA Mobili	Characteristic	-230036.13	355000	44672.20	204959.35	242700.30	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	242490.41	360000	Verified
Concio 4 Hvar	426	J[867]	ST RARA Mobili	Characteristic	193451.15	355000	45474.57	204959.35	199645.02	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	136909.85	360000	Verified
Concio 4 Hvar	426	J[868]	ST RARA Mobili	Characteristic	167403.94	355000	36583.11	204959.35	178994.56	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	105249.50	360000	Verified
Concio 4 Hvar	427	J[868]	ST RARA Mobili	Characteristic	168554.04	355000	33752.95	204959.35	178404.74	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	100477.27	360000	Verified
Concio 4 Hvar	427	J[869]	ST RARA Mobili	Characteristic	166814.44	355000	35769.39	204959.35	177947.76	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	97694.00	360000	Verified
Concio 4 H-20	428	J[869]	ST RARA Mobili	Characteristic	165427.53	355000	35633.86	204959.35	176566.09	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	100837.41	360000	Verified
Concio 4 H-20	428	J[870]	ST RARA Mobili	Characteristic	177973.00	355000	36899.21	204959.35	189100.62	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	119413.00	360000	Verified
Concio 4 H-20	429	J[870]	ST RARA Mobili	Characteristic	177314.17	355000	41866.02	204959.35	191568.81	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	113143.45	360000	Verified
Concio 4 H-20	429	J[871]	ST RARA Mobili	Characteristic	164411.85	355000	40631.15	204959.35	178840.51	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	104599.54	360000	Verified
Concio 4 Hvar	430	J[871]	ST RARA Mobili	Characteristic	165864.01	355000	40417.71	204959.35	180032.34	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	99337.84	360000	Verified
Concio 4 Hvar	430	J[872]	ST RARA Mobili	Characteristic	164671.03	355000	41661.22	204959.35	179787.43	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	95045.94	360000	Verified
Concio 4 Hvar	431	J[872]	ST RARA Mobili	Characteristic	164031.43	355000	42661.66	204959.35	179906.54	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	100274.80	360000	Verified
Concio 4 Hvar	431	J[873]	ST RARA Mobili	Characteristic	167998.70	355000	54963.00	204959.35	193096.75	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	123330.44	360000	Verified
Concio 3	432	J[873]	ST RARA Mobili	Characteristic	-209223.71	355000	54960.97	204959.35	229576.26	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	221851.62	360000	Verified
Concio 3	432	J[874]	ST RARA Mobili	Characteristic	-187855.35	355000	52394.08	204959.35	208626.58	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	259915.75	360000	Verified
Concio 3	433	J[874]	ST RARA Mobili	Characteristic	-188705.74	355000	55605.11	204959.35	218621.31	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	170274.50	360000	Verified
Concio 3	433	J[875]	ST RARA Mobili	Characteristic	-72706.35	355000	29518.60	204959.35	88883.39	355000	ST RARA Mobili	Characteristic	3853.42	19200	ST RARA Mobili	Characteristic	69754.27	360000	Verified
Concio 3	434	J[875]	ST RARA Mobili	Characteristic	-64487.10	355000	44547.92	204959.35	100599.12	355000	ST RARA Mobili	Characteristic	3320.51	19200	ST RARA Mobili	Characteristic	70468.91	360000	Verified
Concio 3	434	J[876]	ST RARA Mobili	Characteristic	-167553.33	355000	29641.27	204959.35	127544.58	355000	ST RARA Mobili	Characteristic	4781.63	19200	ST RARA Mobili	Characteristic	50991.30	360000	Verified
Concio 2	435	J[876]	ST RARA Mobili	Characteristic	-121299.13	355000	39521.69	204959.35	139281.62	355000	ST RARA Mobili	Characteristic	4806.21	19200	ST RARA Mobili	Characteristic	62585.35	360000	Verified
Concio 2	435	J[877]	ST RARA Mobili	Characteristic	-163918.61	355000	11491.53	204959.35	155122.62	355000	ST RARA Mobili	Characteristic	6332.49	19200	ST RARA Mobili	Characteristic	35167.44	360000	Verified
Concio 2	436	J[877]	ST RARA Mobili	Characteristic	-160474.73	355000	13551.74	204959.35	182192.27	355000	ST RARA Mobili	Characteristic	6219.68	19200	ST RARA Mobili	Characteristic	41008.795	360000	Verified
Concio 2	436	J[878]	ST RARA Mobili	Characteristic	-192544.92	355000	34524.93	204959.35	192089.98	355000	ST RARA Mobili	Characteristic	6605.84	19200	ST RARA Mobili	Characteristic	36567.85	360000	Verified
Concio 2	437	J[878]	ST RARA Mobili	Characteristic	-163922.28	355000	15626.21	204959.35	185903.04	355000	ST RARA Mobili	Characteristic	6910.73	19200	ST RARA Mobili	Characteristic	38686.88	360000	Verified
Concio 2	437	J[879]	ST RARA Mobili	Characteristic	-177498.76	355000	33191.14	204959.35	186576.43	355000	ST RARA Mobili	Characteristic	6453.98	19200	ST RARA Mobili	Characteristic	35636.53	360000	Verified
Concio 1	438	J[879]	ST RARA Mobili	Characteristic	-218947.97	355000	32838.04	204959.35	226215.00	355000	ST RARA Mobili	Characteristic	6944.19	19200	ST RARA Mobili	Characteristic	37751.65	360000	Verified
Concio 1	438	J[880]	ST RARA Mobili	Characteristic	-158509.58	355000	62736.70	204959.35	192179.52	355000	ST RARA Mobili	Characteristic	6263.95	19200	ST RARA Mobili	Characteristic	28855.47	360000	Verified
Concio 1	439	J[880]	ST RARA Mobili	Characteristic	-168693.34	355000	45646.44	204959.35	186301.47	355000	ST RARA Mobili	Characteristic	5767.21	19200	ST RARA Mobili	Characteristic	31616.62	360000	Verified
Concio 1	439	J[881]	ST RARA Mobili	Characteristic	57485.95	355000	69227.08	204959.35	32972.94	355000	ST RARA Mobili	Characteristic	867.40	19200	ST RARA Mobili	Characteristic	159871.92	360000	Verified
Concio 1	440	J[881]	ST RARA Mobili	Characteristic	12803.61	355000	25893.17	204959.35	46140.69	355000	ST RARA Mobili	Characteristic	0.00	19200	ST RARA Mobili	Characteristic	15797.59	360000	Verified
Concio 1	440	J[882]	ST RARA Frenamento	Characteristic	0.00	355000	0.00	204959.35	0.00	355000	ST RARA Frenamento	Characteristic	0.00	19200	ST RARA Frenamento	Characteristic	0.00	360000	Verified



## 8.5 VERIFICHE A FATICA

Si effettuano le verifiche allo stato limite di fatica per vita illimitata dei profilati metallici e delle saldature anima/piattabanda.

L'impalcato si considera caricato secondo il modello di carico a fatica 2, applicato sulla corsia lenta; il veicolo più gravoso è risultato essere il 3°: semirimorchio a 5 assi per un peso complessivo di 630 kN (ved. 3.4.1).

Si assume che le strutture siano sensibili alla rottura per fatica e che essa produca conseguenze significative; il coefficiente parziale di sicurezza per le verifiche è quindi pari a:  $\gamma_{Mf} = 1.35$ .

Le caratteristiche resistenti (elastiche) tengono conto del contributo del calcestruzzo della soletta se compressa, se tesa tengono conto solamente dell'armatura.

La verifica viene eseguita per ogni tipologia di trave (concio) per:

- a) profilato metallico: tensioni normali:  $\Delta\sigma_c = 112 \text{ N/mm}^2$
- b) profilato metallico: tensioni tangenziali:  $\Delta\tau_c = 100 \text{ N/mm}^2$
- c) saldature: tensioni tangenziali:  $\Delta\tau_c = 80 \text{ N/mm}^2$

Seguono le verifiche eseguite per ogni tipologia di sezione all'interno della quale si sono individuate le condizioni:

- 1)  $\Delta M$  massimo e  $\Delta T$  associato (per verifica a)
- 2)  $\Delta T$  massimo e  $\Delta M$  associato (per verifiche b, c)

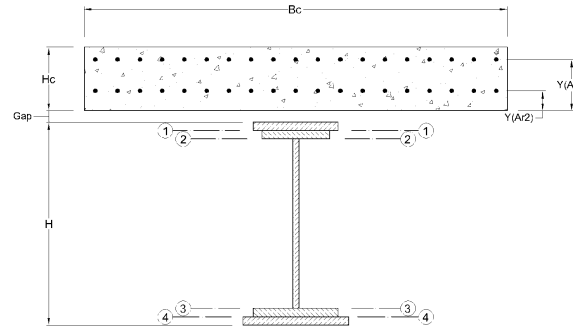
VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):

CONCI C1

Legenda

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in clc.)
- Hc 24 cm (altezza soletta in clc.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



Caratteristiche geometriche

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jc/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	471.0	1.5	46.91	875818	100.0	98.88	2678605	-2396436	27089	24421.6	1.097	CLS in compressione
Teso	471.0	1.5	46.91	875818	100.0	55.33	1159041	-25948	20947	11743.6	0.987	CLS in trazione

Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	94.0	1.5	141.0	48.88					ANIMA
sez1	0.0	0.0	0.0	-1.12	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	0.38	24421.2	1.097	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	3.0	180.0	97.38	17528.8	1.528	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	98.88	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Caratteristiche con sezione avente il CLS totalmente in trazione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	94.0	1.5	141.0	5.33					ANIMA
sez1	0.0	0.0	0.0	-44.67	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-43.17	10441.4	1.110	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	3.0	180.0	53.83	9689.7	1.196	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	55.33	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Sollecitazioni flettenti e taglianti associate (con segno)

TIPO DI COMBINAZIONE	M(max)	M(min)	V(max)	V(min)	Elemento
	kNm	kNm	kN	kN	n°
DM(max) - DT(ass)	2952.5	1901.0	-65.1	-67.0	203
DM(ass) - DT(max)	-13.6	-317.6	34.1	414.1	201

VERIFICA (tensioni normali) DEL PROFILATO METALLICO

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma_c$	$\Delta\sigma D/\gamma_{Mf,s}$	
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	109.0	70.2	38.82	112.0	61.14	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$ VERIFICATO

<-Tab. C.4.2.XIV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau_c$	$\Delta\tau D/\gamma_{Mf,s}$	
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	2.30	27.97	25.67	100.0	33.85	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$ VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	2.17	26.38	24.21	80.0	27.08	VERIFICATO
sez3	2.01	24.48	22.46	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<-Tab. C.4.2.XIIb (6)  $\Delta\tau_c=100$   
<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

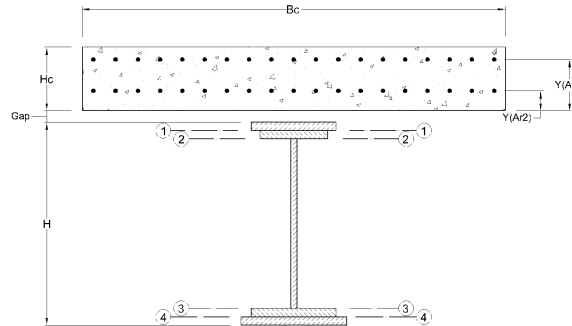
VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):

CONCI C2

Legenda

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in clc.)
- Hc 24 cm (altezza soletta in clc.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



Caratteristiche geometriche

STATO	As	Sp	Yg_inf(s)	Jc	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jc/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	529.5	1.5	42.11	973886	100.0	95.80	3193890	-761155	33338	28833.9	1.108	CLS in compressione
Teso	529.5	1.5	42.11	973886	100.0	50.22	1300762	-26131	25900	13175.6	0.987	CLS in trazione

Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	1.5	139.5	45.30					ANIMA
sez1	0.0	0.0	0.0	-4.20	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-2.70	28832.8	1.108	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	4.0	240.0	93.80	22512.9	1.419	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	95.80	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Caratteristiche con sezione avente il CLS totalmente in trazione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	1.5	139.5	-0.28					ANIMA
sez1	0.0	0.0	0.0	-49.78	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-48.28	11534.4	1.128	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	4.0	240.0	48.22	11573.2	1.124	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	50.22	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Sollecitazioni flettenti e taglianti associate (con segno)

TIPO DI COMBINAZIONE	M(max) kNm	M(min) kNm	V(max) kN	V(min) kN	Elemento
DM(max) - DT(ass)	1624.8	372.9	495.7	383.0	206
DM(ass) - DT(max)	2424.3	2638.7	-126.2	-434.1	236

VERIFICA (tensioni normali) DEL PROFILATO METALLICO

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma_c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	48.7	11.2	37.55	112.0	61.14	VERIFICATO

<-Tab. C.4.2.XIV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau_c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	-7.59	-26.13	18.53	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	-8.05	-27.71	19.66	80.0	27.08	VERIFICATO
sez3	-6.29	-21.64	15.35	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<-Tab. C.4.2.XIIb (6)  $\Delta\tau_c=100$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
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 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

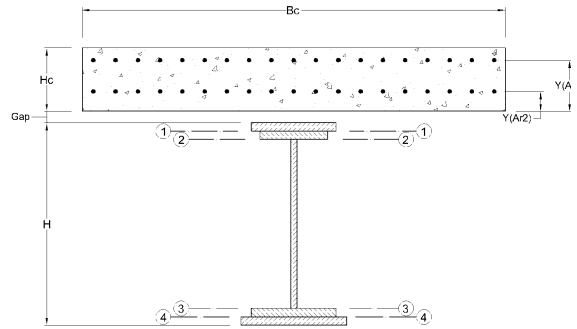
VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):

CONCI C3

Legenda

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i-esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\sigma$  e  $\Delta\sigma_c$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in clc.)
- Hc 24 cm (altezza soletta in clc.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



Caratteristiche geometriche

STATO	As	Sp	Yg_inf(s)	Jc	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jc/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	576.0	2.0	42.79	1010400	100.0	94.67	3320287	-623177	35071	30462.7	1.090	CLS in compressione
Teso	576.0	2.0	42.79	1010400	100.0	50.24	1334247	-26816	26555	13717.2	0.973	CLS in trazione

Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	93.0	2.0	186.0	44.17					ANIMA
sez1	0.0	0.0	0.0	-5.33	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-3.83	30457.3	1.090	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	4.0	240.0	92.67	22241.3	1.493	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	94.67	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Caratteristiche con sezione avente il CLS totalmente in trazione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	93.0	2.0	186.0	-0.26					ANIMA
sez1	0.0	0.0	0.0	-49.76	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-48.26	11531.1	1.157	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	4.0	240.0	48.24	11578.7	1.152	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	50.24	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Sollecitazioni flettenti e taglianti associate (con segno)

TIPO DI COMBINAZIONE	M(max) kNm	M(min) kNm	V(max) kN	V(min) kN	Elemento
DM(max) - DT(ass)	1623.9	371.7	495.7	383.0	207
DM(ass) - DT(max)	-2571.0	-2488.6	-636.4	-984.4	233

VERIFICA (tensioni normali) DEL PROFILATO METALLICO

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma_c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
sez.max	46.3	10.6	35.70	112.0	61.14	VERIFICATO

<-Tab. C.4.2.XIV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau_c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
sez.max	-32.71	-50.60	17.89	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	-38.89	-60.16	21.27	80.0	27.08	VERIFICATO
sez3	-39.05	-60.41	21.36	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<-Tab. C.4.2.XIIb (6)  $\Delta\tau_c=100$

<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

<-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

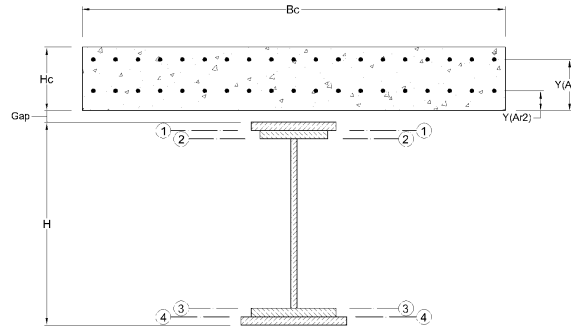
VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):

CONCI C4

Legenda

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in clc.)
- Hc 24 cm (altezza soletta in clc.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 100.48 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 100.48 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



Caratteristiche geometriche

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jc/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	904.0	2.0	95.66	6157420	200.0	170.33	14481460	-488151	85018	74800.6	1.936	CLS in compressione
Teso	904.0	2.0	95.66	6157420	200.0	117.73	8594389	-104463	73002	45338.0	1.896	CLS in trazione

Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	192.0	2.0	384.0	70.33					ANIMA
sez1	0.0	0.0	0.0	-29.67	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	60.0	4.0	240.0	-27.67	74141.8	1.953	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	70.0	4.0	280.0	168.33	47133.5	3.072	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	170.33	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Caratteristiche con sezione avente il CLS totalmente in trazione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	192.0	2.0	384.0	17.73					ANIMA
sez1	0.0	0.0	0.0	-82.27	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	60.0	4.0	240.0	-80.27	39211.5	2.192	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	70.0	4.0	280.0	115.73	32403.9	2.652	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	117.73	0.0		C.ANG.	0.0	PIATTO RINFORZO INF.

Sollecitazioni flettenti e taglianti associate (con segno)

TIPO DI COMBINAZIONE	M(max) kNm	M(min) kNm	V(max) kN	V(min) kN	Elemento
DM(max) - DT(ass)	-7871.7	-8931.9	-879.1	-1139.0	213
DM(ass) - DT(max)	-8498.5	-7852.3	1275.7	876.9	228

VERIFICA (tensioni normali) DEL PROFILATO METALLICO

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma_c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	-107.8	-122.4	14.52	112.0	61.14	VERIFICATO

<-Tab. C.4.2.XIV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau_c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	33.65	23.13	10.52	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	41.16	28.29	12.86	80.0	27.08	VERIFICATO
sez3	34.01	23.38	10.63	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<-Tab. C.4.2.XIIb (6)  $\Delta\tau_c=100$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
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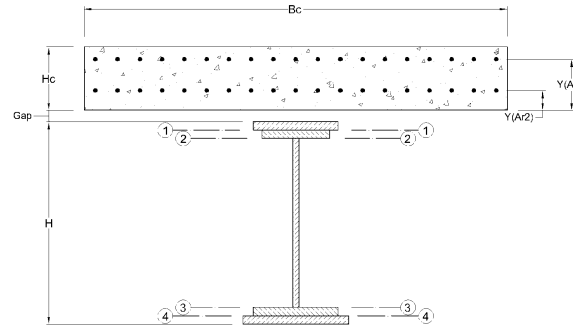
VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):

CONCI C5

Legenda

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in clc.)
- Hc 24 cm (altezza soletta in clc.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



Caratteristiche geometriche

STATO	As	Sp	Yg_inf(s)	Jc	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jc/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	576.0	2.0	42.79	1010407	100.0	94.67	3320294	-623178	35072	30462.7	1.090	CLS in compressione
Teso	576.0	2.0	42.79	1010407	100.0	50.24	1334254	-26816	26555	13717.2	0.973	CLS in trazione

Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	93.0	2.0	186.0	44.17					ANIMA
sez1	0.0	0.0	0.0	-5.33	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-3.83	30457.3	1.090	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	4.0	240.0	92.67	22241.3	1.493	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	94.67	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

Caratteristiche con sezione avente il CLS totalmente in trazione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
sez.max.	93.0	2.0	186.0	-0.26					ANIMA
sez1	0.0	0.0	0.0	-49.76	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-48.26	11531.1	1.157	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	4.0	240.0	48.24	11578.7	1.152	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	50.24	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

Sollecitazioni flettenti e taglianti associate (con segno)

TIPO DI COMBINAZIONE	M(max) kNm	M(min) kNm	V(max) kN	V(min) kN	Elemento
DM(max) - DT(ass)	551.4	-616.8	374.9	268.1	223
DM(ass) - DT(max)	-3128.3	-3084.9	-546.0	-896.5	217

VERIFICA (tensioni normali) DEL PROFILATO METALLICO

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma_c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
sez_max	15.7	-23.2	38.95	112.0	61.14	VERIFICATO

<-Tab. C.4.2.XIV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau_c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
sez_max	28.07	-46.08	18.02	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	-33.37	-54.79	21.42	80.0	27.08	VERIFICATO
sez3	-33.50	-55.01	21.51	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<-Tab. C.4.2.XIIb (6)  $\Delta\tau_c=100$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$



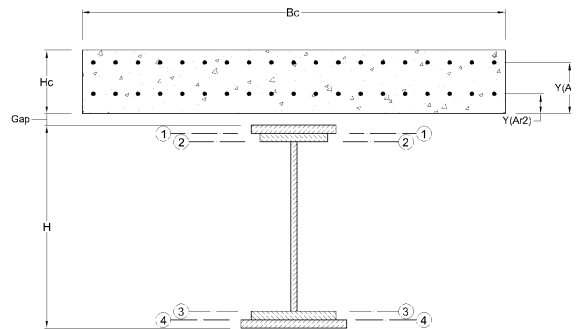
VERIFICA ALLO STATO LIMITE DI FATICA (VERIFICA PER VITA ILLIMITATA):

CONCI C6

Legenda

- As Area profilo metallico
- Yg\_inf(s) Baricentro profilo metallico
- H Altezza profilo metallico
- Yg\_inf Baricentro della sezione composta
- Jc Momento d'inerzia della sezione composta
- Sc\_max Momento statico massimo della sezione del profilo
- Sp Spessore anima in corrispondenza della sezione con momento statico massimo Sc\_max
- b Larghezza dei piatti (piattabande, anime, ecc.)
- t Spessore dei piatti (piattabande, anime, ecc.)
- A Sezione resistente dei piatti (piattabande, anime, ecc.)
- Z Distanza baricentro dei piatti (piattabande, anime, ecc.) dal baricentro della sezione composta
- S\_i Momento statico della sezione i\_esima di attacco dei piatti in esame (piattabande, anime, ecc.)
- $\Delta\tau$  e  $\Delta\sigma$  Delta di tensione tangenziale e normale degli elementi da verificare
- C.ANG Saldatura a cordone d'angolo
- a Larghezza della gola del cordone d'angolo (somma)
- P.PEN Saldatura a piena penetrazione (spessore resistente= spessore anima)
- $\gamma_{M,s}$  1.35 (coeff. di sicurezza a fatica)
- Es/Ec 6.30 (coeff. di omogeneizzazione acciaio/clc.)
- Bc 320 cm (base efficace soletta in cls.)
- Hc 24 cm (altezza soletta in cls.)
- Gap 6 cm (distanza tra profilo metallico e soletta)
- Ar1 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar1) 20 cm (distanza dal lembo inferiore della soletta dell'armatura Ar1)
- Ar2 32.16 cm<sup>2</sup> (area di armatura soletta)
- Y(Ar2) 2 cm (distanza dal lembo inferiore della soletta dell'armatura Ar2)
- Ar\_compr? SI (considerare o no le armature della soletta se in compressione)

SCHEMA SEZIONE COMPOSTA



Caratteristiche geometriche

STATO	As	Sp	Yg_inf(s)	Js	H	Yg_inf	Jc	Wsup	Winf	Sc_max	Jc/Sc_max	DESCRIZIONE
CLS	cm <sup>2</sup>	cm	cm	cm <sup>4</sup>	cm	cm	cm <sup>4</sup>	cm <sup>3</sup>	cm <sup>3</sup>	cm <sup>3</sup>	m	
Compr.	501.6	1.2	41.65	951707	100.0	96.51	3115259	-893577	32278	27818.6	1.120	CLS in compressione
Teso	501.6	1.2	41.65	951707	100.0	50.21	1280596	-25722	25503	12852.8	0.996	CLS in trazione

Caratteristiche con sezione avente il CLS totalmente o parzialmente in compressione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	1.2	111.6	46.01					ANIMA
sez1	0.0	0.0	0.0	-3.49	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-1.99	27818.4	1.120	C.ANG.	14.1	ANIMA-PIATTABANDA SUP. (2 SALD.10x10 mm)
sez3	60.0	4.0	240.0	94.51	22683.3	1.373	C.ANG.	14.1	ANIMA-PIATTABANDA INF. (2 SALD.10x10 mm)
sez4	0.0	0.0	0.0	96.51	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

Caratteristiche con sezione avente il CLS totalmente in trazione

POSIZ.	b	t	A	Z	S_i	Jc/S_i	TIPO SALDAT.	a	DESCRIZIONE
	cm	cm	cm <sup>2</sup>	cm	cm <sup>3</sup>	m		mm	
sez.max.	93.0	1.2	111.6	-0.29					ANIMA
sez1	0.0	0.0	0.0	-49.79	0.0		C.ANG.	0.0	PIATTO RINFORZO SUP.
sez2	50.0	3.0	150.0	-48.29	11539.4	1.110	C.ANG.	14.1	ANIMA-PIATTABANDA SUP.
sez3	60.0	4.0	240.0	48.21	11571.3	1.107	C.ANG.	14.1	ANIMA-PIATTABANDA INF.
sez4	0.0	0.0	0.0	50.21	0.0		C.ANG.	0.0	PIATTO RINFORZO INF

Sollecitazioni flettenti e taglianti associate (con segno)

TIPO DI COMBINAZIONE	M(max) kNm	M(min) kNm	V(max) kN	V(min) kN	Elemento n°
DM(max) - DT(ass)	553.7	-613.7	374.9	268.1	222
DM(ass) - DT(max)	992.4	1220.7	-16.1	-331.4	220

VERIFICA (tensioni normali) DEL PROFILATO METALLICO

POSIZ.	$\sigma$ inf(+)	$\sigma$ inf(-)	$\Delta\sigma$	$\Delta\sigma_c$	$\Delta\sigma D/\gamma_{Mf,s}$	$\Delta\sigma < \Delta\sigma D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	17.2	-24.1	41.22	112.0	61.14	VERIFICATO

<-Tab. C.4.2.XIV (3)  $\Delta\sigma_c=112$ ; Tab. C.4.2.XV (2)  $\Delta\sigma_c=112$

VERIFICA (tensioni tangenziali) DEL PROFILATO E DEI COLLEGAMENTI SALDATI

POSIZ.	$\tau$ (+)	$\tau$ (-)	$\Delta\tau$	$\Delta\tau_c$	$\Delta\tau D/\gamma_{Mf,s}$	$\Delta\tau < \Delta\tau D/\gamma_{Mf,s}$
	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	
sez. max	-1.20	-24.66	23.46	100.0	33.85	VERIFICATO
sez1	0.00	0.00	0.00	80.0	27.08	VERIFICATO
sez2	-1.02	-20.93	19.91	80.0	27.08	VERIFICATO
sez3	-0.83	-17.06	16.23	80.0	27.08	VERIFICATO
sez4	0.00	0.00	0.00	80.0	27.08	VERIFICATO

<-Tab. C.4.2.XIIIb (6)  $\Delta\tau_c=100$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$   
 <-Tab. C.4.2.XVIIb (8)  $\Delta\tau_c=80$

## 9 VERIFICHE DEI CONNETTORI "NELSON"

Dati connettori:

numero per fila:	3 (C2, C6); 4 (C1, C3, C4, C5)
interasse file:	0.20 m
diametro:	22 mm (7/8")
altezza:	175 mm
resistenza a rottura:	450 N/mm <sup>2</sup>

### 9.1 STATO LIMITE ULTIMO – RESISTENZA AL TAGLIO LONGITUDINALE

Si effettuano le verifiche allo stato limite ultimo per taglio longitudinale dei connettori trave-soletta. I coefficienti parziali per SLU assunti sono i seguenti:

$$\gamma_V = 1.25 : \text{connettori: resistenza allo SLU}$$

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose. Dati tabulati:

Elem property:	nome delle caratteristiche geometriche dell'elemento
Elem:	numero dell'elemento
Position:	nodo iniziale (I) o finale (J) dell'elemento
Lcom:	combinazione di carico più gravosa
Type:	sollecitazione (massima o minima)
V <sub>L,Ed</sub> :	sforzo di taglio longitudinale agente sulla larghezza della regione inelastica
v <sub>L,Ed</sub> :	sforzo di taglio longitudinale di calcolo per unità di lunghezza (m) all'interfaccia trave-soletta
P <sub>Rd</sub> :	resistenza al taglio del singolo connettore "Nelson"
v <sub>L,Rd</sub> :	resistenza al taglio longitudinale per trave e per unità di lunghezza
$v_{Ed} = \frac{v_{L,Ed}}{2 \times t_c}$	(t <sub>c</sub> = spessore della soletta)

Condizione di verifica: Verification Ratio:  $\frac{v_{L,Ed}}{v_{L,Rd}} \leq 1$

Elem property	Elem number	Position	Lcom	Type	V <sub>L,Ed</sub> (kN)	v <sub>L,Ed</sub> (kN/m)	P <sub>Rd</sub> (kN)	v <sub>L,Rd</sub> (kN/m)	Verif. Ratio
Concio 1	101	I[719]	ST SLV Vert	FX-MAX	1.95	1.77	111.48	2229.56	0.001
Concio 1	101	J[720]	ST SLU Mobili	FZ-MAX	454.57	412.07	111.48	2229.56	0.185
Concio 1	102	I[720]	ST SLU Mobili	FZ-MIN	-1359.96	1232.82	111.48	2229.56	0.553
Concio 1	102	J[721]	ST SLU Mobili	FZ-MIN	-610.91	553.79	111.48	2229.56	0.248
Concio 1	103	I[721]	ST SLU Mobili	FZ-MIN	-976.25	884.98	111.48	2229.56	0.397
Concio 1	103	J[722]	ST SLU Mobili	FZ-MIN	-519.15	470.62	111.48	2229.56	0.211
Concio 2	104	I[722]	ST SLU Mobili	FZ-MIN	-519.15	459.95	111.48	1672.17	0.275
Concio 2	104	J[723]	ST SLU Mobili	FZ-MAX	581.58	515.26	111.48	1672.17	0.308
Concio 2	105	I[723]	ST SLU Mobili	FZ-MIN	-655.53	580.78	111.48	1672.17	0.347
Concio 2	105	J[724]	ST SLU Mobili	FZ-MAX	860.04	761.97	111.48	1672.17	0.456
Concio 2	106	I[724]	ST SLU Mobili	FZ-MAX	469.38	415.86	111.48	1672.17	0.249
Concio 2	106	J[725]	ST SLU Mobili	FZ-MAX	875.47	775.64	111.48	1672.17	0.464
Concio 3	107	I[725]	ST SLU Mobili	FZ-MAX	875.47	784.50	111.48	2229.56	0.352
Concio 3	107	J[726]	ST SLU Mobili	FZ-MAX	1168.60	1047.16	111.48	2229.56	0.470
Concio 3	108	I[726]	ST SLU Mobili	FZ-MAX	791.12	708.91	111.48	2229.56	0.318
Concio 3	108	J[727]	ST SLU Mobili	FZ-MAX	1476.21	1322.81	111.48	2229.56	0.593

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Concio 3	109	I[727]	ST SLU Mobili	FZ-MAX	1151.31	1031.67	111.48	2229.56	0.463
Concio 3	109	J[728]	ST SLU Mobili	FZ-MAX	1233.79	1105.58	111.48	2229.56	0.496
Concio 4 H=var	110	I[728]	ST SLU Mobili	FZ-MAX	1222.33	1069.33	111.48	2229.56	0.480
Concio 4 H=var	110	J[729]	ST SLU Mobili	FZ-MAX	1610.08	971.14	111.48	2229.56	0.436
Concio 4 H=var	111	I[729]	ST SLU Mobili	FZ-MAX	1368.07	825.16	111.48	2229.56	0.370
Concio 4 H=var	111	J[730]	ST SLU Mobili	FZ-MAX	1739.82	798.20	111.48	2229.56	0.358
Concio 4 H=200	112	I[730]	ST SLU Mobili	FZ-MAX	1742.14	799.26	111.48	2229.56	0.358
Concio 4 H=200	112	J[731]	ST SLU Mobili	FZ-MAX	1817.20	833.70	111.48	2229.56	0.374
Concio 4 H=200	113	I[731]	ST SLU Mobili	FZ-MIN	-1732.95	795.04	111.48	2229.56	0.357
Concio 4 H=200	113	J[732]	ST SLU Mobili	FZ-MIN	-1657.84	760.59	111.48	2229.56	0.341
Concio 4 H=var	114	I[732]	ST SLU Mobili	FZ-MIN	-1657.12	760.25	111.48	2229.56	0.341
Concio 4 H=var	114	J[733]	ST SLU Mobili	FZ-MIN	-1284.80	774.94	111.48	2229.56	0.348
Concio 4 H=var	115	I[733]	ST SLU Mobili	FZ-MIN	-1523.31	918.80	111.48	2229.56	0.412
Concio 4 H=var	115	J[734]	ST SLU Mobili	FZ-MIN	-1135.69	993.53	111.48	2229.56	0.446
Concio 5	116	I[734]	ST SLU Mobili	FZ-MIN	-1145.39	1026.37	111.48	2229.56	0.460
Concio 5	116	J[735]	ST SLU Mobili	FZ-MIN	-1062.92	952.47	111.48	2229.56	0.427
Concio 5	117	I[735]	ST SLU Mobili	FZ-MIN	-1389.77	1245.35	111.48	2229.56	0.559
Concio 5	117	J[736]	ST SLU Mobili	FZ-MIN	-686.77	615.41	111.48	2229.56	0.276
Concio 5	118	I[736]	ST SLU Mobili	FZ-MIN	-1076.31	964.46	111.48	2229.56	0.433
Concio 5	118	J[737]	ST SLU Mobili	FZ-MIN	-745.41	667.95	111.48	2229.56	0.300
Concio 6	119	I[737]	ST SLU Mobili	FZ-MIN	-745.41	655.19	111.48	1672.17	0.392
Concio 6	119	J[738]	ST SLU Mobili	FZ-MAX	477.56	419.75	111.48	1672.17	0.251
Concio 6	120	I[738]	ST SLU Mobili	FZ-MIN	-759.17	667.28	111.48	1672.17	0.399
Concio 6	120	J[739]	ST SLU Mobili	FZ-MIN	-380.57	334.50	111.48	1672.17	0.200
Concio 6	121	I[739]	ST SLU Mobili	FZ-MIN	-380.57	334.50	111.48	1672.17	0.200
Concio 6	121	J[740]	ST SLU Mobili	FZ-MAX	758.99	667.12	111.48	1672.17	0.399
Concio 6	122	I[740]	ST SLU Mobili	FZ-MIN	-477.80	419.97	111.48	1672.17	0.251
Concio 6	122	J[741]	ST SLU Mobili	FZ-MAX	745.27	655.06	111.48	1672.17	0.392
Concio 5	123	I[741]	ST SLU Mobili	FZ-MAX	745.27	667.82	111.48	2229.56	0.300
Concio 5	123	J[742]	ST SLU Mobili	FZ-MAX	1076.20	964.36	111.48	2229.56	0.433
Concio 5	124	I[742]	ST SLU Mobili	FZ-MAX	686.73	615.37	111.48	2229.56	0.276
Concio 5	124	J[743]	ST SLU Mobili	FZ-MAX	1389.55	1245.15	111.48	2229.56	0.558
Concio 5	125	I[743]	ST SLU Mobili	FZ-MAX	1062.79	952.35	111.48	2229.56	0.427
Concio 5	125	J[744]	ST SLU Mobili	FZ-MAX	1145.26	1026.25	111.48	2229.56	0.460
Concio 4 H=var	126	I[744]	ST SLU Mobili	FZ-MAX	1135.54	993.40	111.48	2229.56	0.446
Concio 4 H=var	126	J[745]	ST SLU Mobili	FZ-MAX	1523.13	918.69	111.48	2229.56	0.412
Concio 4 H=var	127	I[745]	ST SLU Mobili	FZ-MAX	1284.47	774.74	111.48	2229.56	0.347
Concio 4 H=var	127	J[746]	ST SLU Mobili	FZ-MAX	1656.73	760.08	111.48	2229.56	0.341
Concio 4 H=200	128	I[746]	ST SLU Mobili	FZ-MAX	1657.44	760.40	111.48	2229.56	0.341
Concio 4 H=200	128	J[747]	ST SLU Mobili	FZ-MAX	1732.54	794.86	111.48	2229.56	0.357
Concio 4 H=200	129	I[747]	ST SLU Mobili	FZ-MIN	-1817.75	833.95	111.48	2229.56	0.374
Concio 4 H=200	129	J[748]	ST SLU Mobili	FZ-MIN	-1742.69	799.51	111.48	2229.56	0.359
Concio 4 H=var	130	I[748]	ST SLU Mobili	FZ-MIN	-1740.34	798.44	111.48	2229.56	0.358
Concio 4 H=var	130	J[749]	ST SLU Mobili	FZ-MIN	-1368.53	825.44	111.48	2229.56	0.370
Concio 4 H=var	131	I[749]	ST SLU Mobili	FZ-MIN	-1610.36	971.30	111.48	2229.56	0.436
Concio 4 H=var	131	J[750]	ST SLU Mobili	FZ-MIN	-1222.57	1069.54	111.48	2229.56	0.480
Concio 3	132	I[750]	ST SLU Mobili	FZ-MIN	-1234.08	1105.84	111.48	2229.56	0.496
Concio 3	132	J[751]	ST SLU Mobili	FZ-MIN	-1151.60	1031.93	111.48	2229.56	0.463
Concio 3	133	I[751]	ST SLU Mobili	FZ-MIN	-1476.56	1323.12	111.48	2229.56	0.593
Concio 3	133	J[752]	ST SLU Mobili	FZ-MIN	-791.28	709.06	111.48	2229.56	0.318
Concio 3	134	I[752]	ST SLU Mobili	FZ-MIN	-1168.86	1047.39	111.48	2229.56	0.470
Concio 3	134	J[753]	ST SLU Mobili	FZ-MIN	-875.78	784.77	111.48	2229.56	0.352
Concio 2	135	I[753]	ST SLU Mobili	FZ-MIN	-875.78	775.92	111.48	1672.17	0.464
Concio 2	135	J[754]	ST SLU Mobili	FZ-MIN	-469.61	416.06	111.48	1672.17	0.249
Concio 2	136	I[754]	ST SLU Mobili	FZ-MIN	-860.43	762.32	111.48	1672.17	0.456
Concio 2	136	J[755]	ST SLU Mobili	FZ-MAX	655.10	580.40	111.48	1672.17	0.347
Concio 2	137	I[755]	ST SLU Mobili	FZ-MIN	-581.87	515.52	111.48	1672.17	0.308
Concio 2	137	J[756]	ST SLU Mobili	FZ-MAX	518.87	459.70	111.48	1672.17	0.275
Concio 1	138	I[756]	ST SLU Mobili	FZ-MAX	518.87	470.36	111.48	2229.56	0.211

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Concio 1	138	J[757]	ST SLU Mobili	FZ-MAX	975.91	884.68	111.48	2229.56	0.397
Concio 1	139	I[757]	ST SLU Mobili	FZ-MAX	610.54	553.46	111.48	2229.56	0.248
Concio 1	139	J[758]	ST SLU Mobili	FZ-MAX	1359.55	1232.44	111.48	2229.56	0.553
Concio 1	140	I[758]	ST SLU Mobili	FZ-MIN	-454.57	412.07	111.48	2229.56	0.185
Concio 1	140	J[759]	ST SLV Vert	FX-MAX	1.95	1.77	111.48	2229.56	0.001
Concio 1	201	I[760]	ST SLV Vert	FX-MAX	1.66	1.51	111.48	2229.56	0.001
Concio 1	201	J[761]	ST SLU Mobili	FZ-MAX	443.76	403.87	111.48	2229.56	0.181
Concio 1	202	I[761]	ST SLU Mobili	FZ-MIN	-1307.81	1190.23	111.48	2229.56	0.534
Concio 1	202	J[762]	ST SLU Mobili	FZ-MIN	-642.47	584.71	111.48	2229.56	0.262
Concio 1	203	I[762]	ST SLU Mobili	FZ-MIN	-909.54	827.77	111.48	2229.56	0.371
Concio 1	203	J[763]	ST SLU Mobili	FZ-MIN	-557.49	507.37	111.48	2229.56	0.228
Concio 2	204	I[763]	ST SLU Mobili	FZ-MIN	-557.49	496.03	111.48	1672.17	0.297
Concio 2	204	J[764]	ST SLU Mobili	FZ-MAX	623.74	554.97	111.48	1672.17	0.332
Concio 2	205	I[764]	ST SLU Mobili	FZ-MIN	-665.79	592.39	111.48	1672.17	0.354
Concio 2	205	J[765]	ST SLU Mobili	FZ-MAX	847.71	754.25	111.48	1672.17	0.451
Concio 2	206	I[765]	ST SLU Mobili	FZ-MAX	579.12	515.28	111.48	1672.17	0.308
Concio 2	206	J[766]	ST SLU Mobili	FZ-MAX	864.97	769.61	111.48	1672.17	0.460
Concio 3	207	I[766]	ST SLU Mobili	FZ-MAX	864.97	778.81	111.48	2229.56	0.349
Concio 3	207	J[767]	ST SLU Mobili	FZ-MAX	1092.11	983.32	111.48	2229.56	0.441
Concio 3	208	I[767]	ST SLU Mobili	FZ-MAX	824.19	742.09	111.48	2229.56	0.333
Concio 3	208	J[768]	ST SLU Mobili	FZ-MAX	1382.18	1244.49	111.48	2229.56	0.558
Concio 3	209	I[768]	ST SLU Mobili	FZ-MAX	1178.16	1060.80	111.48	2229.56	0.476
Concio 3	209	J[769]	ST SLU Mobili	FZ-MAX	1231.55	1108.87	111.48	2229.56	0.497
Concio 4 H=var	210	I[769]	ST SLU Mobili	FZ-MAX	1218.98	1074.36	111.48	2229.56	0.482
Concio 4 H=var	210	J[770]	ST SLU Mobili	FZ-MAX	1533.45	934.44	111.48	2229.56	0.419
Concio 4 H=var	211	I[770]	ST SLU Mobili	FZ-MAX	1418.38	864.32	111.48	2229.56	0.388
Concio 4 H=var	211	J[771]	ST SLU Mobili	FZ-MAX	1741.23	808.66	111.48	2229.56	0.363
Concio 4 H=200	212	I[771]	ST SLU Mobili	FZ-MAX	1737.84	807.08	111.48	2229.56	0.362
Concio 4 H=200	212	J[772]	ST SLU Mobili	FZ-MAX	1799.04	835.50	111.48	2229.56	0.375
Concio 4 H=200	213	I[772]	ST SLU Mobili	FZ-MIN	-1711.92	795.04	111.48	2229.56	0.357
Concio 4 H=200	213	J[773]	ST SLU Mobili	FZ-MIN	-1650.76	766.64	111.48	2229.56	0.344
Concio 4 H=var	214	I[773]	ST SLU Mobili	FZ-MIN	-1654.03	768.16	111.48	2229.56	0.345
Concio 4 H=var	214	J[774]	ST SLU Mobili	FZ-MIN	-1331.32	811.26	111.48	2229.56	0.364
Concio 4 H=var	215	I[774]	ST SLU Mobili	FZ-MIN	-1448.50	882.67	111.48	2229.56	0.396
Concio 4 H=var	215	J[775]	ST SLU Mobili	FZ-MIN	-1135.02	1000.35	111.48	2229.56	0.449
Concio 5	216	I[775]	ST SLU Mobili	FZ-MIN	-1147.83	1033.49	111.48	2229.56	0.464
Concio 5	216	J[776]	ST SLU Mobili	FZ-MIN	-1094.53	985.50	111.48	2229.56	0.442
Concio 5	217	I[776]	ST SLU Mobili	FZ-MIN	-1304.65	1174.69	111.48	2229.56	0.527
Concio 5	217	J[777]	ST SLU Mobili	FZ-MIN	-765.62	689.35	111.48	2229.56	0.309
Concio 5	218	I[777]	ST SLU Mobili	FZ-MIN	-1007.91	907.51	111.48	2229.56	0.407
Concio 5	218	J[778]	ST SLU Mobili	FZ-MIN	-747.37	672.92	111.48	2229.56	0.302
Concio 6	219	I[778]	ST SLU Mobili	FZ-MIN	-747.37	659.49	111.48	1672.17	0.394
Concio 6	219	J[779]	ST SLU Mobili	FZ-MAX	533.39	470.68	111.48	1672.17	0.281
Concio 6	220	I[779]	ST SLU Mobili	FZ-MIN	-760.40	670.99	111.48	1672.17	0.401
Concio 6	220	J[780]	ST SLU Mobili	FZ-MIN	-464.45	409.84	111.48	1672.17	0.245
Concio 6	221	I[780]	ST SLU Mobili	FZ-MIN	-464.45	409.84	111.48	1672.17	0.245
Concio 6	221	J[781]	ST SLU Mobili	FZ-MAX	760.22	670.84	111.48	1672.17	0.401
Concio 6	222	I[781]	ST SLU Mobili	FZ-MIN	-533.59	470.85	111.48	1672.17	0.282
Concio 6	222	J[782]	ST SLU Mobili	FZ-MAX	747.19	659.34	111.48	1672.17	0.394
Concio 5	223	I[782]	ST SLU Mobili	FZ-MAX	747.19	672.76	111.48	2229.56	0.302
Concio 5	223	J[783]	ST SLU Mobili	FZ-MAX	1007.76	907.37	111.48	2229.56	0.407
Concio 5	224	I[783]	ST SLU Mobili	FZ-MAX	765.70	689.43	111.48	2229.56	0.309
Concio 5	224	J[784]	ST SLU Mobili	FZ-MAX	1304.45	1174.50	111.48	2229.56	0.527
Concio 5	225	I[784]	ST SLU Mobili	FZ-MAX	1094.30	985.29	111.48	2229.56	0.442
Concio 5	225	J[785]	ST SLU Mobili	FZ-MAX	1147.59	1033.27	111.48	2229.56	0.463
Concio 4 H=var	226	I[785]	ST SLU Mobili	FZ-MAX	1134.79	1000.16	111.48	2229.56	0.449
Concio 4 H=var	226	J[786]	ST SLU Mobili	FZ-MAX	1448.34	882.57	111.48	2229.56	0.396
Concio 4 H=var	227	I[786]	ST SLU Mobili	FZ-MAX	1331.04	811.10	111.48	2229.56	0.364
Concio 4 H=var	227	J[787]	ST SLU Mobili	FZ-MAX	1653.74	768.02	111.48	2229.56	0.344

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Concio 4 H=200	228	I[787]	ST SLU Mobili	FZ-MAX	1650.45	766.50	111.48	2229.56	0.344
Concio 4 H=200	228	J[788]	ST SLU Mobili	FZ-MAX	1711.62	794.90	111.48	2229.56	0.357
Concio 4 H=200	229	I[788]	ST SLU Mobili	FZ-MIN	-1799.40	835.67	111.48	2229.56	0.375
Concio 4 H=200	229	J[789]	ST SLU Mobili	FZ-MIN	-1738.20	807.25	111.48	2229.56	0.362
Concio 4 H=var	230	I[789]	ST SLU Mobili	FZ-MIN	-1741.57	808.81	111.48	2229.56	0.363
Concio 4 H=var	230	J[790]	ST SLU Mobili	FZ-MIN	-1418.70	864.51	111.48	2229.56	0.388
Concio 4 H=var	231	I[790]	ST SLU Mobili	FZ-MIN	-1533.66	934.56	111.48	2229.56	0.419
Concio 4 H=var	231	J[791]	ST SLU Mobili	FZ-MIN	-1219.25	1074.59	111.48	2229.56	0.482
Concio 3	232	I[791]	ST SLU Mobili	FZ-MIN	-1231.82	1109.12	111.48	2229.56	0.497
Concio 3	232	J[792]	ST SLU Mobili	FZ-MIN	-1178.43	1061.04	111.48	2229.56	0.476
Concio 3	233	I[792]	ST SLU Mobili	FZ-MIN	-1382.43	1244.72	111.48	2229.56	0.558
Concio 3	233	J[793]	ST SLU Mobili	FZ-MIN	-824.33	742.21	111.48	2229.56	0.333
Concio 3	234	I[793]	ST SLU Mobili	FZ-MIN	-1092.35	983.53	111.48	2229.56	0.441
Concio 3	234	J[794]	ST SLU Mobili	FZ-MIN	-865.25	779.05	111.48	2229.56	0.349
Concio 2	235	I[794]	ST SLU Mobili	FZ-MIN	-865.25	769.86	111.48	1672.17	0.460
Concio 2	235	J[795]	ST SLU Mobili	FZ-MIN	-579.28	515.41	111.48	1672.17	0.308
Concio 2	236	I[795]	ST SLU Mobili	FZ-MIN	-848.00	754.51	111.48	1672.17	0.451
Concio 2	236	J[796]	ST SLU Mobili	FZ-MAX	665.49	592.13	111.48	1672.17	0.354
Concio 2	237	I[796]	ST SLU Mobili	FZ-MIN	-623.96	555.17	111.48	1672.17	0.332
Concio 2	237	J[797]	ST SLU Mobili	FZ-MAX	557.29	495.85	111.48	1672.17	0.297
Concio 1	238	I[797]	ST SLU Mobili	FZ-MAX	557.29	507.19	111.48	2229.56	0.227
Concio 1	238	J[798]	ST SLU Mobili	FZ-MAX	909.29	827.55	111.48	2229.56	0.371
Concio 1	239	I[798]	ST SLU Mobili	FZ-MAX	642.27	584.53	111.48	2229.56	0.262
Concio 1	239	J[799]	ST SLU Mobili	FZ-MAX	1307.43	1189.89	111.48	2229.56	0.534
Concio 1	240	I[799]	ST SLU Mobili	FZ-MIN	-443.76	403.86	111.48	2229.56	0.181
Concio 1	240	J[800]	ST SLV Vert	FX-MAX	1.66	1.51	111.48	2229.56	0.001
Concio 1	301	I[801]	ST SLV Vert	FX-MAX	1.66	1.51	111.48	2229.56	0.001
Concio 1	301	J[802]	ST SLU Mobili	FZ-MAX	443.76	403.87	111.48	2229.56	0.181
Concio 1	302	I[802]	ST SLU Mobili	FZ-MIN	-1308.74	1191.08	111.48	2229.56	0.534
Concio 1	302	J[803]	ST SLU Mobili	FZ-MIN	-643.41	585.57	111.48	2229.56	0.263
Concio 1	303	I[803]	ST SLU Mobili	FZ-MIN	-910.09	828.27	111.48	2229.56	0.371
Concio 1	303	J[804]	ST SLU Mobili	FZ-MIN	-558.04	507.87	111.48	2229.56	0.228
Concio 2	304	I[804]	ST SLU Mobili	FZ-MIN	-558.04	496.52	111.48	1672.17	0.297
Concio 2	304	J[805]	ST SLU Mobili	FZ-MAX	624.29	555.46	111.48	1672.17	0.332
Concio 2	305	I[805]	ST SLU Mobili	FZ-MIN	-665.86	592.45	111.48	1672.17	0.354
Concio 2	305	J[806]	ST SLU Mobili	FZ-MAX	847.82	754.35	111.48	1672.17	0.451
Concio 2	306	I[806]	ST SLU Mobili	FZ-MAX	579.88	515.95	111.48	1672.17	0.309
Concio 2	306	J[807]	ST SLU Mobili	FZ-MAX	865.73	770.29	111.48	1672.17	0.461
Concio 3	307	I[807]	ST SLU Mobili	FZ-MAX	865.73	779.49	111.48	2229.56	0.350
Concio 3	307	J[808]	ST SLU Mobili	FZ-MAX	1092.87	984.00	111.48	2229.56	0.441
Concio 3	308	I[808]	ST SLU Mobili	FZ-MAX	825.38	743.16	111.48	2229.56	0.333
Concio 3	308	J[809]	ST SLU Mobili	FZ-MAX	1383.36	1245.56	111.48	2229.56	0.559
Concio 3	309	I[809]	ST SLU Mobili	FZ-MAX	1179.56	1062.05	111.48	2229.56	0.476
Concio 3	309	J[810]	ST SLU Mobili	FZ-MAX	1232.95	1110.13	111.48	2229.56	0.498
Concio 4 H=var	310	I[810]	ST SLU Mobili	FZ-MAX	1221.33	1076.42	111.48	2229.56	0.483
Concio 4 H=var	310	J[811]	ST SLU Mobili	FZ-MAX	1535.80	935.86	111.48	2229.56	0.420
Concio 4 H=var	311	I[811]	ST SLU Mobili	FZ-MAX	1421.31	866.10	111.48	2229.56	0.388
Concio 4 H=var	311	J[812]	ST SLU Mobili	FZ-MAX	1744.17	810.02	111.48	2229.56	0.363
Concio 4 H=200	312	I[812]	ST SLU Mobili	FZ-MAX	1739.72	807.95	111.48	2229.56	0.362
Concio 4 H=200	312	J[813]	ST SLU Mobili	FZ-MAX	1800.92	836.38	111.48	2229.56	0.375
Concio 4 H=200	313	I[813]	ST SLU Mobili	FZ-MIN	-1713.79	795.91	111.48	2229.56	0.357
Concio 4 H=200	313	J[814]	ST SLU Mobili	FZ-MIN	-1652.63	767.51	111.48	2229.56	0.344
Concio 4 H=var	314	I[814]	ST SLU Mobili	FZ-MIN	-1657.01	769.54	111.48	2229.56	0.345
Concio 4 H=var	314	J[815]	ST SLU Mobili	FZ-MIN	-1334.30	813.08	111.48	2229.56	0.365
Concio 4 H=var	315	I[815]	ST SLU Mobili	FZ-MIN	-1450.99	884.19	111.48	2229.56	0.397
Concio 4 H=var	315	J[816]	ST SLU Mobili	FZ-MIN	-1137.51	1002.55	111.48	2229.56	0.450
Concio 5	316	I[816]	ST SLU Mobili	FZ-MIN	-1149.22	1034.74	111.48	2229.56	0.464
Concio 5	316	J[817]	ST SLU Mobili	FZ-MIN	-1095.92	986.75	111.48	2229.56	0.443
Concio 5	317	I[817]	ST SLU Mobili	FZ-MIN	-1305.85	1175.77	111.48	2229.56	0.527



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Concio 5	317	J[818]	ST SLU Mobili	FZ-MIN	-766.82	690.43	111.48	2229.56	0.310
Concio 5	318	I[818]	ST SLU Mobili	FZ-MIN	-1008.58	908.11	111.48	2229.56	0.407
Concio 5	318	J[819]	ST SLU Mobili	FZ-MIN	-748.03	673.52	111.48	2229.56	0.302
Concio 6	319	I[819]	ST SLU Mobili	FZ-MIN	-748.03	660.08	111.48	1672.17	0.395
Concio 6	319	J[820]	ST SLU Mobili	FZ-MAX	534.06	471.26	111.48	1672.17	0.282
Concio 6	320	I[820]	ST SLU Mobili	FZ-MIN	-760.40	670.99	111.48	1672.17	0.401
Concio 6	320	J[821]	ST SLU Mobili	FZ-MIN	-464.46	409.84	111.48	1672.17	0.245
Concio 6	321	I[821]	ST SLU Mobili	FZ-MIN	-464.46	409.84	111.48	1672.17	0.245
Concio 6	321	J[822]	ST SLU Mobili	FZ-MAX	760.23	670.84	111.48	1672.17	0.401
Concio 6	322	I[822]	ST SLU Mobili	FZ-MIN	-534.27	471.45	111.48	1672.17	0.282
Concio 6	322	J[823]	ST SLU Mobili	FZ-MAX	747.87	659.94	111.48	1672.17	0.395
Concio 5	323	I[823]	ST SLU Mobili	FZ-MAX	747.87	673.37	111.48	2229.56	0.302
Concio 5	323	J[824]	ST SLU Mobili	FZ-MAX	1008.44	907.98	111.48	2229.56	0.407
Concio 5	324	I[824]	ST SLU Mobili	FZ-MAX	766.88	690.49	111.48	2229.56	0.310
Concio 5	324	J[825]	ST SLU Mobili	FZ-MAX	1305.62	1175.56	111.48	2229.56	0.527
Concio 5	325	I[825]	ST SLU Mobili	FZ-MAX	1095.67	986.53	111.48	2229.56	0.442
Concio 5	325	J[826]	ST SLU Mobili	FZ-MAX	1148.97	1034.51	111.48	2229.56	0.464
Concio 4 H=var	326	I[826]	ST SLU Mobili	FZ-MAX	1137.26	1002.33	111.48	2229.56	0.450
Concio 4 H=var	326	J[827]	ST SLU Mobili	FZ-MAX	1450.81	884.08	111.48	2229.56	0.397
Concio 4 H=var	327	I[827]	ST SLU Mobili	FZ-MAX	1334.10	812.96	111.48	2229.56	0.365
Concio 4 H=var	327	J[828]	ST SLU Mobili	FZ-MAX	1656.79	769.44	111.48	2229.56	0.345
Concio 4 H=200	328	I[828]	ST SLU Mobili	FZ-MAX	1652.41	767.40	111.48	2229.56	0.344
Concio 4 H=200	328	J[829]	ST SLU Mobili	FZ-MAX	1713.57	795.81	111.48	2229.56	0.357
Concio 4 H=200	329	I[829]	ST SLU Mobili	FZ-MIN	-1801.21	836.51	111.48	2229.56	0.375
Concio 4 H=200	329	J[830]	ST SLU Mobili	FZ-MIN	-1740.02	808.09	111.48	2229.56	0.362
Concio 4 H=var	330	I[830]	ST SLU Mobili	FZ-MIN	-1744.44	810.15	111.48	2229.56	0.363
Concio 4 H=var	330	J[831]	ST SLU Mobili	FZ-MIN	-1421.58	866.26	111.48	2229.56	0.389
Concio 4 H=var	331	I[831]	ST SLU Mobili	FZ-MIN	-1536.05	936.02	111.48	2229.56	0.420
Concio 4 H=var	331	J[832]	ST SLU Mobili	FZ-MIN	-1221.64	1076.70	111.48	2229.56	0.483
Concio 3	332	I[832]	ST SLU Mobili	FZ-MIN	-1233.25	1110.40	111.48	2229.56	0.498
Concio 3	332	J[833]	ST SLU Mobili	FZ-MIN	-1179.85	1062.32	111.48	2229.56	0.476
Concio 3	333	I[833]	ST SLU Mobili	FZ-MIN	-1383.63	1245.80	111.48	2229.56	0.559
Concio 3	333	J[834]	ST SLU Mobili	FZ-MIN	-825.54	743.30	111.48	2229.56	0.333
Concio 3	334	I[834]	ST SLU Mobili	FZ-MIN	-1093.09	984.20	111.48	2229.56	0.441
Concio 3	334	J[835]	ST SLU Mobili	FZ-MIN	-865.99	779.72	111.48	2229.56	0.350
Concio 2	335	I[835]	ST SLU Mobili	FZ-MIN	-865.99	770.51	111.48	1672.17	0.461
Concio 2	335	J[836]	ST SLU Mobili	FZ-MIN	-580.02	516.07	111.48	1672.17	0.309
Concio 2	336	I[836]	ST SLU Mobili	FZ-MIN	-848.09	754.59	111.48	1672.17	0.451
Concio 2	336	J[837]	ST SLU Mobili	FZ-MAX	665.58	592.20	111.48	1672.17	0.354
Concio 2	337	I[837]	ST SLU Mobili	FZ-MIN	-624.50	555.65	111.48	1672.17	0.332
Concio 2	337	J[838]	ST SLU Mobili	FZ-MAX	557.84	496.34	111.48	1672.17	0.297
Concio 1	338	I[838]	ST SLU Mobili	FZ-MAX	557.84	507.68	111.48	2229.56	0.228
Concio 1	338	J[839]	ST SLU Mobili	FZ-MAX	909.84	828.05	111.48	2229.56	0.371
Concio 1	339	I[839]	ST SLU Mobili	FZ-MAX	643.21	585.38	111.48	2229.56	0.263
Concio 1	339	J[840]	ST SLU Mobili	FZ-MAX	1308.37	1190.75	111.48	2229.56	0.534
Concio 1	340	I[840]	ST SLU Mobili	FZ-MIN	-443.76	403.86	111.48	2229.56	0.181
Concio 1	340	J[841]	ST SLV Vert	FX-MAX	1.66	1.51	111.48	2229.56	0.001
Concio 1	401	I[842]	ST SLV Vert	FX-MAX	1.96	1.77	111.48	2229.56	0.001
Concio 1	401	J[843]	ST SLU Mobili	FZ-MAX	454.57	412.07	111.48	2229.56	0.185
Concio 1	402	I[843]	ST SLU Mobili	FZ-MIN	-1357.68	1230.75	111.48	2229.56	0.552
Concio 1	402	J[844]	ST SLU Mobili	FZ-MIN	-608.79	551.87	111.48	2229.56	0.248
Concio 1	403	I[844]	ST SLU Mobili	FZ-MIN	-975.83	884.60	111.48	2229.56	0.397
Concio 1	403	J[845]	ST SLU Mobili	FZ-MIN	-518.73	470.24	111.48	2229.56	0.211
Concio 2	404	I[845]	ST SLU Mobili	FZ-MIN	-518.73	459.58	111.48	1672.17	0.275
Concio 2	404	J[846]	ST SLU Mobili	FZ-MAX	581.16	514.89	111.48	1672.17	0.308
Concio 2	405	I[846]	ST SLU Mobili	FZ-MIN	-655.17	580.47	111.48	1672.17	0.347
Concio 2	405	J[847]	ST SLU Mobili	FZ-MAX	859.71	761.68	111.48	1672.17	0.456
Concio 2	406	I[847]	ST SLU Mobili	FZ-MAX	468.52	415.09	111.48	1672.17	0.248
Concio 2	406	J[848]	ST SLU Mobili	FZ-MAX	874.61	774.88	111.48	1672.17	0.463

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Concio 3	407	I[848]	ST SLU Mobili	FZ-MAX	874.61	783.73	111.48	2229.56	0.352
Concio 3	407	J[849]	ST SLU Mobili	FZ-MAX	1167.74	1046.39	111.48	2229.56	0.469
Concio 3	408	I[849]	ST SLU Mobili	FZ-MAX	788.33	706.41	111.48	2229.56	0.317
Concio 3	408	J[850]	ST SLU Mobili	FZ-MAX	1473.42	1320.30	111.48	2229.56	0.592
Concio 3	409	I[850]	ST SLU Mobili	FZ-MAX	1147.84	1028.56	111.48	2229.56	0.461
Concio 3	409	J[851]	ST SLU Mobili	FZ-MAX	1230.32	1102.47	111.48	2229.56	0.494
Concio 4 H=var	410	I[851]	ST SLU Mobili	FZ-MAX	1219.43	1066.79	111.48	2229.56	0.478
Concio 4 H=var	410	J[852]	ST SLU Mobili	FZ-MAX	1607.19	969.39	111.48	2229.56	0.435
Concio 4 H=var	411	I[852]	ST SLU Mobili	FZ-MAX	1364.90	823.25	111.48	2229.56	0.369
Concio 4 H=var	411	J[853]	ST SLU Mobili	FZ-MAX	1736.65	796.74	111.48	2229.56	0.357
Concio 4 H=200	412	I[853]	ST SLU Mobili	FZ-MAX	1738.54	797.61	111.48	2229.56	0.358
Concio 4 H=200	412	J[854]	ST SLU Mobili	FZ-MAX	1813.60	832.05	111.48	2229.56	0.373
Concio 4 H=200	413	I[854]	ST SLU Mobili	FZ-MIN	-1729.26	793.35	111.48	2229.56	0.356
Concio 4 H=200	413	J[855]	ST SLU Mobili	FZ-MIN	-1654.16	758.90	111.48	2229.56	0.340
Concio 4 H=var	414	I[855]	ST SLU Mobili	FZ-MIN	-1653.88	758.77	111.48	2229.56	0.340
Concio 4 H=var	414	J[856]	ST SLU Mobili	FZ-MIN	-1281.56	772.98	111.48	2229.56	0.347
Concio 4 H=var	415	I[856]	ST SLU Mobili	FZ-MIN	-1520.49	917.10	111.48	2229.56	0.411
Concio 4 H=var	415	J[857]	ST SLU Mobili	FZ-MIN	-1132.88	991.07	111.48	2229.56	0.445
Concio 5	416	I[857]	ST SLU Mobili	FZ-MIN	-1141.98	1023.31	111.48	2229.56	0.459
Concio 5	416	J[858]	ST SLU Mobili	FZ-MIN	-1059.51	949.41	111.48	2229.56	0.426
Concio 5	417	I[858]	ST SLU Mobili	FZ-MIN	-1387.13	1242.98	111.48	2229.56	0.558
Concio 5	417	J[859]	ST SLU Mobili	FZ-MIN	-684.13	613.04	111.48	2229.56	0.275
Concio 5	418	I[859]	ST SLU Mobili	FZ-MIN	-1075.64	963.86	111.48	2229.56	0.432
Concio 5	418	J[860]	ST SLU Mobili	FZ-MIN	-744.74	667.35	111.48	2229.56	0.299
Concio 6	419	I[860]	ST SLU Mobili	FZ-MIN	-744.74	654.60	111.48	1672.17	0.391
Concio 6	419	J[861]	ST SLU Mobili	FZ-MAX	476.89	419.17	111.48	1672.17	0.251
Concio 6	420	I[861]	ST SLU Mobili	FZ-MIN	-759.17	667.27	111.48	1672.17	0.399
Concio 6	420	J[862]	ST SLU Mobili	FZ-MIN	-380.56	334.50	111.48	1672.17	0.200
Concio 6	421	I[862]	ST SLU Mobili	FZ-MIN	-380.56	334.50	111.48	1672.17	0.200
Concio 6	421	J[863]	ST SLU Mobili	FZ-MAX	758.99	667.12	111.48	1672.17	0.399
Concio 6	422	I[863]	ST SLU Mobili	FZ-MIN	-477.12	419.37	111.48	1672.17	0.251
Concio 6	422	J[864]	ST SLU Mobili	FZ-MAX	744.59	654.46	111.48	1672.17	0.391
Concio 5	423	I[864]	ST SLU Mobili	FZ-MAX	744.59	667.21	111.48	2229.56	0.299
Concio 5	423	J[865]	ST SLU Mobili	FZ-MAX	1075.51	963.75	111.48	2229.56	0.432
Concio 5	424	I[865]	ST SLU Mobili	FZ-MAX	684.04	612.95	111.48	2229.56	0.275
Concio 5	424	J[866]	ST SLU Mobili	FZ-MAX	1386.86	1242.74	111.48	2229.56	0.557
Concio 5	425	I[866]	ST SLU Mobili	FZ-MAX	1059.37	949.28	111.48	2229.56	0.426
Concio 5	425	J[867]	ST SLU Mobili	FZ-MAX	1141.83	1023.18	111.48	2229.56	0.459
Concio 4 H=var	426	I[867]	ST SLU Mobili	FZ-MAX	1132.70	990.92	111.48	2229.56	0.444
Concio 4 H=var	426	J[868]	ST SLU Mobili	FZ-MAX	1520.29	916.98	111.48	2229.56	0.411
Concio 4 H=var	427	I[868]	ST SLU Mobili	FZ-MAX	1281.37	772.87	111.48	2229.56	0.347
Concio 4 H=var	427	J[869]	ST SLU Mobili	FZ-MAX	1653.62	758.65	111.48	2229.56	0.340
Concio 4 H=200	428	I[869]	ST SLU Mobili	FZ-MAX	1653.91	758.78	111.48	2229.56	0.340
Concio 4 H=200	428	J[870]	ST SLU Mobili	FZ-MAX	1729.01	793.24	111.48	2229.56	0.356
Concio 4 H=200	429	I[870]	ST SLU Mobili	FZ-MIN	-1814.01	832.23	111.48	2229.56	0.373
Concio 4 H=200	429	J[871]	ST SLU Mobili	FZ-MIN	-1738.94	797.79	111.48	2229.56	0.358
Concio 4 H=var	430	I[871]	ST SLU Mobili	FZ-MIN	-1737.03	796.92	111.48	2229.56	0.357
Concio 4 H=var	430	J[872]	ST SLU Mobili	FZ-MIN	-1365.22	823.45	111.48	2229.56	0.369
Concio 4 H=var	431	I[872]	ST SLU Mobili	FZ-MIN	-1607.49	969.57	111.48	2229.56	0.435
Concio 4 H=var	431	J[873]	ST SLU Mobili	FZ-MIN	-1219.70	1067.03	111.48	2229.56	0.479
Concio 3	432	I[873]	ST SLU Mobili	FZ-MIN	-1230.62	1102.74	111.48	2229.56	0.495
Concio 3	432	J[874]	ST SLU Mobili	FZ-MIN	-1148.14	1028.83	111.48	2229.56	0.461
Concio 3	433	I[874]	ST SLU Mobili	FZ-MIN	-1473.82	1320.67	111.48	2229.56	0.592
Concio 3	433	J[875]	ST SLU Mobili	FZ-MIN	-788.55	706.61	111.48	2229.56	0.317
Concio 3	434	I[875]	ST SLU Mobili	FZ-MIN	-1167.98	1046.61	111.48	2229.56	0.469
Concio 3	434	J[876]	ST SLU Mobili	FZ-MIN	-874.91	783.99	111.48	2229.56	0.352
Concio 2	435	I[876]	ST SLU Mobili	FZ-MIN	-874.91	775.14	111.48	1672.17	0.464
Concio 2	435	J[877]	ST SLU Mobili	FZ-MIN	-468.74	415.29	111.48	1672.17	0.248
Concio 2	436	I[877]	ST SLU Mobili	FZ-MIN	-860.09	762.02	111.48	1672.17	0.456

Concio 2	436	J[878]	ST SLU Mobili	FZ-MAX	654.75	580.09	111.48	1672.17	0.347
Concio 2	437	I[878]	ST SLU Mobili	FZ-MIN	-581.46	515.15	111.48	1672.17	0.308
Concio 2	437	J[879]	ST SLU Mobili	FZ-MAX	518.45	459.33	111.48	1672.17	0.275
Concio 1	438	I[879]	ST SLU Mobili	FZ-MAX	518.45	469.98	111.48	2229.56	0.211
Concio 1	438	J[880]	ST SLU Mobili	FZ-MAX	975.50	884.30	111.48	2229.56	0.397
Concio 1	439	I[880]	ST SLU Mobili	FZ-MAX	608.26	551.40	111.48	2229.56	0.247
Concio 1	439	J[881]	ST SLU Mobili	FZ-MAX	1357.27	1230.38	111.48	2229.56	0.552
Concio 1	440	I[881]	ST SLU Mobili	MZ-MAX	-454.57	412.07	111.48	2229.56	0.185
Concio 1	440	J[882]	ST SLV Vert	FX-MAX	1.96	1.77	111.48	2229.56	0.001

## 9.2 STATO LIMITE DI ESERCIZIO – RESISTENZA AL TAGLIO LONGITUDINALE

Si effettuano le verifiche allo stato limite di esercizio per taglio longitudinale dei connettori trave-soletta.

I coefficienti parziali per SLE assunti sono i seguenti:

$$k_s = 0.60 \quad \text{connettori: resistenza allo SLE}$$

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem property:	nome delle caratteristiche geometriche dell'elemento
Elem:	numero dell'elemento
Position:	nodo iniziale (I) o finale (J) dell'elemento
Lcom:	combinazione di carico più gravosa
Type:	tipo combinazione (caratteristica, frequente, quasi permanente)
V_c,Ed:	sforzio di taglio longitudinale agente sulla larghezza della regione inelastica
v_L,Ed:	sforzio di taglio longitudinale di calcolo per unità di lunghezza (m) all'interfaccia trave-soletta
P_Rd,ser:	resistenza al taglio (SLE) del singolo connettore "Nelson"
v_L,Rd:	resistenza al taglio longitudinale per trave e per unità di lunghezza

Condizione di verifica: 
$$\text{Verification Ratio: } \frac{v_{L,Ed}}{v_{L,Rd}} \leq 1$$

Elem property	Elem number	Position	Lcom	Type	V_c,Ed (kN)	v_L,Ed (kN/m)	P_Rd_ser (kN)	v_L,Rd (kN/m)	Verif. Ratio
Concio 1	101	I[719]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	101	J[720]	ST RARA Mobili	Characteristic	335.45	304.09	66.89	1337.73	0.227
Concio 1	102	I[720]	ST RARA Mobili	Characteristic	-987.37	895.06	66.89	1337.73	0.669
Concio 1	102	J[721]	ST RARA Mobili	Characteristic	-440.47	399.29	66.89	1337.73	0.298
Concio 1	103	I[721]	ST RARA Mobili	Characteristic	-709.80	643.44	66.89	1337.73	0.481
Concio 1	103	J[722]	ST RARA Mobili	Characteristic	-375.98	340.83	66.89	1337.73	0.255
Concio 2	104	I[722]	ST RARA Mobili	Characteristic	-375.98	333.11	66.89	1003.30	0.332
Concio 2	104	J[723]	ST RARA Mobili	Characteristic	437.68	387.77	66.89	1003.30	0.386
Concio 2	105	I[723]	ST RARA Mobili	Characteristic	-479.13	424.50	66.89	1003.30	0.423
Concio 2	105	J[724]	ST RARA Mobili	Characteristic	637.34	564.66	66.89	1003.30	0.563
Concio 2	106	I[724]	ST RARA Mobili	Characteristic	349.11	309.30	66.89	1003.30	0.308
Concio 2	106	J[725]	ST RARA Mobili	Characteristic	645.15	571.59	66.89	1003.30	0.570
Concio 3	107	I[725]	ST RARA Mobili	Characteristic	645.15	578.11	66.89	1337.73	0.432
Concio 3	107	J[726]	ST RARA Mobili	Characteristic	859.10	769.83	66.89	1337.73	0.575
Concio 3	108	I[726]	ST RARA Mobili	Characteristic	580.27	519.97	66.89	1337.73	0.389
Concio 3	108	J[727]	ST RARA Mobili	Characteristic	1079.79	967.58	66.89	1337.73	0.723
Concio 3	109	I[727]	ST RARA Mobili	Characteristic	840.80	753.42	66.89	1337.73	0.563

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Concio 3	109	J[728]	ST RARA Mobili	Characteristic	901.10	807.46	66.89	1337.73	0.604
Concio 4 H=var	110	I[728]	ST RARA Mobili	Characteristic	892.46	780.75	66.89	1337.73	0.584
Concio 4 H=var	110	J[729]	ST RARA Mobili	Characteristic	1175.71	709.14	66.89	1337.73	0.530
Concio 4 H=var	111	I[729]	ST RARA Mobili	Characteristic	998.93	602.51	66.89	1337.73	0.450
Concio 4 H=var	111	J[730]	ST RARA Mobili	Characteristic	1270.32	582.80	66.89	1337.73	0.436
Concio 4 H=200	112	I[730]	ST RARA Mobili	Characteristic	1272.18	583.65	66.89	1337.73	0.436
Concio 4 H=200	112	J[731]	ST RARA Mobili	Characteristic	1326.99	608.80	66.89	1337.73	0.455
Concio 4 H=200	113	I[731]	ST RARA Mobili	Characteristic	-1259.61	577.88	66.89	1337.73	0.432
Concio 4 H=200	113	J[732]	ST RARA Mobili	Characteristic	-1204.77	552.73	66.89	1337.73	0.413
Concio 4 H=var	114	I[732]	ST RARA Mobili	Characteristic	-1204.27	552.50	66.89	1337.73	0.413
Concio 4 H=var	114	J[733]	ST RARA Mobili	Characteristic	-932.46	562.42	66.89	1337.73	0.420
Concio 4 H=var	115	I[733]	ST RARA Mobili	Characteristic	-1106.50	667.40	66.89	1337.73	0.499
Concio 4 H=var	115	J[734]	ST RARA Mobili	Characteristic	-823.35	720.29	66.89	1337.73	0.538
Concio 5	116	I[734]	ST RARA Mobili	Characteristic	-830.52	744.22	66.89	1337.73	0.556
Concio 5	116	J[735]	ST RARA Mobili	Characteristic	-770.23	690.19	66.89	1337.73	0.516
Concio 5	117	I[735]	ST RARA Mobili	Characteristic	-1010.59	905.57	66.89	1337.73	0.677
Concio 5	117	J[736]	ST RARA Mobili	Characteristic	-498.11	446.35	66.89	1337.73	0.334
Concio 5	118	I[736]	ST RARA Mobili	Characteristic	-785.94	704.26	66.89	1337.73	0.526
Concio 5	118	J[737]	ST RARA Mobili	Characteristic	-544.48	487.90	66.89	1337.73	0.365
Concio 6	119	I[737]	ST RARA Mobili	Characteristic	-544.48	478.58	66.89	1003.30	0.477
Concio 6	119	J[738]	ST RARA Mobili	Characteristic	356.73	313.55	66.89	1003.30	0.313
Concio 6	120	I[738]	ST RARA Mobili	Characteristic	-558.21	490.64	66.89	1003.30	0.489
Concio 6	120	J[739]	ST RARA Mobili	Characteristic	-281.90	247.78	66.89	1003.30	0.247
Concio 6	121	I[739]	ST RARA Mobili	Characteristic	-281.90	247.78	66.89	1003.30	0.247
Concio 6	121	J[740]	ST RARA Mobili	Characteristic	558.08	490.53	66.89	1003.30	0.489
Concio 6	122	I[740]	ST RARA Mobili	Characteristic	-356.91	313.71	66.89	1003.30	0.313
Concio 6	122	J[741]	ST RARA Mobili	Characteristic	544.38	478.48	66.89	1003.30	0.477
Concio 5	123	I[741]	ST RARA Mobili	Characteristic	544.38	487.81	66.89	1337.73	0.365
Concio 5	123	J[742]	ST RARA Mobili	Characteristic	785.85	704.19	66.89	1337.73	0.526
Concio 5	124	I[742]	ST RARA Mobili	Characteristic	498.08	446.32	66.89	1337.73	0.334
Concio 5	124	J[743]	ST RARA Mobili	Characteristic	1010.41	905.42	66.89	1337.73	0.677
Concio 5	125	I[743]	ST RARA Mobili	Characteristic	770.13	690.10	66.89	1337.73	0.516
Concio 5	125	J[744]	ST RARA Mobili	Characteristic	830.42	744.12	66.89	1337.73	0.556
Concio 4 H=var	126	I[744]	ST RARA Mobili	Characteristic	823.25	720.20	66.89	1337.73	0.538
Concio 4 H=var	126	J[745]	ST RARA Mobili	Characteristic	1106.37	667.32	66.89	1337.73	0.499
Concio 4 H=var	127	I[745]	ST RARA Mobili	Characteristic	932.23	562.28	66.89	1337.73	0.420
Concio 4 H=var	127	J[746]	ST RARA Mobili	Characteristic	1203.99	552.37	66.89	1337.73	0.413
Concio 4 H=200	128	I[746]	ST RARA Mobili	Characteristic	1204.49	552.60	66.89	1337.73	0.413
Concio 4 H=200	128	J[747]	ST RARA Mobili	Characteristic	1259.32	577.75	66.89	1337.73	0.432
Concio 4 H=200	129	I[747]	ST RARA Mobili	Characteristic	-1327.38	608.98	66.89	1337.73	0.455
Concio 4 H=200	129	J[748]	ST RARA Mobili	Characteristic	-1272.57	583.83	66.89	1337.73	0.436
Concio 4 H=var	130	I[748]	ST RARA Mobili	Characteristic	-1270.68	582.97	66.89	1337.73	0.436
Concio 4 H=var	130	J[749]	ST RARA Mobili	Characteristic	-999.25	602.71	66.89	1337.73	0.451
Concio 4 H=var	131	I[749]	ST RARA Mobili	Characteristic	-1175.91	709.26	66.89	1337.73	0.530
Concio 4 H=var	131	J[750]	ST RARA Mobili	Characteristic	-892.64	780.90	66.89	1337.73	0.584
Concio 3	132	I[750]	ST RARA Mobili	Characteristic	-901.31	807.65	66.89	1337.73	0.604
Concio 3	132	J[751]	ST RARA Mobili	Characteristic	-841.01	753.61	66.89	1337.73	0.563
Concio 3	133	I[751]	ST RARA Mobili	Characteristic	-1080.05	967.82	66.89	1337.73	0.723
Concio 3	133	J[752]	ST RARA Mobili	Characteristic	-580.40	520.08	66.89	1337.73	0.389
Concio 3	134	I[752]	ST RARA Mobili	Characteristic	-859.30	770.00	66.89	1337.73	0.576
Concio 3	134	J[753]	ST RARA Mobili	Characteristic	-645.38	578.32	66.89	1337.73	0.432
Concio 2	135	I[753]	ST RARA Mobili	Characteristic	-645.38	571.79	66.89	1003.30	0.570
Concio 2	135	J[754]	ST RARA Mobili	Characteristic	-349.28	309.46	66.89	1003.30	0.308
Concio 2	136	I[754]	ST RARA Mobili	Characteristic	-637.63	564.92	66.89	1003.30	0.563
Concio 2	136	J[755]	ST RARA Mobili	Characteristic	478.81	424.21	66.89	1003.30	0.423
Concio 2	137	I[755]	ST RARA Mobili	Characteristic	-437.90	387.96	66.89	1003.30	0.387
Concio 2	137	J[756]	ST RARA Mobili	Characteristic	375.77	332.92	66.89	1003.30	0.332
Concio 1	138	I[756]	ST RARA Mobili	Characteristic	375.77	340.64	66.89	1337.73	0.255
Concio 1	138	J[757]	ST RARA Mobili	Characteristic	709.55	643.22	66.89	1337.73	0.481



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Concio 1	139	I[757]	ST RARA Mobili	Characteristic	440.20	399.05	66.89	1337.73	0.298
Concio 1	139	J[758]	ST RARA Mobili	Characteristic	987.07	894.79	66.89	1337.73	0.669
Concio 1	140	I[758]	ST RARA Mobili	Characteristic	-335.45	304.09	66.89	1337.73	0.227
Concio 1	140	J[759]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	201	I[760]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	201	J[761]	ST RARA Mobili	Characteristic	386.40	351.66	66.89	1337.73	0.263
Concio 1	202	I[761]	ST RARA Mobili	Characteristic	-954.38	868.58	66.89	1337.73	0.649
Concio 1	202	J[762]	ST RARA Mobili	Characteristic	-465.98	424.09	66.89	1337.73	0.317
Concio 1	203	I[762]	ST RARA Mobili	Characteristic	-663.29	603.66	66.89	1337.73	0.451
Concio 1	203	J[763]	ST RARA Mobili	Characteristic	-405.18	368.75	66.89	1337.73	0.276
Concio 2	204	I[763]	ST RARA Mobili	Characteristic	-405.18	360.51	66.89	1003.30	0.359
Concio 2	204	J[764]	ST RARA Mobili	Characteristic	469.87	418.07	66.89	1003.30	0.417
Concio 2	205	I[764]	ST RARA Mobili	Characteristic	-488.28	434.45	66.89	1003.30	0.433
Concio 2	205	J[765]	ST RARA Mobili	Characteristic	630.04	560.58	66.89	1003.30	0.559
Concio 2	206	I[765]	ST RARA Mobili	Characteristic	430.08	382.66	66.89	1003.30	0.381
Concio 2	206	J[766]	ST RARA Mobili	Characteristic	639.15	568.69	66.89	1003.30	0.567
Concio 3	207	I[766]	ST RARA Mobili	Characteristic	639.15	575.48	66.89	1337.73	0.430
Concio 3	207	J[767]	ST RARA Mobili	Characteristic	805.63	725.37	66.89	1337.73	0.542
Concio 3	208	I[767]	ST RARA Mobili	Characteristic	607.53	547.01	66.89	1337.73	0.409
Concio 3	208	J[768]	ST RARA Mobili	Characteristic	1016.41	915.16	66.89	1337.73	0.684
Concio 3	209	I[768]	ST RARA Mobili	Characteristic	866.19	779.90	66.89	1337.73	0.583
Concio 3	209	J[769]	ST RARA Mobili	Characteristic	905.29	815.11	66.89	1337.73	0.609
Concio 4 H=var	210	I[769]	ST RARA Mobili	Characteristic	895.21	789.00	66.89	1337.73	0.590
Concio 4 H=var	210	J[770]	ST RARA Mobili	Characteristic	1125.93	686.10	66.89	1337.73	0.513
Concio 4 H=var	211	I[770]	ST RARA Mobili	Characteristic	1043.45	635.84	66.89	1337.73	0.475
Concio 4 H=var	211	J[771]	ST RARA Mobili	Characteristic	1280.37	594.63	66.89	1337.73	0.445
Concio 4 H=200	212	I[771]	ST RARA Mobili	Characteristic	1279.08	594.03	66.89	1337.73	0.444
Concio 4 H=200	212	J[772]	ST RARA Mobili	Characteristic	1323.97	614.87	66.89	1337.73	0.460
Concio 4 H=200	213	I[772]	ST RARA Mobili	Characteristic	-1254.54	582.63	66.89	1337.73	0.436
Concio 4 H=200	213	J[773]	ST RARA Mobili	Characteristic	-1209.68	561.80	66.89	1337.73	0.420
Concio 4 H=var	214	I[773]	ST RARA Mobili	Characteristic	-1210.92	562.37	66.89	1337.73	0.420
Concio 4 H=var	214	J[774]	ST RARA Mobili	Characteristic	-974.10	593.58	66.89	1337.73	0.444
Concio 4 H=var	215	I[774]	ST RARA Mobili	Characteristic	-1058.23	644.85	66.89	1337.73	0.482
Concio 4 H=var	215	J[775]	ST RARA Mobili	Characteristic	-828.24	729.98	66.89	1337.73	0.546
Concio 5	216	I[775]	ST RARA Mobili	Characteristic	-838.48	754.96	66.89	1337.73	0.564
Concio 5	216	J[776]	ST RARA Mobili	Characteristic	-799.44	719.81	66.89	1337.73	0.538
Concio 5	217	I[776]	ST RARA Mobili	Characteristic	-954.32	859.25	66.89	1337.73	0.642
Concio 5	217	J[777]	ST RARA Mobili	Characteristic	-559.66	503.91	66.89	1337.73	0.377
Concio 5	218	I[777]	ST RARA Mobili	Characteristic	-738.71	665.12	66.89	1337.73	0.497
Concio 5	218	J[778]	ST RARA Mobili	Characteristic	-547.76	493.19	66.89	1337.73	0.369
Concio 6	219	I[778]	ST RARA Mobili	Characteristic	-547.76	483.35	66.89	1003.30	0.482
Concio 6	219	J[779]	ST RARA Mobili	Characteristic	398.67	351.80	66.89	1003.30	0.351
Concio 6	220	I[779]	ST RARA Mobili	Characteristic	-560.95	494.99	66.89	1003.30	0.493
Concio 6	220	J[780]	ST RARA Mobili	Characteristic	-344.04	303.59	66.89	1003.30	0.303
Concio 6	221	I[780]	ST RARA Mobili	Characteristic	-344.04	303.59	66.89	1003.30	0.303
Concio 6	221	J[781]	ST RARA Mobili	Characteristic	560.82	494.88	66.89	1003.30	0.493
Concio 6	222	I[781]	ST RARA Mobili	Characteristic	-398.82	351.93	66.89	1003.30	0.351
Concio 6	222	J[782]	ST RARA Mobili	Characteristic	547.63	483.24	66.89	1003.30	0.482
Concio 5	223	I[782]	ST RARA Mobili	Characteristic	547.63	493.07	66.89	1337.73	0.369
Concio 5	223	J[783]	ST RARA Mobili	Characteristic	738.60	665.02	66.89	1337.73	0.497
Concio 5	224	I[783]	ST RARA Mobili	Characteristic	559.72	503.96	66.89	1337.73	0.377
Concio 5	224	J[784]	ST RARA Mobili	Characteristic	954.17	859.12	66.89	1337.73	0.642
Concio 5	225	I[784]	ST RARA Mobili	Characteristic	799.27	719.65	66.89	1337.73	0.538
Concio 5	225	J[785]	ST RARA Mobili	Characteristic	838.31	754.80	66.89	1337.73	0.564
Concio 4 H=var	226	I[785]	ST RARA Mobili	Characteristic	828.08	729.83	66.89	1337.73	0.546
Concio 4 H=var	226	J[786]	ST RARA Mobili	Characteristic	1058.11	644.78	66.89	1337.73	0.482
Concio 4 H=var	227	I[786]	ST RARA Mobili	Characteristic	973.90	593.47	66.89	1337.73	0.444
Concio 4 H=var	227	J[787]	ST RARA Mobili	Characteristic	1210.71	562.28	66.89	1337.73	0.420
Concio 4 H=200	228	I[787]	ST RARA Mobili	Characteristic	1209.47	561.70	66.89	1337.73	0.420



Concio 4 H=200	228	J[788]	ST RARA Mobili	Characteristic	1254.33	582.53	66.89	1337.73	0.435
Concio 4 H=200	229	I[788]	ST RARA Mobili	Characteristic	-1324.23	614.99	66.89	1337.73	0.460
Concio 4 H=200	229	J[789]	ST RARA Mobili	Characteristic	-1279.34	594.15	66.89	1337.73	0.444
Concio 4 H=var	230	I[789]	ST RARA Mobili	Characteristic	-1280.61	594.74	66.89	1337.73	0.445
Concio 4 H=var	230	J[790]	ST RARA Mobili	Characteristic	-1043.67	635.98	66.89	1337.73	0.475
Concio 4 H=var	231	I[790]	ST RARA Mobili	Characteristic	-1126.08	686.20	66.89	1337.73	0.513
Concio 4 H=var	231	J[791]	ST RARA Mobili	Characteristic	-895.41	789.17	66.89	1337.73	0.590
Concio 3	232	I[791]	ST RARA Mobili	Characteristic	-905.49	815.29	66.89	1337.73	0.609
Concio 3	232	J[792]	ST RARA Mobili	Characteristic	-866.39	780.08	66.89	1337.73	0.583
Concio 3	233	I[792]	ST RARA Mobili	Characteristic	-1016.60	915.33	66.89	1337.73	0.684
Concio 3	233	J[793]	ST RARA Mobili	Characteristic	-607.64	547.11	66.89	1337.73	0.409
Concio 3	234	I[793]	ST RARA Mobili	Characteristic	-805.80	725.53	66.89	1337.73	0.542
Concio 3	234	J[794]	ST RARA Mobili	Characteristic	-639.35	575.66	66.89	1337.73	0.430
Concio 2	235	I[794]	ST RARA Mobili	Characteristic	-639.35	568.87	66.89	1003.30	0.567
Concio 2	235	J[795]	ST RARA Mobili	Characteristic	-430.19	382.77	66.89	1003.30	0.382
Concio 2	236	I[795]	ST RARA Mobili	Characteristic	-630.25	560.77	66.89	1003.30	0.559
Concio 2	236	J[796]	ST RARA Mobili	Characteristic	488.06	434.26	66.89	1003.30	0.433
Concio 2	237	I[796]	ST RARA Mobili	Characteristic	-470.03	418.21	66.89	1003.30	0.417
Concio 2	237	J[797]	ST RARA Mobili	Characteristic	405.03	360.38	66.89	1003.30	0.359
Concio 1	238	I[797]	ST RARA Mobili	Characteristic	405.03	368.61	66.89	1337.73	0.276
Concio 1	238	J[798]	ST RARA Mobili	Characteristic	663.11	603.49	66.89	1337.73	0.451
Concio 1	239	I[798]	ST RARA Mobili	Characteristic	465.83	423.95	66.89	1337.73	0.317
Concio 1	239	J[799]	ST RARA Mobili	Characteristic	954.10	868.33	66.89	1337.73	0.649
Concio 1	240	I[799]	ST RARA Mobili	Characteristic	-386.40	351.66	66.89	1337.73	0.263
Concio 1	240	J[800]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	301	I[801]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	301	J[802]	ST RARA Mobili	Characteristic	386.40	351.66	66.89	1337.73	0.263
Concio 1	302	I[802]	ST RARA Mobili	Characteristic	-955.00	869.15	66.89	1337.73	0.650
Concio 1	302	J[803]	ST RARA Mobili	Characteristic	-466.61	424.66	66.89	1337.73	0.317
Concio 1	303	I[803]	ST RARA Mobili	Characteristic	-663.66	603.99	66.89	1337.73	0.452
Concio 1	303	J[804]	ST RARA Mobili	Characteristic	-405.55	369.09	66.89	1337.73	0.276
Concio 2	304	I[804]	ST RARA Mobili	Characteristic	-405.55	360.84	66.89	1003.30	0.360
Concio 2	304	J[805]	ST RARA Mobili	Characteristic	470.24	418.40	66.89	1003.30	0.417
Concio 2	305	I[805]	ST RARA Mobili	Characteristic	-488.33	434.49	66.89	1003.30	0.433
Concio 2	305	J[806]	ST RARA Mobili	Characteristic	630.12	560.65	66.89	1003.30	0.559
Concio 2	306	I[806]	ST RARA Mobili	Characteristic	430.58	383.11	66.89	1003.30	0.382
Concio 2	306	J[807]	ST RARA Mobili	Characteristic	639.66	569.14	66.89	1003.30	0.567
Concio 3	307	I[807]	ST RARA Mobili	Characteristic	639.66	575.94	66.89	1337.73	0.431
Concio 3	307	J[808]	ST RARA Mobili	Characteristic	806.13	725.83	66.89	1337.73	0.543
Concio 3	308	I[808]	ST RARA Mobili	Characteristic	608.32	547.72	66.89	1337.73	0.409
Concio 3	308	J[809]	ST RARA Mobili	Characteristic	1017.20	915.87	66.89	1337.73	0.685
Concio 3	309	I[809]	ST RARA Mobili	Characteristic	867.12	780.74	66.89	1337.73	0.584
Concio 3	309	J[810]	ST RARA Mobili	Characteristic	906.22	815.95	66.89	1337.73	0.610
Concio 4 H=var	310	I[810]	ST RARA Mobili	Characteristic	896.77	790.38	66.89	1337.73	0.591
Concio 4 H=var	310	J[811]	ST RARA Mobili	Characteristic	1127.49	687.06	66.89	1337.73	0.514
Concio 4 H=var	311	I[811]	ST RARA Mobili	Characteristic	1045.40	637.03	66.89	1337.73	0.476
Concio 4 H=var	311	J[812]	ST RARA Mobili	Characteristic	1282.33	595.53	66.89	1337.73	0.445
Concio 4 H=200	312	I[812]	ST RARA Mobili	Characteristic	1280.33	594.61	66.89	1337.73	0.444
Concio 4 H=200	312	J[813]	ST RARA Mobili	Characteristic	1325.22	615.46	66.89	1337.73	0.460
Concio 4 H=200	313	I[813]	ST RARA Mobili	Characteristic	-1255.79	583.21	66.89	1337.73	0.436
Concio 4 H=200	313	J[814]	ST RARA Mobili	Characteristic	-1210.93	562.38	66.89	1337.73	0.420
Concio 4 H=var	314	I[814]	ST RARA Mobili	Characteristic	-1212.91	563.29	66.89	1337.73	0.421
Concio 4 H=var	314	J[815]	ST RARA Mobili	Characteristic	-976.09	594.79	66.89	1337.73	0.445
Concio 4 H=var	315	I[815]	ST RARA Mobili	Characteristic	-1059.89	645.86	66.89	1337.73	0.483
Concio 4 H=var	315	J[816]	ST RARA Mobili	Characteristic	-829.90	731.44	66.89	1337.73	0.547
Concio 5	316	I[816]	ST RARA Mobili	Characteristic	-839.41	755.79	66.89	1337.73	0.565
Concio 5	316	J[817]	ST RARA Mobili	Characteristic	-800.37	720.64	66.89	1337.73	0.539
Concio 5	317	I[817]	ST RARA Mobili	Characteristic	-955.12	859.97	66.89	1337.73	0.643
Concio 5	317	J[818]	ST RARA Mobili	Characteristic	-560.46	504.63	66.89	1337.73	0.377

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Concio 5	318	I[818]	ST RARA Mobili	Characteristic	-739.15	665.52	66.89	1337.73	0.497
Concio 5	318	J[819]	ST RARA Mobili	Characteristic	-548.20	493.59	66.89	1337.73	0.369
Concio 6	319	I[819]	ST RARA Mobili	Characteristic	-548.20	483.74	66.89	1003.30	0.482
Concio 6	319	J[820]	ST RARA Mobili	Characteristic	399.12	352.19	66.89	1003.30	0.351
Concio 6	320	I[820]	ST RARA Mobili	Characteristic	-560.95	494.99	66.89	1003.30	0.493
Concio 6	320	J[821]	ST RARA Mobili	Characteristic	-344.04	303.59	66.89	1003.30	0.303
Concio 6	321	I[821]	ST RARA Mobili	Characteristic	-344.04	303.59	66.89	1003.30	0.303
Concio 6	321	J[822]	ST RARA Mobili	Characteristic	560.82	494.88	66.89	1003.30	0.493
Concio 6	322	I[822]	ST RARA Mobili	Characteristic	-399.27	352.33	66.89	1003.30	0.351
Concio 6	322	J[823]	ST RARA Mobili	Characteristic	548.08	483.64	66.89	1003.30	0.482
Concio 5	323	I[823]	ST RARA Mobili	Characteristic	548.08	493.48	66.89	1337.73	0.369
Concio 5	323	J[824]	ST RARA Mobili	Characteristic	739.05	665.43	66.89	1337.73	0.497
Concio 5	324	I[824]	ST RARA Mobili	Characteristic	560.50	504.67	66.89	1337.73	0.377
Concio 5	324	J[825]	ST RARA Mobili	Characteristic	954.95	859.82	66.89	1337.73	0.643
Concio 5	325	I[825]	ST RARA Mobili	Characteristic	800.19	720.48	66.89	1337.73	0.539
Concio 5	325	J[826]	ST RARA Mobili	Characteristic	839.23	755.63	66.89	1337.73	0.565
Concio 4 H=var	326	I[826]	ST RARA Mobili	Characteristic	829.72	731.28	66.89	1337.73	0.547
Concio 4 H=var	326	J[827]	ST RARA Mobili	Characteristic	1059.76	645.78	66.89	1337.73	0.483
Concio 4 H=var	327	I[827]	ST RARA Mobili	Characteristic	975.94	594.71	66.89	1337.73	0.445
Concio 4 H=var	327	J[828]	ST RARA Mobili	Characteristic	1212.75	563.22	66.89	1337.73	0.421
Concio 4 H=200	328	I[828]	ST RARA Mobili	Characteristic	1210.77	562.30	66.89	1337.73	0.420
Concio 4 H=200	328	J[829]	ST RARA Mobili	Characteristic	1255.63	583.14	66.89	1337.73	0.436
Concio 4 H=200	329	I[829]	ST RARA Mobili	Characteristic	-1325.43	615.55	66.89	1337.73	0.460
Concio 4 H=200	329	J[830]	ST RARA Mobili	Characteristic	-1280.55	594.71	66.89	1337.73	0.445
Concio 4 H=var	330	I[830]	ST RARA Mobili	Characteristic	-1282.53	595.63	66.89	1337.73	0.445
Concio 4 H=var	330	J[831]	ST RARA Mobili	Characteristic	-1045.59	637.15	66.89	1337.73	0.476
Concio 4 H=var	331	I[831]	ST RARA Mobili	Characteristic	-1127.67	687.17	66.89	1337.73	0.514
Concio 4 H=var	331	J[832]	ST RARA Mobili	Characteristic	-897.00	790.58	66.89	1337.73	0.591
Concio 3	332	I[832]	ST RARA Mobili	Characteristic	-906.44	816.15	66.89	1337.73	0.610
Concio 3	332	J[833]	ST RARA Mobili	Characteristic	-867.34	780.94	66.89	1337.73	0.584
Concio 3	333	I[833]	ST RARA Mobili	Characteristic	-1017.40	916.05	66.89	1337.73	0.685
Concio 3	333	J[834]	ST RARA Mobili	Characteristic	-608.44	547.83	66.89	1337.73	0.410
Concio 3	334	I[834]	ST RARA Mobili	Characteristic	-806.30	725.98	66.89	1337.73	0.543
Concio 3	334	J[835]	ST RARA Mobili	Characteristic	-639.85	576.11	66.89	1337.73	0.431
Concio 2	335	I[835]	ST RARA Mobili	Characteristic	-639.85	569.31	66.89	1003.30	0.567
Concio 2	335	J[836]	ST RARA Mobili	Characteristic	-430.69	383.21	66.89	1003.30	0.382
Concio 2	336	I[836]	ST RARA Mobili	Characteristic	-630.31	560.82	66.89	1003.30	0.559
Concio 2	336	J[837]	ST RARA Mobili	Characteristic	488.12	434.31	66.89	1003.30	0.433
Concio 2	337	I[837]	ST RARA Mobili	Characteristic	-470.40	418.54	66.89	1003.30	0.417
Concio 2	337	J[838]	ST RARA Mobili	Characteristic	405.39	360.70	66.89	1003.30	0.360
Concio 1	338	I[838]	ST RARA Mobili	Characteristic	405.39	368.95	66.89	1337.73	0.276
Concio 1	338	J[839]	ST RARA Mobili	Characteristic	663.47	603.83	66.89	1337.73	0.451
Concio 1	339	I[839]	ST RARA Mobili	Characteristic	466.46	424.52	66.89	1337.73	0.317
Concio 1	339	J[840]	ST RARA Mobili	Characteristic	954.73	868.90	66.89	1337.73	0.650
Concio 1	340	I[840]	ST RARA Mobili	Characteristic	-386.40	351.66	66.89	1337.73	0.263
Concio 1	340	J[841]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	401	I[842]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000
Concio 1	401	J[843]	ST RARA Mobili	Characteristic	335.45	304.09	66.89	1337.73	0.227
Concio 1	402	I[843]	ST RARA Mobili	Characteristic	-985.85	893.69	66.89	1337.73	0.668
Concio 1	402	J[844]	ST RARA Mobili	Characteristic	-439.07	398.02	66.89	1337.73	0.298
Concio 1	403	I[844]	ST RARA Mobili	Characteristic	-709.52	643.19	66.89	1337.73	0.481
Concio 1	403	J[845]	ST RARA Mobili	Characteristic	-375.70	340.58	66.89	1337.73	0.255
Concio 2	404	I[845]	ST RARA Mobili	Characteristic	-375.70	332.86	66.89	1003.30	0.332
Concio 2	404	J[846]	ST RARA Mobili	Characteristic	437.40	387.53	66.89	1003.30	0.386
Concio 2	405	I[846]	ST RARA Mobili	Characteristic	-478.89	424.28	66.89	1003.30	0.423
Concio 2	405	J[847]	ST RARA Mobili	Characteristic	637.12	564.47	66.89	1003.30	0.563
Concio 2	406	I[847]	ST RARA Mobili	Characteristic	348.54	308.80	66.89	1003.30	0.308
Concio 2	406	J[848]	ST RARA Mobili	Characteristic	644.58	571.08	66.89	1003.30	0.569
Concio 3	407	I[848]	ST RARA Mobili	Characteristic	644.58	577.60	66.89	1337.73	0.432

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Concio 3	407	J[849]	ST RARA Mobili	Characteristic	858.53	769.31	66.89	1337.73	0.575
Concio 3	408	I[849]	ST RARA Mobili	Characteristic	578.41	518.30	66.89	1337.73	0.387
Concio 3	408	J[850]	ST RARA Mobili	Characteristic	1077.93	965.92	66.89	1337.73	0.722
Concio 3	409	I[850]	ST RARA Mobili	Characteristic	838.48	751.35	66.89	1337.73	0.562
Concio 3	409	J[851]	ST RARA Mobili	Characteristic	898.78	805.38	66.89	1337.73	0.602
Concio 4 H=var	410	I[851]	ST RARA Mobili	Characteristic	890.53	779.06	66.89	1337.73	0.582
Concio 4 H=var	410	J[852]	ST RARA Mobili	Characteristic	1173.77	707.97	66.89	1337.73	0.529
Concio 4 H=var	411	I[852]	ST RARA Mobili	Characteristic	996.81	601.24	66.89	1337.73	0.449
Concio 4 H=var	411	J[853]	ST RARA Mobili	Characteristic	1268.20	581.83	66.89	1337.73	0.435
Concio 4 H=200	412	I[853]	ST RARA Mobili	Characteristic	1269.78	582.55	66.89	1337.73	0.435
Concio 4 H=200	412	J[854]	ST RARA Mobili	Characteristic	1324.59	607.70	66.89	1337.73	0.454
Concio 4 H=200	413	I[854]	ST RARA Mobili	Characteristic	-1257.15	576.76	66.89	1337.73	0.431
Concio 4 H=200	413	J[855]	ST RARA Mobili	Characteristic	-1202.32	551.60	66.89	1337.73	0.412
Concio 4 H=var	414	I[855]	ST RARA Mobili	Characteristic	-1202.11	551.50	66.89	1337.73	0.412
Concio 4 H=var	414	J[856]	ST RARA Mobili	Characteristic	-930.30	561.12	66.89	1337.73	0.419
Concio 4 H=var	415	I[856]	ST RARA Mobili	Characteristic	-1104.63	666.26	66.89	1337.73	0.498
Concio 4 H=var	415	J[857]	ST RARA Mobili	Characteristic	-821.48	718.65	66.89	1337.73	0.537
Concio 5	416	I[857]	ST RARA Mobili	Characteristic	-828.24	742.18	66.89	1337.73	0.555
Concio 5	416	J[858]	ST RARA Mobili	Characteristic	-767.95	688.15	66.89	1337.73	0.514
Concio 5	417	I[858]	ST RARA Mobili	Characteristic	-1008.82	903.99	66.89	1337.73	0.676
Concio 5	417	J[859]	ST RARA Mobili	Characteristic	-496.35	444.77	66.89	1337.73	0.332
Concio 5	418	I[859]	ST RARA Mobili	Characteristic	-785.49	703.86	66.89	1337.73	0.526
Concio 5	418	J[860]	ST RARA Mobili	Characteristic	-544.04	487.50	66.89	1337.73	0.364
Concio 6	419	I[860]	ST RARA Mobili	Characteristic	-544.04	478.19	66.89	1003.30	0.477
Concio 6	419	J[861]	ST RARA Mobili	Characteristic	356.29	313.16	66.89	1003.30	0.312
Concio 6	420	I[861]	ST RARA Mobili	Characteristic	-558.21	490.64	66.89	1003.30	0.489
Concio 6	420	J[862]	ST RARA Mobili	Characteristic	-281.90	247.78	66.89	1003.30	0.247
Concio 6	421	I[862]	ST RARA Mobili	Characteristic	-281.90	247.78	66.89	1003.30	0.247
Concio 6	421	J[863]	ST RARA Mobili	Characteristic	558.08	490.52	66.89	1003.30	0.489
Concio 6	422	I[863]	ST RARA Mobili	Characteristic	-356.46	313.31	66.89	1003.30	0.312
Concio 6	422	J[864]	ST RARA Mobili	Characteristic	543.92	478.08	66.89	1003.30	0.477
Concio 5	423	I[864]	ST RARA Mobili	Characteristic	543.92	487.40	66.89	1337.73	0.364
Concio 5	423	J[865]	ST RARA Mobili	Characteristic	785.39	703.78	66.89	1337.73	0.526
Concio 5	424	I[865]	ST RARA Mobili	Characteristic	496.28	444.71	66.89	1337.73	0.332
Concio 5	424	J[866]	ST RARA Mobili	Characteristic	1008.62	903.81	66.89	1337.73	0.676
Concio 5	425	I[866]	ST RARA Mobili	Characteristic	767.84	688.05	66.89	1337.73	0.514
Concio 5	425	J[867]	ST RARA Mobili	Characteristic	828.13	742.08	66.89	1337.73	0.555
Concio 4 H=var	426	I[867]	ST RARA Mobili	Characteristic	821.35	718.54	66.89	1337.73	0.537
Concio 4 H=var	426	J[868]	ST RARA Mobili	Characteristic	1104.48	666.17	66.89	1337.73	0.498
Concio 4 H=var	427	I[868]	ST RARA Mobili	Characteristic	930.16	561.03	66.89	1337.73	0.419
Concio 4 H=var	427	J[869]	ST RARA Mobili	Characteristic	1201.92	551.42	66.89	1337.73	0.412
Concio 4 H=200	428	I[869]	ST RARA Mobili	Characteristic	1202.14	551.52	66.89	1337.73	0.412
Concio 4 H=200	428	J[870]	ST RARA Mobili	Characteristic	1256.97	576.67	66.89	1337.73	0.431
Concio 4 H=200	429	I[870]	ST RARA Mobili	Characteristic	-1324.88	607.83	66.89	1337.73	0.454
Concio 4 H=200	429	J[871]	ST RARA Mobili	Characteristic	-1270.07	582.68	66.89	1337.73	0.436
Concio 4 H=var	430	I[871]	ST RARA Mobili	Characteristic	-1268.48	581.95	66.89	1337.73	0.435
Concio 4 H=var	430	J[872]	ST RARA Mobili	Characteristic	-997.04	601.38	66.89	1337.73	0.450
Concio 4 H=var	431	I[872]	ST RARA Mobili	Characteristic	-1174.00	708.11	66.89	1337.73	0.529
Concio 4 H=var	431	J[873]	ST RARA Mobili	Characteristic	-890.73	779.23	66.89	1337.73	0.583
Concio 3	432	I[873]	ST RARA Mobili	Characteristic	-899.00	805.58	66.89	1337.73	0.602
Concio 3	432	J[874]	ST RARA Mobili	Characteristic	-838.70	751.54	66.89	1337.73	0.562
Concio 3	433	I[874]	ST RARA Mobili	Characteristic	-1078.23	966.18	66.89	1337.73	0.722
Concio 3	433	J[875]	ST RARA Mobili	Characteristic	-578.57	518.45	66.89	1337.73	0.388
Concio 3	434	I[875]	ST RARA Mobili	Characteristic	-858.71	769.48	66.89	1337.73	0.575
Concio 3	434	J[876]	ST RARA Mobili	Characteristic	-644.80	577.79	66.89	1337.73	0.432
Concio 2	435	I[876]	ST RARA Mobili	Characteristic	-644.80	571.27	66.89	1003.30	0.569
Concio 2	435	J[877]	ST RARA Mobili	Characteristic	-348.70	308.94	66.89	1003.30	0.308
Concio 2	436	I[877]	ST RARA Mobili	Characteristic	-637.40	564.72	66.89	1003.30	0.563
Concio 2	436	J[878]	ST RARA Mobili	Characteristic	478.58	424.01	66.89	1003.30	0.423

Concio 2	437	I[878]	ST RARA Mobili	Characteristic	-437.62	387.72	66.89	1003.30	0.386
Concio 2	437	J[879]	ST RARA Mobili	Characteristic	375.49	332.68	66.89	1003.30	0.332
Concio 1	438	I[879]	ST RARA Mobili	Characteristic	375.49	340.39	66.89	1337.73	0.254
Concio 1	438	J[880]	ST RARA Mobili	Characteristic	709.28	642.97	66.89	1337.73	0.481
Concio 1	439	I[880]	ST RARA Mobili	Characteristic	438.68	397.67	66.89	1337.73	0.297
Concio 1	439	J[881]	ST RARA Mobili	Characteristic	985.55	893.41	66.89	1337.73	0.668
Concio 1	440	I[881]	ST RARA Mobili	Characteristic	-335.45	304.09	66.89	1337.73	0.227
Concio 1	440	J[882]	ST RARA Mobili	Characteristic	0.00	0.00	66.89	1337.73	0.000

### 9.3 STATO LIMITE ULTIMO DI FATICA – RESISTENZA AL TAGLIO LONGITUDINALE

Si effettuano le verifiche allo stato limite ultimo per fatica a “danneggiamento accettabile” dei connettori trave-soletta.

L’impalcato si considera caricato secondo il modello di carico a fatica 3, applicato sulla corsia lenta (ved. 3.4.2).

Il coefficiente parziale di sicurezza per le verifiche dei connettori è pari a:

$$\gamma_{Mf,s} = 1 \quad \text{EN 1994-1-1:2005; § 6.8.2}$$

I coefficienti di equivalenza assumono i seguenti valori:

$$\lambda_{v1} = 1.55: \quad \text{EN 1994-2:2005, § 6.8.6.2(4)}$$

$$\lambda_{v2} = \frac{Q_{M1}}{Q_0} \left( \frac{N_{obs}}{N_0} \right)^{1/8} = 1.10: \quad \text{EN 1993-2, § 9.5.2}$$

$Q_{M1} = 440 \text{ kN}$ : massa complessiva a pieno carico autotreno o autoarticolato a 5 assi

$Q_0 = 480 \text{ kN}$

$N_{obs} = 2 \times 10^6$ : flusso annuo di veicoli pesanti sulla corsia lenta per strade ed autostrade caratterizzate da intenso traffico pesante.

$N_0 = 2 \times 10^6$

$$\lambda_{v3} = \left( \frac{t_{id}}{100} \right)^{1/8} = 0.871$$

$t_{id} = 50$  anni: vita di progetto del ponte

$\lambda_{v4} = 1$ : fattore per traffico pesante sulle altre corsie

$$\lambda_v = \lambda_{v1} \cdot \lambda_{v2} \cdot \lambda_{v3} \cdot \lambda_{v4} = 1.484$$

Seguono i tabulati di calcolo per ogni asta considerata, per le combinazioni di carichi più gravose.

Dati tabulati:

Elem property:	nome delle caratteristiche geometriche dell’elemento
Elem:	numero dell’elemento
Position:	nodo iniziale (I) o finale (J) dell’elemento
Lcom:	combinazione di carico più gravosa
Type:	sollecitazione (massima o minima)
Lamda_v:	coefficienti di danno equivalente
Delta_Tau:	ampiezza delle tensioni tangenziali per il carico da fatica
Delta_Tau_E,2 = Lamda_v x Delta_Tau:	ampiezza costante delle tensioni tangenziali relative a $2 \times 10^6$ cicli di carico all’anno
Delta_Tau_c:	tensione tangenziale limite (resistenza a fatica connettori a piolo)

$$\text{Verification Ratio} = \gamma_{Mf,s} \frac{\text{Delta\_Tau\_E\_2}}{\text{Delta\_Tau\_c}} \quad \text{verificato se } \leq 1$$



Elem property	Elem number	Position	Lcom	Type	Lamda_v	Delta_Tau (kN/m^2)	Delta_Tau_E_2 (kN/m^2)	Delta_Tau_c (kN/m^2)	Verif. Ratio
Concio 1	101	I[719]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	101	J[720]	ST FATICA DANN.ACC.	FX-MAX	1.484	1340.530	1989.733	90000.000	0.022
Concio 1	102	I[720]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25686.225	38125.774	90000.000	0.424
Concio 1	102	J[721]	ST FATICA DANN.ACC.	FZ-MIN	1.484	17307.913	25689.940	90000.000	0.285
Concio 1	103	I[721]	ST FATICA DANN.ACC.	FY-MIN	1.484	13094.068	19435.377	90000.000	0.216
Concio 1	103	J[722]	ST FATICA DANN.ACC.	FY-MIN	1.484	8067.081	11973.877	90000.000	0.133
Concio 2	104	I[722]	ST FATICA DANN.ACC.	FY-MIN	1.484	10512.408	15603.449	90000.000	0.173
Concio 2	104	J[723]	ST FATICA DANN.ACC.	MY-MIN	1.484	9091.624	13494.595	90000.000	0.150
Concio 2	105	I[723]	ST FATICA DANN.ACC.	FY-MAX	1.484	7472.608	11091.507	90000.000	0.123
Concio 2	105	J[724]	ST FATICA DANN.ACC.	MZ-MIN	1.484	18846.335	27973.402	90000.000	0.311
Concio 2	106	I[724]	ST FATICA DANN.ACC.	FX-MIN	1.484	22584.508	33521.930	90000.000	0.373
Concio 2	106	J[725]	ST FATICA DANN.ACC.	MZ-MIN	1.484	29148.470	43264.746	90000.000	0.481
Concio 3	107	I[725]	ST FATICA DANN.ACC.	MZ-MIN	1.484	22110.854	32818.891	90000.000	0.365
Concio 3	107	J[726]	ST FATICA DANN.ACC.	MZ-MIN	1.484	26185.267	38866.496	90000.000	0.432
Concio 3	108	I[726]	ST FATICA DANN.ACC.	FZ-MAX	1.484	26936.646	39981.760	90000.000	0.444
Concio 3	108	J[727]	ST FATICA DANN.ACC.	MZ-MIN	1.484	37076.412	55032.100	90000.000	0.612
Concio 3	109	I[727]	ST FATICA DANN.ACC.	FZ-MAX	1.484	35035.588	52002.927	90000.000	0.578
Concio 3	109	J[728]	ST FATICA DANN.ACC.	FZ-MAX	1.484	35863.782	53232.206	90000.000	0.592
Concio 4 H=var	110	I[728]	ST FATICA DANN.ACC.	MX-MIN	1.484	35318.228	52422.447	90000.000	0.583
Concio 4 H=var	110	J[729]	ST FATICA DANN.ACC.	MZ-MIN	1.484	29390.096	43623.388	90000.000	0.485
Concio 4 H=var	111	I[729]	ST FATICA DANN.ACC.	FZ-MAX	1.484	27098.383	40221.824	90000.000	0.447
Concio 4 H=var	111	J[730]	ST FATICA DANN.ACC.	FZ-MAX	1.484	22940.947	34050.988	90000.000	0.378
Concio 4 H=200	112	I[730]	ST FATICA DANN.ACC.	MX-MIN	1.484	23794.002	35317.169	90000.000	0.392
Concio 4 H=200	112	J[731]	ST FATICA DANN.ACC.	MZ-MIN	1.484	24884.262	36935.429	90000.000	0.410
Concio 4 H=200	113	I[731]	ST FATICA DANN.ACC.	FY-MIN	1.484	21859.004	32445.073	90000.000	0.361
Concio 4 H=200	113	J[732]	ST FATICA DANN.ACC.	FY-MIN	1.484	21393.438	31754.038	90000.000	0.353
Concio 4 H=var	114	I[732]	ST FATICA DANN.ACC.	FY-MIN	1.484	21634.393	32111.686	90000.000	0.357
Concio 4 H=var	114	J[733]	ST FATICA DANN.ACC.	FY-MIN	1.484	25380.660	37672.228	90000.000	0.419
Concio 4 H=var	115	I[733]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25294.055	37543.680	90000.000	0.417
Concio 4 H=var	115	J[734]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32244.083	47859.529	90000.000	0.532
Concio 5	116	I[734]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32497.590	48235.806	90000.000	0.536
Concio 5	116	J[735]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31669.396	47006.527	90000.000	0.522
Concio 5	117	I[735]	ST FATICA DANN.ACC.	MZ-MIN	1.484	31593.476	46893.839	90000.000	0.521
Concio 5	117	J[736]	ST FATICA DANN.ACC.	FZ-MIN	1.484	22920.355	34020.425	90000.000	0.378
Concio 5	118	I[736]	ST FATICA DANN.ACC.	FZ-MIN	1.484	20410.672	30295.330	90000.000	0.337
Concio 5	118	J[737]	ST FATICA DANN.ACC.	FZ-MIN	1.484	16600.979	24640.646	90000.000	0.274
Concio 6	119	I[737]	ST FATICA DANN.ACC.	FZ-MIN	1.484	21711.611	32226.299	90000.000	0.358
Concio 6	119	J[738]	ST FATICA DANN.ACC.	FZ-MIN	1.484	15429.315	22901.558	90000.000	0.255
Concio 6	120	I[738]	ST FATICA DANN.ACC.	FZ-MIN	1.484	12045.473	17878.960	90000.000	0.199
Concio 6	120	J[739]	ST FATICA DANN.ACC.	FZ-MIN	1.484	6413.070	9518.847	90000.000	0.106
Concio 6	121	I[739]	ST FATICA DANN.ACC.	FZ-MIN	1.484	6413.070	9518.847	90000.000	0.106
Concio 6	121	J[740]	ST FATICA DANN.ACC.	MZ-MIN	1.484	11828.741	17557.266	90000.000	0.195
Concio 6	122	I[740]	ST FATICA DANN.ACC.	FX-MIN	1.484	13413.492	19909.494	90000.000	0.221
Concio 6	122	J[741]	ST FATICA DANN.ACC.	MZ-MIN	1.484	20774.952	30836.026	90000.000	0.343
Concio 5	123	I[741]	ST FATICA DANN.ACC.	MZ-MIN	1.484	15884.797	23577.626	90000.000	0.262
Concio 5	123	J[742]	ST FATICA DANN.ACC.	MZ-MIN	1.484	20225.767	30020.878	90000.000	0.334
Concio 5	124	I[742]	ST FATICA DANN.ACC.	FZ-MAX	1.484	20280.306	30101.829	90000.000	0.335
Concio 5	124	J[743]	ST FATICA DANN.ACC.	MZ-MIN	1.484	31019.101	46041.301	90000.000	0.512
Concio 5	125	I[743]	ST FATICA DANN.ACC.	FZ-MAX	1.484	28580.183	42421.243	90000.000	0.471
Concio 5	125	J[744]	ST FATICA DANN.ACC.	FZ-MAX	1.484	29408.377	43650.522	90000.000	0.485
Concio 4 H=var	126	I[744]	ST FATICA DANN.ACC.	MX-MIN	1.484	29164.747	43288.905	90000.000	0.481
Concio 4 H=var	126	J[745]	ST FATICA DANN.ACC.	MZ-MIN	1.484	25147.567	37326.249	90000.000	0.415
Concio 4 H=var	127	I[745]	ST FATICA DANN.ACC.	FZ-MAX	1.484	22962.677	34083.243	90000.000	0.379
Concio 4 H=var	127	J[746]	ST FATICA DANN.ACC.	MZ-MIN	1.484	21440.012	31823.168	90000.000	0.354
Concio 4 H=200	128	I[746]	ST FATICA DANN.ACC.	MZ-MIN	1.484	21293.918	31606.323	90000.000	0.351
Concio 4 H=200	128	J[747]	ST FATICA DANN.ACC.	MZ-MIN	1.484	21698.704	32207.141	90000.000	0.358

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Concio 4 H=200	129	I[747]	ST FATICA DANN.ACC.	FY-MIN	1.484	25061.161	37197.999	90000.000	0.413
Concio 4 H=200	129	J[748]	ST FATICA DANN.ACC.	FY-MIN	1.484	24595.595	36506.964	90000.000	0.406
Concio 4 H=var	130	I[748]	ST FATICA DANN.ACC.	FY-MIN	1.484	24782.205	36783.947	90000.000	0.409
Concio 4 H=var	130	J[749]	ST FATICA DANN.ACC.	FY-MIN	1.484	29519.081	43814.839	90000.000	0.487
Concio 4 H=var	131	I[749]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29519.736	43815.811	90000.000	0.487
Concio 4 H=var	131	J[750]	ST FATICA DANN.ACC.	FZ-MIN	1.484	38373.058	56956.697	90000.000	0.633
Concio 3	132	I[750]	ST FATICA DANN.ACC.	FZ-MIN	1.484	38952.278	57816.426	90000.000	0.642
Concio 3	132	J[751]	ST FATICA DANN.ACC.	FZ-MIN	1.484	38124.084	56587.147	90000.000	0.629
Concio 3	133	I[751]	ST FATICA DANN.ACC.	MZ-MIN	1.484	37807.795	56117.683	90000.000	0.624
Concio 3	133	J[752]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29479.774	43756.495	90000.000	0.486
Concio 3	134	I[752]	ST FATICA DANN.ACC.	MZ-MIN	1.484	26646.452	39551.028	90000.000	0.440
Concio 3	134	J[753]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23302.856	34588.166	90000.000	0.384
Concio 2	135	I[753]	ST FATICA DANN.ACC.	FZ-MIN	1.484	30719.872	45597.159	90000.000	0.507
Concio 2	135	J[754]	ST FATICA DANN.ACC.	FZ-MIN	1.484	24169.083	35873.898	90000.000	0.399
Concio 2	136	I[754]	ST FATICA DANN.ACC.	FY-MIN	1.484	19410.049	28810.117	90000.000	0.320
Concio 2	136	J[755]	ST FATICA DANN.ACC.	FY-MIN	1.484	8492.068	12604.681	90000.000	0.140
Concio 2	137	I[755]	ST FATICA DANN.ACC.	FZ-MIN	1.484	9080.597	13478.227	90000.000	0.150
Concio 2	137	J[756]	ST FATICA DANN.ACC.	FX-MIN	1.484	8979.964	13328.859	90000.000	0.148
Concio 1	138	I[756]	ST FATICA DANN.ACC.	FX-MIN	1.484	6891.104	10228.387	90000.000	0.114
Concio 1	138	J[757]	ST FATICA DANN.ACC.	MZ-MIN	1.484	12826.399	19038.079	90000.000	0.212
Concio 1	139	I[757]	ST FATICA DANN.ACC.	FX-MIN	1.484	15913.302	23619.934	90000.000	0.262
Concio 1	139	J[758]	ST FATICA DANN.ACC.	MZ-MIN	1.484	24816.542	36834.914	90000.000	0.409
Concio 1	140	I[758]	ST FATICA DANN.ACC.	FX-MAX	1.484	1340.530	1989.733	90000.000	0.022
Concio 1	140	J[759]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	201	I[760]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	201	J[761]	ST FATICA DANN.ACC.	FZ-MAX	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	202	I[761]	ST FATICA DANN.ACC.	FZ-MIN	1.484	39471.667	58587.350	90000.000	0.651
Concio 1	202	J[762]	ST FATICA DANN.ACC.	MX-MAX	1.484	14478.741	21490.632	90000.000	0.239
Concio 1	203	I[762]	ST FATICA DANN.ACC.	FY-MIN	1.484	30212.998	44844.812	90000.000	0.498
Concio 1	203	J[763]	ST FATICA DANN.ACC.	FY-MIN	1.484	14930.316	22160.899	90000.000	0.246
Concio 2	204	I[763]	ST FATICA DANN.ACC.	FY-MIN	1.484	19462.107	28887.385	90000.000	0.321
Concio 2	204	J[764]	ST FATICA DANN.ACC.	MX-MIN	1.484	30646.395	45488.099	90000.000	0.505
Concio 2	205	I[764]	ST FATICA DANN.ACC.	FZ-MIN	1.484	18761.810	27847.943	90000.000	0.309
Concio 2	205	J[765]	ST FATICA DANN.ACC.	MX-MIN	1.484	44182.711	65579.899	90000.000	0.729
Concio 2	206	I[765]	ST FATICA DANN.ACC.	MX-MIN	1.484	30019.125	44557.048	90000.000	0.495
Concio 2	206	J[766]	ST FATICA DANN.ACC.	MX-MIN	1.484	45293.443	67228.547	90000.000	0.747
Concio 3	207	I[766]	ST FATICA DANN.ACC.	MX-MIN	1.484	34375.915	51023.783	90000.000	0.567
Concio 3	207	J[767]	ST FATICA DANN.ACC.	MX-MIN	1.484	43402.482	64421.815	90000.000	0.716
Concio 3	208	I[767]	ST FATICA DANN.ACC.	MX-MIN	1.484	33266.055	49376.430	90000.000	0.549
Concio 3	208	J[768]	ST FATICA DANN.ACC.	MX-MIN	1.484	54346.026	80665.192	90000.000	0.896
Concio 3	209	I[768]	ST FATICA DANN.ACC.	FZ-MAX	1.484	42742.694	63442.499	90000.000	0.705
Concio 3	209	J[769]	ST FATICA DANN.ACC.	MX-MIN	1.484	45688.719	67815.250	90000.000	0.754
Concio 4 H=var	210	I[769]	ST FATICA DANN.ACC.	MX-MIN	1.484	46642.661	69231.175	90000.000	0.769
Concio 4 H=var	210	J[770]	ST FATICA DANN.ACC.	MX-MIN	1.484	40559.600	60202.156	90000.000	0.669
Concio 4 H=var	211	I[770]	ST FATICA DANN.ACC.	FZ-MAX	1.484	34712.643	51523.585	90000.000	0.573
Concio 4 H=var	211	J[771]	ST FATICA DANN.ACC.	MX-MIN	1.484	34517.899	51234.527	90000.000	0.569
Concio 4 H=200	212	I[771]	ST FATICA DANN.ACC.	MX-MIN	1.484	34377.352	51025.916	90000.000	0.567
Concio 4 H=200	212	J[772]	ST FATICA DANN.ACC.	MX-MIN	1.484	35726.282	53028.116	90000.000	0.589
Concio 4 H=200	213	I[772]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29643.797	43999.954	90000.000	0.489
Concio 4 H=200	213	J[773]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28124.259	41744.520	90000.000	0.464
Concio 4 H=var	214	I[773]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28517.591	42328.339	90000.000	0.470
Concio 4 H=var	214	J[774]	ST FATICA DANN.ACC.	FX-MIN	1.484	28560.595	42392.168	90000.000	0.471
Concio 4 H=var	215	I[774]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32548.832	48311.864	90000.000	0.537
Concio 4 H=var	215	J[775]	ST FATICA DANN.ACC.	FX-MIN	1.484	38871.418	57696.407	90000.000	0.641
Concio 5	216	I[775]	ST FATICA DANN.ACC.	FX-MIN	1.484	39661.600	58869.266	90000.000	0.654
Concio 5	216	J[776]	ST FATICA DANN.ACC.	FX-MIN	1.484	39196.377	58178.739	90000.000	0.646
Concio 5	217	I[776]	ST FATICA DANN.ACC.	FZ-MIN	1.484	42677.023	63345.023	90000.000	0.704
Concio 5	217	J[777]	ST FATICA DANN.ACC.	FX-MIN	1.484	27574.516	40928.543	90000.000	0.455
Concio 5	218	I[777]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31280.501	46429.294	90000.000	0.516

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Concio 5	218	J[778]	ST FATICA DANN.ACC.	FZ-MIN	1.484	19544.647	29009.899	90000.000	0.322
Concio 6	219	I[778]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25539.571	37908.097	90000.000	0.421
Concio 6	219	J[779]	ST FATICA DANN.ACC.	MX-MIN	1.484	23355.022	34665.595	90000.000	0.385
Concio 6	220	I[779]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28092.565	41697.477	90000.000	0.463
Concio 6	220	J[780]	ST FATICA DANN.ACC.	MX-MIN	1.484	21439.220	31821.993	90000.000	0.354
Concio 6	221	I[780]	ST FATICA DANN.ACC.	MX-MIN	1.484	21439.220	31821.993	90000.000	0.354
Concio 6	221	J[781]	ST FATICA DANN.ACC.	MX-MIN	1.484	36231.889	53778.584	90000.000	0.598
Concio 6	222	I[781]	ST FATICA DANN.ACC.	MX-MIN	1.484	21739.445	32267.613	90000.000	0.359
Concio 6	222	J[782]	ST FATICA DANN.ACC.	MX-MIN	1.484	35698.277	52986.549	90000.000	0.589
Concio 5	223	I[782]	ST FATICA DANN.ACC.	MX-MIN	1.484	27318.792	40548.974	90000.000	0.451
Concio 5	223	J[783]	ST FATICA DANN.ACC.	MX-MIN	1.484	37472.447	55619.931	90000.000	0.618
Concio 5	224	I[783]	ST FATICA DANN.ACC.	MX-MIN	1.484	26860.199	39868.290	90000.000	0.443
Concio 5	224	J[784]	ST FATICA DANN.ACC.	MX-MIN	1.484	48662.629	72229.391	90000.000	0.803
Concio 5	225	I[784]	ST FATICA DANN.ACC.	FZ-MAX	1.484	36659.139	54412.746	90000.000	0.605
Concio 5	225	J[785]	ST FATICA DANN.ACC.	MX-MIN	1.484	39558.097	58715.636	90000.000	0.652
Concio 4 H=var	226	I[785]	ST FATICA DANN.ACC.	MX-MIN	1.484	40765.157	60507.262	90000.000	0.672
Concio 4 H=var	226	J[786]	ST FATICA DANN.ACC.	MX-MIN	1.484	36472.757	54136.101	90000.000	0.602
Concio 4 H=var	227	I[786]	ST FATICA DANN.ACC.	FZ-MAX	1.484	30506.645	45280.668	90000.000	0.503
Concio 4 H=var	227	J[787]	ST FATICA DANN.ACC.	MX-MIN	1.484	31309.859	46472.870	90000.000	0.516
Concio 4 H=200	228	I[787]	ST FATICA DANN.ACC.	MX-MIN	1.484	31144.582	46227.551	90000.000	0.514
Concio 4 H=200	228	J[788]	ST FATICA DANN.ACC.	MX-MIN	1.484	32491.567	48226.866	90000.000	0.536
Concio 4 H=200	229	I[788]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32888.108	48815.447	90000.000	0.542
Concio 4 H=200	229	J[789]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31366.224	46556.532	90000.000	0.517
Concio 4 H=var	230	I[789]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31729.694	47096.026	90000.000	0.523
Concio 4 H=var	230	J[790]	ST FATICA DANN.ACC.	FX-MIN	1.484	32838.376	48741.631	90000.000	0.542
Concio 4 H=var	231	I[790]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36649.916	54399.057	90000.000	0.604
Concio 4 H=var	231	J[791]	ST FATICA DANN.ACC.	FX-MIN	1.484	44780.109	66466.611	90000.000	0.739
Concio 3	232	I[791]	ST FATICA DANN.ACC.	FX-MIN	1.484	45880.059	68099.254	90000.000	0.757
Concio 3	232	J[792]	ST FATICA DANN.ACC.	FX-MIN	1.484	45414.835	67408.728	90000.000	0.749
Concio 3	233	I[792]	ST FATICA DANN.ACC.	FZ-MIN	1.484	48364.034	71786.190	90000.000	0.798
Concio 3	233	J[793]	ST FATICA DANN.ACC.	FX-MIN	1.484	33703.255	50025.360	90000.000	0.556
Concio 3	234	I[793]	ST FATICA DANN.ACC.	FZ-MIN	1.484	37252.823	55293.944	90000.000	0.614
Concio 3	234	J[794]	ST FATICA DANN.ACC.	FZ-MIN	1.484	26909.890	39942.046	90000.000	0.444
Concio 2	235	I[794]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35456.265	52627.335	90000.000	0.585
Concio 2	235	J[795]	ST FATICA DANN.ACC.	FY-MIN	1.484	24131.030	35817.415	90000.000	0.398
Concio 2	236	I[795]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36229.745	53775.402	90000.000	0.598
Concio 2	236	J[796]	ST FATICA DANN.ACC.	MX-MIN	1.484	26906.177	39936.535	90000.000	0.444
Concio 2	237	I[796]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23187.661	34417.184	90000.000	0.382
Concio 2	237	J[797]	ST FATICA DANN.ACC.	MX-MIN	1.484	21676.391	32174.022	90000.000	0.358
Concio 1	238	I[797]	ST FATICA DANN.ACC.	MX-MIN	1.484	16628.999	24682.236	90000.000	0.274
Concio 1	238	J[798]	ST FATICA DANN.ACC.	MX-MIN	1.484	30109.376	44691.006	90000.000	0.497
Concio 1	239	I[798]	ST FATICA DANN.ACC.	FZ-MAX	1.484	16855.280	25018.102	90000.000	0.278
Concio 1	239	J[799]	ST FATICA DANN.ACC.	MX-MIN	1.484	43191.979	64109.367	90000.000	0.712
Concio 1	240	I[799]	ST FATICA DANN.ACC.	FZ-MIN	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	240	J[800]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	301	I[801]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	301	J[802]	ST FATICA DANN.ACC.	FZ-MAX	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	302	I[802]	ST FATICA DANN.ACC.	FZ-MIN	1.484	39471.671	58587.355	90000.000	0.651
Concio 1	302	J[803]	ST FATICA DANN.ACC.	MX-MIN	1.484	19372.016	28753.664	90000.000	0.320
Concio 1	303	I[803]	ST FATICA DANN.ACC.	FZ-MIN	1.484	24085.483	35749.811	90000.000	0.397
Concio 1	303	J[804]	ST FATICA DANN.ACC.	FZ-MAX	1.484	13670.483	20290.943	90000.000	0.226
Concio 2	304	I[804]	ST FATICA DANN.ACC.	FZ-MAX	1.484	17819.877	26449.842	90000.000	0.294
Concio 2	304	J[805]	ST FATICA DANN.ACC.	FZ-MAX	1.484	29548.348	43858.279	90000.000	0.487
Concio 2	305	I[805]	ST FATICA DANN.ACC.	FZ-MIN	1.484	18761.805	27847.936	90000.000	0.309
Concio 2	305	J[806]	ST FATICA DANN.ACC.	FZ-MAX	1.484	41736.055	61948.355	90000.000	0.688
Concio 2	306	I[806]	ST FATICA DANN.ACC.	FZ-MAX	1.484	28662.116	42542.855	90000.000	0.473
Concio 2	306	J[807]	ST FATICA DANN.ACC.	FZ-MAX	1.484	42894.205	63667.384	90000.000	0.707
Concio 3	307	I[807]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32554.989	48321.002	90000.000	0.537
Concio 3	307	J[808]	ST FATICA DANN.ACC.	FZ-MAX	1.484	40758.746	60497.746	90000.000	0.672



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Concio 3	308	I[808]	ST FATICA DANN.ACC.	FZ-MAX	1.484	31433.279	46656.062	90000.000	0.518
Concio 3	308	J[809]	ST FATICA DANN.ACC.	FZ-MAX	1.484	50810.420	75417.333	90000.000	0.838
Concio 3	309	I[809]	ST FATICA DANN.ACC.	FZ-MAX	1.484	42742.694	63442.499	90000.000	0.705
Concio 3	309	J[810]	ST FATICA DANN.ACC.	FZ-MAX	1.484	44532.632	66099.284	90000.000	0.734
Concio 4 H=var	310	I[810]	ST FATICA DANN.ACC.	FZ-MAX	1.484	43937.202	65215.493	90000.000	0.725
Concio 4 H=var	310	J[811]	ST FATICA DANN.ACC.	FZ-MAX	1.484	38067.346	56502.932	90000.000	0.628
Concio 4 H=var	311	I[811]	ST FATICA DANN.ACC.	FZ-MAX	1.484	34712.643	51523.585	90000.000	0.573
Concio 4 H=var	311	J[812]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32434.021	48141.451	90000.000	0.535
Concio 4 H=200	312	I[812]	ST FATICA DANN.ACC.	FZ-MAX	1.484	32157.630	47731.207	90000.000	0.530
Concio 4 H=200	312	J[813]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33374.747	49537.761	90000.000	0.550
Concio 4 H=200	313	I[813]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29643.797	43999.954	90000.000	0.489
Concio 4 H=200	313	J[814]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28124.259	41744.520	90000.000	0.464
Concio 4 H=var	314	I[814]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28517.590	42328.336	90000.000	0.470
Concio 4 H=var	314	J[815]	ST FATICA DANN.ACC.	FX-MIN	1.484	28560.595	42392.168	90000.000	0.471
Concio 4 H=var	315	I[815]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32548.831	48311.862	90000.000	0.537
Concio 4 H=var	315	J[816]	ST FATICA DANN.ACC.	FX-MIN	1.484	38871.417	57696.405	90000.000	0.641
Concio 5	316	I[816]	ST FATICA DANN.ACC.	FX-MIN	1.484	39661.600	58869.266	90000.000	0.654
Concio 5	316	J[817]	ST FATICA DANN.ACC.	FX-MIN	1.484	39196.377	58178.739	90000.000	0.646
Concio 5	317	I[817]	ST FATICA DANN.ACC.	FZ-MIN	1.484	42677.023	63345.023	90000.000	0.704
Concio 5	317	J[818]	ST FATICA DANN.ACC.	MX-MIN	1.484	30781.081	45688.012	90000.000	0.508
Concio 5	318	I[818]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31280.499	46429.291	90000.000	0.516
Concio 5	318	J[819]	ST FATICA DANN.ACC.	FZ-MIN	1.484	19544.649	29009.902	90000.000	0.322
Concio 6	319	I[819]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25539.574	37908.101	90000.000	0.421
Concio 6	319	J[820]	ST FATICA DANN.ACC.	FZ-MAX	1.484	21508.946	31925.485	90000.000	0.355
Concio 6	320	I[820]	ST FATICA DANN.ACC.	FZ-MIN	1.484	28092.565	41697.477	90000.000	0.463
Concio 6	320	J[821]	ST FATICA DANN.ACC.	FZ-MAX	1.484	19916.963	29562.523	90000.000	0.329
Concio 6	321	I[821]	ST FATICA DANN.ACC.	FZ-MAX	1.484	19916.963	29562.523	90000.000	0.329
Concio 6	321	J[822]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33504.590	49730.484	90000.000	0.553
Concio 6	322	I[822]	ST FATICA DANN.ACC.	FZ-MAX	1.484	20152.447	29912.050	90000.000	0.332
Concio 6	322	J[823]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33281.533	49399.403	90000.000	0.549
Concio 5	323	I[823]	ST FATICA DANN.ACC.	FZ-MAX	1.484	25469.332	37803.842	90000.000	0.420
Concio 5	323	J[824]	ST FATICA DANN.ACC.	FZ-MAX	1.484	34748.127	51576.253	90000.000	0.573
Concio 5	324	I[824]	ST FATICA DANN.ACC.	FZ-MAX	1.484	25141.387	37317.077	90000.000	0.415
Concio 5	324	J[825]	ST FATICA DANN.ACC.	FZ-MAX	1.484	45069.710	66896.462	90000.000	0.743
Concio 5	325	I[825]	ST FATICA DANN.ACC.	FZ-MAX	1.484	36659.139	54412.746	90000.000	0.605
Concio 5	325	J[826]	ST FATICA DANN.ACC.	FZ-MAX	1.484	38440.636	57057.002	90000.000	0.634
Concio 4 H=var	326	I[826]	ST FATICA DANN.ACC.	FZ-MAX	1.484	38035.897	56456.252	90000.000	0.627
Concio 4 H=var	326	J[827]	ST FATICA DANN.ACC.	FZ-MAX	1.484	33966.328	50415.838	90000.000	0.560
Concio 4 H=var	327	I[827]	ST FATICA DANN.ACC.	FZ-MAX	1.484	30506.645	45280.668	90000.000	0.503
Concio 4 H=var	327	J[828]	ST FATICA DANN.ACC.	FZ-MAX	1.484	29222.207	43374.192	90000.000	0.482
Concio 4 H=200	328	I[828]	ST FATICA DANN.ACC.	FZ-MAX	1.484	28915.669	42919.202	90000.000	0.477
Concio 4 H=200	328	J[829]	ST FATICA DANN.ACC.	FZ-MAX	1.484	30130.899	44722.953	90000.000	0.497
Concio 4 H=200	329	I[829]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32888.108	48815.447	90000.000	0.542
Concio 4 H=200	329	J[830]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31366.224	46556.532	90000.000	0.517
Concio 4 H=var	330	I[830]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31729.692	47096.024	90000.000	0.523
Concio 4 H=var	330	J[831]	ST FATICA DANN.ACC.	FX-MIN	1.484	32838.376	48741.631	90000.000	0.542
Concio 4 H=var	331	I[831]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36649.916	54399.057	90000.000	0.604
Concio 4 H=var	331	J[832]	ST FATICA DANN.ACC.	FX-MIN	1.484	44780.108	66466.608	90000.000	0.739
Concio 3	332	I[832]	ST FATICA DANN.ACC.	FX-MIN	1.484	45880.058	68099.251	90000.000	0.757
Concio 3	332	J[833]	ST FATICA DANN.ACC.	FX-MIN	1.484	45414.834	67408.725	90000.000	0.749
Concio 3	333	I[833]	ST FATICA DANN.ACC.	FZ-MIN	1.484	48364.034	71786.190	90000.000	0.798
Concio 3	333	J[834]	ST FATICA DANN.ACC.	FX-MIN	1.484	33703.255	50025.360	90000.000	0.556
Concio 3	334	I[834]	ST FATICA DANN.ACC.	FZ-MIN	1.484	37252.825	55293.947	90000.000	0.614
Concio 3	334	J[835]	ST FATICA DANN.ACC.	FZ-MIN	1.484	26909.890	39942.046	90000.000	0.444
Concio 2	335	I[835]	ST FATICA DANN.ACC.	FZ-MIN	1.484	35456.265	52627.335	90000.000	0.585
Concio 2	335	J[836]	ST FATICA DANN.ACC.	FY-MAX	1.484	20898.299	31019.109	90000.000	0.345
Concio 2	336	I[836]	ST FATICA DANN.ACC.	FZ-MIN	1.484	36229.745	53775.402	90000.000	0.598
Concio 2	336	J[837]	ST FATICA DANN.ACC.	FZ-MAX	1.484	24315.101	36090.630	90000.000	0.401
Concio 2	337	I[837]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23187.661	34417.184	90000.000	0.382

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Concio 2	337	J[838]	ST FATICA DANN.ACC.	FZ-MAX	1.484	19676.453	29205.537	90000.000	0.325
Concio 1	338	I[838]	ST FATICA DANN.ACC.	FZ-MAX	1.484	15094.751	22404.969	90000.000	0.249
Concio 1	338	J[839]	ST FATICA DANN.ACC.	FZ-MAX	1.484	27521.682	40850.122	90000.000	0.454
Concio 1	339	I[839]	ST FATICA DANN.ACC.	FZ-MAX	1.484	16855.280	25018.102	90000.000	0.278
Concio 1	339	J[840]	ST FATICA DANN.ACC.	FZ-MAX	1.484	39847.351	59144.973	90000.000	0.657
Concio 1	340	I[840]	ST FATICA DANN.ACC.	FZ-MIN	1.484	14859.676	22056.049	90000.000	0.245
Concio 1	340	J[841]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	401	I[842]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000
Concio 1	401	J[843]	ST FATICA DANN.ACC.	FX-MAX	1.484	1340.530	1989.733	90000.000	0.022
Concio 1	402	I[843]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25686.225	38125.774	90000.000	0.424
Concio 1	402	J[844]	ST FATICA DANN.ACC.	FZ-MIN	1.484	17307.913	25689.940	90000.000	0.285
Concio 1	403	I[844]	ST FATICA DANN.ACC.	FZ-MIN	1.484	13094.067	19435.375	90000.000	0.216
Concio 1	403	J[845]	ST FATICA DANN.ACC.	FZ-MIN	1.484	8067.080	11973.875	90000.000	0.133
Concio 2	404	I[845]	ST FATICA DANN.ACC.	FZ-MIN	1.484	10512.407	15603.447	90000.000	0.173
Concio 2	404	J[846]	ST FATICA DANN.ACC.	MY-MIN	1.484	9091.624	13494.595	90000.000	0.150
Concio 2	405	I[846]	ST FATICA DANN.ACC.	MX-MIN	1.484	7995.824	11868.111	90000.000	0.132
Concio 2	405	J[847]	ST FATICA DANN.ACC.	MX-MIN	1.484	18913.805	28073.547	90000.000	0.312
Concio 2	406	I[847]	ST FATICA DANN.ACC.	MZ-MIN	1.484	23833.186	35375.329	90000.000	0.393
Concio 2	406	J[848]	ST FATICA DANN.ACC.	FY-MIN	1.484	30341.785	45035.970	90000.000	0.500
Concio 3	407	I[848]	ST FATICA DANN.ACC.	FY-MIN	1.484	23016.055	34162.470	90000.000	0.380
Concio 3	407	J[849]	ST FATICA DANN.ACC.	FY-MIN	1.484	26328.831	39079.586	90000.000	0.434
Concio 3	408	I[849]	ST FATICA DANN.ACC.	FY-MIN	1.484	29368.259	43590.975	90000.000	0.484
Concio 3	408	J[850]	ST FATICA DANN.ACC.	FY-MIN	1.484	37650.199	55883.765	90000.000	0.621
Concio 3	409	I[850]	ST FATICA DANN.ACC.	FY-MIN	1.484	37470.759	55617.424	90000.000	0.618
Concio 3	409	J[851]	ST FATICA DANN.ACC.	FY-MIN	1.484	38298.953	56846.703	90000.000	0.632
Concio 4 H=var	410	I[851]	ST FATICA DANN.ACC.	FY-MIN	1.484	37558.973	55748.359	90000.000	0.619
Concio 4 H=var	410	J[852]	ST FATICA DANN.ACC.	FY-MIN	1.484	28958.457	42982.711	90000.000	0.478
Concio 4 H=var	411	I[852]	ST FATICA DANN.ACC.	FY-MIN	1.484	29120.105	43222.643	90000.000	0.480
Concio 4 H=var	411	J[853]	ST FATICA DANN.ACC.	FY-MIN	1.484	24478.731	36333.504	90000.000	0.404
Concio 4 H=200	412	I[853]	ST FATICA DANN.ACC.	FY-MIN	1.484	24337.004	36123.141	90000.000	0.401
Concio 4 H=200	412	J[854]	ST FATICA DANN.ACC.	FY-MIN	1.484	24802.571	36814.176	90000.000	0.409
Concio 4 H=200	413	I[854]	ST FATICA DANN.ACC.	FZ-MIN	1.484	21814.549	32379.088	90000.000	0.360
Concio 4 H=200	413	J[855]	ST FATICA DANN.ACC.	FZ-MIN	1.484	21348.982	31688.054	90000.000	0.352
Concio 4 H=var	414	I[855]	ST FATICA DANN.ACC.	FZ-MIN	1.484	21585.191	32038.656	90000.000	0.356
Concio 4 H=var	414	J[856]	ST FATICA DANN.ACC.	MZ-MIN	1.484	25342.486	37615.565	90000.000	0.418
Concio 4 H=var	415	I[856]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25294.055	37543.680	90000.000	0.417
Concio 4 H=var	415	J[857]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32244.083	47859.529	90000.000	0.532
Concio 5	416	I[857]	ST FATICA DANN.ACC.	FZ-MIN	1.484	32497.592	48235.808	90000.000	0.536
Concio 5	416	J[858]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31669.398	47006.529	90000.000	0.522
Concio 5	417	I[858]	ST FATICA DANN.ACC.	FZ-MIN	1.484	31533.573	46804.926	90000.000	0.520
Concio 5	417	J[859]	ST FATICA DANN.ACC.	FZ-MIN	1.484	22920.355	34020.425	90000.000	0.378
Concio 5	418	I[859]	ST FATICA DANN.ACC.	FZ-MIN	1.484	20410.672	30295.330	90000.000	0.337
Concio 5	418	J[860]	ST FATICA DANN.ACC.	FZ-MIN	1.484	16600.979	24640.646	90000.000	0.274
Concio 6	419	I[860]	ST FATICA DANN.ACC.	FZ-MIN	1.484	21711.611	32226.299	90000.000	0.358
Concio 6	419	J[861]	ST FATICA DANN.ACC.	FZ-MIN	1.484	15429.315	22901.558	90000.000	0.255
Concio 6	420	I[861]	ST FATICA DANN.ACC.	FZ-MIN	1.484	12045.473	17878.960	90000.000	0.199
Concio 6	420	J[862]	ST FATICA DANN.ACC.	FZ-MIN	1.484	6413.070	9518.847	90000.000	0.106
Concio 6	421	I[862]	ST FATICA DANN.ACC.	FZ-MIN	1.484	6413.070	9518.847	90000.000	0.106
Concio 6	421	J[863]	ST FATICA DANN.ACC.	FY-MIN	1.484	11828.740	17557.265	90000.000	0.195
Concio 6	422	I[863]	ST FATICA DANN.ACC.	FY-MIN	1.484	15226.400	22600.373	90000.000	0.251
Concio 6	422	J[864]	ST FATICA DANN.ACC.	FY-MIN	1.484	21508.696	31925.115	90000.000	0.355
Concio 5	423	I[864]	ST FATICA DANN.ACC.	FY-MIN	1.484	16445.828	24410.357	90000.000	0.271
Concio 5	423	J[865]	ST FATICA DANN.ACC.	FY-MIN	1.484	20255.520	30065.040	90000.000	0.334
Concio 5	424	I[865]	ST FATICA DANN.ACC.	FY-MIN	1.484	22773.016	33801.730	90000.000	0.376
Concio 5	424	J[866]	ST FATICA DANN.ACC.	FY-MIN	1.484	31386.233	46586.232	90000.000	0.518
Concio 5	425	I[866]	ST FATICA DANN.ACC.	FY-MIN	1.484	30923.473	45899.362	90000.000	0.510
Concio 5	425	J[867]	ST FATICA DANN.ACC.	FY-MIN	1.484	31751.667	47128.641	90000.000	0.524
Concio 4 H=var	426	I[867]	ST FATICA DANN.ACC.	FY-MIN	1.484	31342.734	46521.666	90000.000	0.517
Concio 4 H=var	426	J[868]	ST FATICA DANN.ACC.	FY-MIN	1.484	24672.611	36621.278	90000.000	0.407



Concio 4 H=var	427	I[868]	ST FATICA DANN.ACC.	FY-MIN	1.484	25009.271	37120.978	90000.000	0.413
Concio 4 H=var	427	J[869]	ST FATICA DANN.ACC.	FY-MIN	1.484	21351.903	31692.388	90000.000	0.352
Concio 4 H=200	428	I[869]	ST FATICA DANN.ACC.	FY-MIN	1.484	21159.193	31406.352	90000.000	0.349
Concio 4 H=200	428	J[870]	ST FATICA DANN.ACC.	FY-MIN	1.484	21624.760	32097.387	90000.000	0.357
Concio 4 H=200	429	I[870]	ST FATICA DANN.ACC.	FZ-MIN	1.484	25020.934	37138.290	90000.000	0.413
Concio 4 H=200	429	J[871]	ST FATICA DANN.ACC.	FZ-MIN	1.484	24555.368	36447.255	90000.000	0.405
Concio 4 H=var	430	I[871]	ST FATICA DANN.ACC.	FZ-MIN	1.484	24742.990	36725.740	90000.000	0.408
Concio 4 H=var	430	J[872]	ST FATICA DANN.ACC.	MZ-MIN	1.484	29477.583	43753.244	90000.000	0.486
Concio 4 H=var	431	I[872]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29519.736	43815.811	90000.000	0.487
Concio 4 H=var	431	J[873]	ST FATICA DANN.ACC.	FZ-MIN	1.484	38373.058	56956.697	90000.000	0.633
Concio 3	432	I[873]	ST FATICA DANN.ACC.	FZ-MIN	1.484	38952.278	57816.426	90000.000	0.642
Concio 3	432	J[874]	ST FATICA DANN.ACC.	FZ-MIN	1.484	38124.084	56587.147	90000.000	0.629
Concio 3	433	I[874]	ST FATICA DANN.ACC.	FZ-MIN	1.484	37761.713	56049.284	90000.000	0.623
Concio 3	433	J[875]	ST FATICA DANN.ACC.	FZ-MIN	1.484	29479.773	43756.494	90000.000	0.486
Concio 3	434	I[875]	ST FATICA DANN.ACC.	FZ-MIN	1.484	26615.632	39505.282	90000.000	0.439
Concio 3	434	J[876]	ST FATICA DANN.ACC.	FZ-MIN	1.484	23302.856	34588.166	90000.000	0.384
Concio 2	435	I[876]	ST FATICA DANN.ACC.	FZ-MIN	1.484	30719.872	45597.159	90000.000	0.507
Concio 2	435	J[877]	ST FATICA DANN.ACC.	FZ-MIN	1.484	24169.083	35873.898	90000.000	0.399
Concio 2	436	I[877]	ST FATICA DANN.ACC.	FZ-MIN	1.484	19410.049	28810.117	90000.000	0.320
Concio 2	436	J[878]	ST FATICA DANN.ACC.	FZ-MIN	1.484	8492.068	12604.681	90000.000	0.140
Concio 2	437	I[878]	ST FATICA DANN.ACC.	FZ-MIN	1.484	9080.597	13478.227	90000.000	0.150
Concio 2	437	J[879]	ST FATICA DANN.ACC.	FY-MIN	1.484	10163.601	15085.719	90000.000	0.168
Concio 1	438	I[879]	ST FATICA DANN.ACC.	FY-MIN	1.484	7799.411	11576.577	90000.000	0.129
Concio 1	438	J[880]	ST FATICA DANN.ACC.	FY-MIN	1.484	12826.398	19038.077	90000.000	0.212
Concio 1	439	I[880]	ST FATICA DANN.ACC.	FY-MIN	1.484	17022.191	25265.846	90000.000	0.281
Concio 1	439	J[881]	ST FATICA DANN.ACC.	FY-MIN	1.484	25400.503	37701.680	90000.000	0.419
Concio 1	440	I[881]	ST FATICA DANN.ACC.	FX-MAX	1.484	1340.530	1989.733	90000.000	0.022
Concio 1	440	J[882]	ST FATICA DANN.ACC.	FX-MAX	1.484	0.000	0.000	90000.000	0.000

## 10 VERIFICHE DI RESISTENZA DEI PROFILATI IN ACCIAIO ALLO S.L.U.

Si effettuano le verifiche di resistenza dei diaframmi e dei controventi allo stato limite ultimo; seguono gli schemi grafici e i tabulati di calcolo estesi degli elementi maggiormente sollecitati.

### 10.1 DIAFRAMMI SU SPALLE E INTEREDI – TRASVERSI INFERIORI

MIDAS/Civil		Steel Checking Result																									
<b>Company</b>		<b>Project Title</b>	CASALPUSTERLENGO																								
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...Asse 80 V_1.mcb																								
<b>1. Design Information</b>																											
Design Code	: Eurocode3-2:05																										
Unit System	: kN, m																										
Member No	: 1110																										
Material	: S355W (No:1) (Fy = 355000, Es = 210000000)																										
Section Name	: HINF_CORR 2L80x10d15 (No:18) (Built-up Section).																										
Member Length	: 1.60000																										
<b>2. Member Forces</b>																											
Axial Force	Fxx = 543.631 (LCB: 19+, POS:I)																										
Bending Moments	My = 1.22405, Mz = 1.01223																										
End Moments	Myi = 1.22405, Myj = 1.67647 (for Lb) Myi = 1.22405, Myj = 1.67647 (for Ly) Mzi = 1.01223, Mzj = 0.25618 (for Lz)																										
Shear Forces	Fyy = 0.79234 (LCB: 19+, POS:I) Fzz = -3.0661 (LCB: 19-, POS:J)																										
	<table border="1"> <tr> <td>Depth</td> <td>0.08000</td> <td>Web Thick</td> <td>0.01000</td> </tr> <tr> <td>Flg Width</td> <td>0.08000</td> <td>Flg Thick</td> <td>0.01000</td> </tr> <tr> <td>BTB Spacing</td> <td>0.01500</td> <td></td> <td></td> </tr> </table>	Depth	0.08000	Web Thick	0.01000	Flg Width	0.08000	Flg Thick	0.01000	BTB Spacing	0.01500																
Depth	0.08000	Web Thick	0.01000																								
Flg Width	0.08000	Flg Thick	0.01000																								
BTB Spacing	0.01500																										
	<table border="1"> <tr> <td>Area</td> <td>0.00300</td> <td>Asz</td> <td>0.00133</td> </tr> <tr> <td>Cyb</td> <td>0.00159</td> <td>Czb</td> <td>0.00320</td> </tr> <tr> <td>Iyy</td> <td>0.00000</td> <td>Izz</td> <td>0.00000</td> </tr> <tr> <td>Ybar</td> <td>0.08750</td> <td>Zbar</td> <td>0.05633</td> </tr> <tr> <td>Wely</td> <td>0.00003</td> <td>Welz</td> <td>0.00005</td> </tr> <tr> <td>ry</td> <td>0.02436</td> <td>rz</td> <td>0.03955</td> </tr> </table>	Area	0.00300	Asz	0.00133	Cyb	0.00159	Czb	0.00320	Iyy	0.00000	Izz	0.00000	Ybar	0.08750	Zbar	0.05633	Wely	0.00003	Welz	0.00005	ry	0.02436	rz	0.03955		
Area	0.00300	Asz	0.00133																								
Cyb	0.00159	Czb	0.00320																								
Iyy	0.00000	Izz	0.00000																								
Ybar	0.08750	Zbar	0.05633																								
Wely	0.00003	Welz	0.00005																								
ry	0.02436	rz	0.03955																								
<b>3. Design Parameters</b>																											
Unbraced Lengths	Ly = 1.60000, Lz = 1.60000, Lb = 1.60000																										
Effective Length Factors	Ky = 0.65, Kz = 1.30																										
Equivalent Uniform Moment Factors	Cmy = 1.00, Cmz = 1.00, CmLT = 1.00																										
<b>4. Checking Results</b>																											
Axial Resistance	$N_{Ed}/N_{t,Rd} = 543.63/1014.29 = 0.536 < 1.000$ ..... O.K																										
Bending Resistance	$M_{Edy}/M_{Rdy} = 1.2241/19.2503 = 0.064 < 1.000$ ..... O.K $M_{Edz}/M_{Rdz} = 1.0122/31.6119 = 0.032 < 1.000$ ..... O.K																										
Combined Resistance	$R_{NRd} = \text{MAX}[ M_{Edy}/M_{ny,Rd}, M_{Edz}/M_{nz,Rd} ]$ $R_{max1} = (M_{Edy}/M_{ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz,Rd})^{\text{Beta}}$ $R_{000} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$ $R_{max} = \text{MAX}[ R_{NRd}, R_{max1}, (R_{000} + R_{bend}) ] = 0.632 < 1.000$ ..... O.K																										
Shear Resistance	$V_{Edy}/V_{y,Rd} = 0.003 < 1.000$ ..... O.K $V_{Edz}/V_{z,Rd} = 0.010 < 1.000$ ..... O.K																										



1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV( 1.350)  
 22- 1 Dead Load( 1.350) +Erection Load 1( 1.500) +Creep Secondary( 1.200)  
 INV( 0.900) +Shrinkage Secondary( 1.200) +Vento Strutture INV( 0.900) +Vento Mobili  
 1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV(

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=
    23+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
    23-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
    24+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
    24-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
    25+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
    25-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
    28+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
    28-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
    29+  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
    29-  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
    30+  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    30-  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    31+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
    31-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
    32+  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    +Mobili SLE INV( 1.000)
    32-  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    +Mobili SLE INV( 1.000)
    33+  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
    33-  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
    34+  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
    34-  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
  
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35+ 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)  
35- 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)

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\*. PROJECT : CASALPUSTERLENGO  
 \*. MEMBER NO = 1110, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19+, MATERIAL NO = 1, SECTION NO = 18  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = HINF\_CORR 2L80x10d15  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.015  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.00000e-003, Avy = 1.60000e-003, Avz = 1.60000e-003  
 Ybar = 8.75000e-002, Zbar = 5.63333e-002, Qyb = 1.58672e-003, Qzb =  
 3.20000e-003  
 Wely = 3.15917e-005, Welz = 5.36429e-005, Wply = 5.69375e-005, Wplz =  
 9.35000e-005  
 Iyy = 1.77967e-006, Izz = 4.69375e-006, Iyz = 0.00000e+000  
 iy = 2.43562e-002, iz = 3.95548e-002  
 J = 1.00000e-007, Cwp = 4.24931e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 1.60000e+000, Lz = 1.60000e+000, Lu = 1.60000e+000  
 Ky = 6.50000e-001, Kz = 1.30000e+000

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (I) POINT :  
 Axial Force Fxx = 5.43631e+002  
 Shear Forces Fyy = 7.92336e-001, Fzz = 1.62507e+000  
 Bending Moments My = 1.22405e+000, Mz = 1.01223e+000  
 End Moments Myi = 1.22405e+000, Myj = 1.67647e+000 (for Lb)  
 Myi = 1.22405e+000, Myj = 1.67647e+000 (for Ly)  
 Mzi = 1.01223e+000, Mzj = 2.56180e-001 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of tension flanges(double angle).  
 -. Not Checking the Section Classification.

( ). Determine classification of tension flanges(double angle).  
 -. Not Checking the Section Classification.

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

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- ( ). Determine classification of tension elements(Double angle).
  - . Not Checking the Section Classification.

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=====
[[[*]]]   APPLIED FACTORS.
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- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).
  - [ Eurocode3:05 Annex A. Table A.1, A.2 ]
  - . Cmy,0 = 0.841
  - . Cmz,0 = 0.758
  - . Cmy (Default or User Defined Value) = 1.000
  - . Cmz (Default or User Defined Value) = 1.000
  - . CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).
  - [ Eurocode3:05 6.1 ]
  - . Gamma\_M0 = 1.05
  - . Gamma\_M1 = 1.10
  - . Gamma\_M2 = 1.25

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=====
[[[*]]]   CHECK AXIAL RESISTANCE.
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- ( ). Check slenderness ratio of axial tension member (l/i).
  - [ Eurocode3:05 6.3.1 ]
  - . l/i = 65.7 < 300.0 ---> O.K.
- ( ). Calculate parameters for combined resistance.
  - . Lambdal = Pi \* SQRT(Es/fy) = 76.409
  - . Lambda\_bz = (KLz/iz) / Lambdal = 0.529
- ( ). Calculate axial tensile resistance (Nt\_Rd).
  - [ Eurocode3:05 6.2.3 ]
  - . Nt\_Rd = fy \* Area / Gamma\_M0 = 1014.29 kN.
- ( ). Check ratio of axial resistance (N\_Ed/Nt\_Rd).
  - .  $\frac{N_{Ed}}{Nt_{Rd}} = \frac{543.63}{1014.29} = 0.536 < 1.000 \text{ ---> O.K.}$

```
=====
[[[*]]]   CHECK SHEAR RESISTANCE.
=====
```

- ( ). Calculate shear area.
  - [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]
  - . Avy = 2\*B\*tf = 0.0016 m^2.
  - . Avz = 2\*h\*tw = 0.0016 m^2.

-----  
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 =====  
 =

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -.  $V_{pl\_Rdy} = [ A_{vy} \cdot f_y / \sqrt{3} ] / \Gamma_{M0} = 312.32 \text{ kN.}$

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19+, POS = I )  
 -. Applied shear force :  $V_{Edy} = 0.79 \text{ kN.}$   

$$\frac{V_{Edy}}{V_{pl\_Rdy}} = \frac{0.79}{312.32} = 0.003 < 1.000 \text{ ---> O.K.}$$

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -.  $V_{pl\_Rdz} = [ A_{vz} \cdot f_y / \sqrt{3} ] / \Gamma_{M0} = 312.32 \text{ kN.}$

( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 -.  $HTR < 72 \cdot e / \eta \text{ ---> No need to check!}$

( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19-, POS = J )  
 -. Applied shear force :  $V_{Edz} = 3.07 \text{ kN.}$   

$$\frac{V_{Edz}}{V_{pl\_Rdz}} = \frac{3.07}{312.32} = 0.010 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -.  $W_{ply} = 5.6938e-005 \text{ m}^3.$   
 -.  $M_{c\_Rdy} = W_{ply} \cdot f_y / \Gamma_{M0} = 19.25 \text{ kN-m.}$

( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  

$$\frac{M_{Edy}}{M_{c\_Rdy}} = \frac{1.22}{19.25} = 0.064 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -.  $W_{plz} = 9.3500e-005 \text{ m}^3.$   
 -.  $M_{c\_Rdz} = W_{plz} \cdot f_y / \Gamma_{M0} = 31.61 \text{ kN-m.}$

-----  
-  
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=====

( ). Check ratio of moment resistance (M\_Edz/Mc\_Rdz).  

$$\frac{M\_Edz}{Mc\_Rdz} = \frac{1.01}{31.61} = 0.032 < 1.000 \text{ ---> O.K.}$$

=====  
[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.  
=====

( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of V\_Edz / Vpl\_Rdz < 0.5  
 -. My\_Rd = Mc\_Rdy = 19.25 kN-m.

( ). Calculate Minor reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of V\_Edy / Vpl\_Rdy < 0.5  
 -. Mz\_Rd = Mc\_Rdz = 31.61 kN-m.

( ). Check general interaction ratio.  
 [ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2  

$$R_{max1} = \frac{N\_Ed}{N\_Rd} + \frac{M\_Edy}{My\_Rd} + \frac{M\_Edz}{Mz\_Rd}$$

$$= 0.632 < 1.000 \text{ ---> O.K.}$$

( ). Check interaction ratio of bending and axial force member.  
 [ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2  
 -. n = N\_Ed / Npl\_Rd = 0.536  
 -. a = MIN[ (Area-2b\*tf)/Area, 0.5 ] = 0.500  
 -. Alpha = 2.000  
 -. Beta = MAX[ 5\*n, 1.0 ] = 2.680  
 -. Mny\_Rd = MIN[ Mply\_Rd\*(1-n)/(1-0.5\*a), Mply\_Rd ] = 11.91 kN-m.  
 -. Rmaxy = M\_Edy / Mny\_Rd = 0.103 < 1.000 ---> O.K.

-. In case of n > a  
 -. Mnz\_Rd = Mplz\_Rd \* [ 1 - ((n-a)/(1-a))^2 ] = 31.45 kN-m.  
 -. Rmaxz = M\_Edz / Mnz\_Rd = 0.032 < 1.000 ---> O.K.

$$R_{max2} = \left[ \frac{|M\_Edy|^{Alpha}}{|Mny\_Rd|} + \frac{|M\_Edz|^{Beta}}{|Mnz\_Rd|} \right]$$

$$= 0.011 < 1.000 \text{ ---> O.K.}$$

-. Rmax = MAX[ Rmax1, Rmax2 ] = 0.632 < 1.000 ---> O.K.



## 10.2 DIAFRAMMI SU SPALLE E INTEREDI – TRASVERSI SUPERIORI

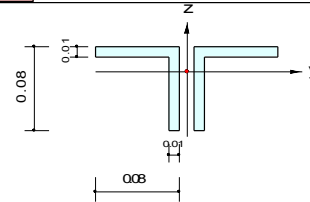
### MIDAS/Civil

### Steel Checking Result

<b>Company</b>		<b>Project Title</b>	CASALPUSTERLENGO
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...Asse 80 V_1.mdb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 1439  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : HSUP\_CORR 2L80x10d15 (No:17)  
 (Built-up Section).  
 Member Length : 3.20000



#### 2. Member Forces

Axial Force : Fxx = -209.74 (LCB: 22-, POS:I)  
 Bending Moments : My = -0.6722, Mz = -0.0637  
 End Moments : Myi = -0.6722, Myj = -0.6672 (for Lb)  
 : Myi = -0.6722, Myj = -0.6672 (for Ly)  
 : Mzi = -0.0637, Mzj = -0.0637 (for Lz)  
 Shear Forces : Fyy = 0.05667 (LCB: 19+, POS:I)  
 : Fzz = 0.93628 (LCB: 19+, POS:J)

Depth	0.08000	Web Thick	0.01000
Flg Width	0.08000	Flg Thick	0.01000
BTB Spacing	0.01500		
Area	0.00300	Asz	0.00133
Cyb	0.00159	Czb	0.00320
Iyy	0.00000	Izz	0.00000
Ybar	0.08750	Zbar	0.06633
Wely	0.00003	Welz	0.00005
ry	0.02436	rz	0.03955

#### 3. Design Parameters

Unbraced Lengths : Ly = 3.20000, Lz = 3.20000, Lb = 3.20000  
 Effective Length Factors : Ky = 0.65, Kz = 0.65  
 Equivalent Uniform Moment Factors : Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

##### Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c,Rd}, N_{b,Rd}] = 209.738/508.054 = 0.413 < 1.000 \dots\dots\dots \text{O.K}$$

##### Bending Resistance

$$M_{Edy}/M_{Rdy} = 0.6722/19.2503 = 0.035 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 0.0637/31.6119 = 0.002 < 1.000 \dots\dots\dots \text{O.K}$$

##### Combined Resistance

$$R_{NRd} = \text{MAX}[M_{Edy}/M_{Ny,Rd}, M_{Edz}/M_{Nz,Rd}]$$

$$R_{\text{max}1} = (M_{Edy}/M_{Ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{Nz,Rd})^{\text{Beta}}$$

$$R_{\text{oom}} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{\text{bend}} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$$

$$R_{c\_LT1} = N_{Ed}/(X_{iy} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT1} = (k_{yy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (k_{yz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{c\_LT2} = N_{Ed}/(X_{iz} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT2} = (K_{zy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (K_{zz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{\text{max}} = \text{MAX}[R_{NRd}, R_{\text{max}1}, (R_{\text{oom}} + R_{\text{bend}}), \text{MAX}(R_{c\_LT1} + R_{b\_LT1}, R_{c\_LT2} + R_{b\_LT2})] = 0.472 < 1.000 \dots\dots \text{O.K}$$

##### Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.000 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.003 < 1.000 \dots\dots\dots \text{O.K}$$



1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV( 1.350)  
 22- 1 Dead Load( 1.350) +Erection Load 1( 1.500) +Creep Secondary( 1.200)  
 INV( 0.900) +Shrinkage Secondary( 1.200) +Vento Strutture INV( 0.900) +Vento Mobili  
 1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV(

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=
    23+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
    23-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
    24+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
    24-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
    25+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
    25-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
    28+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
    28-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
    29+  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
    29-  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
    30+  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    30-  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    31+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
    31-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
    32+  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    +Mobili SLE INV( 1.000)
    32-  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    +Mobili SLE INV( 1.000)
    33+  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
    33-  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
    34+  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
    34-  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)

```

35+ 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
35- 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)

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\*. PROJECT : CASALPUSTERLENGO  
 \*. MEMBER NO = 1439, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 22-, MATERIAL NO = 1, SECTION NO = 17  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = HSUP\_CORR 2L80x10d15  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.015  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.00000e-003, Avy = 1.60000e-003, Avz = 1.60000e-003  
 Ybar = 8.75000e-002, Zbar = 5.63333e-002, Qyb = 1.58672e-003, Qzb =  
 3.20000e-003  
 Wely = 3.15917e-005, Welz = 5.36429e-005, Wply = 5.69375e-005, Wplz =  
 9.35000e-005  
 Iyy = 1.77967e-006, Izz = 4.69375e-006, Iyz = 0.00000e+000  
 iy = 2.43562e-002, iz = 3.95548e-002  
 J = 1.00000e-007, Cwp = 4.24931e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.20000e+000, Lz = 3.20000e+000, Lu = 3.20000e+000  
 Ky = 6.50000e-001, Kz = 6.50000e-001

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (I) POINT :  
 Axial Force Fxx = -2.09738e+002  
 Shear Forces Fyy = -3.87634e-002, Fzz = -8.14924e-001  
 Bending Moments My = -6.72164e-001, Mz = -6.37363e-002  
 End Moments Myi = -6.72164e-001, Myj = -6.67178e-001 (for Lb)  
 Myi = -6.72164e-001, Myj = -6.67178e-001 (for Ly)  
 Mzi = -6.37363e-002, Mzj = -6.37202e-002 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -. e = SQRT( 235/fy ) = 0.81  
 -. b/t = BTR = 7.00  
 -. sigma1 = 62162.105 KPa.  
 -. sigma2 = 61075.786 KPa.  
 -. BTR < 9\*e ( Class 1 : Plastic ).

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```
=====  

[[[*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  

=====
```

- ```
( ). Determine classification of compression flanges(Double angle).  

[ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  

-. e      = SQRT( 235/fy ) =    0.81  

-. b/t    = BTR =    7.00  

-. sigma1 = 60872.102 KPa.  

-. sigma2 = 59785.783 KPa.  

-. BTR < 9*e ( Class 1 : Plastic ).
```

```
=====  

[[[*]]] CLASSIFY WEB OF SECTION (HTR).  

=====
```

- ```
( ). Determine classification of compression element(Double angles).  

[ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  

-. e      = SQRT( 235/fy ) =    0.81  

-. d/t    = HTR =    8.00  

-. sigma1 = 91426.864 KPa.  

-. sigma2 = 64988.487 KPa.  

-. HTR < 10*e ( Class 2 : Compact ).
```

```
=====  

[[[*]]] APPLIED FACTORS.  

=====
```

- ```
( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).  

[ Eurocode3:05 Annex A. Table A.1, A.2 ]  

-. Cmy,0  = 0.754  

-. Cmz,0  = 0.907  

-. Cmy (Default or User Defined Value) = 1.000  

-. Cmz (Default or User Defined Value) = 1.000  

-. CmLT (Default or User Defined Value) = 1.000  
  

( ). Partial Factors (Gamma_Mi).  

[ Eurocode3:05 6.1 ]  

-. Gamma_M0 = 1.05  

-. Gamma_M1 = 1.10  

-. Gamma_M2 = 1.25
```

```
=====  

[[[*]]] CHECK AXIAL RESISTANCE.  

=====
```

- ```
( ). Check slenderness ratio of axial compression member (Kl/i).  

[ Eurocode3:05 6.3.1 ]  

-. Kl/i = 85.4 < 200.0 ---> O.K.
```

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 -  
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 =====  
 =

- ( ). Calculate axial compressive resistance (Nc\_Rd).  
 [ Eurocode3:05 6.1, 6.2.4 ]  
 -. Nc\_Rd =  $f_y \cdot Area / \Gamma_{M0}$  = 1014.29 kN.
- ( ). Check ratio of axial resistance (N\_Ed/Nc\_Rd).  
 $\frac{N_{Ed}}{Nc_{Rd}} = \frac{209.74}{1014.29} = 0.207 < 1.000 \text{ ---> O.K.}$
- ( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).  
 [ Eurocode3:05 6.3.1.1, 6.3.1.2 ]  
 -. Beta\_A =  $A_{eff} / Area$  = 1.000  
 -. Lambdal =  $\pi \cdot \sqrt{Es/fy}$  = 76.409  
 -. Lambda\_by =  $\{(KLy/iy)/Lambdal\} \cdot \sqrt{Beta\_A}$  = 1.118  
 -. Ncry =  $\pi^2 \cdot Es \cdot Ryy / KLy^2$  = 852.57 kN.  
 -. Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.  
 -. Alphay = 0.340  
 -. Phiy =  $0.5 \cdot [1 + Alphay \cdot (Lambda\_by - 0.2) + Lambda\_by^2]$  = 1.281  
 -. Xiy =  $\text{MIN} [ 1 / [Phiy + \sqrt{Phiy^2 - Lambda\_by^2}], 1.0 ]$  = 0.525  
 -. Nb\_Rdy =  $Xiy \cdot Beta\_A \cdot Area \cdot fy / \Gamma_{M1}$  = 508.05 kN.
- . Lambdabz =  $\{(KLz/iz)/Lambdal\} \cdot \sqrt{Beta\_A}$  = 0.688  
 -. Ncrz =  $\pi^2 \cdot Es \cdot Rzz / KLz^2$  = 2248.60 kN.  
 -. Lambdabz > 0.2 and N\_Ed/Ncrz > 0.04 --> Need to check.  
 -. Alphaz = 0.340  
 -. Phiz =  $0.5 \cdot [1 + Alphaz \cdot (Lambdabz - 0.2) + Lambdabz^2]$  = 0.820  
 -. Xiz =  $\text{MIN} [ 1 / [Phiz + \sqrt{Phiz^2 - Lambdabz^2}], 1.0 ]$  = 0.790  
 -. Nb\_Rdz =  $Xiz \cdot Beta\_A \cdot Area \cdot fy / \Gamma_{M1}$  = 765.18 kN.
- ( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).  
 -. Nb\_Rd =  $\text{MIN} [ Nb\_Rdy, Nb\_Rdz ]$  = 508.05 kN.  
 $\frac{N_{Ed}}{Nb_{Rd}} = \frac{209.74}{508.05} = 0.413 < 1.000 \text{ ---> O.K.}$

=====  
 [[[\*]]] CHECK SHEAR RESISTANCE.  
 =====

- ( ). Calculate shear area.  
 [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]  
 -. Avy =  $2 \cdot B \cdot tf$  = 0.0016 m<sup>2</sup>.  
 -. Avz =  $2 \cdot h \cdot tw$  = 0.0016 m<sup>2</sup>.
- ( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdy =  $[ Avy \cdot fy / \sqrt{3} ] / \Gamma_{M0}$  = 312.32 kN.

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 =====  
 =

- ( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19-, POS = J )  
 -. Applied shear force : V\_Edy = 0.06 kN.  

$$\frac{V_{Edy}}{V_{pl\_Rdy}} = \frac{0.06}{312.32} = 1.814e-004 < 1.000 \text{ ---> O.K.}$$
- ( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 312.32 kN.
- ( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 -. HTR < 72\*e/Eta ---> No need to check!
- ( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19+, POS = J )  
 -. Applied shear force : V\_Edz = 0.94 kN.  

$$\frac{V_{Edz}}{V_{pl\_Rdz}} = \frac{0.94}{312.32} = 0.003 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

- ( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wply = 5.6938e-005 m^3.  
 -. Mc\_Rdy = Wply \* fy / Gamma\_M0 = 19.25 kN-m.
- ( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  

$$\frac{M_{Edy}}{Mc_{Rdy}} = \frac{0.67}{19.25} = 0.035 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

- ( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wplz = 9.3500e-005 m^3.  
 -. Mc\_Rdz = Wplz \* fy / Gamma\_M0 = 31.61 kN-m.
- ( ). Check ratio of moment resistance (M\_Edz/Mc\_Rdz).  

$$\frac{M_{Edz}}{Mc_{Rdz}} = \frac{0.06}{31.61} = 0.002 < 1.000 \text{ ---> O.K.}$$

-----  
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MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
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=====  
[[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.  
=====

- ( ). Calculate Major reduced design resistance of bending and shear.  
[ Eurocode3:05 6.2.8 (6.30) ]  
-. In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$   
-.  $M_{y,Rd} = M_{c,Rdy} = 19.25$  kN-m.
- ( ). Calculate Minor reduced design resistance of bending and shear.  
[ Eurocode3:05 6.2.8 (6.30) ]  
-. In case of  $V_{Edy} / V_{pl,Rdy} < 0.5$   
-.  $M_{z,Rd} = M_{c,Rdz} = 31.61$  kN-m.
- ( ). Check general interaction ratio.  
[ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2  
$$R_{max1} = \frac{N_{Ed}}{N_{Rd}} + \frac{M_{Edy}}{M_{y,Rd}} + \frac{M_{Edz}}{M_{z,Rd}}$$
  
= 0.244 < 1.000 ---> O.K.
- ( ). Check interaction ratio of bending and axial force member.  
[ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2  
-.  $n = N_{Ed} / N_{pl,Rd} = 0.207$   
-.  $a = \text{MIN}[(Area - 2b \cdot t_f) / Area, 0.5] = 0.500$   
-.  $\alpha = 2.000$   
-.  $\beta = \text{MAX}[5 \cdot n, 1.0] = 1.034$   
-.  $M_{ny,Rd} = \text{MIN}[M_{ply,Rd} \cdot (1-n) / (1-0.5 \cdot a), M_{ply,Rd}] = 19.25$  kN-m.  
-.  $R_{maxy} = M_{Edy} / M_{ny,Rd} = 0.035 < 1.000$  ---> O.K.
- . In case of  $n < a$   
-.  $M_{nz,Rd} = M_{plz,Rd} = 31.61$  kN-m.  
-.  $R_{maxz} = M_{Edz} / M_{nz,Rd} = 0.002 < 1.000$  ---> O.K.
- .  $R_{max2} = \left[ \frac{|M_{Edy}|^{\alpha}}{|M_{ny,Rd}|} + \frac{|M_{Edz}|^{\beta}}{|M_{nz,Rd}|} \right]$   
= 0.003 < 1.000 ---> O.K.



-----  
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 =

( ). Check interaction ratio of bending and axial compression member.  
 [ Eurocode3:05 6.3.1, 6.2.9.3 (6.61, 6.62), Annex A ]

- . N\_Ed = -209.74 kN.  
 - . M\_Edy = -0.67 kN-m.  
 - . M\_Edz = -0.06 kN-m.  
 - . kyy = 1.217  
 - . kyz = 0.598  
 - . kzy = 0.805  
 - . kzz = 1.214  
 - . Xiy = 0.525  
 - . Xiz = 0.790  
 - . XiLT = 0.772  
 - . N\_Rk = A\*fy = 1065.00 kN.  
 - . My\_Rk = Wply\*fy = 20.21 kN-m.  
 - . Mz\_Rk = Wplz\*fy = 33.19 kN-m.  
 - . N\_Ed\*eNy = 0.0 (Not Slender)  
 - . N\_Ed\*eNZ = 0.0 (Not Slender)

N\_Ed\*eNz  
 -----  
 - . Rmax\_LT1 = ----- + kyy \* ----- + kyz \* -----  
 Xiy\*N\_Rk/Gamma\_M1 XiLT\*My\_Rk/Gamma\_M1  
 Mz\_Rk/Gamma\_M1  
 = 0.472 < 1.000 ----> O.K.  
 N\_Ed\*eNz  
 -----  
 - . Rmax\_LT2 = ----- + kzy \* ----- + kzz \* -----  
 Xiz\*N\_Rk/Gamma\_M1 XiLT\*My\_Rk/Gamma\_M1  
 Mz\_Rk/Gamma\_M1  
 = 0.315 < 1.000 ----> O.K.  
 - . Rmax = MAX[ MAX(Rmax1, Rmax2), MAX(Rmax\_LT1, Rmax\_LT2) ] = 0.472 <  
 1.000 ----> O.K.

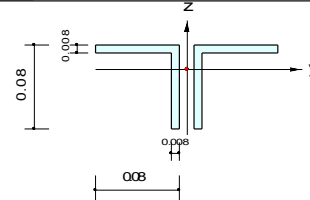
### 10.3 DIAFRAMMI SU SPALLE E INTERMEDI – DIAGONALI

#### MIDAS/Civil Steel Checking Result

<b>Company</b>		<b>Project Title</b>	CASALPUSTERLENGO
<b>Author</b>	Renato Vaira	<b>File Name</b>	D:\...\Asse 80 V_1.mcb

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 1422  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : DIAG\_CORR 2L80x8d15 (No:64)  
 (Built-up Section).  
 Member Length : 1.70880



#### 2. Member Forces

Axial Force Fxx = -149.58 (LCB: 19-, POS:I)  
 Bending Moments My = -2.0074, Mz = -0.4795  
 End Moments Myi = -2.0074, Mj = -0.7248 (for Lb)  
 Myi = -2.0074, Mj = -0.7248 (for Ly)  
 Mzi = -0.4795, Mzj = -0.2886 (for Lz)  
 Shear Forces Fyy = -0.1119 (LCB: 19-, POS:I)  
 Fzz = -2.0999 (LCB: 19-, POS:J)

Depth	0.08000	Web Thick	0.00800
Flg Width	0.08000	Flg Thick	0.00800
BTB Spacing	0.01500		
Area	0.00243	Asz	0.00107
Cyb	0.00163	Czb	0.00320
Iyy	0.00000	Izz	0.00000
Ybar	0.08750	Zbar	0.05705
Wely	0.00003	Welz	0.00004
ry	0.02462	rz	0.03916

#### 3. Design Parameters

Unbraced Lengths Ly = 1.70880, Lz = 1.70880, Lb = 1.70880  
 Effective Length Factors Ky = 0.65, Kz = 1.30  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

##### Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c,Rd}, N_{b,Rd}] = 149.581/595.881 = 0.251 < 1.000 \dots\dots\dots \text{O.K}$$

##### Bending Resistance

$$M_{Edy}/M_{Rdy} = 2.00740/8.34129 = 0.241 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 0.4795/11.6551 = 0.041 < 1.000 \dots\dots\dots \text{O.K}$$

##### Combined Resistance

$$R_{oom} = N_{Ed}/(A_{eff} \cdot f_y / \Gamma_{M0}), R_{bend} = (M_{Edy} + N_{Ed} \cdot e_{Ny}) / M_{y,Rd} + (M_{Edz} + N_{Ed} \cdot e_{Nz}) / M_{z,Rd}$$

$$R_{c\_LT1} = N_{Ed} / (X_{iy} \cdot A_{eff} \cdot f_y / \Gamma_{M1})$$

$$R_{b\_LT1} = k_{yy} \cdot (M_{Edy} + N_{Ed} \cdot e_{Ny}) / (X_{i\_LT} \cdot W_{effy} \cdot f_y / \Gamma_{M1}) + k_{yz} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (W_{effz} \cdot f_y / \Gamma_{M1})$$

$$R_{c\_LT2} = N_{Ed} / (X_{iz} \cdot A_{eff} \cdot f_y / \Gamma_{M1})$$

$$R_{b\_LT2} = k_{zy} \cdot (M_{Edy} + N_{Ed} \cdot e_{Ny}) / (X_{i\_LT} \cdot W_{effy} \cdot f_y / \Gamma_{M1}) + k_{zz} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (W_{effz} \cdot f_y / \Gamma_{M1})$$

$$R_{max} = \text{MAX}[R_{oom} + R_{bend}, \text{MAX}(R_{c\_LT1} + R_{b\_LT1}, R_{c\_LT2} + R_{b\_LT2})] = 0.565 < 1.000 \dots\dots \text{O.K}$$

##### Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.000 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.008 < 1.000 \dots\dots\dots \text{O.K}$$



1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV( 1.350)  
 22- 1 Dead Load( 1.350) +Erection Load 1( 1.500) +Creep Secondary( 1.200)  
 INV( 0.900) +Shrinkage Secondary( 1.200) +Vento Strutture INV( 0.900) +Vento Mobili  
 1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV(

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=
    23+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    23-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    24+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    24-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    25+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    25-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    28+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    28-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
1.000)
    29+  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    29-  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    30+  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    30-  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    31+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    31-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
1.000)
    32+  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    32-  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    33+  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    33-  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    34+  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
    34-  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)

```

35+ 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
35- 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
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*. PROJECT      : CASALPUSTERLENGO
*. MEMBER NO   = 1422, ELEMENT TYPE = Beam
*. LOADCOMB NO = 19-, MATERIAL NO  = 1, SECTION NO  = 64
*. UNIT SYSTEM : kN, m

*. SECTION PROPERTIES : Designation = DIAG_CORR 2L80x8d15
  Shape        = 2L - Section. (Built-up)
  Depth        = 0.080, Flg Width   = 0.080, BTB Spacing = 0.015
  Web Thick   = 0.008, Flg Thick  = 0.008

  Area = 2.43200e-003, Avy = 1.28000e-003, Avz = 1.28000e-003
  Ybar = 8.75000e-002, Zbar = 5.70526e-002, Qyb = 1.62750e-003, Qzb =
3.20000e-003
  Wely = 2.58462e-005, Welz = 4.26190e-005, Wply = 4.65664e-005, Wplz =
7.40480e-005
  Iyy = 1.47460e-006, Izz = 3.72916e-006, Iyz = 0.00000e+000
  iy  = 2.46238e-002, iz  = 3.91583e-002
  J   = 5.18827e-008, Cwp = 2.31033e-011

*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :
  Ly = 1.70880e+000, Lz = 1.70880e+000, Lu = 1.70880e+000
  Ky = 6.50000e-001, Kz = 1.30000e+000

*. MATERIAL PROPERTIES :
  Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

*. FORCES AND MOMENTS AT (I) POINT :
  Axial Force      Fxx =-1.49581e+002
  Shear Forces     Fyy =-1.11881e-001, Fzz =-1.63483e+000
  Bending Moments   My =-2.00740e+000, Mz =-4.79512e-001
  End Moments       Myi =-2.00740e+000, Myj =-7.24751e-001 (for Lb)
                  Myi =-2.00740e+000, Myj =-7.24751e-001 (for Ly)
                  Mzi =-4.79512e-001, Mzj =-2.88619e-001 (for Lz)

*. Sign conventions for stress and axial force.
  - Stress : Compression positive.
  - Axial force: Tension positive.
  
```

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

```

( ). Determine classification of compression flanges(Double angle).
  [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]
  -. e      = SQRT( 235/fy ) = 0.81
  -. b/t    = BTR = 9.00
  -. sigma1 = 41517.900 KPa.
  -. sigma2 = 31231.144 KPa.
  -. BTR_L = (H+B) / (2*tf) = 10.00
  -. BTR > 15*e or BTR_L > 11.5*e ( Class 4 : Slender ).
  
```

```
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MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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```

```
=====  
[[[*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  
=====
```

```
( ). Determine classification of compression flanges(Double angle).  
[ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
-. e      = SQRT( 235/fy ) =    0.81  
-. b/t    = BTR =    9.00  
-. sigma1 = 29302.377 KPa.  
-. sigma2 = 19015.622 KPa.  
-. BTR_L = (H+B) / (2*tf) = 10.00  
-. BTR > 15*e or BTR_L > 11.5*e ( Class 4 : Slender ).
```

```
=====  
[[[*]]] CLASSIFY WEB OF SECTION (HTR).  
=====
```

```
( ). Determine classification of compression element(Double angles).  
[ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
-. e      = SQRT( 235/fy ) =    0.81  
-. d/t    = HTR = 10.00  
-. sigma1 = 141165.378 KPa.  
-. sigma2 = 43150.375 KPa.  
-. HTR_L = (h+B) / (2*tw) = 10.00  
-. HTR > 15*e or HTR_L > 11.5*e ( Class 4 : Slender ).
```

```
=====  
[[[*]]] CALCULATE EFFECTIVE AREA.  
=====
```

```
( ). Calculate buckling factor of outstand compression element.  
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]  
-. In case of Psi = 1.0  
-. k_sigma = 0.4300
```

```
( ). Calculate effective cross-section properties of left flange flange of Class 4  
(Outstand element).
```

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]  
-. RatT    = 9.0000  
-. Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5940  
-. Rho      = 1.0  
-. sigma_max = MAX( sigma1, sigma2 ) = 61505.460 KPa.  
-. sigma_min = MIN( sigma1, sigma2 ) = 61505.460 KPa.  
-. r        = 0.000 m.  
-. bc       = 0.080 m.  
-. beff     = Rho*bc + r = 0.080 m.  
-. Aeff     = beff * tf = 6.400e-004 m^2.  
-. yeff     = beff/2    = 0.040 m.
```

```
( ). Calculate buckling factor of outstand compression element.  
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
```

```
-----
-
MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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```

```
- . In case of Psi = 1.0
- . k_sigma = 0.4300
```

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT      = 9.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5940
- . Rho      = 1.0
- . sigma_max = MAX( sigma1, sigma2 ) = 61505.460 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = 61505.460 KPa.
- . r        = 0.000 m.
- . bc      = 0.080 m.
- . beff    = Rho*bc + r = 0.080 m.
- . Aeff    = beff * tf = 6.400e-004 m^2.
- . yeff    = beff/2 = 0.040 m.
```

( ). Calculate buckling factor of outstand compression element.

```
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- . In case of Psi = 1.0
- . k_sigma = 0.4300
```

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT      = 10.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.6600
- . Rho      = 1.0
- . sigma_max = MAX( sigma1, sigma2 ) = 61505.460 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = 61505.460 KPa.
- . r        = 0.000 m.
- . Ar      = 0.000 m^2.
- . dc      = 0.072 m.
- . deff1    = 2*(Rho*dc) / [ 5 - sigma_min/sigma_max ] + r = 0.036 m.
- . Aeff1    = deff1 * tw + 2*Ar = 2.880e-004 m^2.
- . zeff1    = deff1/2 + tf = 0.018 m.
- . deff2    = (Rho*dc) - deff1 + r = 0.036 m.
- . Aeff2    = deff2 * tw + 2*Ar = 2.880e-004 m^2.
- . zeff2    = (h+2*r) - deff2/2 + tf = 0.054 m.
```

( ). Calculate buckling factor of outstand compression element.

```
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- . In case of Psi = 1.0
- . k_sigma = 0.4300
```

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT      = 10.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.6600
- . Rho      = 1.0
```

```
-----
-
MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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```

```
- . sigma_max = MAX( sigma1, sigma2 ) = 61505.460 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = 61505.460 KPa.
- . r = 0.000 m.
- . Ar = 0.000 m^2.
- . dc = 0.072 m.
- . deff1 = 2*(Rho*dc) / [ 5 - sigma_min/sigma_max ] + r = 0.036 m.
- . Aeff1 = deff1 * tw + 2*Ar = 2.880e-004 m^2.
- . zeff1 = deff1/2 + tf = 0.018 m.
- . deff2 = (Rho*dc) - deff1 + r = 0.036 m.
- . Aeff2 = deff2 * tw + 2*Ar = 2.880e-004 m^2.
- . zeff2 = (h+2*r) - deff2/2 + tf = 0.054 m.
```

```
=====[[*]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MAJOR AXIS.=====[[*]]
```

( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . Rho = 1.0 (Only tensile stresses).
- . sigma_max = MAX( sigma1, sigma2 ) = -31238.700 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = -31238.700 KPa.
- . r = 0.000 m.
- . bc = 0.080 m.
- . beff = Rho*bc + r = 0.080 m.
- . Aeff = beff * tf = 6.400e-004 m^2.
- . yeff = beff/2 = 0.040 m.
```

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . Rho = 1.0 (Only tensile stresses).
- . sigma_max = MAX( sigma1, sigma2 ) = -31238.700 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = -31238.700 KPa.
- . r = 0.000 m.
- . bc = 0.080 m.
- . beff = Rho*bc + r = 0.080 m.
- . Aeff = beff * tf = 6.400e-004 m^2.
- . yeff = beff/2 = 0.040 m.
```

( ). Calculate buckling factor of outstand compression element.

```
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- . k_sigma = 0.57 - 0.21*Psi + 0.07*Psi^2 = 0.6298
```

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT = 10.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5453
- . Rho = 1.0
```

```
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-
MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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```

```
-. sigma_max = MAX( sigma1, sigma2 ) = 77666.859 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = -20348.144 KPa.
-. r          = 0.000 m.
-. Ar         = 0.000 m^2.
-. dc         = (h*sigma_max) / (sigma_max-sigma_min) = 0.057 m.
-. deff1      = 0.4*Rho*dc + r = 0.023 m.
-. Aeff1      = deff1 * tw + 2*Ar = 1.826e-004 m^2.
-. zeff1      = deff1/2 + tf = 0.011 m.
-. deff2      = 0.6*Rho*dc + (h-dc) + r = 0.049 m.
-. Aeff2      = deff2 * tw + 2*Ar = 3.934e-004 m^2.
-. zeff2      = (h+2*r) - deff2/2 + tf = 0.047 m.
```

```
( ). Calculate buckling factor of outstand compression element.
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
-. k_sigma = 0.57 - 0.21*Psi + 0.07*Psi^2 = 0.6298
```

```
( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).
```

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
-. RatT      = 10.0000
-. Lambda_p  = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5453
-. Rho       = 1.0
-. sigma_max = MAX( sigma1, sigma2 ) = 77666.859 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = -20348.144 KPa.
-. r         = 0.000 m.
-. Ar        = 0.000 m^2.
-. dc        = (h*sigma_max) / (sigma_max-sigma_min) = 0.057 m.
-. deff1     = 0.4*Rho*dc + r = 0.023 m.
-. Aeff1     = deff1 * tw + 2*Ar = 1.826e-004 m^2.
-. zeff1     = deff1/2 + tf = 0.011 m.
-. deff2     = 0.6*Rho*dc + (h-dc) + r = 0.049 m.
-. Aeff2     = deff2 * tw + 2*Ar = 3.934e-004 m^2.
-. zeff2     = (h+2*r) - deff2/2 + tf = 0.047 m.
```

```
=====[[*]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MINOR AXIS.====
```

```
( ). Calculate buckling factor of outstand compression element.
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
-. k_sigma = 0.57 - 0.21*Psi + 0.07*Psi^2 = 0.5525
```

```
( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).
```

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
-. RatT      = 9.0000
-. Lambda_p  = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5240
-. Rho       = 1.0
```

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- . sigma\_max = MAX( sigma1, sigma2 ) = 11251.139 KPa.  
 - . sigma\_min = MIN( sigma1, sigma2 ) = 964.383 KPa.  
 - . r = 0.000 m.  
 - . bc = 0.080 m.  
 - . beff = Rho\*bc + r = 0.080 m.  
 - . Aeff = beff \* tf = 6.400e-004 m^2.  
 - . yeff = beff/2 = 0.040 m.

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]  
 - . Rho = 1.0 (Only tensile stresses).  
 - . sigma\_max = MAX( sigma1, sigma2 ) = -964.383 KPa.  
 - . sigma\_min = MIN( sigma1, sigma2 ) = -11251.139 KPa.  
 - . r = 0.000 m.  
 - . bc = 0.080 m.  
 - . beff = Rho\*bc + r = 0.080 m.  
 - . Aeff = beff \* tf = 6.400e-004 m^2.  
 - . yeff = beff/2 = 0.040 m.

( ). Calculate buckling factor of outstand compression element.

[ Eurocode3 Part 1-5 4.4, Table 4.2 ]  
 - . In case of Psi = 1.0  
 - . k\_sigma = 0.4300

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]  
 - . RatT = 10.0000  
 - . Lambda\_p = RatT / [ 28.4\*Eps\*SQRT(k\_sigma) ] = 0.6600  
 - . Rho = 1.0  
 - . sigma\_max = MAX( sigma1, sigma2 ) = 1993.059 KPa.  
 - . sigma\_min = MIN( sigma1, sigma2 ) = 1993.059 KPa.  
 - . r = 0.000 m.  
 - . Ar = 0.000 m^2.  
 - . dc = 0.072 m.  
 - . deff1 = 2\*(Rho\*dc) / [ 5 - sigma\_min/sigma\_max ] + r = 0.036 m.  
 - . Aeff1 = deff1 \* tw + 2\*Ar = 2.880e-004 m^2.  
 - . zeff1 = deff1/2 + tf = 0.018 m.  
 - . deff2 = (Rho\*dc) - deff1 + r = 0.036 m.  
 - . Aeff2 = deff2 \* tw + 2\*Ar = 2.880e-004 m^2.  
 - . zeff2 = (h+2\*r) - deff2/2 + tf = 0.054 m.

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]  
 - . Rho = 1.0 (Only tensile stresses).  
 - . sigma\_max = MAX( sigma1, sigma2 ) = -1993.059 KPa.  
 - . sigma\_min = MIN( sigma1, sigma2 ) = -1993.059 KPa.  
 - . r = 0.000 m.  
 - . Ar = 0.000 m^2.  
 - . dc = 0.072 m.  
 - . deff = dc + r = 0.072 m.  
 - . Aeff = deff \* tw + 4\*Ar = 5.760e-004 m^2.  
 - . zeff = (h+2\*r) - deff/2 = 0.036 m.



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```

```
=====  

[[[*]]] EFFECTIVE SECTION PPOPERTIES.  

=====
```

- ```
( ). Calculated effective cross-section properties of Class4 cross-section.
-. Aeff = 0.0024 m^2. (for calculating axial resistance)
-. Aeffy = 0.0024 m^2.
-. Weffy = 2.5846e-005 m^3.
-. Aeffz = 0.0024 m^2.
-. Weffz = 3.6115e-005 m^3.
-. eNy =1.4211e-017 m.
-. eNz = 0.0000 m.
```

```
=====  

[[[*]]] APPLIED FACTORS.  

=====
```

- ```
( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).
[ Eurocode3:05 Annex A. Table A.1, A.2 ]
-. Cmy,0 = 0.940
-. Cmz,0 = 0.926
-. Cmy (Default or User Defined Value) = 1.000
-. Cmz (Default or User Defined Value) = 1.000
-. CmLT (Default or User Defined Value) = 1.000

( ). Partial Factors (Gamma_Mi).
[ Eurocode3:05 6.1 ]
-. Gamma_M0 = 1.05
-. Gamma_M1 = 1.10
-. Gamma_M2 = 1.25
```

```
=====  

[[[*]]] CHECK AXIAL RESISTANCE.  

=====
```

- ```
( ). Check slenderness ratio of axial compression member (Kl/i).
[ Eurocode3:05 6.3.1 ]
-. Kl/i = 56.7 < 200.0 ---> O.K.

( ). Calculate axial compressive resistance (Nc_Rd).
[ Eurocode3:05 6.1, 6.2.4 ]
-. Nc_Rd = fy * Aeff / Gamma_M0 = 822.25 kN.

( ). Check ratio of axial resistance (N_Ed/Nc_Rd).
N_Ed 149.58
-. ---- = ----- = 0.182 < 1.000 ---> O.K.
Nc_Rd 822.25
```

-----  
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=====

( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).  
[ Eurocode3:05 6.3.1.1, 6.3.1.2 ]

- . Beta\_A = Aeff / Area = 1.000
- . Lambdal = Pi \* SQRT(Es/fy) = 76.409
- . Lambda\_by = { (KLy/iy)/Lambdal } \* SQRT(Beta\_A) = 0.590
- . Ncry = Pi^2\*Es\*Ryy / KLy^2 = 2477.32 kN.
- . Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.
- . Alphay = 0.340
- . Phiy = 0.5 \* [ 1 + Alphay\*(Lambda\_by-0.2) + Lambda\_by^2 ] = 0.741
- . Xiy = MIN [ 1 / [Phiy + SQRT(Phiy^2 - Lambda\_by^2)], 1.0 ] = 0.842
- . Nb\_Rdy = Xiy\*Beta\_A\*Area\*fy / Gamma\_M1 = 660.77 kN.

- . Lambda\_bz = { (KLz/iz)/Lambdal } \* SQRT(Beta\_A) = 0.742
- . Ncrz = Pi^2\*Es\*Rzz / KLz^2 = 1566.25 kN.
- . Lambda\_bz > 0.2 and N\_Ed/Ncrz > 0.04 --> Need to check.
- . Alphaz = 0.340
- . Phiz = 0.5 \* [ 1 + Alphaz\*(Lambda\_bz-0.2) + Lambda\_bz^2 ] = 0.868
- . Xiz = MIN [ 1 / [Phiz + SQRT(Phiz^2 - Lambda\_bz^2)], 1.0 ] = 0.759
- . Nb\_Rdz = Xiz\*Beta\_A\*Area\*fy / Gamma\_M1 = 595.88 kN.

( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).

- . Nb\_Rd = MIN[ Nb\_Rdy, Nb\_Rdz ] = 595.88 kN.
- .  $\frac{N_{Ed}}{Nb_{Rd}} = \frac{149.58}{595.88} = 0.251 < 1.000 \text{ ---> O.K.}$

=====  
[[[\*]]] CHECK SHEAR RESISTANCE.  
=====

( ). Calculate shear area.  
[ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]

- . Avy = 2\*B\*tf = 0.0013 m^2.
- . Avz = 2\*h\*tw = 0.0013 m^2.

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
[ Eurocode3:05 6.1, 6.2.6 ]

- . Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 249.86 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
( LCB = 19-, POS = I )

- . Applied shear force : V\_Edy = 0.11 kN.
- .  $\frac{V_{Edy}}{V_{pl\_Rdy}} = \frac{0.11}{249.86} = 4.478e-004 < 1.000 \text{ ---> O.K.}$

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
[ Eurocode3:05 6.1, 6.2.6 ]

- . Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 249.86 kN.

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- ( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 -.  $HTR < 72 \cdot e / \eta$  ---> No need to check!
- ( ). Check ratio of shear resistance ( $V_{Edz} / V_{pl,Rdz}$ ).  
 ( LCB = 19-, POS = J )  
 -. Applied shear force :  $V_{Edz} = 2.10$  kN.  

$$\frac{V_{Edz}}{V_{pl,Rdz}} = \frac{2.10}{249.86} = 0.008 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

- ( ). Calculate local buckling resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -.  $W_{effy} = 2.5846e-005 \text{ m}^3$ .  
 -.  $M_{c,Rdy} = W_{effy} \cdot f_y / \gamma_{M1} = 8.34$  kN-m.
- ( ). Check ratio of moment resistance ( $M_{Edy} / M_{c,Rdy}$ ).  

$$\frac{M_{Edy}}{M_{c,Rdy}} = \frac{2.01}{8.34} = 0.241 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

- ( ). Calculate local buckling resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -.  $W_{effz} = 3.6115e-005 \text{ m}^3$ .  
 -.  $M_{c,Rdz} = W_{effz} \cdot f_y / \gamma_{M1} = 11.66$  kN-m.
- ( ). Check ratio of moment resistance ( $M_{Edz} / M_{c,Rdz}$ ).  

$$\frac{M_{Edz}}{M_{c,Rdz}} = \frac{0.48}{11.66} = 0.041 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.  
 =====

- ( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$   
 -.  $M_{y,Rd} = M_{c,Rdy} = 8.34$  kN-m.

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( ). Calculate Minor reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of  $V_{Edy} / V_{pl\_Rdy} < 0.5$   
 -.  $Mz\_Rd = Mc\_Rdz = 11.66$  kN-m.

( ). Check interaction ratio of bending and axial force.  
 [ Eurocode3:05 6.2.9.3 (6.44) ] - Class4  

$$R_{max1} = \frac{N_{Ed}}{A_{eff} \cdot f_y / \gamma_{M0}} + \frac{M_{Edy} + N_{Ed} \cdot e_{Nz}}{M_{y\_Rd}} + \frac{M_{Edz} + N_{Ed} \cdot e_{Ny}}{M_{z\_Rd}}$$
 = 0.464 < 1.000 ----> O.K.

( ). Check interaction ratio of bending and axial compression member.  
 [ Eurocode3:05 6.3.1, 6.2.9.3 (6.61, 6.62), Annex A ]

-.  $N_{Ed} = -149.58$  kN.  
 -.  $M_{Edy} = -2.01$  kN-m.  
 -.  $M_{Edz} = -0.48$  kN-m.  
 -.  $k_{yy} = 1.054$   
 -.  $k_{yz} = 1.094$   
 -.  $k_{zy} = 1.038$   
 -.  $k_{zz} = 1.078$   
 -.  $X_{iy} = 0.842$   
 -.  $X_{iz} = 0.759$   
 -.  $X_{iLT} = 0.927$   
 -.  $A_{eff} = 0.0024$  m<sup>2</sup>.  
 -.  $W_{effy} = 2.5846e-005$  m<sup>3</sup>.  
 -.  $W_{effz} = 3.6115e-005$  m<sup>3</sup>.  
 -.  $e_{Ny} = 1.4211e-017$  m.  
 -.  $e_{Nz} = 0.0000$  m.  
 -.  $N_{Rk} = A_{eff} \cdot f_y = 863.36$  kN.  
 -.  $My_{Rk} = W_{effy} \cdot f_y = 9.18$  kN-m.  
 -.  $Mz_{Rk} = W_{effz} \cdot f_y = 12.82$  kN-m.  
 -.  $N_{Ed} \cdot e_{Ny} = 2.13e-015$  kN-m.  
 -.  $N_{Ed} \cdot e_{Nz} = 0.00$  kN-m.

$$R_{max\_LT1} = \frac{N_{Ed} \cdot e_{Nz}}{M_{z\_Rk} / \gamma_{M1}} + k_{yy} \cdot \frac{M_{Edy} + N_{Ed} \cdot e_{Ny}}{M_{y\_Rk} / \gamma_{M1}} + k_{yz} \cdot \frac{M_{Edz} + N_{Ed} \cdot e_{Nz}}{M_{z\_Rk} / \gamma_{M1}}$$
 = 0.545 < 1.000 ----> O.K.

$$R_{max\_LT2} = \frac{N_{Ed} \cdot e_{Nz}}{M_{z\_Rk} / \gamma_{M1}} + k_{zy} \cdot \frac{M_{Edy} + N_{Ed} \cdot e_{Ny}}{M_{y\_Rk} / \gamma_{M1}} + k_{zz} \cdot \frac{M_{Edz} + N_{Ed} \cdot e_{Nz}}{M_{z\_Rk} / \gamma_{M1}}$$
 = 0.565 < 1.000 ----> O.K.

-.  $R_{max} = \text{MAX}[ R_{max1}, \text{MAX}(R_{max\_LT1}, R_{max\_LT2}) ] = 0.565 < 1.000$  ---->  
 O.K.

## 10.4 DIAFRAMMI SU PILE – TRASVERSI INFERIORI

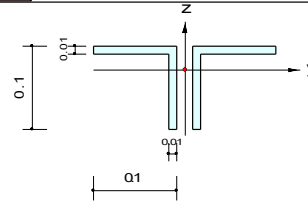
### MIDAS/Civil

### Steel Checking Result

|                |              |                      |                       |
|----------------|--------------|----------------------|-----------------------|
| <b>Company</b> |              | <b>Project Title</b> | CASALPUSTERLENGO      |
| <b>Author</b>  | Renato Vaira | <b>File Name</b>     | D:\...Asse 80 V_1.mcb |

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 2001  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : HINF\_PILE 2L100x10d20 (No:65)  
 (Built-up Section).  
 Member Length : 3.20000



#### 2. Member Forces

Axial Force Fxx = 365.496 (LCB: 19+, POS:J)  
 Bending Moments My = 1.51133, Mz = 1.03364  
 End Moments Myi = 0.88192, Myj = 1.51133 (for Lb)  
 Myi = 0.88192, Myj = 1.51133 (for Ly)  
 Mzi = 0.97428, Mzj = 1.03364 (for Lz)  
 Shear Forces Fyy = -0.6443 (LCB: 19-, POS:I)  
 Fzz = -1.5638 (LCB: 19-, POS:J)

|             |         |           |         |
|-------------|---------|-----------|---------|
| Depth       | 0.10000 | Web Thick | 0.01000 |
| Flg Width   | 0.10000 | Flg Thick | 0.01000 |
| BTB Spacing | 0.02000 |           |         |
| Area        | 0.00380 | Asz       | 0.00167 |
| Cyb         | 0.00254 | Czb       | 0.00500 |
| Iyy         | 0.00000 | Izz       | 0.00001 |
| Ybar        | 0.11000 | Zbar      | 0.07132 |
| Wely        | 0.00005 | Welz      | 0.00008 |
| ry          | 0.03078 | rz        | 0.04944 |

#### 3. Design Parameters

Unbraced Lengths Ly = 3.20000, Lz = 3.20000, Lb = 3.20000  
 Effective Length Factors Ky = 0.65, Kz = 0.65  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

##### Axial Resistance

$$N_{Ed}/N_{t,Rd} = 365.50/1284.76 = 0.284 < 1.000 \dots\dots\dots \text{O.K}$$

##### Bending Resistance

$$M_{Edy}/M_{Rdy} = 1.5113/30.7498 = 0.049 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 1.0336/49.7000 = 0.021 < 1.000 \dots\dots\dots \text{O.K}$$

##### Combined Resistance

$$R_{NRd} = \text{MAX}[ M_{Edy}/M_{ny,Rd}, M_{Edz}/M_{nz,Rd} ]$$

$$R_{max1} = (M_{Edy}/M_{ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz,Rd})^{\text{Beta}}$$

$$R_{oom} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$$

$$R_{max} = \text{MAX}[ R_{NRd}, R_{max1}, (R_{oom} + R_{bend}) ] = 0.354 < 1.000 \dots\dots\dots \text{O.K}$$

##### Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.002 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.004 < 1.000 \dots\dots\dots \text{O.K}$$





1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV( 1.350)  
 22- 1 Dead Load( 1.350) +Erection Load 1( 1.500) +Creep Secondary( 1.200)  
 INV( 0.900) +Shrinkage Secondary( 1.200) +Vento Strutture INV( 0.900) +Vento Mobili  
 1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV(

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    23+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    23-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    24+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    24-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    25+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    25-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    28+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    28-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    29+  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    29-  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    30+  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    30-  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    31+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    31-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    32+  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    32-  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    33+  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    33-  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    34+  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
    34-  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
  
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35+ 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)  
35- 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
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\*. PROJECT : CASALPUSTERLENGO  
 \*. MEMBER NO = 2001, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19+, MATERIAL NO = 1, SECTION NO = 65  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = HINF\_PILE 2L100x10d20  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.100, Flg Width = 0.100, BTB Spacing = 0.020  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.80000e-003, Avy = 2.00000e-003, Avz = 2.00000e-003  
 Ybar = 1.10000e-001, Zbar = 7.13158e-002, Qyb = 2.54297e-003, Qzb =  
 5.00000e-003  
 Wely = 5.04809e-005, Welz = 8.44242e-005, Wply = 9.09500e-005, Wplz =  
 1.47000e-004  
 Iyy = 3.60009e-006, Izz = 9.28667e-006, Iyz = 0.00000e+000  
 iy = 3.07797e-002, iz = 4.94354e-002  
 J = 1.26667e-007, Cwp = 8.81319e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.20000e+000, Lz = 3.20000e+000, Lu = 3.20000e+000  
 Ky = 6.50000e-001, Kz = 6.50000e-001

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = 3.65496e+002  
 Shear Forces Fyy = 6.12634e-001, Fzz = -3.75782e-001  
 Bending Moments My = 1.51133e+000, Mz = 1.03364e+000  
 End Moments Myi = 8.81925e-001, Myj = 1.51133e+000 (for Lb)  
 Myi = 8.81925e-001, Myj = 1.51133e+000 (for Ly)  
 Mzi = 9.74282e-001, Mzj = 1.03364e+000 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of tension flanges(double angle).  
 -. Not Checking the Section Classification.

( ). Determine classification of tension flanges(double angle).  
 -. Not Checking the Section Classification.

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

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 - MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
 =====  
 =

- ( ). Determine classification of tension elements(Double angle).
  - . Not Checking the Section Classification.

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).
  - [ Eurocode3:05 Annex A. Table A.1, A.2 ]
  - . Cmy,0 = 0.788
  - . Cmz,0 = 0.918
  - . Cmy (Default or User Defined Value) = 1.000
  - . Cmz (Default or User Defined Value) = 1.000
  - . CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).
  - [ Eurocode3:05 6.1 ]
  - . Gamma\_M0 = 1.05
  - . Gamma\_M1 = 1.10
  - . Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial tension member (l/i).
  - [ Eurocode3:05 6.3.1 ]
  - . l/i = 104.0 < 300.0 ---> O.K.
- ( ). Calculate parameters for combined resistance.
  - . Lambdal = Pi \* SQRT(Es/fy) = 76.409
  - . Lambda\_bz = (KLz/iz) / Lambdal = 0.847
- ( ). Calculate axial tensile resistance (Nt\_Rd).
  - [ Eurocode3:05 6.2.3 ]
  - . Nt\_Rd = fy \* Area / Gamma\_M0 = 1284.76 kN.
- ( ). Check ratio of axial resistance (N\_Ed/Nt\_Rd).
  - .  $\frac{N_{Ed}}{Nt_{Rd}} = \frac{365.50}{1284.76} = 0.284 < 1.000 \text{ ---> O.K.}$

=====  
 [[[\*]]] CHECK SHEAR RESISTANCE.  
 =====

- ( ). Calculate shear area.
  - [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]
  - . Avy = 2\*B\*tf = 0.0020 m^2.
  - . Avz = 2\*h\*tw = 0.0020 m^2.

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 =====  
 =

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 390.40 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19-, POS = J )  
 -. Applied shear force : V\_Edy = 0.64 kN.  

$$\frac{V\_Edy}{Vpl\_Rdy} = \frac{0.64}{390.40} = 0.002 < 1.000 \text{ ---> O.K.}$$

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 390.40 kN.

( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 -. HTR < 72\*e/Eta ---> No need to check!

( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19-, POS = J )  
 -. Applied shear force : V\_Edz = 1.56 kN.  

$$\frac{V\_Edz}{Vpl\_Rdz} = \frac{1.56}{390.40} = 0.004 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wply = 9.0950e-005 m^3.  
 -. Mc\_Rdy = Wply \* fy / Gamma\_M0 = 30.75 kN-m.

( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  

$$\frac{M\_Edy}{Mc\_Rdy} = \frac{1.51}{30.75} = 0.049 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wplz = 0.0001 m^3.  
 -. Mc\_Rdz = Wplz \* fy / Gamma\_M0 = 49.70 kN-m.



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( ). Check ratio of moment resistance (M\_Edz/Mc\_Rdz).  

$$\frac{M\_Edz}{Mc\_Rdz} = \frac{1.03}{49.70} = 0.021 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.  
 =====

( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of V\_Edz / Vpl\_Rdz < 0.5  
 -. My\_Rd = Mc\_Rdy = 30.75 kN-m.

( ). Calculate Minor reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of V\_Edy / Vpl\_Rdy < 0.5  
 -. Mz\_Rd = Mc\_Rdz = 49.70 kN-m.

( ). Check general interaction ratio.  
 [ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2  

$$R_{max1} = \frac{N\_Ed}{N\_Rd} + \frac{M\_Edy}{My\_Rd} + \frac{M\_Edz}{Mz\_Rd}$$

$$= 0.354 < 1.000 \text{ ---> O.K.}$$

( ). Check interaction ratio of bending and axial force member.  
 [ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2  
 -. n = N\_Ed / Npl\_Rd = 0.284  
 -. a = MIN[ (Area-2b\*tf)/Area, 0.5 ] = 0.500  
 -. Alpha = 2.000  
 -. Beta = MAX[ 5\*n, 1.0 ] = 1.422  
 -. Mny\_Rd = MIN[ Mply\_Rd\*(1-n)/(1-0.5\*a), Mply\_Rd ] = 29.34 kN-m.  
 -. Rmaxy = M\_Edy / Mny\_Rd = 0.052 < 1.000 ---> O.K.

-. In case of n < a  
 -. Mnz\_Rd = Mplz\_Rd = 49.70 kN-m.  
 -. Rmaxz = M\_Edz / Mnz\_Rd = 0.021 < 1.000 ---> O.K.

$$R_{max2} = \frac{\left[ \frac{M\_Edy}{Mny\_Rd} \right]^{Alpha} + \left[ \frac{M\_Edz}{Mnz\_Rd} \right]^{Beta}}{1}$$

$$= 0.007 < 1.000 \text{ ---> O.K.}$$

-. Rmax = MAX[ Rmax1, Rmax2 ] = 0.354 < 1.000 ---> O.K.

## 10.5 DIAFRAMMI SU PILE – TRASVERSI SUPERIORI

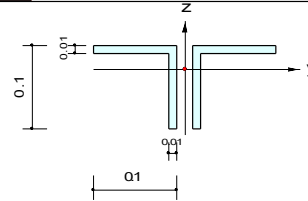
MIDAS/Civil

### Steel Checking Result

|                |              |                      |                       |
|----------------|--------------|----------------------|-----------------------|
| <b>Company</b> |              | <b>Project Title</b> | CASALPUSTERLENGO      |
| <b>Author</b>  | Renato Vaira | <b>File Name</b>     | D:\...Asse 80 V_1.mcb |

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 2045  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : HSUP\_PILE 2L100x10d20 (No:19)  
 (Built-up Section).  
 Member Length : 3.20000



#### 2. Member Forces

Axial Force Fxx = -136.35 (LCB: 22-, POS:J)  
 Bending Moments My = -1.3657, Mz = -0.1477  
 End Moments Myi = -0.8838, Myj = -1.3657 (for Lb)  
 Mzi = -0.8838, Mzj = -1.3657 (for Ly)  
 Mzi = -0.0782, Mzj = -0.1477 (for Lz)  
 Shear Forces Fyy = 0.11250 (LCB: 19+, POS:I)  
 Fzz = 1.56387 (LCB: 19+, POS:J)

|             |         |           |         |
|-------------|---------|-----------|---------|
| Depth       | 0.10000 | Web Thick | 0.01000 |
| Flg Width   | 0.10000 | Flg Thick | 0.01000 |
| BTB Spacing | 0.02000 |           |         |
| Area        | 0.00380 | Asz       | 0.00167 |
| Cyb         | 0.00254 | Czb       | 0.00500 |
| Iyy         | 0.00000 | Izz       | 0.00001 |
| Ybar        | 0.11000 | Zbar      | 0.07132 |
| Wely        | 0.00005 | Welz      | 0.00008 |
| ry          | 0.03078 | rz        | 0.04944 |

#### 3. Design Parameters

Unbraced Lengths Ly = 3.20000, Lz = 3.20000, Lb = 3.20000  
 Effective Length Factors Ky = 0.65, Kz = 0.65  
 Equivalent Uniform Moment Factors Cmy = 1.00, Crnz = 1.00, CmLT = 1.00

#### 4. Checking Results

Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c,Rd}, N_{b,Rd}] = 136.353/823.126 = 0.166 < 1.000 \dots\dots\dots \text{O.K}$$

Bending Resistance

$$M_{Edy}/M_{Rdy} = 1.3657/16.2916 = 0.084 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 0.1477/22.8706 = 0.006 < 1.000 \dots\dots\dots \text{O.K}$$

Combined Resistance

$$R_{oom} = N_{Ed}/(A_{eff} \cdot f_y / \Gamma_{M0}), R_{bend} = (M_{Edy} + N_{Ed} \cdot e_{Ny}) / M_{y,Rd} + (M_{Edz} + N_{Ed} \cdot e_{Nz}) / M_{z,Rd}$$

$$R_{c\_LT1} = N_{Ed} / (X_{iy} \cdot A_{eff} \cdot f_y / \Gamma_{M1})$$

$$R_{b\_LT1} = k_{yy} \cdot (M_{Edy} + N_{Ed} \cdot e_{Ny}) / (X_{i\_LT} \cdot W_{effy} \cdot f_y / \Gamma_{M1}) + k_{yz} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (W_{effz} \cdot f_y / \Gamma_{M1})$$

$$R_{c\_LT2} = N_{Ed} / (X_{iz} \cdot A_{eff} \cdot f_y / \Gamma_{M1})$$

$$R_{b\_LT2} = k_{zy} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (X_{i\_LT} \cdot W_{effy} \cdot f_y / \Gamma_{M1}) + k_{zz} \cdot (M_{Edz} + N_{Ed} \cdot e_{Nz}) / (W_{effz} \cdot f_y / \Gamma_{M1})$$

$$R_{max} = \text{MAX}[R_{oom} + R_{bend}, \text{MAX}(R_{c\_LT1} + R_{b\_LT1}, R_{c\_LT2} + R_{b\_LT2})] = 0.275 < 1.000 \dots\dots \text{O.K}$$

Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.000 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.004 < 1.000 \dots\dots\dots \text{O.K}$$



1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV( 1.350)  
 22- 1 Dead Load( 1.350) +Erection Load 1( 1.500) +Creep Secondary( 1.200)  
 INV( 0.900) +Shrinkage Secondary( 1.200) +Vento Strutture INV( 0.900) +Vento Mobili  
 1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV(

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    23+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    23-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    24+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    24-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    25+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    25-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    28+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    28-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    29+  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    29-  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    30+  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    30-  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    31+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    31-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    32+  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    32-  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    33+  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    33-  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    34+  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
    34-  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
  
```

35+ 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)  
35- 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
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\*. PROJECT : CASALPUSTERLENGO  
 \*. MEMBER NO = 2045, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 22-, MATERIAL NO = 1, SECTION NO = 19  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = HSUP\_PILE 2L100x10d20  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.100, Flg Width = 0.100, BTB Spacing = 0.020  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.80000e-003, Avy = 2.00000e-003, Avz = 2.00000e-003  
 Ybar = 1.10000e-001, Zbar = 7.13158e-002, Qyb = 2.54297e-003, Qzb =  
 5.00000e-003  
 Wely = 5.04809e-005, Welz = 8.44242e-005, Wply = 9.09500e-005, Wplz =  
 1.47000e-004  
 Iyy = 3.60009e-006, Izz = 9.28667e-006, Iyz = 0.00000e+000  
 iy = 3.07797e-002, iz = 4.94354e-002  
 J = 1.26667e-007, Cwp = 8.81319e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.20000e+000, Lz = 3.20000e+000, Lu = 3.20000e+000  
 Ky = 6.50000e-001, Kz = 6.50000e-001

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = -1.36353e+002  
 Shear Forces Fyy = -4.96787e-002, Fzz = 4.09770e-001  
 Bending Moments My = -1.36568e+000, Mz = -1.47676e-001  
 End Moments Myi = -8.83830e-001, Myj = -1.36568e+000 (for Lb)  
 Myi = -8.83830e-001, Myj = -1.36568e+000 (for Ly)  
 Mzi = -7.81600e-002, Mzj = -1.47676e-001 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -. e = SQRT( 235/fy ) = 0.81  
 -. b/t = BTR = 9.00  
 -. sigma1 = 26750.304 KPa.  
 -. sigma2 = 25160.106 KPa.  
 -. BTR\_L = (H+B) / (2\*tf) = 10.00  
 -. BTR > 15\*e or BTR\_L > 11.5\*e ( Class 4 : Slender ).

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MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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=====  

[[[*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  

=====
```

```
( ). Determine classification of compression flanges(Double angle).  

[ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  

-. e      = SQRT( 235/fy ) =    0.81  

-. b/t    = BTR =    9.00  

-. sigma1 = 24842.067 KPa.  

-. sigma2 = 23251.870 KPa.  

-. BTR_L = (H+B) / (2*tf) = 10.00  

-. BTR > 15*e or BTR_L > 11.5*e ( Class 4 : Slender ).
```

```
=====  

[[[*]]] CLASSIFY WEB OF SECTION (HTR).  

=====
```

```
( ). Determine classification of compression element(Double angles).  

[ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  

-. e      = SQRT( 235/fy ) =    0.81  

-. d/t    = HTR = 10.00  

-. sigma1 = 63253.766 KPa.  

-. sigma2 = 29112.590 KPa.  

-. HTR_L = (h+B) / (2*tw) = 10.00  

-. HTR > 15*e or HTR_L > 11.5*e ( Class 4 : Slender ).
```

```
=====  

[[[*]]] CALCULATE EFFECTIVE AREA.  

=====
```

```
( ). Calculate buckling factor of outstand compression element.  

[ Eurocode3 Part 1-5 4.4, Table 4.2 ]  

-. In case of Psi = 1.0  

-. k_sigma = 0.4300
```

```
( ). Calculate effective cross-section properties of left flange flange of Class 4  

(Outstand element).
```

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]  

-. RatT    = 9.0000  

-. Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5940  

-. Rho     = 1.0  

-. sigma_max = MAX( sigma1, sigma2 ) = 35882.339 KPa.  

-. sigma_min = MIN( sigma1, sigma2 ) = 35882.339 KPa.  

-. r        = 0.000 m.  

-. bc      = 0.100 m.  

-. beff = Rho*bc + r = 0.100 m.  

-. Aeff = beff * tf = 0.001 m^2.  

-. yeff = beff/2 = 0.050 m.
```

```
( ). Calculate buckling factor of outstand compression element.  

[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
```

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MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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```
- . In case of Psi = 1.0
- . k_sigma = 0.4300
```

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT      = 9.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5940
- . Rho      = 1.0
- . sigma_max = MAX( sigma1, sigma2 ) = 35882.339 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = 35882.339 KPa.
- . r        = 0.000 m.
- . bc      = 0.100 m.
- . beff    = Rho*bc + r = 0.100 m.
- . Aeff    = beff * tf = 0.001 m^2.
- . yeff    = beff/2 = 0.050 m.
```

( ). Calculate buckling factor of outstand compression element.

```
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- . In case of Psi = 1.0
- . k_sigma = 0.4300
```

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT      = 10.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.6600
- . Rho      = 1.0
- . sigma_max = MAX( sigma1, sigma2 ) = 35882.339 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = 35882.339 KPa.
- . r        = 0.000 m.
- . Ar      = 0.000 m^2.
- . dc      = 0.090 m.
- . deff1    = 2*(Rho*dc) / [ 5 - sigma_min/sigma_max ] + r = 0.045 m.
- . Aeff1    = deff1 * tw + 2*Ar = 4.500e-004 m^2.
- . zeff1    = deff1/2 + tf = 0.022 m.
- . deff2    = (Rho*dc) - deff1 + r = 0.045 m.
- . Aeff2    = deff2 * tw + 2*Ar = 4.500e-004 m^2.
- . zeff2    = (h+2*r) - deff2/2 + tf = 0.068 m.
```

( ). Calculate buckling factor of outstand compression element.

```
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- . In case of Psi = 1.0
- . k_sigma = 0.4300
```

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT      = 10.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.6600
- . Rho      = 1.0
```

```
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MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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```
- . sigma_max = MAX( sigma1, sigma2 ) = 35882.339 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = 35882.339 KPa.
- . r = 0.000 m.
- . Ar = 0.000 m^2.
- . dc = 0.090 m.
- . deff1 = 2*(Rho*dc) / [ 5 - sigma_min/sigma_max ] + r = 0.045 m.
- . Aeff1 = deff1 * tw + 2*Ar = 4.500e-004 m^2.
- . zeff1 = deff1/2 + tf = 0.022 m.
- . deff2 = (Rho*dc) - deff1 + r = 0.045 m.
- . Aeff2 = deff2 * tw + 2*Ar = 4.500e-004 m^2.
- . zeff2 = (h+2*r) - deff2/2 + tf = 0.068 m.
```

```
=====[[*]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MAJOR AXIS.=====[[*]]
```

( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . Rho = 1.0 (Only tensile stresses).
- . sigma_max = MAX( sigma1, sigma2 ) = -10881.252 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = -10881.252 KPa.
- . r = 0.000 m.
- . bc = 0.100 m.
- . beff = Rho*bc + r = 0.100 m.
- . Aeff = beff * tf = 0.001 m^2.
- . yeff = beff/2 = 0.050 m.
```

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . Rho = 1.0 (Only tensile stresses).
- . sigma_max = MAX( sigma1, sigma2 ) = -10881.252 KPa.
- . sigma_min = MIN( sigma1, sigma2 ) = -10881.252 KPa.
- . r = 0.000 m.
- . bc = 0.100 m.
- . beff = Rho*bc + r = 0.100 m.
- . Aeff = beff * tf = 0.001 m^2.
- . yeff = beff/2 = 0.050 m.
```

( ). Calculate buckling factor of outstand compression element.

```
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
- . k_sigma = 0.57 - 0.21*Psi + 0.07*Psi^2 = 0.6298
```

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

```
[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
- . RatT = 10.0000
- . Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5453
- . Rho = 1.0
```

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```

-. sigma_max = MAX( sigma1, sigma2 ) = 27053.388 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = -7087.788 KPa.
-. r = 0.000 m.
-. Ar = 0.000 m^2.
-. dc = (h*sigma_max) / (sigma_max-sigma_min) = 0.071 m.
-. deff1 = 0.4*Rho*dc + r = 0.029 m.
-. Aeff1 = deff1 * tw + 2*Ar = 2.853e-004 m^2.
-. zeff1 = deff1/2 + tf = 0.014 m.
-. deff2 = 0.6*Rho*dc + (h-dc) + r = 0.061 m.
-. Aeff2 = deff2 * tw + 2*Ar = 6.147e-004 m^2.
-. zeff2 = (h+2*r) - deff2/2 + tf = 0.059 m.

```

( ). Calculate buckling factor of outstand compression element.  
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]  
-. k\_sigma = 0.57 - 0.21\*Psi + 0.07\*Psi^2 = 0.6298

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).

```

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
-. RatT = 10.0000
-. Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5453
-. Rho = 1.0
-. sigma_max = MAX( sigma1, sigma2 ) = 27053.388 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = -7087.788 KPa.
-. r = 0.000 m.
-. Ar = 0.000 m^2.
-. dc = (h*sigma_max) / (sigma_max-sigma_min) = 0.071 m.
-. deff1 = 0.4*Rho*dc + r = 0.029 m.
-. Aeff1 = deff1 * tw + 2*Ar = 2.853e-004 m^2.
-. zeff1 = deff1/2 + tf = 0.014 m.
-. deff2 = 0.6*Rho*dc + (h-dc) + r = 0.061 m.
-. Aeff2 = deff2 * tw + 2*Ar = 6.147e-004 m^2.
-. zeff2 = (h+2*r) - deff2/2 + tf = 0.059 m.

```

=====  
[[[\*]]] CALCULATE EFFECTIVE SECTION MODULUS ABOUT MINOR AXIS.  
=====

( ). Calculate buckling factor of outstand compression element.  
[ Eurocode3 Part 1-5 4.4, Table 4.2 ]  
-. k\_sigma = 0.57 - 0.21\*Psi + 0.07\*Psi^2 = 0.5515

( ). Calculate effective cross-section properties of left flange flange of Class 4 (Outstand element).

```

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
-. RatT = 9.0000
-. Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.5245
-. Rho = 1.0

```

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```

-. sigma_max = MAX( sigma1, sigma2 ) = 1749.217 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = 159.020 KPa.
-. r = 0.000 m.
-. bc = 0.100 m.
-. beff = Rho*bc + r = 0.100 m.
-. Aeff = beff * tf = 0.001 m^2.
-. yeff = beff/2 = 0.050 m.
  
```

( ). Calculate effective cross-section properties of right flange flange of Class 4 (Outstand element).

```

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
-. Rho = 1.0 (Only tensile stresses).
-. sigma_max = MAX( sigma1, sigma2 ) = -159.020 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = -1749.217 KPa.
-. r = 0.000 m.
-. bc = 0.100 m.
-. beff = Rho*bc + r = 0.100 m.
-. Aeff = beff * tf = 0.001 m^2.
-. yeff = beff/2 = 0.050 m.
  
```

( ). Calculate buckling factor of outstand compression element.

```

[ Eurocode3 Part 1-5 4.4, Table 4.2 ]
-. In case of Psi = 1.0
-. k_sigma = 0.4300
  
```

( ). Calculate effective cross-section properties of left web of Class 4 (Internal element).

```

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
-. RatT = 10.0000
-. Lambda_p = RatT / [ 28.4*Eps*SQRT(k_sigma) ] = 0.6600
-. Rho = 1.0
-. sigma_max = MAX( sigma1, sigma2 ) = 318.039 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = 318.039 KPa.
-. r = 0.000 m.
-. Ar = 0.000 m^2.
-. dc = 0.090 m.
-. deff1 = 2*(Rho*dc) / [ 5 - sigma_min/sigma_max ] + r = 0.045 m.
-. Aeff1 = deff1 * tw + 2*Ar = 4.500e-004 m^2.
-. zeff1 = deff1/2 + tf = 0.022 m.
-. deff2 = (Rho*dc) - deff1 + r = 0.045 m.
-. Aeff2 = deff2 * tw + 2*Ar = 4.500e-004 m^2.
-. zeff2 = (h+2*r) - deff2/2 + tf = 0.068 m.
  
```

( ). Calculate effective cross-section properties of right web of Class 4 (Internal element).

```

[ Eurocode3 Part 1-5 4.4, Table 4.1, 4.2 ]
-. Rho = 1.0 (Only tensile stresses).
-. sigma_max = MAX( sigma1, sigma2 ) = -318.039 KPa.
-. sigma_min = MIN( sigma1, sigma2 ) = -318.039 KPa.
-. r = 0.000 m.
-. Ar = 0.000 m^2.
-. dc = 0.090 m.
-. deff = dc + r = 0.090 m.
-. Aeff = deff * tw + 4*Ar = 9.000e-004 m^2.
-. zeff = (h+2*r) - deff/2 = 0.045 m.
  
```



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=====  
 [[[\*]]] EFFECTIVE SECTION PPOPERTIES.  
 =====

- ( ). Calculated effective cross-section properties of Class4 cross-section.  
 -. Aeff = 0.0038 m<sup>2</sup>. (for calculating axial resistance)  
 -. Aeffy = 0.0038 m<sup>2</sup>.  
 -. Weffy = 5.0481e-005 m<sup>3</sup>.  
 -. Aeffz = 0.0038 m<sup>2</sup>.  
 -. Weffz = 7.0867e-005 m<sup>3</sup>.  
 -. eNy = 0.0000 m.  
 -. eNz = 0.0000 m.

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).  
 [ Eurocode3:05 Annex A. Table A.1, A.2 ]  
 -. Cmy,0 = 0.921  
 -. Cmz,0 = 0.969  
 -. Cmy (Default or User Defined Value) = 1.000  
 -. Cmz (Default or User Defined Value) = 1.000  
 -. CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).  
 [ Eurocode3:05 6.1 ]  
 -. Gamma\_M0 = 1.05  
 -. Gamma\_M1 = 1.10  
 -. Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial compression member (Kl/i).  
 [ Eurocode3:05 6.3.1 ]  
 -. Kl/i = 67.6 < 200.0 ---> O.K.
- ( ). Calculate axial compressive resistance (Nc\_Rd).  
 [ Eurocode3:05 6.1, 6.2.4 ]  
 -. Nc\_Rd = fy \* Aeff / Gamma\_M0 = 1284.76 kN.
- ( ). Check ratio of axial resistance (N\_Ed/Nc\_Rd).  
 N\_Ed 136.35  
 -. ----- = ----- = 0.106 < 1.000 ---> O.K.  
 Nc\_Rd 1284.76

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=====

( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).  
[ Eurocode3:05 6.3.1.1, 6.3.1.2 ]  
-. Beta\_A = Aeff / Area = 1.000  
-. Lambdal = Pi \* SQRT(Es/fy) = 76.409  
-. Lambda\_by = { (KLy/iy)/Lambdal } \* SQRT(Beta\_A) = 0.884  
-. Ncry = Pi^2\*Es\*Ryy / KLy^2 = 1724.67 kN.  
-. Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.  
-. Alphay = 0.340  
-. Phiy = 0.5 \* [ 1 + Alphay\*(Lambda\_by-0.2) + Lambda\_by^2 ] = 1.007  
-. Xiy = MIN [ 1 / [Phiy + SQRT(Phiy^2 - Lambda\_by^2)], 1.0 ] = 0.671  
-. Nb\_Rdy = Xiy\*Beta\_A\*Area\*fy / Gamma\_M1 = 823.13 kN.  
  
-. Lambda\_bz = { (KLz/iz)/Lambdal } \* SQRT(Beta\_A) = 0.551  
-. Ncrz = Pi^2\*Es\*Rzz / KLz^2 = 4448.90 kN.  
-. Lambda\_bz < 0.2 or N\_Ed/Ncrz < 0.04 --> No need to check.  
  
( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).  
-. Nb\_Rd = MIN[ Nb\_Rdy, Nb\_Rdz ] = 823.13 kN.  
N\_Ed = 136.35  
-. ----- = ----- = 0.166 < 1.000 ---> O.K.  
Nb\_Rd = 823.13

=====  
[[[\*]]] CHECK SHEAR RESISTANCE.  
=====

( ). Calculate shear area.  
[ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]  
-. Avy = 2\*B\*tf = 0.0020 m^2.  
-. Avz = 2\*h\*tw = 0.0020 m^2.  
  
( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
[ Eurocode3:05 6.1, 6.2.6 ]  
-. Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 390.40 kN.  
  
( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
( LCB = 19+, POS = J )  
-. Applied shear force : V\_Edy = 0.11 kN.  
V\_Edy = 0.11  
-. ----- = ----- = 2.882e-004 < 1.000 ---> O.K.  
Vpl\_Rdy = 390.40  
  
( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
[ Eurocode3:05 6.1, 6.2.6 ]  
-. Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 390.40 kN.  
  
( ). Shear Buckling Check.  
[ Eurocode3:05 6.2.6 ]  
-. HTR < 72\*e/Eta ---> No need to check!

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( ). Check ratio of shear resistance ( $V_{Edz}/V_{pl,Rdz}$ ).  
 ( LCB = 19+, POS = J )  
 -. Applied shear force :  $V_{Edz} = 1.56$  kN.  

$$\frac{V_{Edz}}{V_{pl,Rdz}} = \frac{1.56}{390.40} = 0.004 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

( ). Calculate local buckling resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -.  $W_{effy} = 5.0481e-005$  m<sup>3</sup>.  
 -.  $Mc_{Rdy} = W_{effy} * f_y / \Gamma_{M1} = 16.29$  kN-m.  
 ( ). Check ratio of moment resistance ( $M_{Edy}/Mc_{Rdy}$ ).  

$$\frac{M_{Edy}}{Mc_{Rdy}} = \frac{1.37}{16.29} = 0.084 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

( ). Calculate local buckling resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -.  $W_{effz} = 7.0867e-005$  m<sup>3</sup>.  
 -.  $Mc_{Rdz} = W_{effz} * f_y / \Gamma_{M1} = 22.87$  kN-m.  
 ( ). Check ratio of moment resistance ( $M_{Edz}/Mc_{Rdz}$ ).  

$$\frac{M_{Edz}}{Mc_{Rdz}} = \frac{0.15}{22.87} = 0.006 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.  
 =====

( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$   
 -.  $M_{y,Rd} = Mc_{Rdy} = 16.29$  kN-m.  
 ( ). Calculate Minor reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of  $V_{Edy} / V_{pl,Rdy} < 0.5$   
 -.  $M_{z,Rd} = Mc_{Rdz} = 22.87$  kN-m.



## 10.6 DIAFRAMMI SU PILE – DIAGONALI

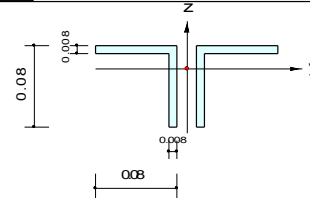
### MIDAS/Civil

### Steel Checking Result

|                |              |                      |                       |
|----------------|--------------|----------------------|-----------------------|
| <b>Company</b> |              | <b>Project Title</b> | CASALPUSTERLENGO      |
| <b>Author</b>  | Renato Vaira | <b>File Name</b>     | D:\...Asse 80 V_1.mdb |

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 2034  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : DIAG\_PILE 2L80x8d20 (No:20)  
 (Built-up Section).  
 Member Length : 1.78220



#### 2. Member Forces

Axial Force Fxx = 268.891 (LCB: 19+, POS:J)  
 Bending Moments My = 0.50471, Mz = 0.11916  
 End Moments Myi = 0.07711, Myj = 0.50471 (for Lb)  
 Myi = 0.07711, Myj = 0.50471 (for Ly)  
 Mzi = 0.19043, Mzj = 0.11916 (for Lz)  
 Shear Forces Fyy = -0.0639 (LCB: 19-, POS:I)  
 Fzz = -0.5883 (LCB: 19-, POS:J)

|             |         |           |         |
|-------------|---------|-----------|---------|
| Depth       | 0.08000 | Web Thick | 0.00800 |
| Flg Width   | 0.08000 | Flg Thick | 0.00800 |
| BTB Spacing | 0.02000 |           |         |
| Area        | 0.00243 | Asz       | 0.00107 |
| Cyb         | 0.00163 | Czb       | 0.00320 |
| Iyy         | 0.00000 | Izz       | 0.00000 |
| Ybar        | 0.09000 | Zbar      | 0.05705 |
| Wely        | 0.00003 | Welz      | 0.00005 |
| ry          | 0.02462 | rz        | 0.04113 |

#### 3. Design Parameters

Unbraced Lengths Ly = 1.78220, Lz = 1.78220, Lb = 1.78220  
 Effective Length Factors Ky = 0.65, Kz = 1.30  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

Axial Resistance  
 $N_{Ed}/N_{t,Rd} = 268.891/822.248 = 0.327 < 1.000$  ..... O.K

Bending Resistance  
 $M_{Edy}/M_{Rdy} = 0.5047/15.7439 = 0.032 < 1.000$  ..... O.K  
 $M_{Edz}/M_{Rdz} = 0.1192/27.0909 = 0.004 < 1.000$  ..... O.K

Combined Resistance  
 $R_{NRd} = \text{MAX}[ M_{Edy}/M_{ny,Rd}, M_{Edz}/M_{nz,Rd} ]$   
 $R_{max1} = (M_{Edy}/M_{ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz,Rd})^{\text{Beta}}$   
 $R_{000} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$   
 $R_{max} = \text{MAX}[ R_{NRd}, R_{max1}, (R_{000} + R_{bend}) ] = 0.363 < 1.000$  ..... O.K

Shear Resistance  
 $V_{Edy}/V_{y,Rd} = 0.000 < 1.000$  ..... O.K  
 $V_{Edz}/V_{z,Rd} = 0.002 < 1.000$  ..... O.K





1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV( 1.350)  
 22- 1 Dead Load( 1.350) +Erection Load 1( 1.500) +Creep Secondary( 1.200)  
 INV( 0.900) +Shrinkage Secondary( 1.200) +Vento Strutture INV( 0.900) +Vento Mobili  
 1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV(

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MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ]                               Version 8.3.5
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23+ 1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
23- 1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
24+ 1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
24- 1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
25+ 1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
25- 1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
28+ 2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
      +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
28- 2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
      +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
29+ 2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
      +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
29- 2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
      +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
30+ 2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
30- 2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
31+ 2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
      +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
31- 2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
      +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
32+ 2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      +Mobili SLE INV( 1.000)
32- 2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
      +Mobili SLE INV( 1.000)
33+ 2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
      +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
33- 2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
      +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
34+ 2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
34- 2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)

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35+ 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)  
35- 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
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\*. PROJECT : CASALPUSTERLENGO  
 \*. MEMBER NO = 2034, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19+, MATERIAL NO = 1, SECTION NO = 20  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = DIAG\_PILE 2L80x8d20  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.020  
 Web Thick = 0.008, Flg Thick = 0.008

Area = 2.43200e-003, Avy = 1.28000e-003, Avz = 1.28000e-003  
 Ybar = 9.00000e-002, Zbar = 5.70526e-002, Qyb = 1.62750e-003, Qzb =  
 3.20000e-003  
 Wely = 2.58462e-005, Welz = 4.57178e-005, Wply = 4.65664e-005, Wplz =  
 8.01280e-005  
 Iyy = 1.47460e-006, Izz = 4.11460e-006, Iyz = 0.00000e+000  
 iy = 2.46238e-002, iz = 4.11322e-002  
 J = 5.18827e-008, Cwp = 2.31033e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 1.78220e+000, Lz = 1.78220e+000, Lu = 1.78220e+000  
 Ky = 6.50000e-001, Kz = 1.30000e+000

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = 2.68891e+002  
 Shear Forces Fyy = 4.00149e-002, Fzz = -1.89426e-001  
 Bending Moments My = 5.04707e-001, Mz = 1.19158e-001  
 End Moments Myi = 7.71066e-002, Myj = 5.04707e-001 (for Lb)  
 Myi = 7.71066e-002, Myj = 5.04707e-001 (for Ly)  
 Mzi = 1.90433e-001, Mzj = 1.19158e-001 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of tension flanges(double angle).  
 -. Not Checking the Section Classification.

( ). Determine classification of tension flanges(double angle).  
 -. Not Checking the Section Classification.

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

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 =

- ( ). Determine classification of tension elements(Double angle).
  - . Not Checking the Section Classification.

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).
  - [ Eurocode3:05 Annex A. Table A.1, A.2 ]
  - . Cmy,0 = 0.882
  - . Cmz,0 = 0.939
  - . Cmy (Default or User Defined Value) = 1.000
  - . Cmz (Default or User Defined Value) = 1.000
  - . CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).
  - [ Eurocode3:05 6.1 ]
  - . Gamma\_M0 = 1.05
  - . Gamma\_M1 = 1.10
  - . Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial tension member (l/i).
  - [ Eurocode3:05 6.3.1 ]
  - . l/i = 72.4 < 300.0 ---> O.K.
- ( ). Calculate parameters for combined resistance.
  - . Lambdal = Pi \* SQRT(Es/fy) = 76.409
  - . Lambda\_bz = (KLz/iz) / Lambdal = 0.567
- ( ). Calculate axial tensile resistance (Nt\_Rd).
  - [ Eurocode3:05 6.2.3 ]
  - . Nt\_Rd = fy \* Area / Gamma\_M0 = 822.25 kN.
- ( ). Check ratio of axial resistance (N\_Ed/Nt\_Rd).
  - .  $\frac{N_{Ed}}{Nt_{Rd}} = \frac{268.89}{822.25} = 0.327 < 1.000 \text{ ---> O.K.}$

=====  
 [[[\*]]] CHECK SHEAR RESISTANCE.  
 =====

- ( ). Calculate shear area.
  - [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]
  - . Avy = 2\*B\*tf = 0.0013 m^2.
  - . Avz = 2\*h\*tw = 0.0013 m^2.

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( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 249.86 kN.

( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19-, POS = J )  
 -. Applied shear force : V\_Edy = 0.06 kN.  

$$\frac{V\_Edy}{Vpl\_Rdy} = \frac{0.06}{249.86} = 2.558e-004 < 1.000 \text{ ---> O.K.}$$

( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 249.86 kN.

( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 -. HTR < 72\*e/Eta ---> No need to check!

( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19-, POS = J )  
 -. Applied shear force : V\_Edz = 0.59 kN.  

$$\frac{V\_Edz}{Vpl\_Rdz} = \frac{0.59}{249.86} = 0.002 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wply = 4.6566e-005 m^3.  
 -. Mc\_Rdy = Wply \* fy / Gamma\_M0 = 15.74 kN-m.

( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  

$$\frac{M\_Edy}{Mc\_Rdy} = \frac{0.50}{15.74} = 0.032 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wplz = 8.0128e-005 m^3.  
 -. Mc\_Rdz = Wplz \* fy / Gamma\_M0 = 27.09 kN-m.



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( ). Check ratio of moment resistance (M\_Edz/Mc\_Rdz).  

$$\frac{M\_Edz}{Mc\_Rdz} = \frac{0.12}{27.09} = 0.004 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.  
 =====

( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of V\_Edz / Vpl\_Rdz < 0.5  
 -. My\_Rd = Mc\_Rdy = 15.74 kN-m.

( ). Calculate Minor reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of V\_Edy / Vpl\_Rdy < 0.5  
 -. Mz\_Rd = Mc\_Rdz = 27.09 kN-m.

( ). Check general interaction ratio.  
 [ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2  

$$R_{max1} = \frac{N\_Ed}{N\_Rd} + \frac{M\_Edy}{My\_Rd} + \frac{M\_Edz}{Mz\_Rd}$$

$$= 0.363 < 1.000 \text{ ---> O.K.}$$

( ). Check interaction ratio of bending and axial force member.  
 [ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2  
 -. n = N\_Ed / Npl\_Rd = 0.327  
 -. a = MIN[ (Area-2b\*tf)/Area, 0.5 ] = 0.500  
 -. Alpha = 2.000  
 -. Beta = MAX[ 5\*n, 1.0 ] = 1.635  
 -. Mny\_Rd = MIN[ Mply\_Rd\*(1-n)/(1-0.5\*a), Mply\_Rd ] = 14.13 kN-m.  
 -. Rmaxy = M\_Edy / Mny\_Rd = 0.036 < 1.000 ---> O.K.

-. In case of n < a  
 -. Mnz\_Rd = Mplz\_Rd = 27.09 kN-m.  
 -. Rmaxz = M\_Edz / Mnz\_Rd = 0.004 < 1.000 ---> O.K.

$$R_{max2} = \frac{\left[ \frac{M\_Edy}{Mny\_Rd} \right]^{Alpha} + \left[ \frac{M\_Edz}{Mnz\_Rd} \right]^{Beta}}{1}$$

$$= 0.001 < 1.000 \text{ ---> O.K.}$$

-. Rmax = MAX[ Rmax1, Rmax2 ] = 0.363 < 1.000 ---> O.K.

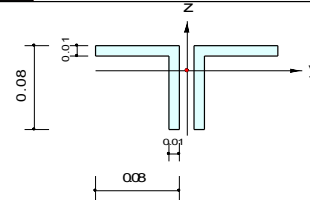
## 10.7 CONTROVENTI

### MIDAS/Civil Steel Checking Result

|                |              |                      |                        |
|----------------|--------------|----------------------|------------------------|
| <b>Company</b> |              | <b>Project Title</b> | CASALPUSTERLENGO       |
| <b>Author</b>  | Renato Vaira | <b>File Name</b>     | D:\...\Asse 80 V_1.mcb |

#### 1. Design Information

Design Code : Eurocode3-2:05  
 Unit System : kN, m  
 Member No : 3148  
 Material : S355W (No:1)  
 (Fy = 355000, Es = 210000000)  
 Section Name : CONTROV 2L80x10d15 (No:15)  
 (Built-up Section).  
 Member Length : 3.05287



#### 2. Member Forces

Axial Force Fxx = -255.42 (LCB: 19-, POS:J)  
 Bending Moments My = -0.2863, Mz = -2.5868  
 End Moments Myi = -0.0084, Myj = -0.2863 (for Lb)  
 Myi = -0.0084, Myj = -0.2863 (for Ly)  
 Mzi = -0.0574, Mzj = -2.5868 (for Lz)  
 Shear Forces Fyy = 1.57734 (LCB: 19+, POS:J)  
 Fzz = 0.19122 (LCB: 19+, POS:3/4)

|             |         |           |         |
|-------------|---------|-----------|---------|
| Depth       | 0.08000 | Web Thick | 0.01000 |
| Flg Width   | 0.08000 | Flg Thick | 0.01000 |
| BTB Spacing | 0.01500 |           |         |
| Area        | 0.00300 | Asz       | 0.00133 |
| Cyb         | 0.00159 | Czb       | 0.00320 |
| Iyy         | 0.00000 | Izz       | 0.00000 |
| Ybar        | 0.08750 | Zbar      | 0.05633 |
| Wely        | 0.00003 | Wetlz     | 0.00005 |
| ry          | 0.02436 | rz        | 0.03955 |

#### 3. Design Parameters

Unbraced Lengths Ly = 3.05287, Lz = 3.05287, Lb = 3.05287  
 Effective Length Factors Ky = 0.65, Kz = 1.30  
 Equivalent Uniform Moment Factors Cmy = 1.00, Cmz = 1.00, CmLT = 1.00

#### 4. Checking Results

##### Axial Resistance

$$N_{Ed}/\text{MIN}[N_{c,Rd}, N_{b,Rd}] = 255.421/407.225 = 0.627 < 1.000 \dots\dots\dots \text{O.K}$$

##### Bending Resistance

$$M_{Edy}/M_{Rdy} = 0.2863/19.2503 = 0.015 < 1.000 \dots\dots\dots \text{O.K}$$

$$M_{Edz}/M_{Rdz} = 2.5868/31.6119 = 0.082 < 1.000 \dots\dots\dots \text{O.K}$$

##### Combined Resistance

$$R_{NRd} = \text{MAX}[M_{Edy}/M_{ny,Rd}, M_{Edz}/M_{nz,Rd}]$$

$$R_{max1} = (M_{Edy}/M_{ny,Rd})^{\text{Alpha}} + (M_{Edz}/M_{nz,Rd})^{\text{Beta}}$$

$$R_{oom} = N_{Ed}/(A \cdot f_y / \text{Gamma}_{M0}), R_{bend} = M_{Edy}/M_{y,Rd} + M_{Edz}/M_{z,Rd}$$

$$R_{c\_LT1} = N_{Ed}/(X_{iy} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT1} = (k_{yy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (k_{yz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{c\_LT2} = N_{Ed}/(X_{iz} \cdot A \cdot f_y / \text{Gamma}_{M1})$$

$$R_{b\_LT2} = (K_{zy} \cdot M_{Edy}) / (X_{i\_LT} \cdot W_{ply} \cdot f_y / \text{Gamma}_{M1}) + (K_{zz} \cdot M_{sdz}) / (W_{plz} \cdot f_y / \text{Gamma}_{M1})$$

$$R_{max} = \text{MAX}[R_{NRd}, R_{max1}, (R_{oom} + R_{bend}), \text{MAX}[R_{c\_LT1} + R_{b\_LT1}, R_{c\_LT2} + R_{b\_LT2}]] = 0.765 < 1.000 \dots\dots \text{O.K}$$

##### Shear Resistance

$$V_{Edy}/V_{y,Rd} = 0.005 < 1.000 \dots\dots\dots \text{O.K}$$

$$V_{Edz}/V_{z,Rd} = 0.001 < 1.000 \dots\dots\dots \text{O.K}$$



1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV( 1.350)  
 22- 1 Dead Load( 1.350) +Erection Load 1( 1.500) +Creep Secondary( 1.200)  
 INV( 0.900) +Shrinkage Secondary( 1.200) +Vento Strutture INV( 0.900) +Vento Mobili  
 1.350) +Termico unif. INV( 1.200) +Termico diff. INV( 1.200) +Mobili SLE INV(

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    23+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
    23-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 1.000) + SLV Trasv INV( 0.300) + SLV Vert INV( 0.300)
    24+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
    24-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 1.000) + SLV Vert INV( 0.300)
    25+  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
    25-  1      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    + SLV Long INV( 0.300) + SLV Trasv INV( 0.300) + SLV Vert INV( 1.000)
    28+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
    28-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLU INV(
1.000)
    29+  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
    29-  2      Summation( 1.000) +Vento Strutture INV( 1.000) +Vento Mobili INV(
1.000)
    +Termico unif. INV( 0.600) +Termico diff. INV( 0.600) +Mobili SLE INV(
1.000)
    30+  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    30-  2      Summation( 1.000) +Frenamento INV( 1.000) +Termico unif. INV( 0.600)
+Termico diff. INV( 0.600) +Mobili SLE INV( 1.000)
    31+  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
    31-  2      Summation( 1.000) +Vento Strutture INV( 0.600) +Vento Mobili INV(
0.600)
    +Termico unif. INV( 1.000) +Termico diff. INV( 1.000) +Mobili SLE INV(
1.000)
    32+  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    +Mobili SLE INV( 1.000)
    32-  2      Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(
0.500)
    +Mobili SLE INV( 1.000)
    33+  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
    33-  2      Summation( 1.000) +Vento Strutture INV( 0.200) +Vento Mobili INV(
0.200)
    +Termico unif. INV( 0.500) +Termico diff. INV( 0.500)
    34+  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)
    34-  2      Summation( 1.000) +Termico unif. INV( 0.600) +Termico diff. INV(
0.600)

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35+ 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)  
35- 2            Summation( 1.000) +Termico unif. INV( 0.500) +Termico diff. INV(  
0.500)

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
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\*. PROJECT : CASALPUSTERLENGO  
 \*. MEMBER NO = 3148, ELEMENT TYPE = Beam  
 \*. LOADCOMB NO = 19-, MATERIAL NO = 1, SECTION NO = 15  
 \*. UNIT SYSTEM : kN, m

\*. SECTION PROPERTIES : Designation = CONTROV 2L80x10d15  
 Shape = 2L - Section. (Built-up)  
 Depth = 0.080, Flg Width = 0.080, BTB Spacing = 0.015  
 Web Thick = 0.010, Flg Thick = 0.010

Area = 3.00000e-003, Avy = 1.60000e-003, Avz = 1.60000e-003  
 Ybar = 8.75000e-002, Zbar = 5.63333e-002, Qyb = 1.58672e-003, Qzb =  
 3.20000e-003  
 Wely = 3.15917e-005, Welz = 5.36429e-005, Wply = 5.69375e-005, Wplz =  
 9.35000e-005  
 Iyy = 1.77967e-006, Izz = 4.69375e-006, Iyz = 0.00000e+000  
 iy = 2.43562e-002, iz = 3.95548e-002  
 J = 1.00000e-007, Cwp = 4.24931e-011

\*. DESIGN PARAMETERS FOR STRENGTH EVALUATION :  
 Ly = 3.05287e+000, Lz = 3.05287e+000, Lu = 3.05287e+000  
 Ky = 6.50000e-001, Kz = 1.30000e+000

\*. MATERIAL PROPERTIES :  
 Fy = 3.55000e+005, Es = 2.10000e+008, MATERIAL NAME = S355W

\*. FORCES AND MOMENTS AT (J) POINT :  
 Axial Force Fxx = -2.55421e+002  
 Shear Forces Fyy = 1.12343e+000, Fzz = -1.03872e-002  
 Bending Moments My = -2.86331e-001, Mz = -2.58681e+000  
 End Moments Myi = -8.38935e-003, Myj = -2.86331e-001 (for Lb)  
 Myi = -8.38935e-003, Myj = -2.86331e-001 (for Ly)  
 Mzi = -5.73645e-002, Mzj = -2.58681e+000 (for Lz)

\*. Sign conventions for stress and axial force.  
 - Stress : Compression positive.  
 - Axial force: Tension positive.

=====  
 [[[\*]]] CLASSIFY LEFT FLANGE OF SECTION (BTR).  
 =====

( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -. e = SQRT( 235/fy ) = 0.81  
 -. b/t = BTR = 7.00  
 -. sigma1 = 129555.495 KPa.  
 -. sigma2 = 85466.049 KPa.  
 -. BTR < 9\*e ( Class 1 : Plastic ).

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=====  
 [[[\*]]] CLASSIFY RIGHT FLANGE OF SECTION (BTR).  
 =====

- ( ). Determine classification of compression flanges(Double angle).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -. e = SQRT( 235/fy ) = 0.81  
 -. b/t = BTR = 7.00  
 -. sigma1 = 77199.278 KPa.  
 -. sigma2 = 33109.832 KPa.  
 -. BTR < 9\*e ( Class 1 : Plastic ).

=====  
 [[[\*]]] CLASSIFY WEB OF SECTION (HTR).  
 =====

- ( ). Determine classification of compression element(Double angles).  
 [ Eurocode3:05 Table 5.2 (Sheet 3 of 3), EN 1993-1-5 ]  
 -. e = SQRT( 235/fy ) = 0.81  
 -. d/t = HTR = 8.00  
 -. sigma1 = 103848.448 KPa.  
 -. sigma2 = 92586.132 KPa.  
 -. HTR < 10\*e ( Class 2 : Compact ).

=====  
 [[[\*]]] APPLIED FACTORS.  
 =====

- ( ). Calculate equivalent uniform moment factors (Cmy,Cmz,CmLT).  
 [ Eurocode3:05 Annex A. Table A.1, A.2 ]  
 -. Cmy,0 = 0.727  
 -. Cmz,0 = 0.586  
 -. Cmy (Default or User Defined Value) = 1.000  
 -. Cmz (Default or User Defined Value) = 1.000  
 -. CmLT (Default or User Defined Value) = 1.000
- ( ). Partial Factors (Gamma\_Mi).  
 [ Eurocode3:05 6.1 ]  
 -. Gamma\_M0 = 1.05  
 -. Gamma\_M1 = 1.10  
 -. Gamma\_M2 = 1.25

=====  
 [[[\*]]] CHECK AXIAL RESISTANCE.  
 =====

- ( ). Check slenderness ratio of axial compression member (Kl/i).  
 [ Eurocode3:05 6.3.1 ]  
 -. Kl/i = 100.3 < 200.0 ---> O.K.

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
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( ). Calculate axial compressive resistance (Nc\_Rd).  
 [ Eurocode3:05 6.1, 6.2.4 ]  
 -. Nc\_Rd = fy \* Area / Gamma\_M0 = 1014.29 kN.

( ). Check ratio of axial resistance (N\_Ed/Nc\_Rd).  

$$\frac{N_{Ed}}{Nc_{Rd}} = \frac{255.42}{1014.29} = 0.252 < 1.000 \text{ ---> O.K.}$$

( ). Calculate buckling resistance of compression member (Nb\_Rdy, Nb\_Rdz).  
 [ Eurocode3:05 6.3.1.1, 6.3.1.2 ]  
 -. Beta\_A = Aeff / Area = 1.000  
 -. Lambdal = Pi \* SQRT(Es/fy) = 76.409  
 -. Lambda\_by = {(KLy/iy)/Lambdal} \* SQRT(Beta\_A) = 1.066  
 -. Ncry = Pi^2\*Es\*Ryy / KLy^2 = 936.73 kN.  
 -. Lambda\_by > 0.2 and N\_Ed/Ncry > 0.04 --> Need to check.  
 -. Alphay = 0.340  
 -. Phiy = 0.5 \* [ 1 + Alphay\*(Lambda\_by-0.2) + Lambda\_by^2 ] = 1.216  
 -. Xiy = MIN [ 1 / [Phiy + SQRT(Phiy^2 - Lambda\_by^2)], 1.0 ] = 0.556  
 -. Nb\_Rdy = Xiy\*Beta\_A\*Area\*fy / Gamma\_M1 = 537.95 kN.

-. Lambdabz = {(KLz/iz)/Lambdal} \* SQRT(Beta\_A) = 1.313  
 -. Ncrz = Pi^2\*Es\*Rzz / KLz^2 = 617.64 kN.  
 -. Lambdabz > 0.2 and N\_Ed/Ncrz > 0.04 --> Need to check.  
 -. Alphaz = 0.340  
 -. Phiz = 0.5 \* [ 1 + Alphaz\*(Lambdabz-0.2) + Lambdabz^2 ] = 1.551  
 -. Xiz = MIN [ 1 / [Phiz + SQRT(Phiz^2 - Lambdabz^2)], 1.0 ] = 0.421  
 -. Nb\_Rdz = Xiz\*Beta\_A\*Area\*fy / Gamma\_M1 = 407.23 kN.

( ). Check ratio of buckling resistance (N\_Ed/Nb\_Rd).  
 -. Nb\_Rd = MIN[ Nb\_Rdy, Nb\_Rdz ] = 407.23 kN.  

$$\frac{N_{Ed}}{Nb_{Rd}} = \frac{255.42}{407.23} = 0.627 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK SHEAR RESISTANCE.  
 =====

( ). Calculate shear area.  
 [ Eurocode3:05 6.2.6, EN1993-1-5:04 5.1 NOTE 2 ]  
 -. Avy = 2\*B\*tf = 0.0016 m^2.  
 -. Avz = 2\*h\*tw = 0.0016 m^2.

( ). Calculate plastic shear resistance in local-y direction (Vpl\_Rdy).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdy = [ Avy\*fy/SQRT(3) ] / Gamma\_M0 = 312.32 kN.

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 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
 =====  
 =

- ( ). Check ratio of shear resistance (V\_Edy/Vpl\_Rdy).  
 ( LCB = 19+, POS = J )  
 -. Applied shear force : V\_Edy = 1.58 kN.  

$$\frac{V\_Edy}{Vpl\_Rdy} = \frac{1.58}{312.32} = 0.005 < 1.000 \text{ ---> O.K.}$$
- ( ). Calculate plastic shear resistance in local-z direction (Vpl\_Rdz).  
 [ Eurocode3:05 6.1, 6.2.6 ]  
 -. Vpl\_Rdz = [ Avz\*fy/SQRT(3) ] / Gamma\_M0 = 312.32 kN.
- ( ). Shear Buckling Check.  
 [ Eurocode3:05 6.2.6 ]  
 -. HTR < 72\*e/Eta ---> No need to check!
- ( ). Check ratio of shear resistance (V\_Edz/Vpl\_Rdz).  
 ( LCB = 19+, POS = J )  
 -. Applied shear force : V\_Edz = 0.19 kN.  

$$\frac{V\_Edz}{Vpl\_Rdz} = \frac{0.19}{312.32} = 6.123e-004 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MAJOR AXIS.  
 =====

- ( ). Calculate plastic resistance moment about major axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wply = 5.6938e-005 m^3.  
 -. Mc\_Rdy = Wply \* fy / Gamma\_M0 = 19.25 kN-m.
- ( ). Check ratio of moment resistance (M\_Edy/Mc\_Rdy).  

$$\frac{M\_Edy}{Mc\_Rdy} = \frac{0.29}{19.25} = 0.015 < 1.000 \text{ ---> O.K.}$$

=====  
 [[[\*]]] CHECK BENDING MOMENT RESISTANCE ABOUT MINOR AXIS.  
 =====

- ( ). Calculate plastic resistance moment about minor axis.  
 [ Eurocode3:05 6.1, 6.2.5 ]  
 -. Wplz = 9.3500e-005 m^3.  
 -. Mc\_Rdz = Wplz \* fy / Gamma\_M0 = 31.61 kN-m.
- ( ). Check ratio of moment resistance (M\_Edz/Mc\_Rdz).  

$$\frac{M\_Edz}{Mc\_Rdz} = \frac{2.59}{31.61} = 0.082 < 1.000 \text{ ---> O.K.}$$

-----  
 -  
 MIDAS/Civil - Steel Code Checking[ Eurocode3-2:05 ] Version 8.3.5  
 =====  
 =

=====  
 [[[\*]]] CHECK INTERACTION OF COMBINED RESISTANCE.  
 =====

- ( ). Calculate Major reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of  $V_{Edz} / V_{pl,Rdz} < 0.5$   
 -.  $M_{y,Rd} = M_{c,Rdy} = 19.25$  kN-m.
- ( ). Calculate Minor reduced design resistance of bending and shear.  
 [ Eurocode3:05 6.2.8 (6.30) ]  
 -. In case of  $V_{Edy} / V_{pl,Rdy} < 0.5$   
 -.  $M_{z,Rd} = M_{c,Rdz} = 31.61$  kN-m.
- ( ). Check general interaction ratio.  
 [ Eurocode3:05 6.2.1 (6.2) ] - Class1 or Class2  

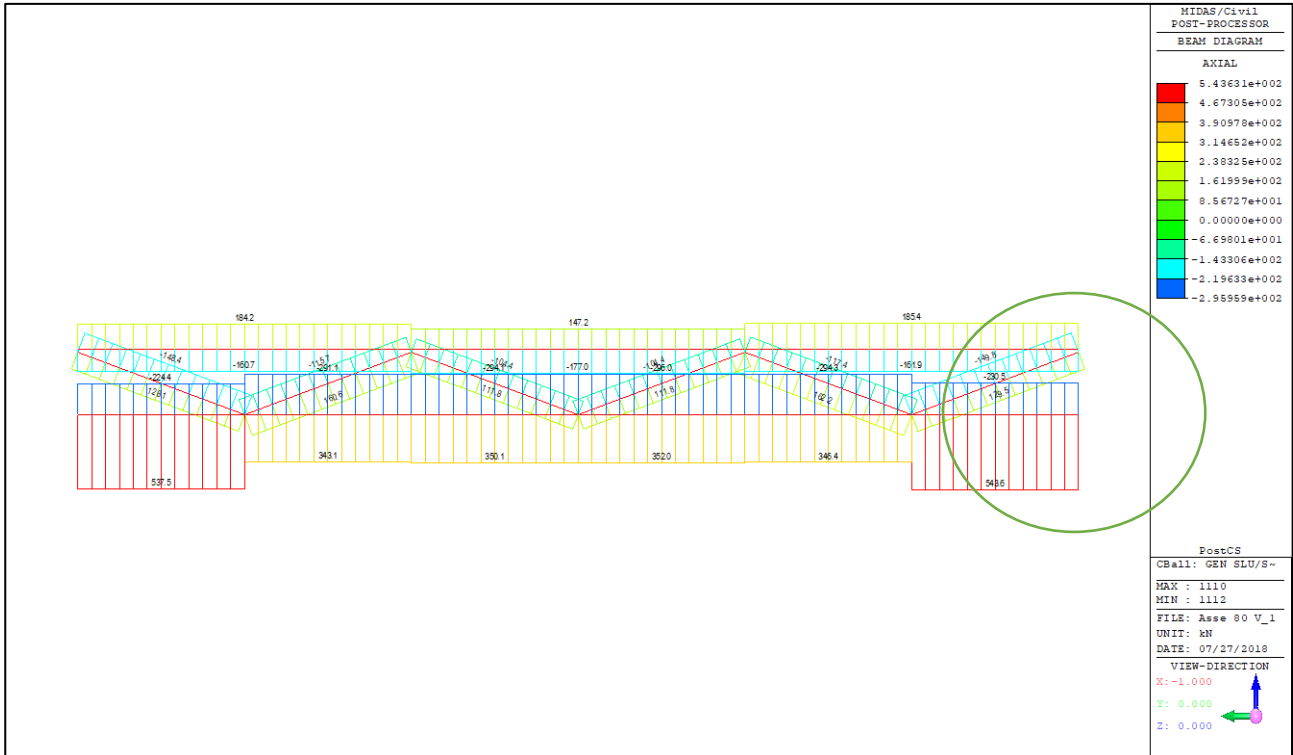
$$R_{max1} = \frac{N_{Ed}}{N_{Rd}} + \frac{M_{Edy}}{M_{y,Rd}} + \frac{M_{Edz}}{M_{z,Rd}}$$

$$= 0.349 < 1.000 \text{ ---> O.K.}$$
- ( ). Check interaction ratio of bending and axial force member.  
 [ Eurocode3:05 6.2.9 (6.31 ~ 6.41) ] - Class1 or Class2  
 -.  $n = N_{Ed} / N_{pl,Rd} = 0.252$   
 -.  $a = \text{MIN}[(Area-2b*tf)/Area, 0.5] = 0.500$   
 -.  $\alpha = 2.000$   
 -.  $\beta = \text{MAX}[5*n, 1.0] = 1.259$   
 -.  $M_{ny,Rd} = \text{MIN}[M_{ply,Rd}*(1-n)/(1-0.5*a), M_{ply,Rd}] = 19.20$  kN-m.  
 -.  $R_{maxy} = M_{Edy} / M_{ny,Rd} = 0.015 < 1.000 \text{ ---> O.K.}$
- . In case of  $n < a$   
 -.  $M_{nz,Rd} = M_{plz,Rd} = 31.61$  kN-m.  
 -.  $R_{maxz} = M_{Edz} / M_{nz,Rd} = 0.082 < 1.000 \text{ ---> O.K.}$
- .  $R_{max2} = \left[ \left| \frac{M_{Edy}}{M_{ny,Rd}} \right|^{\alpha} + \left| \frac{M_{Edz}}{M_{nz,Rd}} \right|^{\beta} \right]$   

$$= 0.043 < 1.000 \text{ ---> O.K.}$$







**COMBINAZIONE INVILUPPO SLU/SLV - MASSIMI SFORZI ASSIALI DIAFRAMMA INTERMEDIO (SPALLA SX)**

Componendo le azioni e trascurando la componente verticale dei diagonali (di compressione), rispetto alla base della piastra si ha:

$$F_x = 185.4 + 129.5 \times \cos 22.1 + 543.6 = 849.0 \text{ kN}$$

$$M_z = -185.4 \times 0.86 - 120.0 \times 0.81 - 543.6 \times 0.16 = -343.62 \text{ kNm}$$

I cordoni di saldatura hanno lato  $2 \times 8 = 16 \text{ mm}$  (altezza totale di gola  $11.3 \text{ mm}$ ) e lunghezza, dedotte le lunette di scarico:

- alle ali della trave:  $240 - 30 = 210 \text{ mm}$
- all' anima della trave:  $930 - 2 \times 30 = 870 \text{ mm}$

La tensione ideale di resistenza vale:  $\sigma_{w,Rd} = f_{tk} / (\beta_w \gamma_{M2}) = 510 / (0.9 \times 1.25) = 453.33 \text{ N/mm}^2$

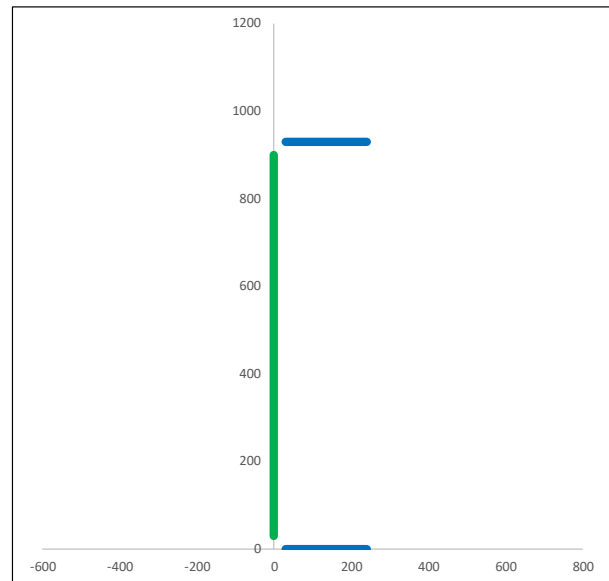
Verifica: la tensione ideale risulta nei 3 cordoni:

- inferiore:  $\sigma_{w,Ed} = 138.71 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- superiore:  $\sigma_{w,Ed} = 62.81 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- anima:  $\sigma_{w,Ed} = 100.82 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )

Segue tabulato foglio di calcolo.

**VERIFICA DI RESISTENZA SALDATURE A CORDONE D'ANGOLO**  
NTC 2008, § 4.2.8.2.4, FORMULA (4.2.75)

|                                                         |                                                                                          |           |                   |
|---------------------------------------------------------|------------------------------------------------------------------------------------------|-----------|-------------------|
| Coefficiente di sicurezza saldature                     | $\gamma_{M2}$                                                                            | 1.25      |                   |
| Spessore nominale massimo:                              | t                                                                                        | 15        | mm                |
| Classe Acciaio:                                         |                                                                                          | S355 W    |                   |
| Tensione di snervamento:                                | $f_{yk}$                                                                                 | 355       | N/mm <sup>2</sup> |
| Tensione di rottura:                                    | $f_{tk}$                                                                                 | 510       | N/mm <sup>2</sup> |
| Coefficiente:                                           | $\beta_w$                                                                                | 0.9       |                   |
| Resistenza (tensione ideale):                           | $\sigma_{w,Rd} = f_{tk} / (\beta \cdot \gamma_{M2})$                                     | 453.33    | N/mm <sup>2</sup> |
| Sollecitazione (tensione ideale):                       | $\sigma_{w,Ed} = [ \sigma_{\perp}^2 + 3 ( \tau_{\perp}^2 + \tau_{\parallel}^2 ) ]^{0.5}$ |           |                   |
| Punto di applicazione sollecitazioni (vuoto=baricentro) | X=                                                                                       | 0         | mm                |
|                                                         | Y=                                                                                       | 0         | mm                |
| Sollecitazioni: (terna destrorsa)                       | FX =                                                                                     | 849000    | N                 |
|                                                         | FY =                                                                                     | 0         | N                 |
|                                                         | FZ =                                                                                     | 0         | N                 |
|                                                         | MX =                                                                                     | 0         | N mm              |
|                                                         | MY =                                                                                     | 0         | N mm              |
|                                                         | MZ =                                                                                     | -34362000 | N mm              |



Cordoni paralleli asse X

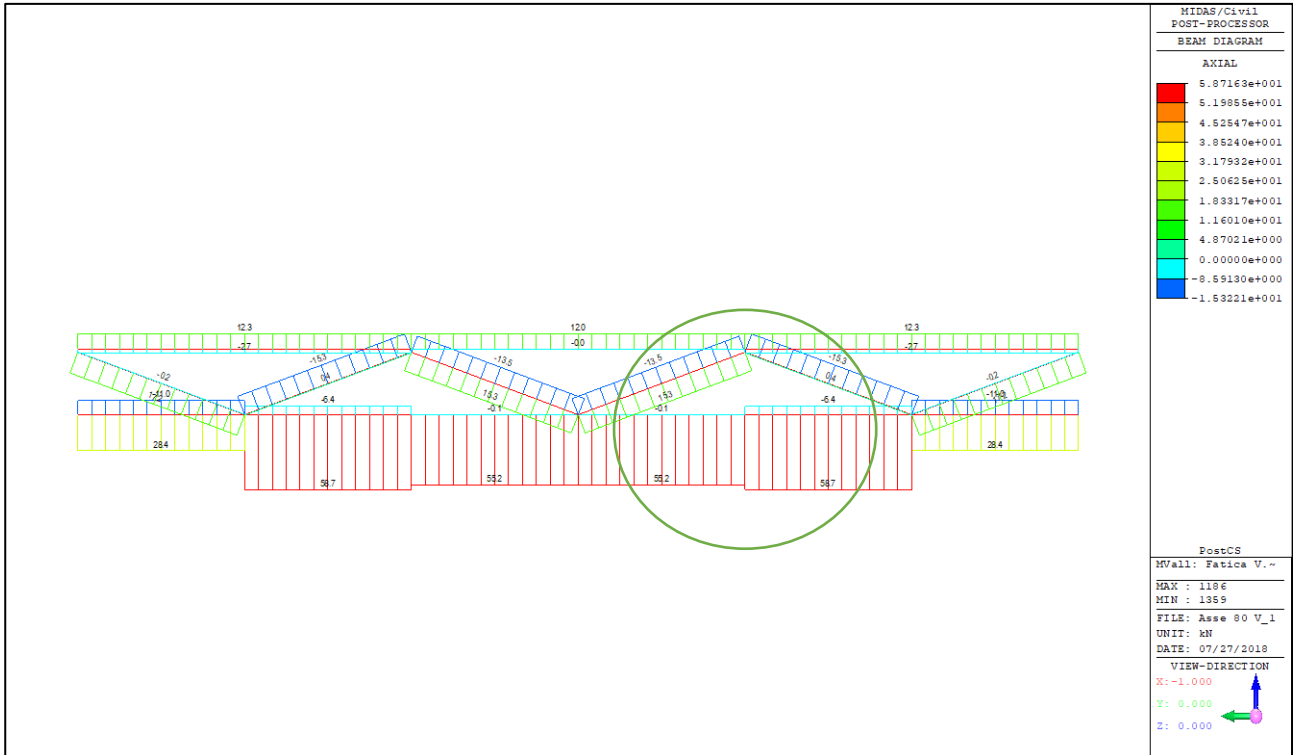
| N° | Y   | Xini | Xfin | bx | ax    | $\sigma_{w,Ed}$ | Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$ |
|----|-----|------|------|----|-------|-----------------|--------------------------------------------------|
| 1  | 0   | 30   | 240  | 16 | 11.31 | 138.707         | 0.306                                            |
| 2  | 930 | 30   | 240  | 16 | 11.31 | 62.806          | 0.139                                            |
| 3  |     |      |      |    |       | 0.00            |                                                  |
| 4  |     |      |      |    |       | 0.00            |                                                  |
| 5  |     |      |      |    |       | 0.00            |                                                  |
| 6  |     |      |      |    |       | 0.00            |                                                  |
| 7  |     |      |      |    |       | 0.00            |                                                  |
| 8  |     |      |      |    |       | 0.00            |                                                  |
| 9  |     |      |      |    |       | 0.00            |                                                  |
| 10 |     |      |      |    |       | 0.00            |                                                  |

Cordoni paralleli asse Y

| N° | X | Yini | Yfin | by | ay    | $\sigma_{w,Ed}$ | Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$ |
|----|---|------|------|----|-------|-----------------|--------------------------------------------------|
| 1  | 0 | 30   | 900  | 16 | 11.31 | 100.820         | 0.222                                            |
| 2  |   |      |      |    |       | 0.00            |                                                  |
| 3  |   |      |      |    |       | 0.00            |                                                  |
| 4  |   |      |      |    |       | 0.00            |                                                  |
| 5  |   |      |      |    |       | 0.00            |                                                  |
| 6  |   |      |      |    |       | 0.00            |                                                  |
| 7  |   |      |      |    |       | 0.00            |                                                  |
| 8  |   |      |      |    |       | 0.00            |                                                  |
| 9  |   |      |      |    |       | 0.00            |                                                  |
| 10 |   |      |      |    |       | 0.00            |                                                  |

**11.3 TRAVI PRINCIPALI - SALDATURE PIASTRE DIAFRAMMI - VERIFICA A FATICA**

Le sollecitazioni massime a cui sono sottoposte le saldature sono ricercate nella condizione di carico mobile "Fatica V.ILL.", componendo le azioni assiali di trazione dei traversi inferiori, superiori e diagonali.



CONDIZIONE DI CARICO FATICA V.I.LL. - MASSIMI SFORZI ASSIALI DIAFRAMMA INTERMEDIO

Componendo le azioni e trascurando la componente verticale dei diagonali (di compressione), rispetto alla base della piastra si ha:

$$F_x = 12.3 + 0.4 \times \cos 22.1 + 58.7 = 71.37 \text{ kN}$$

$$M_z = -12.3 \times 0.86 - 0.37 \times 0.81 - 58.7 \times 0.16 = -20.270 \text{ kNm}$$

I cordoni di saldatura hanno lato  $2 \times 8 = 16 \text{ mm}$  (altezza di gola  $11.3 \text{ mm}$ ) e lunghezza, dedotte le lunette di scarico:

- alle ali della trave:  $240 - 30 = 210 \text{ mm}$
- all' anima della trave:  $930 - 2 \times 30 = 870 \text{ mm}$

Caratteristiche di resistenza:

- classe del dettaglio:  $\Delta\sigma_c = 80 \text{ N/mm}^2$  (Tabella C.4.2.XVI, part. 7)
- limite di fatica ad ampiezza costante:  $\Delta\sigma_D = 0.737 \times 80 = 58.96 \text{ N/mm}^2$
- coefficiente di sicurezza a fatica:  $\gamma_{Mf} = 1.35$

La tensione ideale di resistenza per lo stato limite di fatica vale:  $\sigma_{w,Rd} = \Delta\sigma_D / \gamma_{Mf} = 43.67 \text{ N/mm}^2$

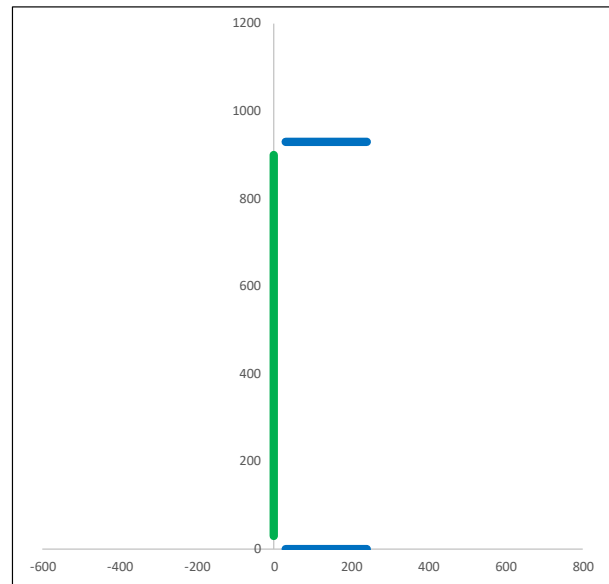
Verifica: la tensione ideale risulta nei 3 cordoni:

- inferiore:  $\sigma_{w,Ed} = 18.05 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- superiore:  $\sigma_{w,Ed} = 1.11 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )
- anima:  $\sigma_{w,Ed} = 8.52 \text{ N/mm}^2$  ( $\sigma_{w,Rd} < \text{verificato}$ )

Segue tabulato foglio di calcolo.

**VERIFICA DI RESISTENZA SALDATURE A CORDONE D'ANGOLO**  
NTC 2008, § 4.2.8.2.4, FORMULA (4.2.75)

|                                                         |                                                                                          |          |                   |
|---------------------------------------------------------|------------------------------------------------------------------------------------------|----------|-------------------|
| Coefficiente di sicurezza saldature                     | $\gamma_{M2}$                                                                            | 1.25     |                   |
| Spessore nominale massimo:                              | t                                                                                        | 15       | mm                |
| Classe Acciaio:                                         |                                                                                          | S355 W   |                   |
| Tensione di snervamento:                                | $f_{yk}$                                                                                 | 355      | N/mm <sup>2</sup> |
| Tensione di rottura:                                    | $f_{tk}$                                                                                 | 510      | N/mm <sup>2</sup> |
| Coefficiente:                                           | $\beta_w$                                                                                | 0.9      |                   |
| Resistenza (tensione ideale):                           | $\sigma_{w,Rd} = f_{tk}/(\beta \cdot \gamma_{M2})$                                       | 453.33   | N/mm <sup>2</sup> |
| Sollecitazione (tensione ideale):                       | $\sigma_{w,Ed} = [ \sigma_{\perp}^2 + 3 ( \tau_{\perp}^2 + \tau_{\parallel}^2 ) ]^{0.5}$ |          |                   |
| Punto di applicazione sollecitazioni (vuoto=baricentro) | X=                                                                                       | 0        | mm                |
|                                                         | Y=                                                                                       | 0        | mm                |
| Sollecitazioni: (terna destrorsa)                       | FX =                                                                                     | 71370    | N                 |
|                                                         | FY =                                                                                     | 0        | N                 |
|                                                         | FZ =                                                                                     | 0        | N                 |
|                                                         | MX =                                                                                     | 0        | N mm              |
|                                                         | MY =                                                                                     | 0        | N mm              |
|                                                         | MZ =                                                                                     | -2027000 | N mm              |



Cordoni paralleli asse X

| N° | Y   | Xini | Xfin | bx | ax    | $\sigma_{w,Ed}$ | Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$ |
|----|-----|------|------|----|-------|-----------------|--------------------------------------------------|
| 1  | 0   | 30   | 240  | 16 | 11.31 | 18.051          | 0.040                                            |
| 2  | 930 | 30   | 240  | 16 | 11.31 | 1.111           | 0.002                                            |
| 3  |     |      |      |    | 0.00  |                 |                                                  |
| 4  |     |      |      |    | 0.00  |                 |                                                  |
| 5  |     |      |      |    | 0.00  |                 |                                                  |
| 6  |     |      |      |    | 0.00  |                 |                                                  |
| 7  |     |      |      |    | 0.00  |                 |                                                  |
| 8  |     |      |      |    | 0.00  |                 |                                                  |
| 9  |     |      |      |    | 0.00  |                 |                                                  |
| 10 |     |      |      |    | 0.00  |                 |                                                  |

Cordoni paralleli asse Y

| N° | X | Yini | Yfin | by | ay    | $\sigma_{w,Ed}$ | Verifica: $\sigma_{w,Ed} / \sigma_{w,Rd} \leq 1$ |
|----|---|------|------|----|-------|-----------------|--------------------------------------------------|
| 1  | 0 | 30   | 900  | 16 | 11.31 | 8.518           | 0.019                                            |
| 2  |   |      |      |    | 0.00  |                 |                                                  |
| 3  |   |      |      |    | 0.00  |                 |                                                  |
| 4  |   |      |      |    | 0.00  |                 |                                                  |
| 5  |   |      |      |    | 0.00  |                 |                                                  |
| 6  |   |      |      |    | 0.00  |                 |                                                  |
| 7  |   |      |      |    | 0.00  |                 |                                                  |
| 8  |   |      |      |    | 0.00  |                 |                                                  |
| 9  |   |      |      |    | 0.00  |                 |                                                  |
| 10 |   |      |      |    | 0.00  |                 |                                                  |

**12 VERIFICA DEI COLLEGAMENTI BULLONATI**

Si verificano a taglio e rifollamento le unioni bullonate dei trasversi e dei controventi. Si utilizzano bulloni di classe 10.9; si suppone che il piano di taglio interessi la parte filettata del bullone e si considera quindi l'area resistente efficace. Seguono i dati generali, comuni a tutti i collegamenti.

Qualità acciaio:

| Tipo      | Classe | Spessore $t \leq 40$ mm       |                               | Spessore $40 \text{ mm} < t \leq 80$ mm |                               |
|-----------|--------|-------------------------------|-------------------------------|-----------------------------------------|-------------------------------|
|           |        | $f_{yk}$ [N/mm <sup>2</sup> ] | $f_{tk}$ [N/mm <sup>2</sup> ] | $f_{yk}$ [N/mm <sup>2</sup> ]           | $f_{tk}$ [N/mm <sup>2</sup> ] |
| Profilati | S355 W | 355                           | 510                           | 335                                     | 490                           |
| Lamiere   | S355 W | 355                           | 510                           | 335                                     | 490                           |

Coefficienti parziali di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):

Resistenza dei bulloni e delle sezioni tese, indebolite dai fori:  $\gamma_{M2} = 1.25$   
 Resistenza allo scorrimento allo SLU:  $\gamma_{M3} = 1.25$   
 Resistenza allo scorrimento allo SLE:  $\gamma_{M3} = 1.10$   
 Precarico bulloni ad alta resistenza (classe 8.8 o 10.9):  $\gamma_{M7} = 1.10$

Dati meccanici e geometrici dei bulloni classe 10.9.

Resistenza di snervamento:  $f_{yb} = 900 \text{ N/mm}^2$

Resistenza di rottura:  $f_{tb} = 1000 \text{ N/mm}^2$

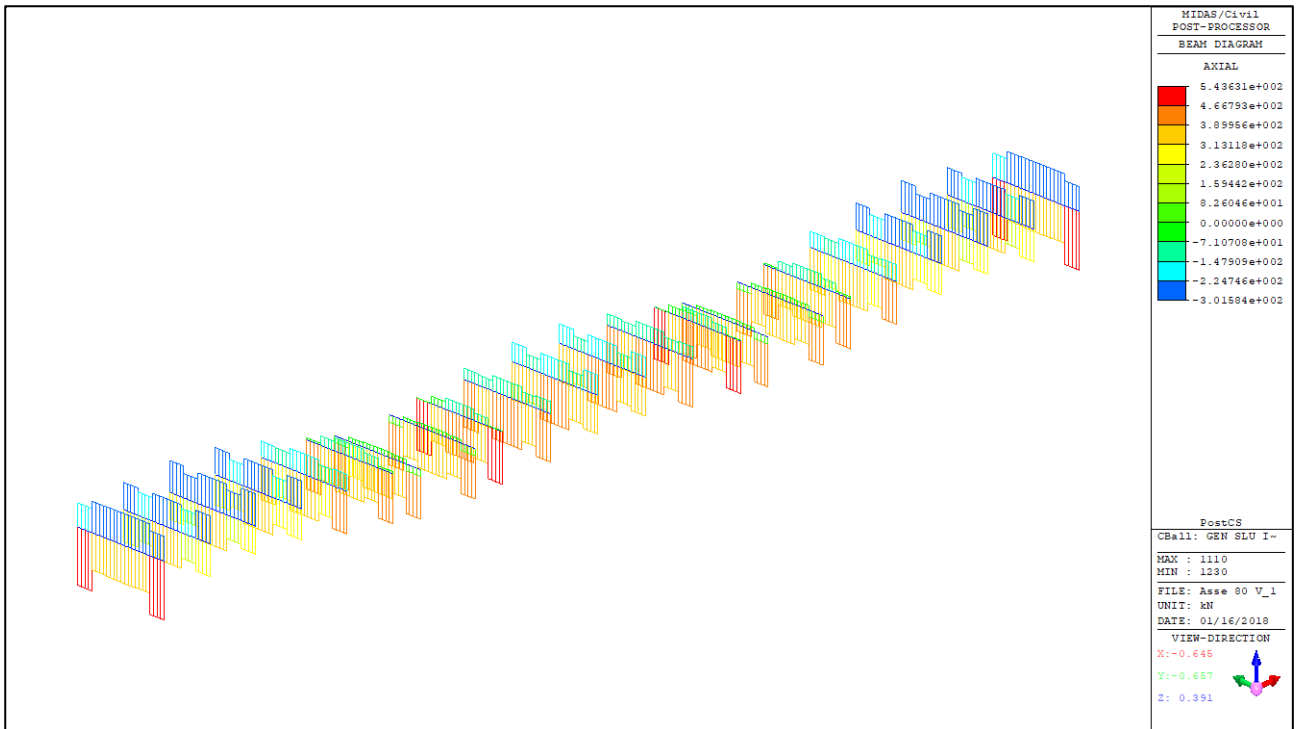
Coefficiente di attrito:  $\mu = 0.30$

Tipo di serraggio: Controllato

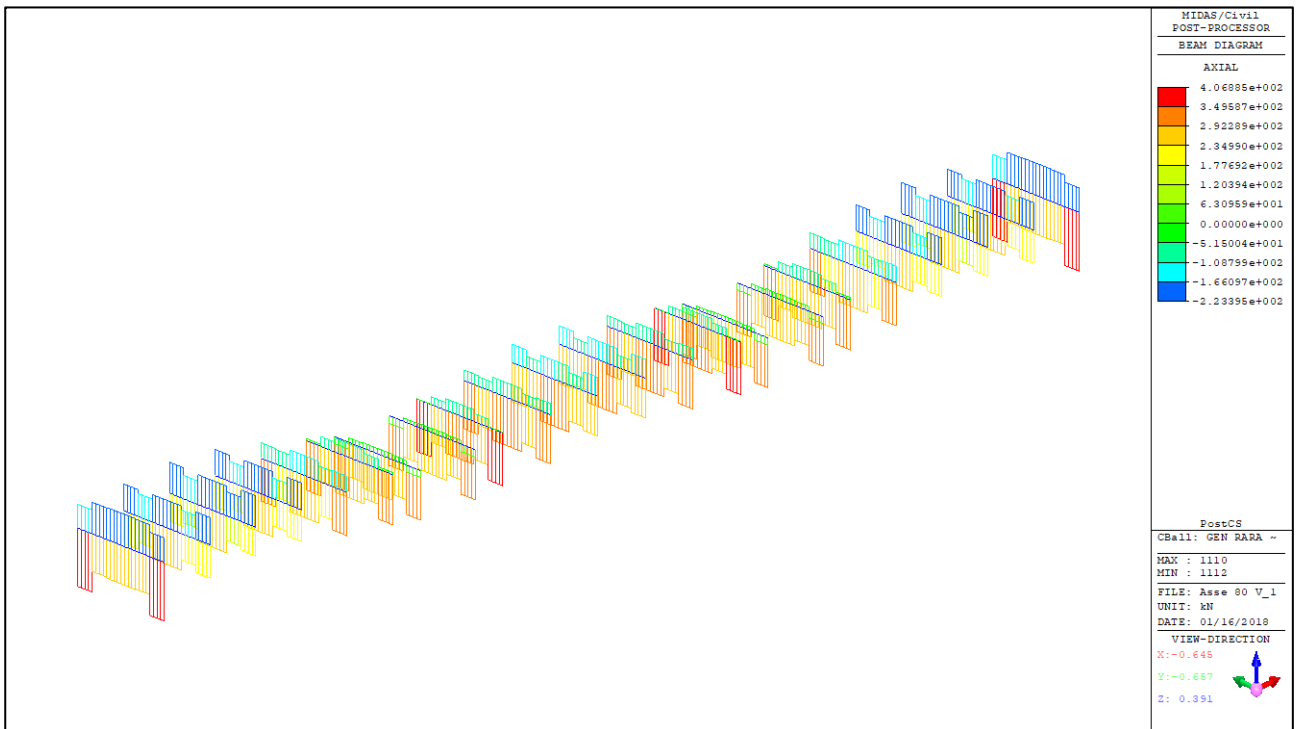
Tutti i nodi bullonati sono verificati a rifollamento e ad attrito allo SLE; si ammette (ipotesi di progetto) che possano non essere verificati ad attrito allo SLU; corrispondono quindi alla Categoria B o C (EN 1993-1-8; § 3.4).

| Diametro nominale                 | d [mm]                       | M16   | M20   | M24   | M27   |
|-----------------------------------|------------------------------|-------|-------|-------|-------|
| Diametro fori (normali)           | $d_0$ [mm]                   | 17    | 21    | 25.5  | 28.5  |
| Area nominale                     | A [mm <sup>2</sup> ]         | 201   | 314   | 452   | 573   |
| Area resistente (gambo filettato) | $A_{res}$ [mm <sup>2</sup> ] | 157   | 245   | 353   | 459   |
| Resistenza a taglio SLU           | $F_{v,Rd}$ [kN]              | 62.7  | 97.9  | 141   | 183.8 |
| Forza di precarico                | $F_{p,Cd}$ [kN]              | 109.7 | 171.3 | 246.7 | 321.6 |
| Resistenza a scorrimento SLU      | $F_{s,Rd}$ [kN]              | 26.3  | 41.1  | 59.2  | 77.2  |
| Resistenza a scorrimento SLE      | $F_{s,Rd,es}$ [kN]           | 29.9  | 46.7  | 67.3  | 87.7  |

## 12.1 DIAFRAMMI SU SPALLE E INTEREDI - TRASVERSI INFERIORI



TRASVERSI INFERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLU/SLV



TRASVERSI INFERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLE

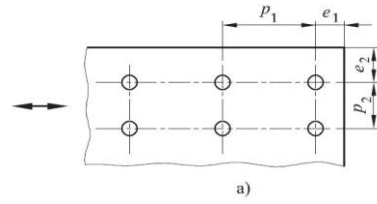


**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI IN CAMPATA - CORRENTI INFERIORI (2 L 80x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):

|                                                                   |                 |      |
|-------------------------------------------------------------------|-----------------|------|
| Resistenza dei bulloni e delle sezioni tese (indebolite dai fori) | $\gamma_{M2} =$ | 1.25 |
| Resistenza a scorrimento allo SLU                                 | $\gamma_{M3} =$ | 1.25 |
| Resistenza a scorrimento allo SLE                                 | $\gamma_{M3} =$ | 1.10 |
| Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)      | $\gamma_{M7} =$ | 1.10 |

|                          |                                    |                    |                           |
|--------------------------|------------------------------------|--------------------|---------------------------|
| Dati geometrici bulloni: | Diametro nominale:                 | d =                | 27 mm                     |
|                          | Tipo fori: accoppiamento           | d <sub>0</sub> =   | 28.5 mm                   |
|                          | Area nominale gambo non filettato: | A =                | 573 mm <sup>2</sup>       |
|                          | Area resistente gambo filettato:   | A <sub>res</sub> = | 459 mm <sup>2</sup>       |
|                          | Classe bulloni:                    | f <sub>yb</sub> =  | 900.00 N/mm <sup>2</sup>  |
|                          |                                    | f <sub>tb</sub> =  | 1000.00 N/mm <sup>2</sup> |



|                           |                                                                        |                           |
|---------------------------|------------------------------------------------------------------------|---------------------------|
| Resistenza a taglio:      | il piano di taglio interessa la parte:                                 | filettata                 |
|                           | resistenza allo SLU (per piano di taglio): F <sub>v,Rd</sub> =         | 183.76 kN                 |
| Resistenza a scorrimento: | coefficiente di attrito: μ =                                           | 0.30 (tutte le superfici) |
| (solo classi 8.8 o 10.9)  | tipo di serraggio:                                                     | controllato               |
|                           | forza di precarico: F <sub>p,Cd</sub> =                                | 321.58 kN                 |
|                           | resistenza allo SLU (per piano di scorrimento): F <sub>s,Rd</sub> =    | 77.18 kN                  |
|                           | resistenza allo SLE (per piano di scorrimento): F <sub>s,Rd,es</sub> = | 87.70 kN                  |

|                         |                                 |                      |                      |
|-------------------------|---------------------------------|----------------------|----------------------|
| Dati profili e lamiere: | Spessore nominale dell'elemento |                      |                      |
|                         | t ≤ 40 mm                       | 40 mm < t ≤ 80 mm    |                      |
|                         | f <sub>yk</sub>                 | f <sub>tk</sub>      | f <sub>yk</sub>      |
|                         | [N/mm <sup>2</sup> ]            | [N/mm <sup>2</sup> ] | [N/mm <sup>2</sup> ] |

|                                        |         |     |     |     |     |                                                                                                       |
|----------------------------------------|---------|-----|-----|-----|-----|-------------------------------------------------------------------------------------------------------|
| Qualità acciaio profilati:             | S 355 W | 355 | 510 | 335 | 490 | UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica |
| Qualità acciaio lamiere / coprigiunti: | S 355 W | 355 | 510 | 335 | 490 | UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica |

|                          |                                              |                       |  |  |  |                          |
|--------------------------|----------------------------------------------|-----------------------|--|--|--|--------------------------|
| Dati geometrici bulloni: | Numero file di bulloni:                      | 1                     |  |  |  |                          |
|                          | Numero bulloni:                              | 3                     |  |  |  | Limiti massimi e minimi  |
|                          | Numero superfici di taglio:                  | 2                     |  |  |  | Min. Max.                |
|                          | Interassi: p <sub>1</sub> =                  | 70 mm                 |  |  |  | 62.7 175 mm              |
|                          | (per fila singola porre: p <sub>2</sub> = 0) | p <sub>2</sub> = 0 mm |  |  |  | 0 0 mm                   |
|                          |                                              |                       |  |  |  | Entro limiti: Verificato |

|                            |                                              |                      |  |  |  |                          |
|----------------------------|----------------------------------------------|----------------------|--|--|--|--------------------------|
| Dati geometrici profilati: | Numero profilati:                            | 2                    |  |  |  |                          |
|                            | Spessore flangia bullonata:                  | 15 mm                |  |  |  | Limiti massimi e minimi  |
|                            | Area sezione singolo profilato:              | 1510 mm <sup>2</sup> |  |  |  | Min. Max.                |
|                            | Distanze bulloni dal bordo: e <sub>1</sub> = | 50 mm                |  |  |  | 34.2 125 mm              |
|                            | e <sub>2</sub> =                             | 40 mm                |  |  |  | 34.2 125 mm              |
|                            |                                              |                      |  |  |  | Entro limiti: Verificato |

|                                        |                                              |       |  |  |  |                          |
|----------------------------------------|----------------------------------------------|-------|--|--|--|--------------------------|
| Dati geometrici lamiere / coprigiunti: | Numero lamiere:                              | 1     |  |  |  |                          |
|                                        | Spessore:                                    | 15 mm |  |  |  | Limiti massimi e minimi  |
|                                        | Distanze bulloni dal bordo: e <sub>1</sub> = | 50 mm |  |  |  | Min. Max.                |
|                                        | e <sub>2</sub> =                             | 40 mm |  |  |  | 34.2 125 mm              |
|                                        |                                              |       |  |  |  | 34.2 125 mm              |
|                                        |                                              |       |  |  |  | Entro limiti: Verificato |

|                                            |        |                      |           |
|--------------------------------------------|--------|----------------------|-----------|
| Sollecitazioni di progetto (trazione > 0): | S.L.U. | N <sub>Ed</sub> =    | 543.60 kN |
|                                            | S.L.E. | N <sub>Ed,es</sub> = | 406.90 kN |

|                                                                     |                     |          |           |                                                                                |
|---------------------------------------------------------------------|---------------------|----------|-----------|--------------------------------------------------------------------------------|
| S.L.U. Verifica di resistenza a taglio bullone, singola superficie: | F <sub>v,Ed</sub> = | 90.60 kN | Verifica: | $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{90.60}{183.76} = 0.493 \leq 1$ : Verificato |
|---------------------------------------------------------------------|---------------------|----------|-----------|--------------------------------------------------------------------------------|

|                                                                          |                     |          |           |                                                                                |
|--------------------------------------------------------------------------|---------------------|----------|-----------|--------------------------------------------------------------------------------|
| S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie: | F <sub>v,Ed</sub> = | 90.60 kN | Verifica: | $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{90.60}{77.18} = 1.174 > 1$ : NON Verificato |
|--------------------------------------------------------------------------|---------------------|----------|-----------|--------------------------------------------------------------------------------|

|                                                                          |                        |          |           |                                                                                     |
|--------------------------------------------------------------------------|------------------------|----------|-----------|-------------------------------------------------------------------------------------|
| S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie: | F <sub>v,Ed,es</sub> = | 67.82 kN | Verifica: | $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{67.82}{87.70} = 0.773 \leq 1$ : Verificato |
|--------------------------------------------------------------------------|------------------------|----------|-----------|-------------------------------------------------------------------------------------|

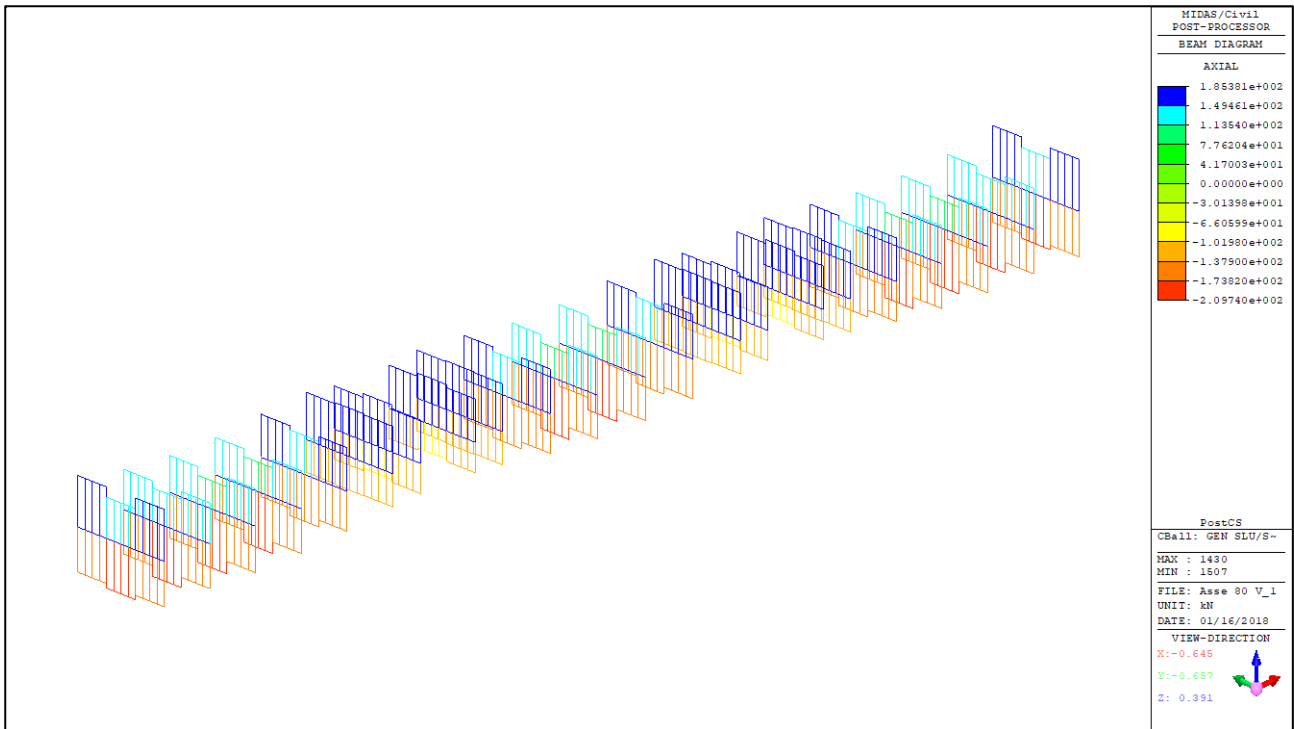
|                                            |                     |                        |           |                                                                               |
|--------------------------------------------|---------------------|------------------------|-----------|-------------------------------------------------------------------------------|
| S.L.U. Verifica al rifollamento profilato: | F <sub>b,Ed</sub> = | 90.60 kN               |           |                                                                               |
|                                            | coefficienti:       | k1 = 2.230             |           |                                                                               |
|                                            |                     | α <sub>b</sub> = 0.569 |           |                                                                               |
| Resistenza al rifollamento:                | F <sub>b,Rd</sub> = | 209.5 kN               | Verifica: | $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{90.60}{209.5} = 0.432 \leq 1$ : Verificato |

|                                          |                     |                        |           |                                                                                |
|------------------------------------------|---------------------|------------------------|-----------|--------------------------------------------------------------------------------|
| S.L.U. Verifica al rifollamento lamiere: | F <sub>b,Ed</sub> = | 181.20 kN              |           |                                                                                |
|                                          | coefficienti:       | k1 = 2.230             |           |                                                                                |
|                                          |                     | α <sub>b</sub> = 0.569 |           |                                                                                |
| Resistenza al rifollamento:              | F <sub>b,Rd</sub> = | 209.5 kN               | Verifica: | $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{181.20}{209.5} = 0.865 \leq 1$ : Verificato |

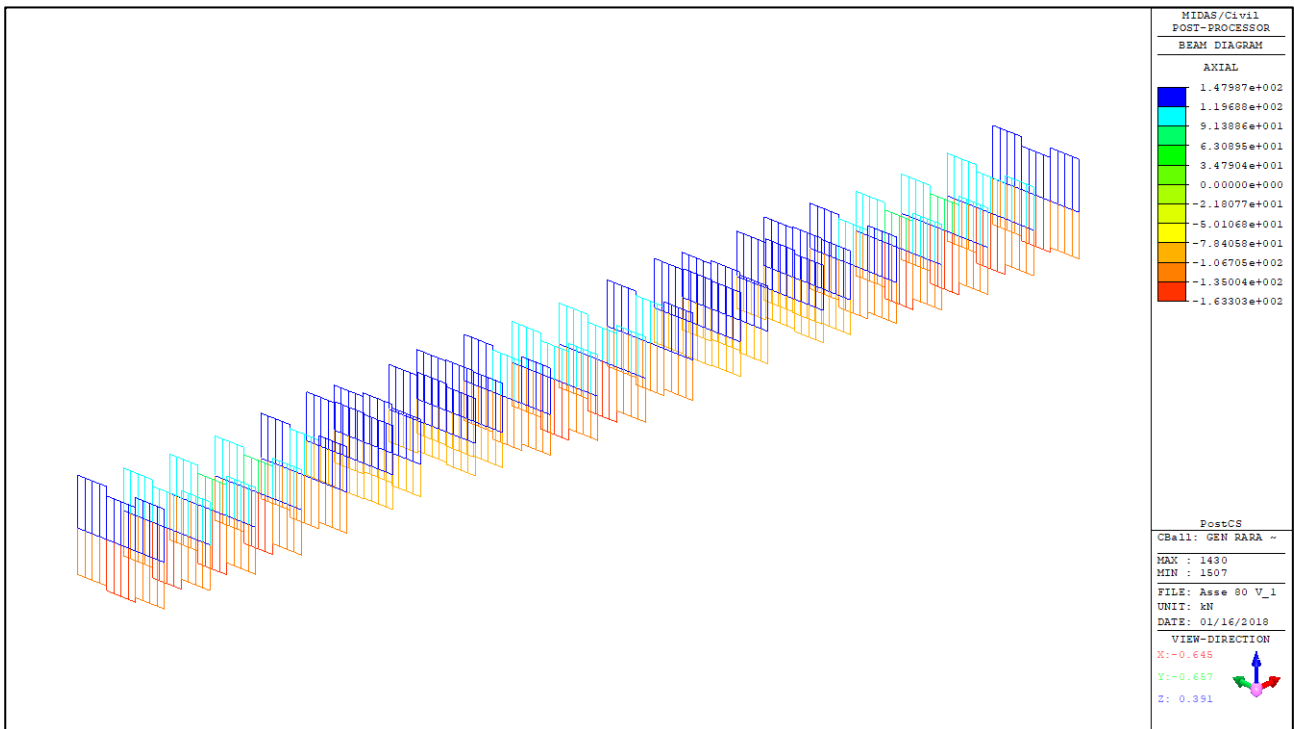
|                                                               |                     |                    |                      |                                                                               |
|---------------------------------------------------------------|---------------------|--------------------|----------------------|-------------------------------------------------------------------------------|
| S.L.U. Verifica a trazione del profilato indebolito dai fori: | Area netta:         | A <sub>net</sub> = | 2165 mm <sup>2</sup> |                                                                               |
| Resistenza della sezione netta:                               | N <sub>u,Rd</sub> = | 794.99 kN          | Verifica:            | $\frac{N_{Ed}}{N_{u,Rd}} = \frac{543.60}{794.99} = 0.684 \leq 1$ : Verificato |

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4): **Categoria B: connessioni ad attrito allo S.L.E.**

## 12.2 DIAFRAMMI SU SPALLE E INTERMEDI - TRASVERSI SUPERIORI



TRASVERSI SUPERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLU



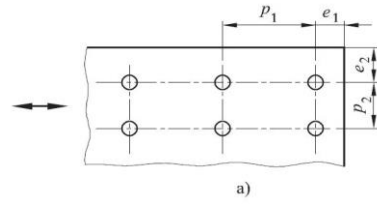
TRASVERSI SUPERIORI CORRENTI – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI IN CAMPATA - CORRENTI SUPERIORI (2 L 80x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):

|                                                                   |                 |      |
|-------------------------------------------------------------------|-----------------|------|
| Resistenza dei bulloni e delle sezioni tese (indebolite dai fori) | $\gamma_{M2} =$ | 1.25 |
| Resistenza a scorrimento allo SLU                                 | $\gamma_{M3} =$ | 1.25 |
| Resistenza a scorrimento allo SLE                                 | $\gamma_{M3} =$ | 1.10 |
| Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)      | $\gamma_{M7} =$ | 1.10 |

|                          |                                    |                    |                           |
|--------------------------|------------------------------------|--------------------|---------------------------|
| Dati geometrici bulloni: | Diametro nominale:                 | d =                | 20 mm                     |
|                          | Tipo fori: accoppiamento           | d <sub>0</sub> =   | 21.0 mm                   |
|                          | Area nominale gambo non filettato: | A =                | 314 mm <sup>2</sup>       |
|                          | Area resistente gambo filettato:   | A <sub>res</sub> = | 245 mm <sup>2</sup>       |
|                          | Classe bulloni:                    | f <sub>yb</sub> =  | 900.00 N/mm <sup>2</sup>  |
|                          |                                    | f <sub>tb</sub> =  | 1000.00 N/mm <sup>2</sup> |



|                           |                                                                        |                           |
|---------------------------|------------------------------------------------------------------------|---------------------------|
| Resistenza a taglio:      | il piano di taglio interessa la parte:                                 | filettata                 |
|                           | resistenza allo SLU (per piano di taglio): F <sub>v,Rd</sub> =         | 97.92 kN                  |
| Resistenza a scorrimento: | coefficiente di attrito: μ =                                           | 0.30 (tutte le superfici) |
| (solo classi 8.8 o 10.9)  | tipo di serraggio:                                                     | controllato               |
|                           | forza di precarico: F <sub>p,Cd</sub> =                                | 171.36 kN                 |
|                           | resistenza allo SLU (per piano di scorrimento): F <sub>s,Rd</sub> =    | 41.13 kN                  |
|                           | resistenza allo SLE (per piano di scorrimento): F <sub>s,Rd,es</sub> = | 46.73 kN                  |

|                         |                                 |                      |                      |
|-------------------------|---------------------------------|----------------------|----------------------|
| Dati profili e lamiere: | Spessore nominale dell'elemento |                      |                      |
|                         | t ≤ 40 mm                       | 40 mm < t ≤ 80 mm    |                      |
|                         | f <sub>yk</sub>                 | f <sub>tk</sub>      | f <sub>yk</sub>      |
|                         | [N/mm <sup>2</sup> ]            | [N/mm <sup>2</sup> ] | [N/mm <sup>2</sup> ] |

|                                        |         |     |     |     |     |                                                                                                       |
|----------------------------------------|---------|-----|-----|-----|-----|-------------------------------------------------------------------------------------------------------|
| Qualità acciaio profilati:             | S 355 W | 355 | 510 | 335 | 490 | UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica |
| Qualità acciaio lamiere / coprigiunti: | S 355 W | 355 | 510 | 335 | 490 | UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica |

|                          |                                              |                       |  |  |  |                          |
|--------------------------|----------------------------------------------|-----------------------|--|--|--|--------------------------|
| Dati geometrici bulloni: | Numero file di bulloni:                      | 1                     |  |  |  |                          |
|                          | Numero bulloni:                              | 3                     |  |  |  | Limiti massimi e minimi  |
|                          | Numero superfici di taglio:                  | 2                     |  |  |  | Min. Max.                |
|                          | Interassi: p <sub>1</sub> =                  | 60 mm                 |  |  |  | 46.2 140 mm              |
|                          | (per fila singola porre: p <sub>2</sub> = 0) | p <sub>2</sub> = 0 mm |  |  |  | 0 0 mm                   |
|                          |                                              |                       |  |  |  | Entro limiti: Verificato |

|                            |                                              |                      |  |  |  |                          |
|----------------------------|----------------------------------------------|----------------------|--|--|--|--------------------------|
| Dati geometrici profilati: | Numero profilati:                            | 2                    |  |  |  |                          |
|                            | Spessore flangia bullonata:                  | 10 mm                |  |  |  | Limiti massimi e minimi  |
|                            | Area sezione singolo profilato:              | 1510 mm <sup>2</sup> |  |  |  | Min. Max.                |
|                            | Distanze bulloni dal bordo: e <sub>1</sub> = | 40 mm                |  |  |  | 25.2 125 mm              |
|                            | e <sub>2</sub> =                             | 40 mm                |  |  |  | 25.2 125 mm              |
|                            |                                              |                      |  |  |  | Entro limiti: Verificato |

|                                        |                                              |       |  |  |  |                          |
|----------------------------------------|----------------------------------------------|-------|--|--|--|--------------------------|
| Dati geometrici lamiere / coprigiunti: | Numero lamiere:                              | 1     |  |  |  |                          |
|                                        | Spessore:                                    | 15 mm |  |  |  | Limiti massimi e minimi  |
|                                        | Distanze bulloni dal bordo: e <sub>1</sub> = | 40 mm |  |  |  | Min. Max.                |
|                                        | e <sub>2</sub> =                             | 40 mm |  |  |  | 25.2 125 mm              |
|                                        |                                              |       |  |  |  | Entro limiti: Verificato |

|                                            |        |                      |            |
|--------------------------------------------|--------|----------------------|------------|
| Sollecitazioni di progetto (trazione > 0): | S.L.U. | N <sub>Ed</sub> =    | -209.70 kN |
|                                            | S.L.E. | N <sub>Ed,es</sub> = | -163.30 kN |

|                                                                     |                     |          |           |                                                                               |
|---------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|
| S.L.U. Verifica di resistenza a taglio bullone, singola superficie: | F <sub>v,Ed</sub> = | 34.95 kN | Verifica: | $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{34.95}{97.92} = 0.357 \leq 1$ : Verificato |
|---------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|

|                                                                          |                     |          |           |                                                                               |
|--------------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|
| S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie: | F <sub>v,Ed</sub> = | 34.95 kN | Verifica: | $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{34.95}{41.13} = 0.850 \leq 1$ : Verificato |
|--------------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|

|                                                                          |                        |          |           |                                                                                     |
|--------------------------------------------------------------------------|------------------------|----------|-----------|-------------------------------------------------------------------------------------|
| S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie: | F <sub>v,Ed,es</sub> = | 27.22 kN | Verifica: | $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{27.22}{46.73} = 0.582 \leq 1$ : Verificato |
|--------------------------------------------------------------------------|------------------------|----------|-----------|-------------------------------------------------------------------------------------|

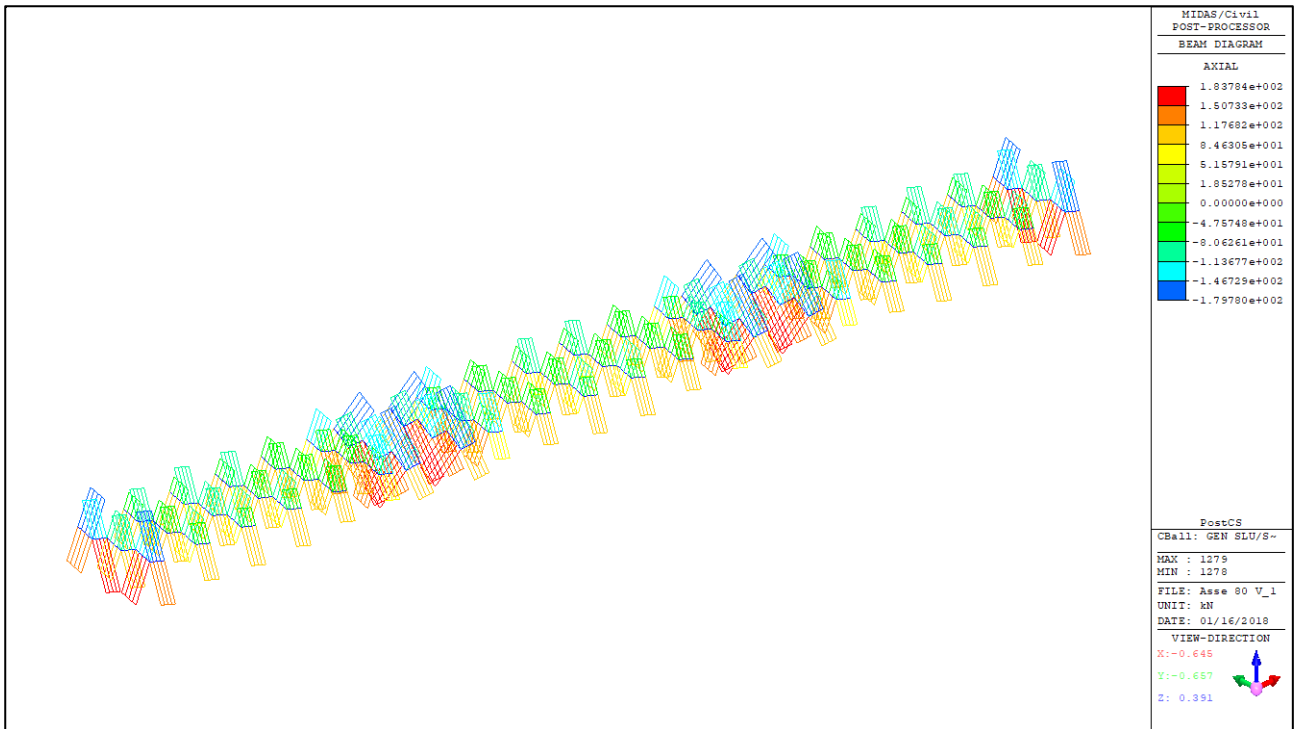
|                                            |                     |                        |           |                                                                               |
|--------------------------------------------|---------------------|------------------------|-----------|-------------------------------------------------------------------------------|
| S.L.U. Verifica al rifollamento profilato: | F <sub>b,Ed</sub> = | 34.95 kN               |           |                                                                               |
|                                            | coefficienti:       | k1 = 2.500             |           |                                                                               |
|                                            |                     | α <sub>b</sub> = 0.635 |           |                                                                               |
| Resistenza al rifollamento:                | F <sub>b,Rd</sub> = | 129.5 kN               | Verifica: | $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{34.95}{129.5} = 0.270 \leq 1$ : Verificato |

|                                          |                     |                        |           |                                                                                |
|------------------------------------------|---------------------|------------------------|-----------|--------------------------------------------------------------------------------|
| S.L.U. Verifica al rifollamento lamiere: | F <sub>b,Ed</sub> = | 69.90 kN               |           |                                                                                |
|                                          | coefficienti:       | k1 = 2.500             |           |                                                                                |
|                                          |                     | α <sub>b</sub> = 0.635 |           |                                                                                |
| Resistenza al rifollamento:              | F <sub>b,Rd</sub> = | 194.3 kN               | Verifica: | $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{69.90}{194.29} = 0.360 \leq 1$ : Verificato |

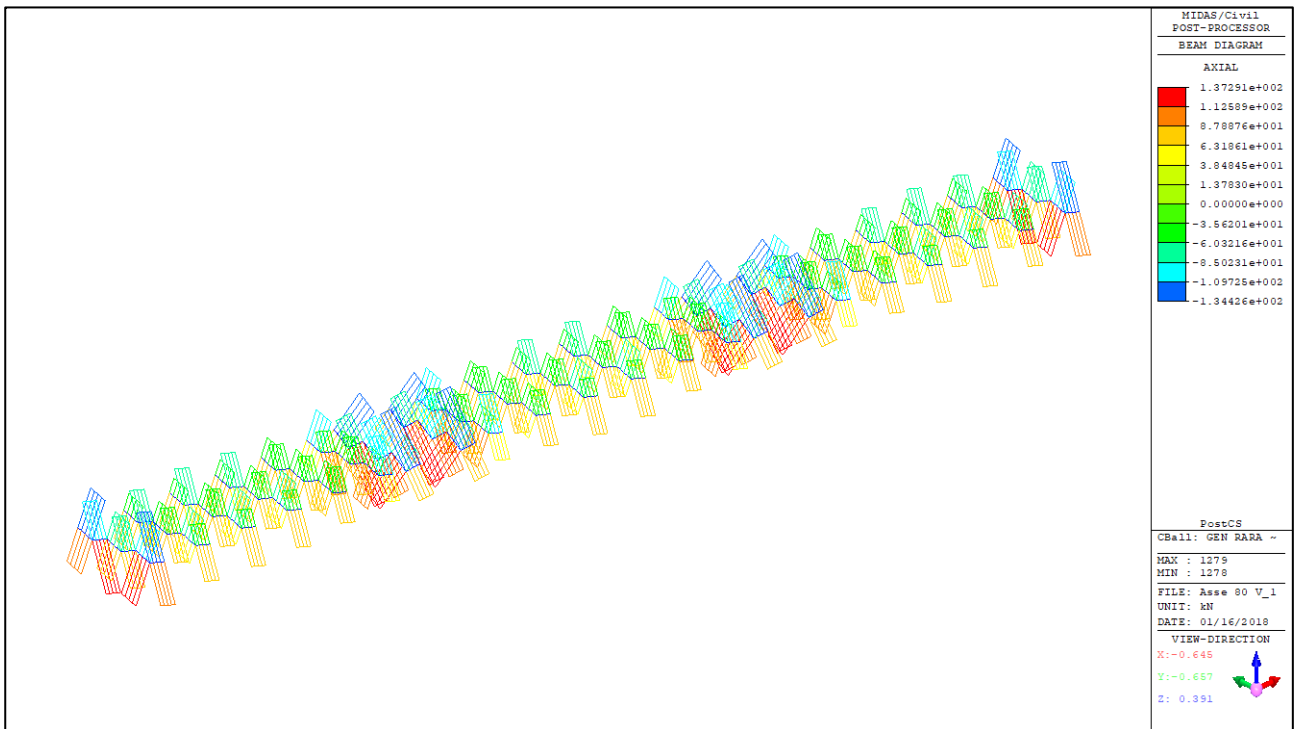
|                                                               |                     |                    |                      |                                                              |
|---------------------------------------------------------------|---------------------|--------------------|----------------------|--------------------------------------------------------------|
| S.L.U. Verifica a trazione del profilato indebolito dai fori: | Area netta:         | A <sub>net</sub> = | 2600 mm <sup>2</sup> |                                                              |
| Resistenza della sezione netta:                               | N <sub>u,Rd</sub> = | 954.72 kN          | Verifica:            | $\frac{N_{Ed}}{N_{u,Rd}} = \frac{0.00}{954.72} = 0.000$ N.A. |

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4): **Categoria C: connessioni ad attrito allo S.L.U.**

### 12.3 DIAFRAMMI SU SPALLE E INTERMEDI - DIAGONALI



DIAGONALI CORRENTI – SFORZO ASSIALE – INVILUPPO SLU

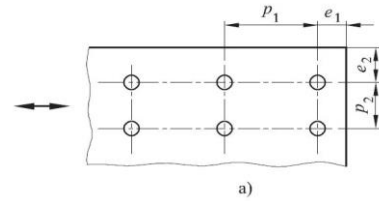


DIAGONALI CORRENTI – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI IN CAMPATA - DIAGONALI (2 L 80x8)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  
 Resistenza a scorrimento allo SLU  
 Resistenza a scorrimento allo SLE  
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)

$\gamma_{M2} = 1.25$   
 $\gamma_{M3} = 1.25$   
 $\gamma_{M3} = 1.10$   
 $\gamma_{M7} = 1.10$



Dati geometrici bulloni: Diametro nominale: **d = 20 mm**  
 Tipo fori: accoppiamento **normale**  $d_0 = 21.0 mm$   
 Area nominale gambo non filettato: **A = 314 mm<sup>2</sup>**  
 Area resistente gambo filettato: **A<sub>res</sub> = 245 mm<sup>2</sup>**  
 Classe bulloni: **10.9**  $f_{yb} = 900.00 N/mm^2$   
 $f_{tb} = 1000.00 N/mm^2$

Resistenza a taglio: il piano di taglio interessa la parte: **filettata**  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 97.92 kN$

Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} = 171.36 kN$   
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 41.13 kN$   
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 46.73 kN$

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40 mm$   $40 mm < t \leq 80 mm$   
 $f_{yk}$   $f_{tk}$   $f_{yk}$   $f_{tk}$   
 [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>] [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 Qualità acciaio lamiere / coprigiunti: **S 355 W** 355 510 335 490 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **2** Limiti massimi e minimi  
 Numero superfici di taglio: **2** Min. Max.  
 Interassi:  $p_1 = 60 mm$  46.2 112 mm Entro limiti: Verificato  
 (per fila singola porre:  $p_2 = 0$ )  $p_2 = 0 mm$  0 0 mm Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2** Limiti massimi e minimi  
 Spessore flangia bullonata: **8 mm** Min. Max.  
 Area sezione singolo profilato: **1230 mm<sup>2</sup>** 25.2 125 mm Entro limiti: Verificato  
 Distanze bulloni dal bordo:  $e_1 = 40 mm$  25.2 125 mm Entro limiti: Verificato  
 $e_2 = 40 mm$

Dati geometrici lamiere / coprigiunti: Numero lamiera: **1** Limiti massimi e minimi  
 Spessore: **15 mm** Min. Max.  
 Distanze bulloni dal bordo:  $e_1 = 40 mm$  25.2 125 mm Entro limiti: Verificato  
 $e_2 = 40 mm$  25.2 125 mm Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = 183.80 kN$   
 S.L.E.  $N_{Ed,es} = 137.30 kN$

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 45.95 kN$  Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{45.95}{97.92} = 0.469 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 45.95 kN$  Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{45.95}{41.13} = 1.117 > 1$ : NON Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 34.33 kN$  Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{34.33}{46.73} = 0.734 \leq 1$ : Verificato

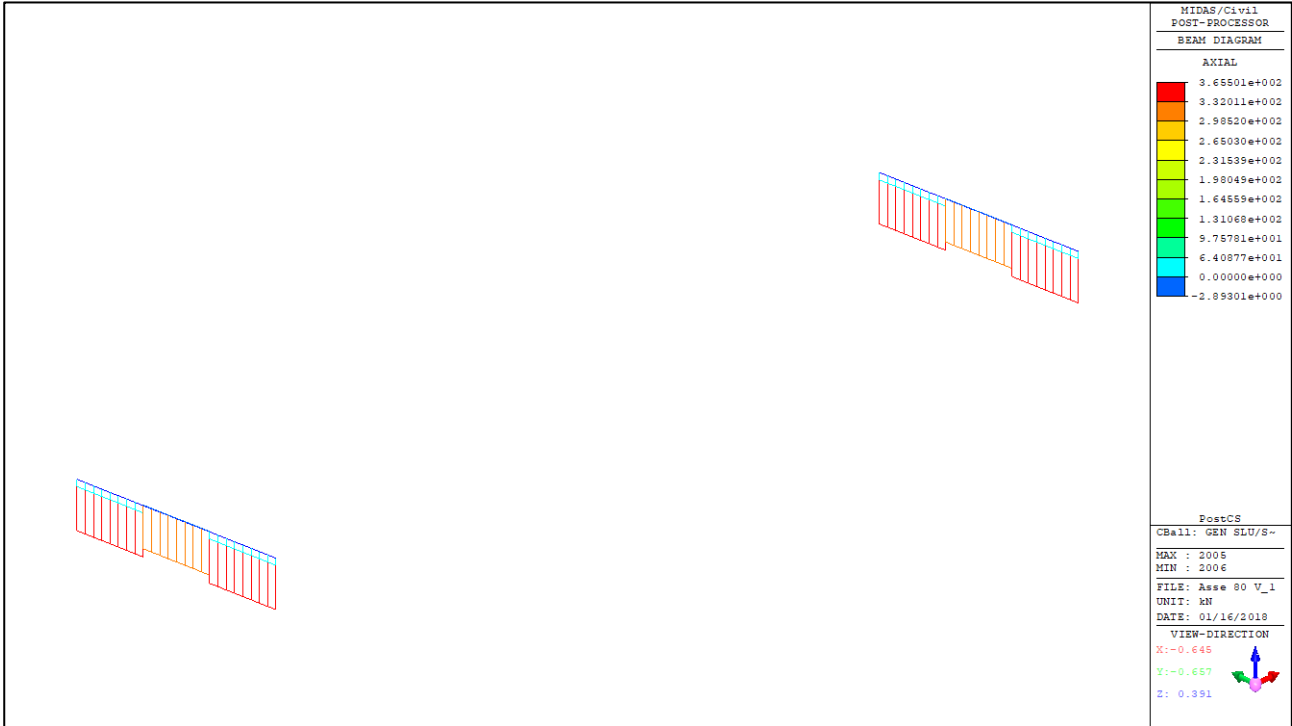
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 45.95 kN$   
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.635$   
 Resistenza al rifollamento:  $F_{b,Rd} = 103.6 kN$  Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{45.95}{103.6} = 0.443 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} = 91.90 kN$   
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.635$   
 Resistenza al rifollamento:  $F_{b,Rd} = 194.3 kN$  Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{91.90}{194.29} = 0.473 \leq 1$ : Verificato

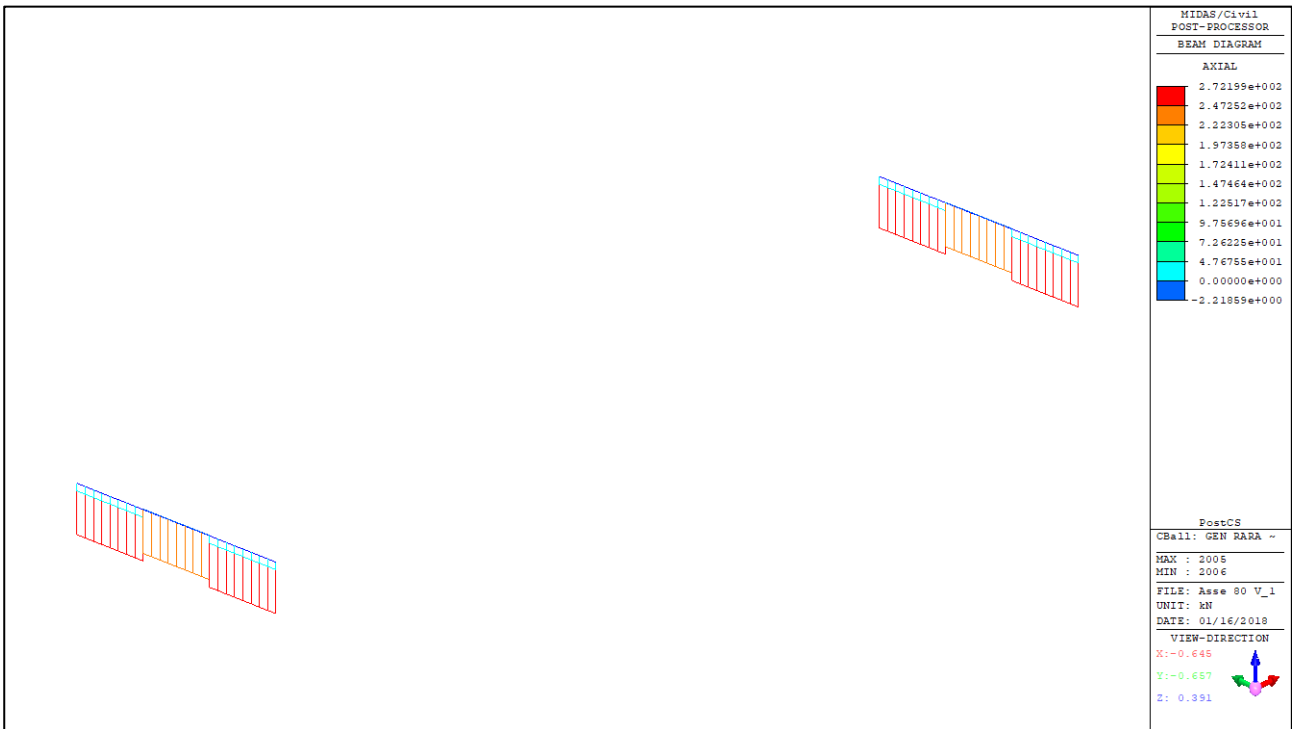
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 2124 mm^2$   
 Resistenza della sezione netta:  $N_{u,Rd} = 779.93 kN$  Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{183.80}{779.93} = 0.236 \leq 1$ : Verificato

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4): **Categoria B: connessioni ad attrito allo S.L.E.**

12.4 DIAFRAMMI SU PILE – TRASVERSI INFERIORI



TRASVERSI INFERIORI PILE – SFORZO ASSIALE – INVILUPPO SLU



TRASVERSI INFERIORI PILE – SFORZO ASSIALE – INVILUPPO SLE

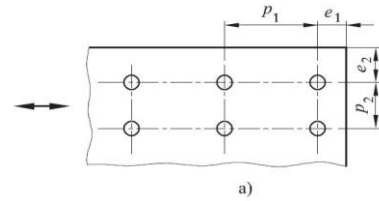


**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI SU PILA - CORRENTI INFERIORI (2 L 100x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  
 Resistenza a scorrimento allo SLU  
 Resistenza a scorrimento allo SLE  
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)

$\gamma_{M2} = 1.25$   
 $\gamma_{M3} = 1.25$   
 $\gamma_{M3} = 1.10$   
 $\gamma_{M7} = 1.10$

Dati geometrici bulloni: Diametro nominale:  $d = 24$  mm  
 Tipo fori: accoppiamento **normale**  
 Area nominale gambo non filettato:  $A = 452$  mm<sup>2</sup>  
 Area resistente gambo filettato:  $A_{res} = 353$  mm<sup>2</sup>  
 Classe bulloni: **10.9**  
 $f_{yb} = 900.00$  N/mm<sup>2</sup>  
 $f_{tb} = 1000.00$  N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte: **filettata**  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 141.00$  kN  
 Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} = 246.75$  kN  
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 59.22$  kN  
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 67.30$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm       $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$        $f_{tk}$        $f_{yk}$        $f_{tk}$   
 [N/mm<sup>2</sup>]      [N/mm<sup>2</sup>]      [N/mm<sup>2</sup>]      [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W**      355      510      335      490      UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 Qualità acciaio lamiere / coprigiunti: **S 355 W**      355      510      335      490      UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **3**      Limiti massimi e minimi  
 Numero superfici di taglio: **2**      Min.      Max.  
 Interassi:  $p_1 = 60$  mm      56.1      140 mm      Entro limiti: Verificato  
 (per fila singola porre:  $p_2 = 0$ )       $p_2 = 0$  mm      0      0 mm      Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2**      Limiti massimi e minimi  
 Spessore flangia bullonata: **10** mm      Min.      Max.  
 Area sezione singolo profilato: **1920** mm<sup>2</sup>      30.6      125 mm      Entro limiti: Verificato  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm      30.6      125 mm      Entro limiti: Verificato  
 $e_2 = 50$  mm

Dati geometrici lamiere / coprigiunti: Numero lamiera: **1**      Limiti massimi e minimi  
 Spessore: **20** mm      Min.      Max.  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm      30.6      160 mm      Entro limiti: Verificato  
 $e_2 = 50$  mm      30.6      160 mm      Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = 365.50$  kN  
 S.L.E.  $N_{Ed,es} = 272.20$  kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 60.92$  kN      Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{60.92}{141.00} = 0.432 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 60.92$  kN      Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{60.92}{59.22} = 1.029 > 1$ : NON Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 45.37$  kN      Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{45.37}{67.30} = 0.674 \leq 1$ : Verificato

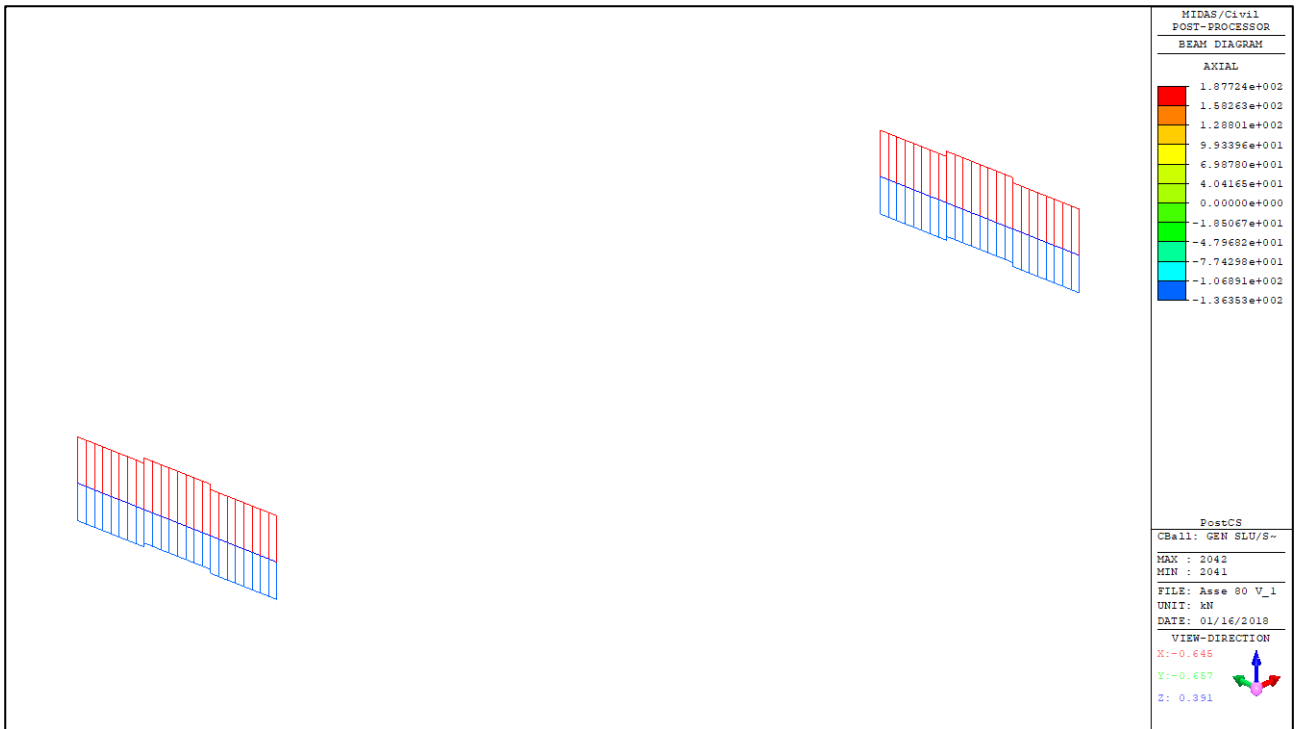
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 60.92$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 128.00$  kN      Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{60.92}{128.00} = 0.476 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiere:  $F_{b,Ed} = 121.83$  kN  
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 256.00$  kN      Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{121.83}{256.00} = 0.476 \leq 1$ : Verificato

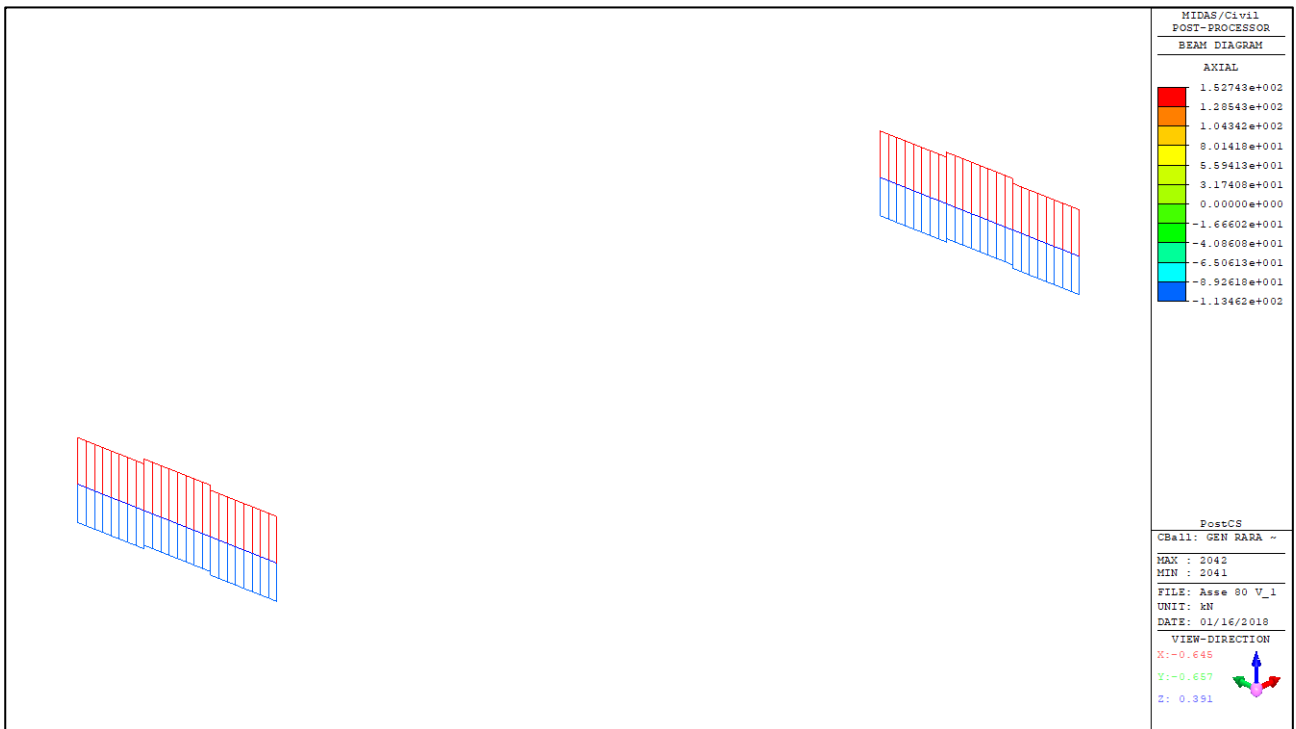
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 3330$  mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} = 1222.78$  kN      Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{365.50}{1222.78} = 0.299 \leq 1$ : Verificato

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4):      Categoria B: connessioni ad attrito allo S.L.E.

## 12.5 DIAFRAMMI SU PILE – TRASVERSI SUPERIORI



TRASVERSI SUPERIORI PILE – SFORZO ASSIALE – INVILUPPO SLU



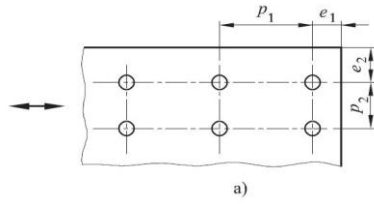
TRASVERSI SUPERIORI PILE – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI SU PILA - CORRENTI SUPERIORI (2 L 100x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):

|                                                                   |                 |      |
|-------------------------------------------------------------------|-----------------|------|
| Resistenza dei bulloni e delle sezioni tese (indebolite dai fori) | $\gamma_{M2} =$ | 1.25 |
| Resistenza a scorrimento allo SLU                                 | $\gamma_{M3} =$ | 1.25 |
| Resistenza a scorrimento allo SLE                                 | $\gamma_{M3} =$ | 1.10 |
| Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)      | $\gamma_{M7} =$ | 1.10 |

|                          |                                    |                    |                           |
|--------------------------|------------------------------------|--------------------|---------------------------|
| Dati geometrici bulloni: | Diametro nominale:                 | d =                | 20 mm                     |
|                          | Tipo fori: accoppiamento           | d <sub>0</sub> =   | 20.3 mm                   |
|                          |                                    | A =                | 314 mm <sup>2</sup>       |
|                          | Area nominale gambo non filettato: | A <sub>res</sub> = | 245 mm <sup>2</sup>       |
|                          | Area resistente gambo filettato:   | f <sub>yb</sub> =  | 900.00 N/mm <sup>2</sup>  |
|                          | Classe bulloni:                    | f <sub>tb</sub> =  | 1000.00 N/mm <sup>2</sup> |



|                           |                                                                        |                           |
|---------------------------|------------------------------------------------------------------------|---------------------------|
| Resistenza a taglio:      | il piano di taglio interessa la parte:                                 | filettata                 |
|                           | resistenza allo SLU (per piano di taglio): F <sub>v,Rd</sub> =         | 97.92 kN                  |
| Resistenza a scorrimento: | coefficiente di attrito: μ =                                           | 0.30 (tutte le superfici) |
| (solo classi 8.8 o 10.9)  | tipo di serraggio:                                                     | controllato               |
|                           | forza di precarico: F <sub>p,Cd</sub> =                                | 171.36 kN                 |
|                           | resistenza allo SLU (per piano di scorrimento): F <sub>s,Rd</sub> =    | 41.13 kN                  |
|                           | resistenza allo SLE (per piano di scorrimento): F <sub>s,Rd,es</sub> = | 46.73 kN                  |

|                         |                                 |                      |                      |
|-------------------------|---------------------------------|----------------------|----------------------|
| Dati profili e lamiere: | Spessore nominale dell'elemento |                      |                      |
|                         | t ≤ 40 mm                       | 40 mm < t ≤ 80 mm    |                      |
|                         | f <sub>yk</sub>                 | f <sub>tk</sub>      | f <sub>yk</sub>      |
|                         | [N/mm <sup>2</sup> ]            | [N/mm <sup>2</sup> ] | [N/mm <sup>2</sup> ] |

|                                        |         |     |     |     |     |                                                                                                       |
|----------------------------------------|---------|-----|-----|-----|-----|-------------------------------------------------------------------------------------------------------|
| Qualità acciaio profilati:             | S 355 W | 355 | 510 | 335 | 490 | UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica |
| Qualità acciaio lamiere / coprigiunti: | S 355 W | 355 | 510 | 335 | 490 | UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica |

|                          |                                              |                  |      |       |                          |
|--------------------------|----------------------------------------------|------------------|------|-------|--------------------------|
| Dati geometrici bulloni: | Numero file di bulloni:                      | 1                |      |       |                          |
|                          | Numero bulloni:                              | 3                |      |       | Limiti massimi e minimi  |
|                          | Numero superfici di taglio:                  | 2                |      |       | Min. Max.                |
|                          | Interassi: p <sub>1</sub> =                  | 60 mm            |      | 44.66 | 140 mm                   |
|                          | (per fila singola porre: p <sub>2</sub> = 0) | p <sub>2</sub> = | 0 mm | 0     | 0 mm                     |
|                          |                                              |                  |      |       | Entro limiti: Verificato |

|                            |                                              |                      |  |       |                          |
|----------------------------|----------------------------------------------|----------------------|--|-------|--------------------------|
| Dati geometrici profilati: | Numero profilati:                            | 2                    |  |       |                          |
|                            | Spessore flangia bullonata:                  | 10 mm                |  |       | Limiti massimi e minimi  |
|                            | Area sezione singolo profilato:              | 1920 mm <sup>2</sup> |  |       | Min. Max.                |
|                            | Distanze bulloni dal bordo: e <sub>1</sub> = | 40 mm                |  | 24.36 | 125 mm                   |
|                            | e <sub>2</sub> =                             | 50 mm                |  | 24.36 | 125 mm                   |
|                            |                                              |                      |  |       | Entro limiti: Verificato |

|                                        |                                              |       |  |       |                          |
|----------------------------------------|----------------------------------------------|-------|--|-------|--------------------------|
| Dati geometrici lamiere / coprigiunti: | Numero lamiere:                              | 1     |  |       |                          |
|                                        | Spessore:                                    | 20 mm |  |       | Limiti massimi e minimi  |
|                                        | Distanze bulloni dal bordo: e <sub>1</sub> = | 40 mm |  | 24.36 | 160 mm                   |
|                                        | e <sub>2</sub> =                             | 50 mm |  | 24.36 | 160 mm                   |
|                                        |                                              |       |  |       | Entro limiti: Verificato |

|                                            |        |                      |           |
|--------------------------------------------|--------|----------------------|-----------|
| Sollecitazioni di progetto (trazione > 0): | S.L.U. | N <sub>Ed</sub> =    | 187.70 kN |
|                                            | S.L.E. | N <sub>Ed,es</sub> = | 152.70 kN |

|                                                                     |                     |          |           |                                                                               |
|---------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|
| S.L.U. Verifica di resistenza a taglio bullone, singola superficie: | F <sub>v,Ed</sub> = | 31.28 kN | Verifica: | $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{31.28}{97.92} = 0.319 \leq 1$ : Verificato |
|---------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|

|                                                                          |                     |          |           |                                                                               |
|--------------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|
| S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie: | F <sub>v,Ed</sub> = | 31.28 kN | Verifica: | $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{31.28}{41.13} = 0.761 \leq 1$ : Verificato |
|--------------------------------------------------------------------------|---------------------|----------|-----------|-------------------------------------------------------------------------------|

|                                                                          |                        |          |           |                                                                                     |
|--------------------------------------------------------------------------|------------------------|----------|-----------|-------------------------------------------------------------------------------------|
| S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie: | F <sub>v,Ed,es</sub> = | 25.45 kN | Verifica: | $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{25.45}{46.73} = 0.545 \leq 1$ : Verificato |
|--------------------------------------------------------------------------|------------------------|----------|-----------|-------------------------------------------------------------------------------------|

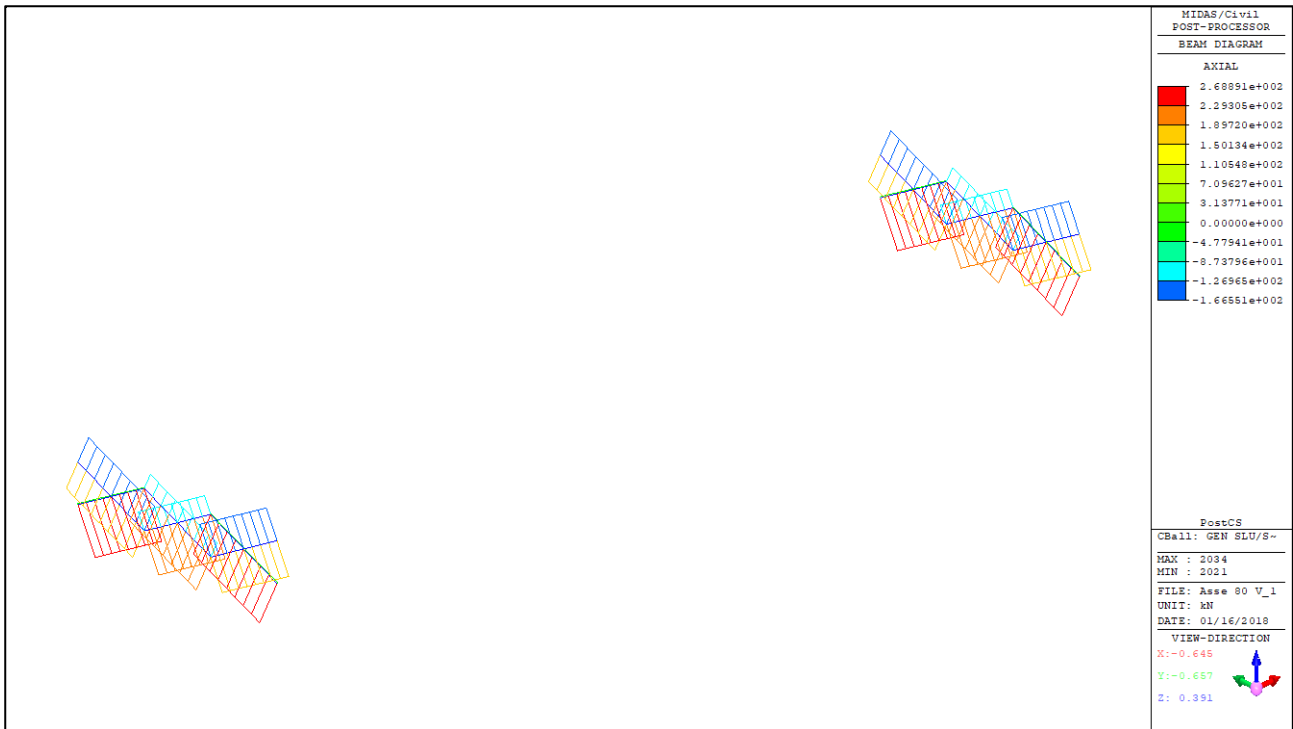
|                                            |                     |                  |           |                                                                                |
|--------------------------------------------|---------------------|------------------|-----------|--------------------------------------------------------------------------------|
| S.L.U. Verifica al rifollamento profilato: | F <sub>b,Ed</sub> = | 31.28 kN         |           |                                                                                |
|                                            | coefficienti:       | k1 =             | 2.500     |                                                                                |
|                                            |                     | α <sub>b</sub> = | 0.657     |                                                                                |
| Resistenza al rifollamento:                | F <sub>b,Rd</sub> = | 134.0 kN         | Verifica: | $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{31.28}{133.99} = 0.233 \leq 1$ : Verificato |

|                                          |                     |                  |           |                                                                                |
|------------------------------------------|---------------------|------------------|-----------|--------------------------------------------------------------------------------|
| S.L.U. Verifica al rifollamento lamiere: | F <sub>b,Ed</sub> = | 62.57 kN         |           |                                                                                |
|                                          | coefficienti:       | k1 =             | 2.500     |                                                                                |
|                                          |                     | α <sub>b</sub> = | 0.657     |                                                                                |
| Resistenza al rifollamento:              | F <sub>b,Rd</sub> = | 268.0 kN         | Verifica: | $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{62.57}{267.98} = 0.233 \leq 1$ : Verificato |

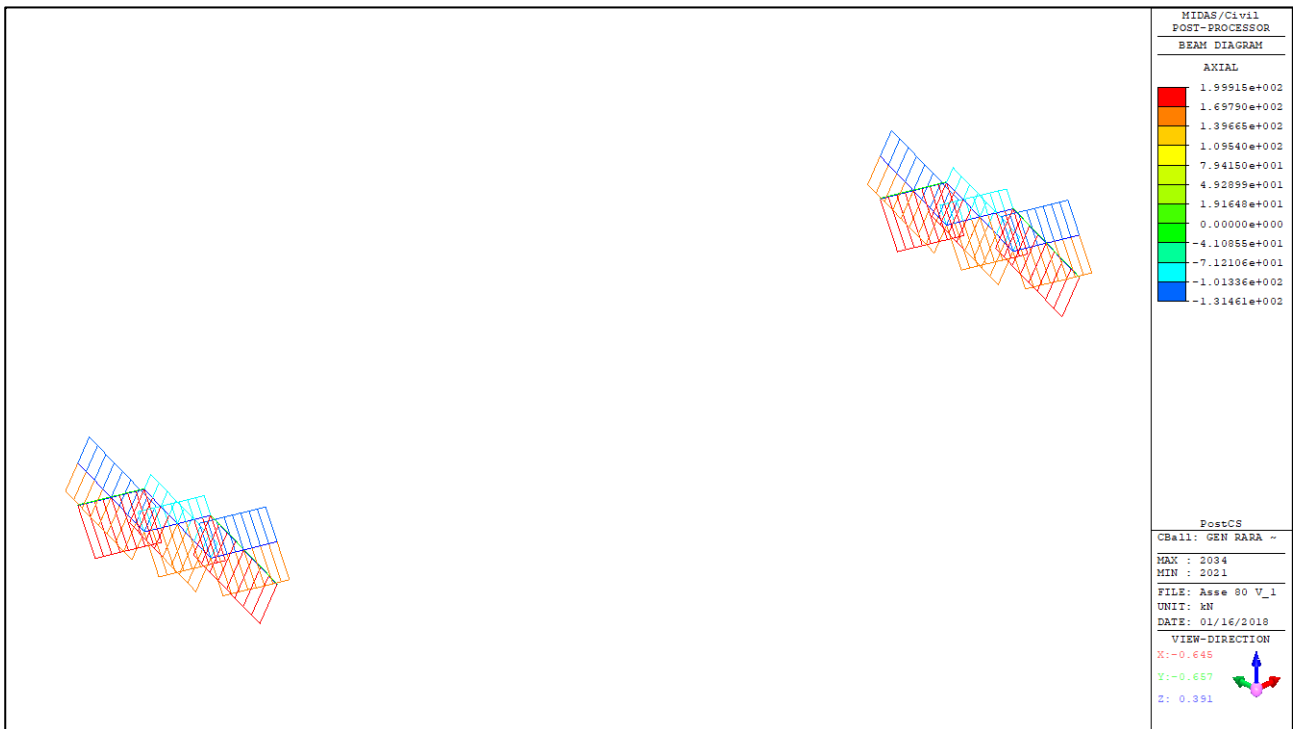
|                                                               |                     |                    |                      |                                                                                |
|---------------------------------------------------------------|---------------------|--------------------|----------------------|--------------------------------------------------------------------------------|
| S.L.U. Verifica a trazione del profilato indebolito dai fori: | Area netta:         | A <sub>net</sub> = | 3434 mm <sup>2</sup> |                                                                                |
| Resistenza della sezione netta:                               | N <sub>u,Rd</sub> = | 1260.96 kN         | Verifica:            | $\frac{N_{Ed}}{N_{u,Rd}} = \frac{187.70}{1260.96} = 0.149 \leq 1$ : Verificato |

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4): **Categoria C: connessioni ad attrito allo S.L.U.**

## 12.6 DIAFRAMMI SU PILE – DIAGONALI



DIAGONALI TRASVERSI SU PILE – SFORZO ASSIALE – INVILUPPO SLU

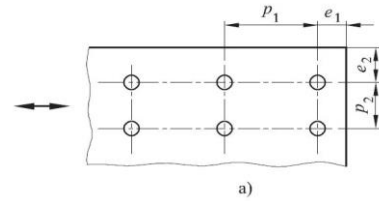


DIAGONALI TRASVERSI SU PILE – SFORZO ASSIALE – INVILUPPO SLE

**VERIFICA CONNESSIONE BULLONATA A TAGLIO: TRASVERSI SU PILA - DIAGONALI (2 L 80x8)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  
 Resistenza a scorrimento allo SLU  
 Resistenza a scorrimento allo SLE  
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)

$\gamma_{M2} = 1.25$   
 $\gamma_{M3} = 1.25$   
 $\gamma_{M3} = 1.10$   
 $\gamma_{M7} = 1.10$



Dati geometrici bulloni: Diametro nominale: **d = 24 mm**  
 Tipo fori: accoppiamento **normale**  $d_0 = 25.5 mm$   
 Area nominale gambo non filettato: **A = 452 mm<sup>2</sup>**  
 Area resistente gambo filettato: **A<sub>res</sub> = 353 mm<sup>2</sup>**  
 Classe bulloni: **10.9**  $f_{yb} = 900.00 N/mm^2$   
 $f_{tb} = 1000.00 N/mm^2$

Resistenza a taglio: il piano di taglio interessa la parte: **filettata**  
 resistenza allo SLU (per piano di taglio): **F<sub>v,Rd</sub> = 141.00 kN**

Resistenza a scorrimento: coefficiente di attrito: **μ = 0.30** (tutte le superfici)  
 tipo di serraggio: **controllato**  
 forza di precarico: **F<sub>p,Cd</sub> = 246.75 kN**  
 resistenza allo SLU (per piano di scorrimento): **F<sub>s,Rd</sub> = 59.22 kN**  
 resistenza allo SLE (per piano di scorrimento): **F<sub>s,Rd,es</sub> = 67.30 kN**

Dati profili e lamiere: Spessore nominale dell'elemento

|  | t ≤ 40 mm                            |                                      | 40 mm < t ≤ 80 mm                    |                                      |
|--|--------------------------------------|--------------------------------------|--------------------------------------|--------------------------------------|
|  | f <sub>yk</sub> [N/mm <sup>2</sup> ] | f <sub>tk</sub> [N/mm <sup>2</sup> ] | f <sub>yk</sub> [N/mm <sup>2</sup> ] | f <sub>tk</sub> [N/mm <sup>2</sup> ] |

Qualità acciaio profilati: **S 355 W** 355 510 335 490  
 Qualità acciaio lamiere / coprigiunti: **S 355 W** 355 510 335 490

UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **2**  
 Numero superfici di taglio: **2**  
 Interassi:  $p_1 = 60 mm$   
 (per fila singola porre:  $p_2 = 0$ )  $p_2 = 0 mm$   
 Limiti massimi e minimi  
 Min. 56.1 Max. 112 mm  
 0 0 mm  
 Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2**  
 Spessore flangia bullonata: **8 mm**  
 Area sezione singolo profilato: **1230 mm<sup>2</sup>**  
 Distanze bulloni dal bordo:  $e_1 = 40 mm$   
 $e_2 = 40 mm$   
 Limiti massimi e minimi  
 Min. 30.6 Max. 125 mm  
 30.6 125 mm  
 Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici lamiere / coprigiunti: Numero lamiera: **1**  
 Spessore: **20 mm**  
 Distanze bulloni dal bordo:  $e_1 = 40 mm$   
 $e_2 = 40 mm$   
 Limiti massimi e minimi  
 Min. 30.6 Max. 160 mm  
 30.6 160 mm  
 Entro limiti: Verificato  
 Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0): S.L.U.  $N_{Ed} = 268.90 kN$   
 S.L.E.  $N_{Ed,es} = 199.90 kN$

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 67.23 kN$   
 Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{67.23}{141.00} = 0.477 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 67.23 kN$   
 Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{67.23}{59.22} = 1.135 > 1$ : NON Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 49.98 kN$   
 Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{49.98}{67.30} = 0.743 \leq 1$ : Verificato

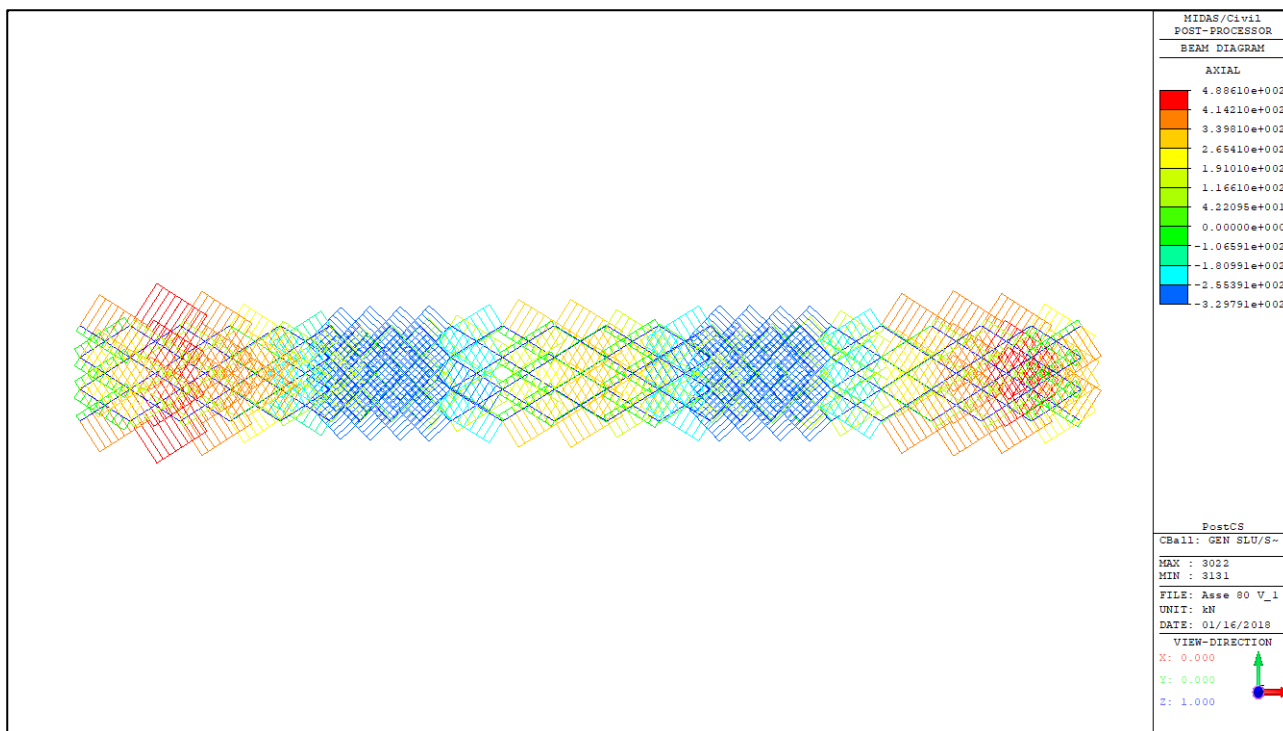
S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 67.23 kN$   
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 102.4 kN$   
 Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{67.23}{102.40} = 0.656 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiere:  $F_{b,Ed} = 134.45 kN$   
 coefficienti:  $k_1 = 2.500$   
 $\alpha_b = 0.523$   
 Resistenza al rifollamento:  $F_{b,Rd} = 256.0 kN$   
 Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{134.45}{256.00} = 0.525 \leq 1$ : Verificato

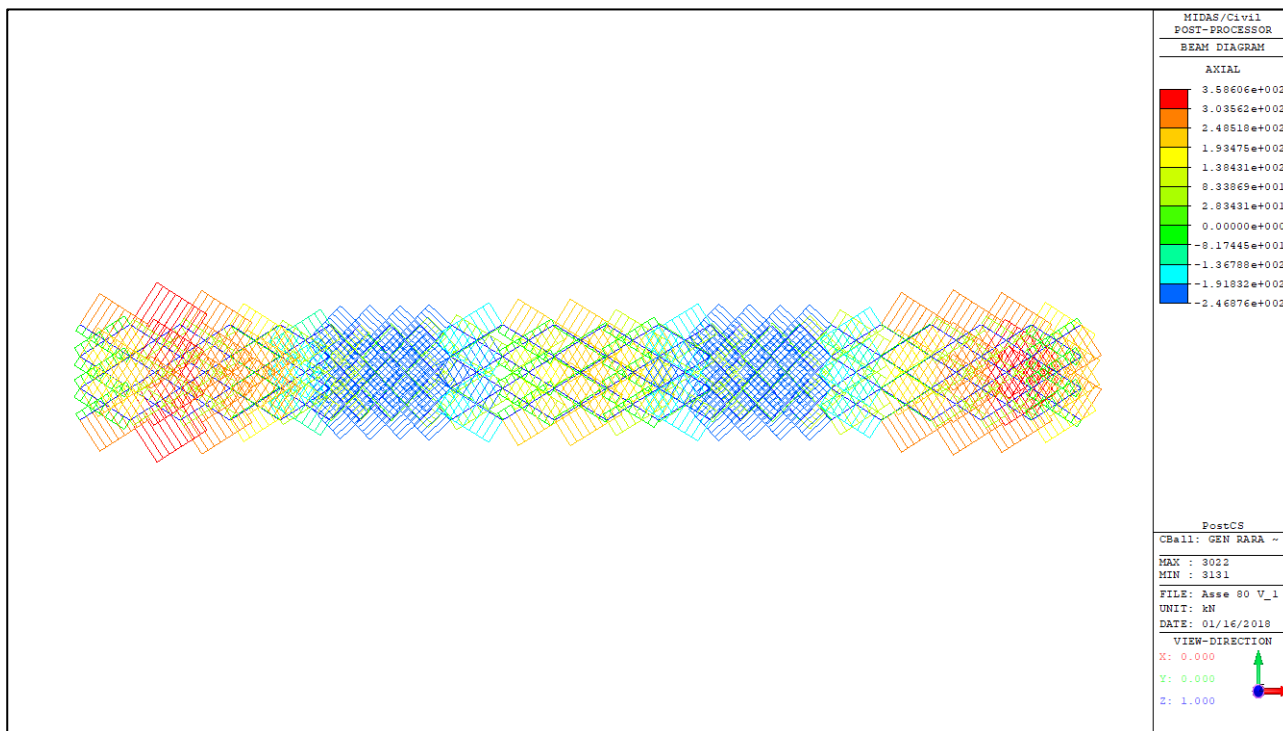
S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 2052 mm^2$   
 Resistenza della sezione netta:  $N_{u,Rd} = 753.49 kN$   
 Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{268.90}{753.49} = 0.357 \leq 1$ : Verificato

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4): **Categoria B: connessioni ad attrito allo S.L.E.**

## 12.7 CONTROVENTI



CONTROVENTI – SFORZO ASSIALE – INVILUPPO SLU



CONTROVENTI – SFORZO ASSIALE – INVILUPPO SLE



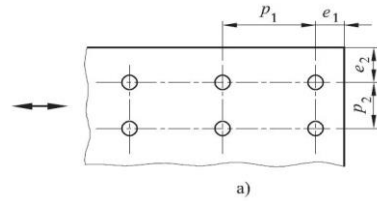
**VERIFICA CONNESSIONE BULLONATA A TAGLIO: CONTROVENTI (2 L 80x10)**

Coefficienti di sicurezza per la verifica delle unioni (NTC 2008; Tab. 4.2.XII):  
 Resistenza dei bulloni e delle sezioni tese (indebolite dai fori)  
 Resistenza a scorrimento allo SLU  
 Resistenza a scorrimento allo SLE  
 Precarico dei bulloni ad alta resistenza (classe 8.8 o 10.9)

$\gamma_{M2} = 1.25$   
 $\gamma_{M3} = 1.25$   
 $\gamma_{M3} = 1.10$   
 $\gamma_{M7} = 1.10$

Dati geometrici bulloni: Diametro nominale:  $d = 27$  mm  
 Tipo fori: accoppiamento **normale**  
 Area nominale gambo non filettato:  $A = 573$  mm<sup>2</sup>  
 Area resistente gambo filettato:  $A_{res} = 459$  mm<sup>2</sup>  
 Classe bulloni: **10.9**

$f_{yk} = 900.00$  N/mm<sup>2</sup>  
 $f_{yk} = 1000.00$  N/mm<sup>2</sup>



Resistenza a taglio: il piano di taglio interessa la parte:  
 resistenza allo SLU (per piano di taglio):  $F_{v,Rd} = 183.76$  kN  
 Resistenza a scorrimento: coefficiente di attrito:  $\mu = 0.30$  (tutte le superfici)  
 (solo classi 8.8 o 10.9) tipo di serraggio: **controllato**  
 forza di precarico:  $F_{p,Cd} = 321.58$  kN  
 resistenza allo SLU (per piano di scorrimento):  $F_{s,Rd} = 77.18$  kN  
 resistenza allo SLE (per piano di scorrimento):  $F_{s,Rd,es} = 87.70$  kN

Dati profili e lamiere: Spessore nominale dell'elemento  
 $t \leq 40$  mm       $40$  mm  $< t \leq 80$  mm  
 $f_{yk}$  [N/mm<sup>2</sup>]       $f_{tk}$  [N/mm<sup>2</sup>]       $f_{yk}$  [N/mm<sup>2</sup>]       $f_{tk}$  [N/mm<sup>2</sup>]

Qualità acciaio profilati: **S 355 W**      355      510      335      490  
 Qualità acciaio lamiere / coprigiunti: **S 355 W**      355      510      335      490

UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica  
 UNI EN 10025-5: acciai per impieghi strutturali con resistenza migliorata alla corrosione atmosferica

Dati geometrici bulloni: Numero file di bulloni: **1**  
 Numero bulloni: **3**  
 Numero superfici di taglio: **2**  
 Interassi:  $p_1 = 70$  mm  
 (per fila singola porre:  $p_2 = 0$ )       $p_2 = 0$  mm

Limiti massimi e minimi  
 Min.      Max.  
 62.7      140 mm  
 0      0 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici profilati: Numero profilati: **2**  
 Spessore flangia bullonata: **10** mm  
 Area sezione singolo profilato: **1510** mm<sup>2</sup>  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm  
 $e_2 = 40$  mm

Limiti massimi e minimi  
 Min.      Max.  
 34.2      125 mm  
 34.2      125 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Dati geometrici lamiere / coprigiunti: Numero lamiera: **1**  
 Spessore: **15** mm  
 Distanze bulloni dal bordo:  $e_1 = 40$  mm  
 $e_2 = 40$  mm

Limiti massimi e minimi  
 Min.      Max.  
 34.2      125 mm  
 34.2      125 mm

Entro limiti: Verificato  
 Entro limiti: Verificato

Sollecitazioni di progetto (trazione > 0):  
 S.L.U.       $N_{Ed} = 488.60$  kN  
 S.L.E.       $N_{Ed,es} = 358.60$  kN

S.L.U. Verifica di resistenza a taglio bullone, singola superficie:  $F_{v,Ed} = 81.43$  kN

Verifica:  $\frac{F_{v,Ed}}{F_{v,Rd}} = \frac{81.43}{183.76} = 0.443 \leq 1$ : Verificato

S.L.U. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed} = 81.43$  kN

Verifica:  $\frac{F_{v,Ed}}{F_{s,Rd}} = \frac{81.43}{77.18} = 1.055 > 1$ : NON Verificato

S.L.E. Verifica di resistenza a scorrimento bullone, singola superficie:  $F_{v,Ed,es} = 59.77$  kN

Verifica:  $\frac{F_{v,Ed,es}}{F_{s,Rd,es}} = \frac{59.77}{87.70} = 0.681 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento profilato:  $F_{b,Ed} = 81.43$  kN

coefficienti:  $k_1 = 2.230$   
 $\alpha_b = 0.468$   
 Resistenza al rifollamento:  $F_{b,Rd} = 114.9$  kN

Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{81.43}{114.92} = 0.709 \leq 1$ : Verificato

S.L.U. Verifica al rifollamento lamiera:  $F_{b,Ed} = 162.87$  kN

coefficienti:  $k_1 = 2.230$   
 $\alpha_b = 0.468$   
 Resistenza al rifollamento:  $F_{b,Rd} = 172.4$  kN

Verifica:  $\frac{F_{b,Ed}}{F_{b,Rd}} = \frac{162.87}{172.38} = 0.945 \leq 1$ : Verificato

S.L.U. Verifica a trazione del profilato indebolito dai fori: Area netta:  $A_{net} = 2450$  mm<sup>2</sup>  
 Resistenza della sezione netta:  $N_{u,Rd} = 899.64$  kN

Verifica:  $\frac{N_{Ed}}{N_{u,Rd}} = \frac{488.60}{899.64} = 0.543 \leq 1$ : Verificato

Categoria (verificata) di connessione bullonata a taglio (EN1993-1-8:2005 § 3.4):

Categoria B: connessioni ad attrito allo S.L.E.

## 13 VERIFICA DEGLI ISOLATORI SISMICI

Si effettuano le verifiche allo stato limite ultimo degli isolatori elastomerici effettuate secondo EN 1337-3 per le fasi statiche e secondo OPCM 3274 (allegato 10.A) per le fasi sismiche.

### CAVALCAVIA ASSE 80

#### VERIFICA APPOGGI ELASTOMERICI ARMATI

##### Dati di progetto:

|                                                                   | SPALLE |          | PILE |          |
|-------------------------------------------------------------------|--------|----------|------|----------|
| Deformazione massima sismica di progetto SLC ( $d/h \leq 2$ )     | Dsp=   | 152.0 mm | Dsp= | 156.0 mm |
| Deformazione massima statica di progetto SLU/SLD ( $d/h \leq 1$ ) | Dp=    | 76.0 mm  | Dp=  | 78.0 mm  |

##### Risultati da modello di calcolo

|                                                  | SPALLE    |               | PILE      |               |
|--------------------------------------------------|-----------|---------------|-----------|---------------|
| Carico verticale massimo statico SLU             | V(max)=   | -1928.8 kN    | V(max)=   | -4626.9 kN    |
| Carico verticale minimo statico SLU              | V(min)=   | -536.0 kN     | V(min)=   | -2028.1 kN    |
| Carico orizzontale massimo statico SLU           | H(max)=   | 55.1 kN       | H(max)=   | 118.2 kN      |
| Rotazione massima statica SLU                    | R (max)=  | 2.304E-02 rad | R (max)=  | 5.407E-03 rad |
| Carico verticale massimo sismico SLC             | Vs(max)=  | -614.3 kN     | Vs(max)=  | -2220.5 kN    |
| Carico verticale minimo sismico SLC              | Vs(min)=  | -380.2 kN     | Vs(min)=  | -1448.4 kN    |
| Carico orizzontale massimo sismico SLC           | Hs(max)=  | 106.5 kN      | Hs(max)=  | 227.5 kN      |
| Rotazione massima sismica SLC                    | Rs (max)= | 1.346E-02 rad | Rs (max)= | 2.466E-03 rad |
| Carico verticale massimo statico SLE (caratt.)   | Ve(max)=  | -1417.7 kN    | V(max)=   | -3410.4 kN    |
| Carico verticale minimo statico SLE (caratt.)    | Ve(min)=  | -367.1 kN     | V(min)=   | -1426.1 kN    |
| Carico orizzontale massimo statico SLE (caratt.) | He(max)=  | 40.6 kN       | H(max)=   | 80.6 kN       |
| Rotazione massima statica SLE (caratt.)          | Re(max)=  | 1.479E-02 rad | R (max)=  | 4.090E-03 rad |

##### Caratteristiche di progetto isolatore

|                                                                                 | SPALLE     |                          | PILE       |                          |
|---------------------------------------------------------------------------------|------------|--------------------------|------------|--------------------------|
| Tensione caratteristica di snervamento lamiera ( $\geq 235$ N/mm <sup>2</sup> ) | fyk=       | 275 N/mm <sup>2</sup>    | fyk=       | 275 N/mm <sup>2</sup>    |
| Modulo di elasticità volumetrica elastomero                                     | Eb=        | 2000 N/mm <sup>2</sup>   | Eb=        | 2000 N/mm <sup>2</sup>   |
| Modulo di elasticità tangenziale dinamico elastomero ( $\nu = D/te = 1$ )       | Gdin=      | 1.4 N/mm <sup>2</sup>    | Gdin=      | 1.4 N/mm <sup>2</sup>    |
| Spostamento massimo di progetto tra le due facce dell'isolatore allo SLC        | de=        | 152 mm                   | de=        | 156 mm                   |
| Diametro elastomero                                                             | Dg=        | 300 mm                   | Dg=        | 450 mm                   |
| Numero di strati di elastomero                                                  | ng=        | 19                       | ng=        | 13                       |
| Spessore singolo strato di elastomero (5 mm < tg < 25 mm)                       | tg=        | 4 mm                     | tg=        | 6 mm                     |
| Spessore singola lamiera di acciaio interna ( $\geq 2$ mm)                      | ts=        | 2 mm                     | ts=        | 3 mm                     |
| Spessore lamiera di acciaio esterna ( $\geq 20$ mm)                             | tse=       | 20 mm                    | tse=       | 20 mm                    |
| Lato piastra di ancoraggio                                                      | Z=         | 350 mm                   | Z=         | 500 mm                   |
| Spessore piastra di ancoraggio                                                  | ta=        | 25 mm                    | ta=        | 25 mm                    |
| Ricoprimento laterale lamiere di acciaio (> 4 mm)                               | rs=        | 10 mm                    | rs=        | 10 mm                    |
| Diametro lamiere di acciaio interne ( $Dg \cdot D' > 2 \times 4 = 8$ mm)        | D'=        | 280 mm                   | D'=        | 430 mm                   |
| Spessore totale elastomero                                                      | te=        | 76 mm                    | te=        | 78 mm                    |
| Spessore complessivo isolatore (escluse piastre ancoraggio)                     | h=         | 152 mm                   | h=         | 154 mm                   |
| Spessore complessivo isolatore (incluse piastre ancoraggio)                     | H=         | 202 mm                   | H=         | 204 mm                   |
| Tensione di trazione massima ( $\min(1,2 \cdot Gdin)$ )                         | Sigma_t=   | -1.00 N/mm <sup>2</sup>  | Sigma_t=   | -1.00 N/mm <sup>2</sup>  |
| Fattore di forma primario                                                       | S1=        | 17.50                    | S1=        | 17.92                    |
|                                                                                 | Fi(de)=    | 2.0790 rad               | Fi(de)=    | 2.4336 rad               |
| Modulo di compressibilità assiale                                               | Ec=        | 947.51 N/mm <sup>2</sup> | Ec=        | 963.83 N/mm <sup>2</sup> |
| <b>Rigidità orizzontale equivalente (<math>\nu = D/te = 1</math>)</b>           | <b>Ke=</b> | <b>1302 N/mm</b>         | <b>Ke=</b> | <b>2855 N/mm</b>         |
| <b>Rigidità verticale</b>                                                       | <b>Kv=</b> | <b>767676 N/mm</b>       | <b>Kv=</b> | <b>1794462 N/mm</b>      |

**Verifiche statiche secondo EN 1337-3:2005**

|                                                    |                          |                           |
|----------------------------------------------------|--------------------------|---------------------------|
| Carico verticale massimo statico SLU               | V(max)= 1928760 N        | V(max)= 4626890 N         |
| Carico orizzontale massimo statico SLU             | H(max)= 55143 N          | H(max)= 118202 N          |
| Carico orizzontale statico a $\gamma = D/te = 1$ : | H( $\gamma=1$ )= 98960 N | H( $\gamma=1$ )= 222660 N |
| Spostamento statico totale                         | D= 37.56 mm              | D= 36.28 mm               |
| Rotazione massima statica SLU                      | R (max)= 2.304E-02 rad   | R (max)= 5.407E-03 rad    |
|                                                    | Fi(de)= 2.8905 rad       | Fi(de)= 2.9802 rad        |
| Area ridotta efficace dell'isolatore               | Ar= 51785 mmq            | Ar= 130330 mmq            |

|                                                             |                       |                       |
|-------------------------------------------------------------|-----------------------|-----------------------|
| 1. Deformazione di taglio massima dovuta alla compressione: | Gamma_c= 2.2803       | Gamma_c= 2.1230       |
| dovuta allo spostamento statico tot (< 1)                   | Gamma_s= 0.4942       | Gamma_s= 0.4651       |
| dovuta alle rotazioni                                       | Gamma_alfa= 2.5578    | Gamma_alfa= 0.8774    |
| totale: (< 7)                                               | Gamma_t= 5.3324       | Gamma_t= 3.4655       |
| 2. Spessore minimo lamiera di acciaio ( $\geq 2$ mm)        | ts(min)= 2.00 mm      | ts(min)= 2.01 mm      |
| 3. Rotazione limite:                                        |                       |                       |
| spostamento verticale massimo:                              | Dz= 2.301 mm          | Dz= 2.349 mm          |
| spostamento dovuto alla rotazione massima                   | D_alfa= 2.150 mm      | D_alfa= 0.775 mm      |
| Verifica: (Dz - D_alfa) > 0                                 | Dz - D_alfa= 0.150 mm | Dz - D_alfa= 1.574 mm |
| 4. Verifica di instabilità: Pmax < Plim                     |                       |                       |
| pressione limite                                            | Plim= 60.175 N/mmq    | Plim= 92.187 N/mmq    |
| pressione massima: (< Plim)                                 | Pmax= 37.245 N/mmq    | Pmax= 35.501 N/mmq    |

**Pressione sulle sottostrutture**

|                                               |                     |                     |
|-----------------------------------------------|---------------------|---------------------|
| Diametro efficace di ripartizione a 60° (< Z) | De= 350 mm          | De= 500 mm          |
| Pressione massima SLU/SLC                     | P'max= 20.047 N/mmq | P'max= 23.565 N/mmq |
| Pressione minima SLU/SLC                      | P'min= 3.951 N/mmq  | P'min= 7.377 N/mmq  |

**Verifiche sismiche secondo OPCM 3274 e s.m. (Allegato 10.A)**

|                                                    |                          |                           |
|----------------------------------------------------|--------------------------|---------------------------|
| Sforzo normale massimo sull'isolatore allo SLC     | V= 614310 N              | V= 2220490 N              |
| Carico orizzontale massimo sismico SLC             | Hs(max)= 106457 N        | Hs(max)= 227515 N         |
| Carico orizzontale sismico a $\gamma = D/te = 1$ : | H( $\gamma=1$ )= 98960 N | H( $\gamma=1$ )= 222660 N |
| Spostamento sismico totale                         | Ds= 81.76 mm (< Dsp)     | Ds= 79.70 mm (< Dsp)      |
| Rotazione massima sismica SLC                      | Rs (max)= 1.346E-02 rad  | Rs (max)= 2.466E-03 rad   |
|                                                    | Fi(de)= 2.58956 rad      | Fi(de)= 2.78549 rad       |
| Area ridotta efficace dell'isolatore               | Ar= 40477 mmq            | Ar= 112644 mmq            |

|                                                            |                    |                    |
|------------------------------------------------------------|--------------------|--------------------|
| Deformazioni di taglio dell'elastomero:                    |                    |                    |
| dovuta alla compressione:                                  | Gamma_c= 0.9292    | Gamma_c= 1.1788    |
| dovuta allo spostamento sismico totale                     | Gamma_s= 1.0758    | Gamma_s= 1.0218    |
| dovuta alle rotazioni                                      | Gamma_alfa= 1.4944 | Gamma_alfa= 0.4001 |
| Deformazione di taglio totale di progetto dell'elastomero: | Gamma_t= 3.4994    | Gamma_t= 2.6007    |
| Carico assiale critico                                     | Vcr= 3914536 N     | Vcr= 16300899 N    |

|                                         |                       |                        |
|-----------------------------------------|-----------------------|------------------------|
| 1. Tensione degli inserti in acciaio    |                       |                        |
| Tensione massimal amiere (<235 N/mmq)   | Sigma_s= 78.920 N/mmq | Sigma_s= 102.505 N/mmq |
| 2. Deformazioni di taglio massime (< 7) | Gamma_s= 1.0758       | Gamma_s= 1.0218        |
|                                         | Gamma_t= 3.4994       | Gamma_t= 2.6007        |
| 3. Instabilità dell'isolatore (Vcr/2.5) | Vmax= 1565815 N       | Vmax= 6520360 N        |

Seguono i tabulati contenenti le reazioni massime e minime per ogni appoggio.

**REAZIONI APPOGGI (ISOLATORI SISMICI ELASTOMERICI)**

**STATI LIMITE ULTIMI - STATICI**

| Tipo isolatore | Struttura Appoggio | No.     | Load                    | Node | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|----------------|--------------------|---------|-------------------------|------|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| 1              | Spalla 1           | 18<br>5 | GEN SLU Mobili(max)     | 77   | -554.03    | 33.22        | 4.54         | 33.53          | 1.302          | 25.75        | 2.29E-02        |
| 1              | Spalla 1           | 18<br>5 | GEN SLU Vento(max)      | 77   | -553.72    | 50.50        | 4.54         | 50.70          | 1.302          | 38.94        | 2.09E-02        |
| 1              | Spalla 1           | 18<br>5 | GEN SLU Frenamento(max) | 77   | -590.15    | 2.84         | 37.89        | 38.00          | 1.302          | 29.18        | 2.09E-02        |
| 1              | Spalla 1           | 18<br>5 | GEN SLU Termico(max)    | 77   | -536.02    | 31.74        | 10.21        | 33.34          | 1.302          | 25.61        | 2.11E-02        |
| 1              | Spalla 1           | 18<br>5 | GEN SLU Mobili(min)     | 77   | -1685.19   | -33.47       | -24.85       | 41.69          | 1.302          | 32.01        | 1.66E-02        |



|   |          |    |   |                         |     |          |         |        |        |       |       |          |
|---|----------|----|---|-------------------------|-----|----------|---------|--------|--------|-------|-------|----------|
| 1 | Spalla 2 | 19 | 2 | GEN SLU Mobili(max)     | 82  | -577.54  | 33.13   | 23.17  | 40.43  | 1.302 | 31.05 | 1.66E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN SLU Vento(max)      | 82  | -572.13  | 50.30   | 20.10  | 54.17  | 1.302 | 41.60 | 1.63E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN SLU Frenamento(max) | 82  | -574.90  | 2.47    | 53.71  | 53.77  | 1.302 | 41.29 | 1.63E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN SLU Termico(max)    | 82  | -537.74  | 31.22   | 25.98  | 40.62  | 1.302 | 31.19 | 1.61E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN SLU Mobili(min)     | 82  | -1927.59 | -32.98  | -3.96  | 33.22  | 1.302 | 25.51 | 2.21E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN SLU Vento(min)      | 82  | -1524.38 | -50.14  | -4.43  | 50.34  | 1.302 | 38.66 | 2.03E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN SLU Frenamento(min) | 82  | -1521.62 | -2.32   | -38.04 | 38.11  | 1.302 | 29.27 | 2.03E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN SLU Termico(min)    | 82  | -1558.78 | -31.06  | -10.32 | 32.73  | 1.302 | 25.14 | 2.06E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Mobili(max)     | 84  | -559.97  | 33.52   | 24.84  | 41.72  | 1.302 | 32.04 | 1.66E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Vento(max)      | 84  | -563.53  | 50.88   | 21.26  | 55.14  | 1.302 | 42.35 | 1.63E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Frenamento(max) | 84  | -590.19  | 3.15    | 54.65  | 54.74  | 1.302 | 42.04 | 1.64E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Termico(max)    | 84  | -541.93  | 32.10   | 26.95  | 41.91  | 1.302 | 32.19 | 1.62E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Mobili(min)     | 84  | -1679.18 | -33.26  | -4.51  | 33.56  | 1.302 | 25.78 | 2.29E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Vento(min)      | 84  | -1357.79 | -50.58  | -4.50  | 50.78  | 1.302 | 39.00 | 2.09E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Frenamento(min) | 84  | -1331.13 | -2.84   | -37.89 | 38.00  | 1.302 | 29.18 | 2.09E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN SLU Termico(min)    | 84  | -1379.39 | -31.79  | -10.18 | 33.38  | 1.302 | 25.64 | 2.11E-02 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Mobili(max)     | 21  | -2073.20 | 74.12   | 32.07  | 80.76  | 2.855 | 28.29 | 7.26E-04 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Vento(max)      | 21  | -2103.32 | 114.19  | 30.30  | 118.14 | 2.855 | 41.39 | 1.22E-03 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Frenamento(max) | 21  | -2225.33 | 7.23    | 103.25 | 103.50 | 2.855 | 36.26 | 1.21E-03 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Termico(max)    | 21  | -2028.15 | 72.63   | 35.17  | 80.70  | 2.855 | 28.27 | 1.17E-03 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Mobili(min)     | 21  | -4345.68 | -67.56  | -5.91  | 67.82  | 2.855 | 23.76 | 5.41E-03 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Vento(min)      | 21  | -3656.09 | -108.71 | -2.16  | 108.73 | 2.855 | 38.09 | 4.81E-03 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Frenamento(min) | 21  | -3534.08 | -1.75   | -75.10 | 75.12  | 2.855 | 26.32 | 4.77E-03 |
| 2 | Pila 1   | 19 | 5 | GEN SLU Termico(min)    | 21  | -3731.26 | -67.15  | -7.02  | 67.52  | 2.855 | 23.65 | 4.92E-03 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Mobili(max)     | 23  | -2508.21 | 70.21   | 30.83  | 76.68  | 2.855 | 26.86 | 9.88E-04 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Vento(max)      | 23  | -2501.91 | 110.96  | 29.17  | 114.73 | 2.855 | 40.19 | 1.37E-03 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Frenamento(max) | 23  | -2516.61 | 3.82    | 102.33 | 102.40 | 2.855 | 35.87 | 1.34E-03 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Termico(max)    | 23  | -2383.81 | 68.45   | 34.11  | 76.48  | 2.855 | 26.79 | 1.30E-03 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Mobili(min)     | 23  | -4623.72 | -68.07  | -3.99  | 68.19  | 2.855 | 23.89 | 5.02E-03 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Vento(min)      | 23  | -3891.56 | -109.37 | -0.76  | 109.37 | 2.855 | 38.31 | 4.49E-03 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Frenamento(min) | 23  | -3876.87 | -2.22   | -73.91 | 73.94  | 2.855 | 25.90 | 4.48E-03 |
| 2 | Pila 1   | 19 | 6 | GEN SLU Termico(min)    | 23  | -4009.67 | -66.85  | -5.70  | 67.09  | 2.855 | 23.50 | 4.57E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Mobili(max)     | 885 | -2505.10 | 68.09   | 30.83  | 74.74  | 2.855 | 26.18 | 1.15E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Vento(max)      | 885 | -2496.73 | 109.39  | 29.17  | 113.21 | 2.855 | 39.66 | 1.45E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Frenamento(max) | 885 | -2516.61 | 2.22    | 102.33 | 102.35 | 2.855 | 35.86 | 1.42E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Termico(max)    | 885 | -2380.70 | 66.87   | 34.11  | 75.07  | 2.855 | 26.30 | 1.39E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Mobili(min)     | 885 | -4626.83 | -70.22  | -4.00  | 70.33  | 2.855 | 24.64 | 4.99E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Vento(min)      | 885 | -3896.75 | -110.99 | -0.76  | 110.99 | 2.855 | 38.88 | 4.46E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Frenamento(min) | 885 | -3876.87 | -3.82   | -73.91 | 74.01  | 2.855 | 25.93 | 4.45E-03 |
| 2 | Pila 1   | 19 | 7 | GEN SLU Termico(min)    | 885 | -4012.78 | -68.46  | -5.70  | 68.70  | 2.855 | 24.07 | 4.54E-03 |
| 2 | Pila 1   | 19 | 8 | GEN SLU Mobili(max)     | 27  | -2081.03 | 67.59   | 32.07  | 74.81  | 2.855 | 26.21 | 1.69E-03 |
| 2 | Pila 1   | 19 | 8 | GEN SLU Vento(max)      | 27  | -2116.37 | 108.76  | 30.30  | 112.90 | 2.855 | 39.55 | 1.78E-03 |
| 2 | Pila 1   | 19 | 8 | GEN SLU Frenamento(max) | 27  | -2225.33 | 1.75    | 103.25 | 103.26 | 2.855 | 36.17 | 1.69E-03 |





STATI LIMITE ULTIMI - SISMICI (SLC)

| Tipo isolatore | Struttura Appoggio | No. | Load               | Node | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|----------------|--------------------|-----|--------------------|------|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| 1              | Spalla 1           | 5   | GEN SLC Long(max)  | 77   | -444.82    | 26.95        | 94.55        | 98.32          | 1.302          | 75.51        | 1.33E-02        |
| 1              | Spalla 1           | 5   | GEN SLC Trasv(max) | 77   | -417.77    | 90.05        | 30.36        | 95.03          | 1.302          | 72.98        | 1.33E-02        |
| 1              | Spalla 1           | 5   | GEN SLC Vert(max)  | 77   | -424.73    | 26.95        | 30.41        | 40.63          | 1.302          | 31.21        | 1.35E-02        |
| 1              | Spalla 1           | 5   | GEN SLC Long(min)  | 77   | -587.25    | -27.81       | -102.76      | 106.46         | 1.302          | 81.76        | 1.25E-02        |
| 1              | Spalla 1           | 5   | GEN SLC Trasv(min) | 77   | -614.31    | -90.91       | -38.57       | 98.75          | 1.302          | 75.84        | 1.25E-02        |
| 1              | Spalla 1           | 5   | GEN SLC Vert(min)  | 77   | -607.34    | -27.82       | -38.62       | 47.60          | 1.302          | 36.55        | 1.23E-02        |
| 1              | Spalla 1           | 6   | GEN SLC Long(max)  | 79   | -395.35    | 27.02        | 94.75        | 98.53          | 1.302          | 75.67        | 1.32E-02        |
| 1              | Spalla 1           | 6   | GEN SLC Trasv(max) | 79   | -401.71    | 90.23        | 30.29        | 95.18          | 1.302          | 73.10        | 1.32E-02        |
| 1              | Spalla 1           | 6   | GEN SLC Vert(max)  | 79   | -380.16    | 27.03        | 30.59        | 40.82          | 1.302          | 31.35        | 1.33E-02        |
| 1              | Spalla 1           | 6   | GEN SLC Long(min)  | 79   | -517.97    | -27.27       | -102.58      | 106.14         | 1.302          | 81.52        | 1.24E-02        |
| 1              | Spalla 1           | 6   | GEN SLC Trasv(min) | 79   | -511.61    | -90.48       | -38.13       | 98.19          | 1.302          | 75.41        | 1.24E-02        |
| 1              | Spalla 1           | 6   | GEN SLC Vert(min)  | 79   | -533.15    | -27.27       | -38.43       | 47.12          | 1.302          | 36.19        | 1.23E-02        |
| 1              | Spalla 1           | 7   | GEN SLC Long(max)  | 81   | -395.36    | 27.27        | 94.75        | 98.60          | 1.302          | 75.72        | 1.32E-02        |
| 1              | Spalla 1           | 7   | GEN SLC Trasv(max) | 81   | -401.72    | 90.48        | 30.29        | 95.42          | 1.302          | 73.28        | 1.32E-02        |
| 1              | Spalla 1           | 7   | GEN SLC Vert(max)  | 81   | -380.19    | 27.27        | 30.59        | 40.98          | 1.302          | 31.47        | 1.33E-02        |
| 1              | Spalla 1           | 7   | GEN SLC Long(min)  | 81   | -517.96    | -27.02       | -102.58      | 106.08         | 1.302          | 81.47        | 1.24E-02        |
| 1              | Spalla 1           | 7   | GEN SLC Trasv(min) | 81   | -511.60    | -90.23       | -38.13       | 97.96          | 1.302          | 75.23        | 1.24E-02        |
| 1              | Spalla 1           | 7   | GEN SLC Vert(min)  | 81   | -533.13    | -27.03       | -38.43       | 46.98          | 1.302          | 36.08        | 1.23E-02        |
| 1              | Spalla 1           | 8   | GEN SLC Long(max)  | 83   | -444.87    | 27.81        | 94.55        | 98.56          | 1.302          | 75.69        | 1.33E-02        |
| 1              | Spalla 1           | 8   | GEN SLC Trasv(max) | 83   | -417.81    | 90.91        | 30.36        | 95.85          | 1.302          | 73.61        | 1.33E-02        |
| 1              | Spalla 1           | 8   | GEN SLC Vert(max)  | 83   | -424.88    | 27.82        | 30.41        | 41.22          | 1.302          | 31.65        | 1.35E-02        |
| 1              | Spalla 1           | 8   | GEN SLC Long(min)  | 83   | -587.21    | -26.95       | -102.76      | 106.24         | 1.302          | 81.59        | 1.25E-02        |
| 1              | Spalla 1           | 8   | GEN SLC Trasv(min) | 83   | -614.26    | -90.05       | -38.57       | 97.96          | 1.302          | 75.23        | 1.25E-02        |
| 1              | Spalla 1           | 8   | GEN SLC Vert(min)  | 83   | -607.19    | -26.95       | -38.62       | 47.09          | 1.302          | 36.17        | 1.23E-02        |
| 1              | Spalla 2           | 0   | GEN SLC Long(max)  | 78   | -444.82    | 26.95        | 102.76       | 106.24         | 1.302          | 81.59        | 1.25E-02        |
| 1              | Spalla 2           | 0   | GEN SLC Trasv(max) | 78   | -417.79    | 90.03        | 38.57        | 97.94          | 1.302          | 75.22        | 1.25E-02        |
| 1              | Spalla 2           | 0   | GEN SLC Vert(max)  | 78   | -424.77    | 26.95        | 38.62        | 47.09          | 1.302          | 36.17        | 1.23E-02        |
| 1              | Spalla 2           | 0   | GEN SLC Long(min)  | 78   | -587.24    | -27.81       | -94.54       | 98.55          | 1.302          | 75.68        | 1.33E-02        |
| 1              | Spalla 2           | 0   | GEN SLC Trasv(min) | 78   | -614.27    | -90.90       | -30.36       | 95.84          | 1.302          | 73.60        | 1.33E-02        |
| 1              | Spalla 2           | 0   | GEN SLC Vert(min)  | 78   | -607.29    | -27.81       | -30.40       | 41.20          | 1.302          | 31.64        | 1.35E-02        |
| 1              | Spalla 2           | 1   | GEN SLC Long(max)  | 80   | -395.34    | 27.02        | 102.59       | 106.09         | 1.302          | 81.47        | 1.24E-02        |
| 1              | Spalla 2           | 1   | GEN SLC Trasv(max) | 80   | -401.70    | 90.22        | 38.13        | 97.95          | 1.302          | 75.22        | 1.24E-02        |
| 1              | Spalla 2           | 1   | GEN SLC Vert(max)  | 80   | -380.16    | 27.02        | 38.43        | 46.98          | 1.302          | 36.08        | 1.23E-02        |
| 1              | Spalla 2           | 1   | GEN SLC Long(min)  | 80   | -517.97    | -27.26       | -94.74       | 98.58          | 1.302          | 75.71        | 1.32E-02        |
| 1              | Spalla 2           | 1   | GEN SLC Trasv(min) | 80   | -511.60    | -90.46       | -30.29       | 95.40          | 1.302          | 73.26        | 1.32E-02        |
| 1              | Spalla 2           | 1   | GEN SLC Vert(min)  | 80   | -533.14    | -27.27       | -30.58       | 40.97          | 1.302          | 31.47        | 1.33E-02        |
| 1              | Spalla 2           | 2   | GEN SLC Long(max)  | 82   | -395.34    | 27.26        | 102.59       | 106.15         | 1.302          | 81.52        | 1.24E-02        |
| 1              | Spalla 2           | 2   | GEN SLC Trasv(max) | 82   | -401.71    | 90.46        | 38.13        | 98.17          | 1.302          | 75.39        | 1.24E-02        |
| 1              | Spalla 2           | 2   | GEN SLC Vert(max)  | 82   | -380.19    | 27.27        | 38.43        | 47.12          | 1.302          | 36.19        | 1.23E-02        |
| 1              | Spalla 2           | 19  | GEN SLC Long(min)  | 82   | -517.97    | -27.02       | -94.74       | 98.52          | 1.302          | 75.66        | 1.32E-02        |

CAVALCAVIA ASSE 80 – IMPALCATO – RELAZIONE DI CALCOLO

|   |          |    |                    |     |          |         |         |        |       |       |          |
|---|----------|----|--------------------|-----|----------|---------|---------|--------|-------|-------|----------|
|   |          | 2  |                    |     |          |         |         |        |       |       |          |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 2  | GEN SLC Trasv(min) | 82  | -511.60  | -90.22  | -30.29  | 95.17  | 1.302 | 73.09 | 1.32E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 2  | GEN SLC Vert(min)  | 82  | -533.12  | -27.02  | -30.58  | 40.81  | 1.302 | 31.34 | 1.33E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 3  | GEN SLC Long(max)  | 84  | -444.86  | 27.81   | 102.76  | 106.46 | 1.302 | 81.76 | 1.25E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 3  | GEN SLC Trasv(max) | 84  | -417.83  | 90.90   | 38.57   | 98.74  | 1.302 | 75.83 | 1.25E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 3  | GEN SLC Vert(max)  | 84  | -424.90  | 27.81   | 38.62   | 47.59  | 1.302 | 36.55 | 1.23E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 3  | GEN SLC Long(min)  | 84  | -587.21  | -26.95  | -94.54  | 98.31  | 1.302 | 75.50 | 1.33E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 3  | GEN SLC Trasv(min) | 84  | -614.24  | -90.03  | -30.36  | 95.01  | 1.302 | 72.97 | 1.33E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 1 | Spalla 2 | 3  | GEN SLC Vert(min)  | 84  | -607.17  | -26.95  | -30.40  | 40.63  | 1.302 | 31.20 | 1.35E-02 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 5  | GEN SLC Long(max)  | 21  | -1575.91 | 63.34   | 218.20  | 227.21 | 2.855 | 79.59 | 2.08E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 5  | GEN SLC Trasv(max) | 21  | -1448.39 | 204.54  | 78.41   | 219.05 | 2.855 | 76.74 | 2.11E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 5  | GEN SLC Vert(max)  | 21  | -1506.63 | 63.38   | 78.21   | 100.67 | 2.855 | 35.26 | 2.11E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 5  | GEN SLC Long(min)  | 21  | -2024.00 | -60.31  | -193.28 | 202.47 | 2.855 | 70.93 | 2.45E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 5  | GEN SLC Trasv(min) | 21  | -2151.51 | -201.52 | -53.49  | 208.50 | 2.855 | 73.04 | 2.47E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 5  | GEN SLC Vert(min)  | 21  | -2093.28 | -60.36  | -53.29  | 80.52  | 2.855 | 28.21 | 2.42E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 6  | GEN SLC Long(max)  | 23  | -1793.63 | 61.43   | 218.04  | 226.53 | 2.855 | 79.35 | 2.06E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 6  | GEN SLC Trasv(max) | 23  | -1783.12 | 202.87  | 77.96   | 217.33 | 2.855 | 76.13 | 2.10E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 6  | GEN SLC Vert(max)  | 23  | -1719.25 | 61.44   | 78.04   | 99.32  | 2.855 | 34.79 | 2.09E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 6  | GEN SLC Long(min)  | 23  | -2146.11 | -60.54  | -193.23 | 202.49 | 2.855 | 70.93 | 2.36E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 6  | GEN SLC Trasv(min) | 23  | -2156.62 | -201.99 | -53.16  | 208.87 | 2.855 | 73.17 | 2.34E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 6  | GEN SLC Vert(min)  | 23  | -2220.49 | -60.55  | -53.23  | 80.62  | 2.855 | 28.24 | 2.33E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 7  | GEN SLC Long(max)  | 885 | -1794.30 | 60.54   | 218.04  | 226.29 | 2.855 | 79.27 | 2.07E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 7  | GEN SLC Trasv(max) | 885 | -1783.40 | 201.99  | 77.96   | 216.51 | 2.855 | 75.85 | 2.12E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 7  | GEN SLC Vert(max)  | 885 | -1719.63 | 60.56   | 78.04   | 98.78  | 2.855 | 34.60 | 2.10E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 7  | GEN SLC Long(min)  | 885 | -2145.44 | -61.43  | -193.23 | 202.76 | 2.855 | 71.03 | 2.36E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 7  | GEN SLC Trasv(min) | 885 | -2156.34 | -202.87 | -53.16  | 209.72 | 2.855 | 73.47 | 2.33E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 7  | GEN SLC Vert(min)  | 885 | -2220.11 | -61.44  | -53.23  | 81.29  | 2.855 | 28.48 | 2.33E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 8  | GEN SLC Long(max)  | 27  | -1577.59 | 60.32   | 218.20  | 226.38 | 2.855 | 79.30 | 2.14E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 8  | GEN SLC Trasv(max) | 27  | -1448.98 | 201.52  | 78.41   | 216.24 | 2.855 | 75.75 | 2.22E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 8  | GEN SLC Vert(max)  | 27  | -1507.45 | 60.36   | 78.21   | 98.79  | 2.855 | 34.61 | 2.17E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 8  | GEN SLC Long(min)  | 27  | -2022.31 | -63.34  | -193.28 | 203.39 | 2.855 | 71.25 | 2.39E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 8  | GEN SLC Trasv(min) | 27  | -2150.93 | -204.54 | -53.49  | 211.42 | 2.855 | 74.06 | 2.37E-03 |
|   |          | 19 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 1   | 8  | GEN SLC Vert(min)  | 27  | -2092.46 | -63.38  | -53.29  | 82.81  | 2.855 | 29.01 | 2.36E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 0  | GEN SLC Long(max)  | 22  | -1577.29 | 63.34   | 193.59  | 203.69 | 2.855 | 71.35 | 2.39E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 0  | GEN SLC Trasv(max) | 22  | -1448.80 | 204.55  | 53.58   | 211.45 | 2.855 | 74.07 | 2.37E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 0  | GEN SLC Vert(max)  | 22  | -1506.93 | 63.38   | 53.39   | 82.87  | 2.855 | 29.03 | 2.36E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 0  | GEN SLC Long(min)  | 22  | -2022.63 | -60.31  | -218.52 | 226.69 | 2.855 | 79.41 | 2.14E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 0  | GEN SLC Trasv(min) | 22  | -2151.13 | -201.52 | -78.52  | 216.28 | 2.855 | 75.76 | 2.22E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 0  | GEN SLC Vert(min)  | 22  | -2093.00 | -60.36  | -78.32  | 98.88  | 2.855 | 34.64 | 2.17E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 1  | GEN SLC Long(max)  | 24  | -1794.17 | 61.43   | 193.54  | 203.06 | 2.855 | 71.13 | 2.35E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 1  | GEN SLC Trasv(max) | 24  | -1783.34 | 202.87  | 53.25   | 209.74 | 2.855 | 73.47 | 2.32E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 1  | GEN SLC Vert(max)  | 24  | -1719.40 | 61.44   | 53.32   | 81.35  | 2.855 | 28.50 | 2.32E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 1  | GEN SLC Long(min)  | 24  | -2145.60 | -60.54  | -218.36 | 226.60 | 2.855 | 79.38 | 2.07E-03 |
|   |          | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2   | 20 | GEN SLC Trasv(min) | 24  | -2156.43 | -201.99 | -78.07  | 216.55 | 2.855 | 75.86 | 2.12E-03 |

|   |        |    |                    |     |          |         |         |        |       |       |          |
|---|--------|----|--------------------|-----|----------|---------|---------|--------|-------|-------|----------|
|   |        | 1  |                    |     |          |         |         |        |       |       |          |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 1  | GEN SLC Vert(min)  | 24  | -2220.37 | -60.55  | -78.14  | 98.85  | 2.855 | 34.63 | 2.10E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 2  | GEN SLC Long(max)  | 886 | -1794.34 | 60.54   | 193.54  | 202.79 | 2.855 | 71.04 | 2.36E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 2  | GEN SLC Trasv(max) | 886 | -1783.38 | 201.99  | 53.25   | 208.89 | 2.855 | 73.18 | 2.34E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 2  | GEN SLC Vert(max)  | 886 | -1719.49 | 60.56   | 53.32   | 80.69  | 2.855 | 28.27 | 2.33E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 2  | GEN SLC Long(min)  | 886 | -2145.44 | -61.43  | -218.36 | 226.84 | 2.855 | 79.46 | 2.06E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 2  | GEN SLC Trasv(min) | 886 | -2156.39 | -202.88 | -78.07  | 217.38 | 2.855 | 76.15 | 2.10E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 2  | GEN SLC Vert(min)  | 886 | -2220.28 | -61.44  | -78.14  | 99.40  | 2.855 | 34.82 | 2.09E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 3  | GEN SLC Long(max)  | 28  | -1577.29 | 60.32   | 193.59  | 202.77 | 2.855 | 71.03 | 2.44E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 3  | GEN SLC Trasv(max) | 28  | -1448.79 | 201.52  | 53.58   | 208.52 | 2.855 | 73.05 | 2.46E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 3  | GEN SLC Vert(max)  | 28  | -1506.96 | 60.36   | 53.39   | 80.58  | 2.855 | 28.23 | 2.42E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 3  | GEN SLC Long(min)  | 28  | -2022.64 | -63.34  | -218.52 | 227.51 | 2.855 | 79.70 | 2.08E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 3  | GEN SLC Trasv(min) | 28  | -2151.14 | -204.55 | -78.52  | 219.10 | 2.855 | 76.75 | 2.11E-03 |
|   |        | 20 |                    |     |          |         |         |        |       |       |          |
| 2 | Pila 2 | 3  | GEN SLC Vert(min)  | 28  | -2092.97 | -63.38  | -78.32  | 100.75 | 2.855 | 35.29 | 2.11E-03 |

|                        |        |  |  | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|------------------------|--------|--|--|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| <b>SFORZI MASSIMI:</b> | Spalle |  |  | -380.16    | 90.91        | 102.76       | 106.46         | 1.302          | 81.76        | 1.35E-02        |
|                        | Pile   |  |  | -1448.39   | 204.55       | 218.20       | 227.51         | 2.855          | 79.70        | 2.47E-03        |
| <b>SFORZI MINIMI:</b>  | Spalle |  |  | -614.31    | -90.91       | -102.76      |                |                |              |                 |
|                        | Pile   |  |  | -2220.49   | -204.55      | -218.52      |                |                |              |                 |

**STATI LIMITE ULTIMI - SISMICI (SLV)**

| Tipo isolatore | Struttura Appoggio | No. | Load               | Node | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|----------------|--------------------|-----|--------------------|------|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| 1              | Spalla 1           | 5   | GEN SLV Long(max)  | 77   | -453.71    | 20.96        | 74.05        | 76.96          | 1.302          | 59.10        | 1.32E-02        |
| 1              | Spalla 1           | 5   | GEN SLV Trasv(max) | 77   | -432.64    | 70.10        | 24.06        | 74.11          | 1.302          | 56.92        | 1.33E-02        |
| 1              | Spalla 1           | 5   | GEN SLV Vert(max)  | 77   | -440.12    | 20.97        | 24.07        | 31.92          | 1.302          | 24.52        | 1.34E-02        |
| 1              | Spalla 1           | 5   | GEN SLV Long(min)  | 77   | -578.37    | -21.82       | -82.26       | 85.10          | 1.302          | 65.36        | 1.25E-02        |
| 1              | Spalla 1           | 5   | GEN SLV Trasv(min) | 77   | -599.43    | -70.96       | -32.27       | 77.95          | 1.302          | 59.87        | 1.25E-02        |
| 1              | Spalla 1           | 5   | GEN SLV Vert(min)  | 77   | -591.96    | -21.83       | -32.27       | 38.96          | 1.302          | 29.92        | 1.24E-02        |
| 1              | Spalla 1           | 6   | GEN SLV Long(max)  | 79   | -401.11    | 21.03        | 74.27        | 77.19          | 1.302          | 59.28        | 1.31E-02        |
| 1              | Spalla 1           | 6   | GEN SLV Trasv(max) | 79   | -406.06    | 70.25        | 24.08        | 74.26          | 1.302          | 57.03        | 1.31E-02        |
| 1              | Spalla 1           | 6   | GEN SLV Vert(max)  | 79   | -391.06    | 21.03        | 24.28        | 32.12          | 1.302          | 24.67        | 1.33E-02        |
| 1              | Spalla 1           | 6   | GEN SLV Long(min)  | 79   | -512.21    | -21.27       | -82.11       | 84.82          | 1.302          | 65.14        | 1.25E-02        |
| 1              | Spalla 1           | 6   | GEN SLV Trasv(min) | 79   | -507.25    | -70.49       | -31.91       | 77.38          | 1.302          | 59.42        | 1.25E-02        |
| 1              | Spalla 1           | 6   | GEN SLV Vert(min)  | 79   | -522.26    | -21.27       | -32.11       | 38.52          | 1.302          | 29.58        | 1.23E-02        |
| 1              | Spalla 1           | 7   | GEN SLV Long(max)  | 81   | -401.12    | 21.27        | 74.27        | 77.26          | 1.302          | 59.33        | 1.31E-02        |
| 1              | Spalla 1           | 7   | GEN SLV Trasv(max) | 81   | -406.07    | 70.49        | 24.08        | 74.49          | 1.302          | 57.21        | 1.31E-02        |
| 1              | Spalla 1           | 7   | GEN SLV Vert(max)  | 81   | -391.08    | 21.27        | 24.28        | 32.28          | 1.302          | 24.79        | 1.33E-02        |
| 1              | Spalla 1           | 7   | GEN SLV Long(min)  | 81   | -512.20    | -21.03       | -82.11       | 84.76          | 1.302          | 65.09        | 1.25E-02        |
| 1              | Spalla 1           | 7   | GEN SLV Trasv(min) | 81   | -507.25    | -70.25       | -31.91       | 77.16          | 1.302          | 59.26        | 1.25E-02        |
| 1              | Spalla 1           | 7   | GEN SLV Vert(min)  | 81   | -522.24    | -21.03       | -32.11       | 38.38          | 1.302          | 29.48        | 1.23E-02        |
| 1              | Spalla 1           | 8   | GEN SLV Long(max)  | 83   | -453.74    | 21.82        | 74.05        | 77.20          | 1.302          | 59.29        | 1.32E-02        |
| 1              | Spalla 1           | 8   | GEN SLV Trasv(max) | 83   | -432.67    | 70.96        | 24.06        | 74.93          | 1.302          | 57.54        | 1.33E-02        |
| 1              | Spalla 1           | 8   | GEN SLV Vert(max)  | 83   | -440.22    | 21.83        | 24.07        | 32.49          | 1.302          | 24.96        | 1.34E-02        |

|   |          |    |                    |     |          |         |         |        |       |       |          |
|---|----------|----|--------------------|-----|----------|---------|---------|--------|-------|-------|----------|
| 1 | Spalla 1 | 18 |                    |     |          |         |         |        |       |       |          |
|   |          | 8  | GEN SLV Long(min)  | 83  | -578.34  | -20.96  | -82.26  | 84.89  | 1.302 | 65.19 | 1.25E-02 |
| 1 | Spalla 1 | 18 |                    |     |          |         |         |        |       |       |          |
|   |          | 8  | GEN SLV Trasv(min) | 83  | -599.40  | -70.10  | -32.27  | 77.17  | 1.302 | 59.27 | 1.25E-02 |
| 1 | Spalla 1 | 18 |                    |     |          |         |         |        |       |       |          |
|   |          | 8  | GEN SLV Vert(min)  | 83  | -591.85  | -20.97  | -32.27  | 38.48  | 1.302 | 29.56 | 1.24E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 0  | GEN SLV Long(max)  | 78  | -453.70  | 20.96   | 82.26   | 84.89  | 1.302 | 65.19 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 0  | GEN SLV Trasv(max) | 78  | -432.66  | 70.09   | 32.27   | 77.16  | 1.302 | 59.26 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 0  | GEN SLV Vert(max)  | 78  | -440.14  | 20.96   | 32.28   | 38.49  | 1.302 | 29.56 | 1.24E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 0  | GEN SLV Long(min)  | 78  | -578.36  | -21.82  | -74.04  | 77.19  | 1.302 | 59.28 | 1.32E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 0  | GEN SLV Trasv(min) | 78  | -599.41  | -70.95  | -24.06  | 74.92  | 1.302 | 57.54 | 1.33E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 0  | GEN SLV Vert(min)  | 78  | -591.92  | -21.82  | -24.06  | 32.48  | 1.302 | 24.94 | 1.34E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 1  | GEN SLV Long(max)  | 80  | -401.10  | 21.03   | 82.11   | 84.76  | 1.302 | 65.09 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 1  | GEN SLV Trasv(max) | 80  | -406.06  | 70.24   | 31.92   | 77.15  | 1.302 | 59.25 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 1  | GEN SLV Vert(max)  | 80  | -391.06  | 21.03   | 32.12   | 38.39  | 1.302 | 29.48 | 1.23E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 1  | GEN SLV Long(min)  | 80  | -512.21  | -21.27  | -74.26  | 77.25  | 1.302 | 59.32 | 1.31E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 1  | GEN SLV Trasv(min) | 80  | -507.25  | -70.48  | -24.07  | 74.48  | 1.302 | 57.20 | 1.31E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 1  | GEN SLV Vert(min)  | 80  | -522.25  | -21.27  | -24.27  | 32.27  | 1.302 | 24.78 | 1.32E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 2  | GEN SLV Long(max)  | 82  | -401.10  | 21.27   | 82.11   | 84.82  | 1.302 | 65.14 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 2  | GEN SLV Trasv(max) | 82  | -406.06  | 70.48   | 31.92   | 77.37  | 1.302 | 59.42 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 2  | GEN SLV Vert(max)  | 82  | -391.08  | 21.27   | 32.12   | 38.52  | 1.302 | 29.59 | 1.23E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 2  | GEN SLV Long(min)  | 82  | -512.21  | -21.03  | -74.26  | 77.18  | 1.302 | 59.27 | 1.31E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 2  | GEN SLV Trasv(min) | 82  | -507.24  | -70.24  | -24.07  | 74.25  | 1.302 | 57.02 | 1.31E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 2  | GEN SLV Vert(min)  | 82  | -522.23  | -21.03  | -24.27  | 32.11  | 1.302 | 24.66 | 1.32E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 3  | GEN SLV Long(max)  | 84  | -453.73  | 21.82   | 82.26   | 85.10  | 1.302 | 65.36 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 3  | GEN SLV Trasv(max) | 84  | -432.68  | 70.95   | 32.27   | 77.94  | 1.302 | 59.86 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 3  | GEN SLV Vert(max)  | 84  | -440.23  | 21.82   | 32.28   | 38.96  | 1.302 | 29.92 | 1.24E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 3  | GEN SLV Long(min)  | 84  | -578.34  | -20.96  | -74.04  | 76.95  | 1.302 | 59.10 | 1.32E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 3  | GEN SLV Trasv(min) | 84  | -599.38  | -70.09  | -24.06  | 74.10  | 1.302 | 56.91 | 1.33E-02 |
| 1 | Spalla 2 | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 3  | GEN SLV Vert(min)  | 84  | -591.83  | -20.96  | -24.06  | 31.91  | 1.302 | 24.51 | 1.34E-02 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 5  | GEN SLV Long(max)  | 21  | -1599.17 | 49.94   | 173.82  | 180.85 | 2.855 | 63.35 | 2.09E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 5  | GEN SLV Trasv(max) | 21  | -1499.91 | 159.90  | 64.96   | 172.59 | 2.855 | 60.46 | 2.12E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 5  | GEN SLV Vert(max)  | 21  | -1550.84 | 49.97   | 64.79   | 81.82  | 2.855 | 28.66 | 2.12E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 5  | GEN SLV Long(min)  | 21  | -2000.74 | -46.92  | -148.90 | 156.12 | 2.855 | 54.69 | 2.43E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 5  | GEN SLV Trasv(min) | 21  | -2099.99 | -156.88 | -40.04  | 161.91 | 2.855 | 56.72 | 2.44E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 5  | GEN SLV Vert(min)  | 21  | -2049.06 | -46.95  | -39.87  | 61.59  | 2.855 | 21.58 | 2.40E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 6  | GEN SLV Long(max)  | 23  | -1806.46 | 48.01   | 173.68  | 180.19 | 2.855 | 63.12 | 2.07E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 6  | GEN SLV Trasv(max) | 23  | -1798.31 | 158.16  | 64.60   | 170.84 | 2.855 | 59.85 | 2.11E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 6  | GEN SLV Vert(max)  | 23  | -1754.54 | 48.02   | 64.64   | 80.52  | 2.855 | 28.21 | 2.10E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 6  | GEN SLV Long(min)  | 23  | -2133.28 | -47.13  | -148.87 | 156.15 | 2.855 | 54.70 | 2.35E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 6  | GEN SLV Trasv(min) | 23  | -2141.43 | -157.28 | -39.79  | 162.24 | 2.855 | 56.83 | 2.33E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 6  | GEN SLV Vert(min)  | 23  | -2185.20 | -47.14  | -39.83  | 61.71  | 2.855 | 21.62 | 2.32E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 7  | GEN SLV Long(max)  | 885 | -1806.99 | 47.13   | 173.68  | 179.96 | 2.855 | 63.04 | 2.08E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 7  | GEN SLV Trasv(max) | 885 | -1798.52 | 157.28  | 64.60   | 170.03 | 2.855 | 59.56 | 2.12E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 7  | GEN SLV Vert(max)  | 885 | -1754.83 | 47.14   | 64.64   | 80.00  | 2.855 | 28.03 | 2.10E-03 |
| 2 | Pila 1   | 19 |                    |     |          |         |         |        |       |       |          |
|   |          | 7  | GEN SLV Long(min)  | 885 | -2132.74 | -48.01  | -148.87 | 156.42 | 2.855 | 54.80 | 2.34E-03 |

|   |        |    |                    |     |          |         |         |        |       |       |          |  |
|---|--------|----|--------------------|-----|----------|---------|---------|--------|-------|-------|----------|--|
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 7  | GEN SLV Trasv(min) | 885 | -2141.21 | -158.16 | -39.79  | 163.09 | 2.855 | 57.13 | 2.32E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 7  | GEN SLV Vert(min)  | 885 | -2184.91 | -48.02  | -39.83  | 62.39  | 2.855 | 21.86 | 2.32E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 8  | GEN SLV Long(max)  | 27  | -1600.50 | 46.92   | 173.82  | 180.04 | 2.855 | 63.07 | 2.16E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 8  | GEN SLV Trasv(max) | 27  | -1500.37 | 156.88  | 64.96   | 169.80 | 2.855 | 59.48 | 2.21E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 8  | GEN SLV Vert(max)  | 27  | -1551.46 | 46.95   | 64.79   | 80.01  | 2.855 | 28.03 | 2.18E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 8  | GEN SLV Long(min)  | 27  | -1999.41 | -49.94  | -148.90 | 157.05 | 2.855 | 55.02 | 2.37E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 8  | GEN SLV Trasv(min) | 27  | -2099.54 | -159.90 | -40.04  | 164.84 | 2.855 | 57.74 | 2.36E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 8  | GEN SLV Vert(min)  | 27  | -2048.44 | -49.97  | -39.87  | 63.93  | 2.855 | 22.39 | 2.35E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 0  | GEN SLV Long(max)  | 22  | -1600.28 | 49.94   | 149.14  | 157.28 | 2.855 | 55.10 | 2.37E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 0  | GEN SLV Trasv(max) | 22  | -1500.24 | 159.90  | 40.11   | 164.85 | 2.855 | 57.75 | 2.35E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 0  | GEN SLV Vert(max)  | 22  | -1551.10 | 49.97   | 39.94   | 63.97  | 2.855 | 22.41 | 2.35E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 0  | GEN SLV Long(min)  | 22  | -1999.65 | -46.92  | -174.07 | 180.28 | 2.855 | 63.15 | 2.16E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 0  | GEN SLV Trasv(min) | 22  | -2099.69 | -156.88 | -65.05  | 169.83 | 2.855 | 59.49 | 2.21E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 0  | GEN SLV Vert(min)  | 22  | -2048.83 | -46.95  | -64.88  | 80.09  | 2.855 | 28.05 | 2.18E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 1  | GEN SLV Long(max)  | 24  | -1806.90 | 48.01   | 149.12  | 156.66 | 2.855 | 54.88 | 2.34E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 1  | GEN SLV Trasv(max) | 24  | -1798.49 | 158.16  | 39.86   | 163.11 | 2.855 | 57.14 | 2.31E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 1  | GEN SLV Vert(max)  | 24  | -1754.67 | 48.02   | 39.91   | 62.44  | 2.855 | 21.87 | 2.31E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 1  | GEN SLV Long(min)  | 24  | -2132.87 | -47.13  | -173.93 | 180.20 | 2.855 | 63.13 | 2.08E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 1  | GEN SLV Trasv(min) | 24  | -2141.28 | -157.28 | -64.68  | 170.06 | 2.855 | 59.57 | 2.12E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 1  | GEN SLV Vert(min)  | 24  | -2185.10 | -47.14  | -64.72  | 80.07  | 2.855 | 28.05 | 2.10E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 2  | GEN SLV Long(max)  | 886 | -1807.03 | 47.13   | 149.12  | 156.39 | 2.855 | 54.79 | 2.34E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 2  | GEN SLV Trasv(max) | 886 | -1798.52 | 157.28  | 39.86   | 162.25 | 2.855 | 56.84 | 2.33E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 2  | GEN SLV Vert(max)  | 886 | -1754.73 | 47.14   | 39.91   | 61.77  | 2.855 | 21.64 | 2.32E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 2  | GEN SLV Long(min)  | 886 | -2132.74 | -48.01  | -173.93 | 180.43 | 2.855 | 63.21 | 2.08E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 2  | GEN SLV Trasv(min) | 886 | -2141.25 | -158.16 | -64.68  | 170.87 | 2.855 | 59.86 | 2.11E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 2  | GEN SLV Vert(min)  | 886 | -2185.04 | -48.02  | -64.72  | 80.59  | 2.855 | 28.23 | 2.10E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 3  | GEN SLV Long(max)  | 28  | -1600.27 | 46.92   | 149.14  | 156.35 | 2.855 | 54.77 | 2.42E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 3  | GEN SLV Trasv(max) | 28  | -1500.23 | 156.88  | 40.11   | 161.93 | 2.855 | 56.72 | 2.44E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 3  | GEN SLV Vert(max)  | 28  | -1551.12 | 46.95   | 39.94   | 61.64  | 2.855 | 21.59 | 2.40E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 3  | GEN SLV Long(min)  | 28  | -1999.66 | -49.94  | -174.07 | 181.09 | 2.855 | 63.44 | 2.09E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 3  | GEN SLV Trasv(min) | 28  | -2099.70 | -159.90 | -65.05  | 172.63 | 2.855 | 60.47 | 2.12E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |         |         |        |       |       |          |  |
|   |        | 3  | GEN SLV Vert(min)  | 28  | -2048.81 | -49.97  | -64.88  | 81.89  | 2.855 | 28.69 | 2.12E-03 |  |

|                        |        |  |  |  | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|------------------------|--------|--|--|--|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| <b>SFORZI MASSIMI:</b> |        |  |  |  |            |              |              |                |                |              |                 |
|                        | Spalle |  |  |  | -391.06    | 70.96        | 82.26        | 85.10          | 1.302          | 65.36        | 1.34E-02        |
|                        | Pile   |  |  |  | -1499.91   | 159.90       | 173.82       | 181.09         | 2.855          | 63.44        | 2.44E-03        |
| <b>SFORZI MINIMI:</b>  |        |  |  |  |            |              |              |                |                |              |                 |
|                        | Spalle |  |  |  | -599.43    | -70.96       | -82.26       |                |                |              |                 |
|                        | Pile   |  |  |  | -2185.20   | -159.90      | -174.07      |                |                |              |                 |

STATI LIMITE DI ESERCIZIO - COMBINAZIONI CARATTERISTICHE (RARE)

| Tipo isolatore | Struttura Appoggio | No. | Load                   | Node | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|----------------|--------------------|-----|------------------------|------|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| 1              | Spalla 1           | 18  | 5 GEN RARA Mobili(max) | 77   | -383.51    | 22.55        | 4.89         | 23.07          | 1.302          | 17.72        | 1.72E-02        |
| 1              | Spalla 1           | 18  | 5 GEN RARA Vento(max)  | 77   | -384.46    | 33.94        | 4.87         | 34.29          | 1.302          | 26.33        | 1.57E-02        |



CAVALCAVIA ASSE 80 – IMPALCATO – RELAZIONE DI CALCOLO

|   |          |    |                       |    |          |        |        |       |       |       |          |
|---|----------|----|-----------------------|----|----------|--------|--------|-------|-------|-------|----------|
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 5  | GEN RARA Frenam(max)  | 77 | -408.50  | 2.17   | 29.60  | 29.68 | 1.302 | 22.79 | 1.57E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 5  | GEN RARA Termico(max) | 77 | -367.06  | 21.49  | 9.62   | 23.54 | 1.302 | 18.08 | 1.59E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 5  | GEN RARA Mobili(min)  | 77 | -1227.21 | -22.70 | -18.47 | 29.26 | 1.302 | 22.47 | 1.24E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 5  | GEN RARA Vento(min)   | 77 | -990.81  | -34.12 | -15.82 | 37.61 | 1.302 | 28.88 | 1.23E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 5  | GEN RARA Frenam(min)  | 77 | -966.76  | -2.35  | -40.56 | 40.63 | 1.302 | 31.20 | 1.23E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 5  | GEN RARA Termico(min) | 77 | -1008.20 | -21.67 | -20.58 | 29.89 | 1.302 | 22.95 | 1.21E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Mobili(max)  | 79 | -406.53  | 22.31  | 4.46   | 22.75 | 1.302 | 17.47 | 1.66E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Vento(max)   | 79 | -402.32  | 33.61  | 4.81   | 33.95 | 1.302 | 26.07 | 1.53E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Frenam(max)  | 79 | -404.82  | 1.73   | 29.72  | 29.77 | 1.302 | 22.86 | 1.53E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Termico(max) | 79 | -373.58  | 20.90  | 9.73   | 23.05 | 1.302 | 17.71 | 1.55E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Mobili(min)  | 79 | -1417.65 | -22.41 | -17.26 | 28.29 | 1.302 | 21.72 | 1.25E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Vento(min)   | 79 | -1119.06 | -33.71 | -14.99 | 36.89 | 1.302 | 28.33 | 1.23E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Frenam(min)  | 79 | -1116.56 | -1.83  | -39.89 | 39.93 | 1.302 | 30.67 | 1.23E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 6  | GEN RARA Termico(min) | 79 | -1147.80 | -21.00 | -19.90 | 28.93 | 1.302 | 22.22 | 1.21E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Mobili(max)  | 81 | -407.10  | 22.41  | 4.46   | 22.85 | 1.302 | 17.55 | 1.66E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Vento(max)   | 81 | -403.27  | 33.72  | 4.81   | 34.06 | 1.302 | 26.16 | 1.53E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Frenam(max)  | 81 | -404.82  | 1.83   | 29.72  | 29.78 | 1.302 | 22.87 | 1.53E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Termico(max) | 81 | -374.15  | 21.00  | 9.73   | 23.14 | 1.302 | 17.77 | 1.55E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Mobili(min)  | 81 | -1417.08 | -22.31 | -17.27 | 28.21 | 1.302 | 21.67 | 1.25E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Vento(min)   | 81 | -1118.11 | -33.61 | -14.99 | 36.80 | 1.302 | 28.26 | 1.23E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Frenam(min)  | 81 | -1116.56 | -1.73  | -39.89 | 39.93 | 1.302 | 30.66 | 1.23E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 7  | GEN RARA Termico(min) | 81 | -1147.23 | -20.90 | -19.91 | 28.87 | 1.302 | 22.17 | 1.21E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Mobili(max)  | 83 | -387.43  | 22.73  | 4.87   | 23.25 | 1.302 | 17.85 | 1.73E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Vento(max)   | 83 | -390.99  | 34.18  | 4.85   | 34.52 | 1.302 | 26.51 | 1.58E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Frenam(max)  | 83 | -408.50  | 2.35   | 29.60  | 29.69 | 1.302 | 22.80 | 1.57E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Termico(max) | 83 | -370.98  | 21.70  | 9.61   | 23.73 | 1.302 | 18.23 | 1.60E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Mobili(min)  | 83 | -1223.29 | -22.58 | -18.46 | 29.17 | 1.302 | 22.40 | 1.23E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Vento(min)   | 83 | -984.28  | -34.00 | -15.80 | 37.49 | 1.302 | 28.79 | 1.22E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Frenam(min)  | 83 | -966.76  | -2.17  | -40.56 | 40.62 | 1.302 | 31.19 | 1.22E-02 |
| 1 | Spalla 1 | 18 |                       |    |          |        |        |       |       |       |          |
|   |          | 8  | GEN RARA Termico(min) | 83 | -1004.29 | -21.52 | -20.56 | 29.76 | 1.302 | 22.86 | 1.20E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Mobili(max)  | 78 | -383.57  | 22.55  | 18.48  | 29.15 | 1.302 | 22.39 | 1.23E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Vento(max)   | 78 | -384.49  | 33.94  | 15.83  | 37.45 | 1.302 | 28.76 | 1.22E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Frenam(max)  | 78 | -408.53  | 2.17   | 40.56  | 40.62 | 1.302 | 31.19 | 1.22E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Termico(max) | 78 | -367.09  | 21.49  | 20.58  | 29.75 | 1.302 | 22.85 | 1.20E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Mobili(min)  | 78 | -1227.10 | -22.70 | -4.88  | 23.22 | 1.302 | 17.83 | 1.73E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Vento(min)   | 78 | -990.76  | -34.12 | -4.87  | 34.47 | 1.302 | 26.47 | 1.58E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Frenam(min)  | 78 | -966.71  | -2.35  | -29.60 | 29.69 | 1.302 | 22.80 | 1.57E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 0  | GEN RARA Termico(min) | 78 | -1008.15 | -21.67 | -9.62  | 23.71 | 1.302 | 18.21 | 1.60E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 1  | GEN RARA Mobili(max)  | 80 | -406.53  | 22.31  | 17.27  | 28.21 | 1.302 | 21.67 | 1.25E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 1  | GEN RARA Vento(max)   | 80 | -402.35  | 33.60  | 14.99  | 36.79 | 1.302 | 28.26 | 1.23E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 1  | GEN RARA Frenam(max)  | 80 | -404.84  | 1.73   | 39.90  | 39.94 | 1.302 | 30.67 | 1.23E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 1  | GEN RARA Termico(max) | 80 | -373.61  | 20.90  | 19.91  | 28.87 | 1.302 | 22.17 | 1.21E-02 |
| 1 | Spalla 2 | 19 |                       |    |          |        |        |       |       |       |          |
|   |          | 1  | GEN RARA Mobili(min)  | 80 | -1417.41 | -22.41 | -4.45  | 22.85 | 1.302 | 17.55 | 1.66E-02 |



CAVALCAVIA ASSE 80 – IMPALCATO – RELAZIONE DI CALCOLO

|   |          |    |   |                       |     |          |        |        |       |       |       |          |
|---|----------|----|---|-----------------------|-----|----------|--------|--------|-------|-------|-------|----------|
| 1 | Spalla 2 | 19 | 1 | GEN RARA Vento(min)   | 80  | -1118.91 | -33.71 | -4.80  | 34.05 | 1.302 | 26.15 | 1.53E-02 |
| 1 | Spalla 2 | 19 | 1 | GEN RARA Frenam(min)  | 80  | -1116.43 | -1.83  | -29.71 | 29.77 | 1.302 | 22.86 | 1.53E-02 |
| 1 | Spalla 2 | 19 | 1 | GEN RARA Termico(min) | 80  | -1147.66 | -21.00 | -9.72  | 23.14 | 1.302 | 17.77 | 1.55E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Mobili(max)  | 82  | -407.10  | 22.41  | 17.27  | 28.29 | 1.302 | 21.73 | 1.24E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Vento(max)   | 82  | -403.30  | 33.72  | 15.00  | 36.91 | 1.302 | 28.34 | 1.23E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Frenam(max)  | 82  | -404.84  | 1.83   | 39.90  | 39.94 | 1.302 | 30.67 | 1.23E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Termico(max) | 82  | -374.18  | 21.00  | 19.91  | 28.94 | 1.302 | 22.22 | 1.21E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Mobili(min)  | 82  | -1416.84 | -22.32 | -4.46  | 22.76 | 1.302 | 17.48 | 1.66E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Vento(min)   | 82  | -1117.97 | -33.61 | -4.81  | 33.95 | 1.302 | 26.07 | 1.53E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Frenam(min)  | 82  | -1116.43 | -1.73  | -29.71 | 29.76 | 1.302 | 22.86 | 1.53E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN RARA Termico(min) | 82  | -1147.09 | -20.90 | -9.72  | 23.05 | 1.302 | 17.70 | 1.55E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Mobili(max)  | 84  | -387.48  | 22.73  | 18.46  | 29.28 | 1.302 | 22.49 | 1.24E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Vento(max)   | 84  | -391.01  | 34.17  | 15.81  | 37.65 | 1.302 | 28.91 | 1.23E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Frenam(max)  | 84  | -408.53  | 2.35   | 40.56  | 40.63 | 1.302 | 31.20 | 1.23E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Termico(max) | 84  | -371.01  | 21.70  | 20.56  | 29.89 | 1.302 | 22.96 | 1.21E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Mobili(min)  | 84  | -1223.19 | -22.58 | -4.86  | 23.10 | 1.302 | 17.74 | 1.72E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Vento(min)   | 84  | -984.24  | -33.99 | -4.84  | 34.33 | 1.302 | 26.37 | 1.57E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Frenam(min)  | 84  | -966.71  | -2.17  | -29.60 | 29.68 | 1.302 | 22.79 | 1.57E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN RARA Termico(min) | 84  | -1004.24 | -21.52 | -9.60  | 23.56 | 1.302 | 18.10 | 1.59E-02 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Mobili(max)  | 21  | -1471.04 | 50.36  | 25.44  | 56.42 | 2.855 | 19.76 | 5.98E-04 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Vento(max)   | 21  | -1496.98 | 76.88  | 24.12  | 80.57 | 2.855 | 28.23 | 9.60E-04 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Frenam(max)  | 21  | -1578.27 | 5.57   | 78.17  | 78.37 | 2.855 | 27.45 | 9.52E-04 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Termico(max) | 21  | -1426.16 | 49.38  | 28.19  | 56.86 | 2.855 | 19.92 | 9.29E-04 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Mobili(min)  | 21  | -3177.96 | -45.42 | -4.05  | 45.60 | 2.855 | 15.97 | 4.09E-03 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Vento(min)   | 21  | -2663.52 | -72.73 | -1.26  | 72.74 | 2.855 | 25.48 | 3.64E-03 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Frenam(min)  | 21  | -2582.23 | -1.42  | -55.32 | 55.34 | 2.855 | 19.39 | 3.62E-03 |
| 2 | Pila 1   | 19 | 5 | GEN RARA Termico(min) | 21  | -2734.34 | -45.23 | -5.34  | 45.54 | 2.855 | 15.95 | 3.74E-03 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Mobili(max)  | 23  | -1808.07 | 47.31  | 24.53  | 53.29 | 2.855 | 18.67 | 7.83E-04 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Vento(max)   | 23  | -1803.88 | 74.32  | 23.29  | 77.88 | 2.855 | 27.28 | 1.07E-03 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Frenam(max)  | 23  | -1813.60 | 2.89   | 77.49  | 77.54 | 2.855 | 27.16 | 1.05E-03 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Termico(max) | 23  | -1704.41 | 46.04  | 27.42  | 53.59 | 2.855 | 18.77 | 1.01E-03 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Mobili(min)  | 23  | -3408.28 | -45.69 | -2.65  | 45.77 | 2.855 | 16.03 | 3.80E-03 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Vento(min)   | 23  | -2865.47 | -73.11 | -0.25  | 73.11 | 2.855 | 25.61 | 3.40E-03 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Frenam(min)  | 23  | -2855.75 | -1.68  | -54.44 | 54.47 | 2.855 | 19.08 | 3.39E-03 |
| 2 | Pila 1   | 19 | 6 | GEN RARA Termico(min) | 23  | -2964.94 | -44.82 | -4.37  | 45.03 | 2.855 | 15.78 | 3.47E-03 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Mobili(max)  | 885 | -1806.00 | 45.70  | 24.53  | 51.87 | 2.855 | 18.17 | 9.02E-04 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Vento(max)   | 885 | -1800.42 | 73.12  | 23.29  | 76.74 | 2.855 | 26.88 | 1.13E-03 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Frenam(max)  | 885 | -1813.60 | 1.68   | 77.49  | 77.51 | 2.855 | 27.15 | 1.11E-03 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Termico(max) | 885 | -1702.33 | 44.83  | 27.42  | 52.55 | 2.855 | 18.41 | 1.08E-03 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Mobili(min)  | 885 | -3410.35 | -47.32 | -2.65  | 47.39 | 2.855 | 16.60 | 3.77E-03 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Vento(min)   | 885 | -2868.93 | -74.34 | -0.25  | 74.34 | 2.855 | 26.04 | 3.38E-03 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Frenam(min)  | 885 | -2855.75 | -2.89  | -54.44 | 54.52 | 2.855 | 19.10 | 3.38E-03 |
| 2 | Pila 1   | 19 | 7 | GEN RARA Termico(min) | 885 | -2967.01 | -46.05 | -4.37  | 46.26 | 2.855 | 16.20 | 3.45E-03 |



|                       |               |                 |               |               |              |              |              |                 |
|-----------------------|---------------|-----------------|---------------|---------------|--------------|--------------|--------------|-----------------|
|                       | <b>Pile</b>   | <b>-1426.14</b> | <b>76.88</b>  | <b>78.17</b>  | <b>80.62</b> | <b>2.855</b> | <b>28.24</b> | <b>4.09E-03</b> |
| <b>SFORZI MINIMI:</b> | <b>Spalle</b> | <b>-1417.65</b> | <b>-34.12</b> | <b>-40.56</b> |              |              |              |                 |
|                       | <b>Pile</b>   | <b>-3410.39</b> | <b>-76.92</b> | <b>-78.27</b> |              |              |              |                 |

**STATI LIMITE DI ESERCIZIO - COMBINAZIONI FREQUENTI**

| Tipo isolatore | Struttura Appoggio | No. | Load                  | Node | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|----------------|--------------------|-----|-----------------------|------|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| 1              | Spalla 1           | 5   | GEN FREQ Mobili(max)  | 77   | -417.98    | 2.10         | 3.42         | 4.01           | 1.302          | 3.08         | 1.56E-02        |
| 1              | Spalla 1           | 5   | GEN FREQ Vento(max)   | 77   | -475.73    | 6.25         | 2.01         | 6.57           | 1.302          | 5.04         | 1.31E-02        |
| 1              | Spalla 1           | 5   | GEN FREQ Termico(max) | 77   | -474.03    | -0.05        | 3.17         | 3.17           | 1.302          | 2.43         | 1.32E-02        |
| 1              | Spalla 1           | 5   | GEN FREQ Mobili(min)  | 77   | -957.29    | -2.28        | -14.37       | 14.55          | 1.302          | 11.17        | 1.24E-02        |
| 1              | Spalla 1           | 5   | GEN FREQ Vento(min)   | 77   | -556.35    | -7.11        | -10.21       | 12.44          | 1.302          | 9.56         | 1.26E-02        |
| 1              | Spalla 1           | 5   | GEN FREQ Termico(min) | 77   | -558.05    | -0.82        | -11.37       | 11.40          | 1.302          | 8.75         | 1.26E-02        |
| 1              | Spalla 1           | 6   | GEN FREQ Mobili(max)  | 79   | -415.58    | 1.72         | 3.50         | 3.90           | 1.302          | 2.99         | 1.52E-02        |
| 1              | Spalla 1           | 6   | GEN FREQ Vento(max)   | 79   | -416.87    | 6.31         | 2.28         | 6.71           | 1.302          | 5.15         | 1.30E-02        |
| 1              | Spalla 1           | 6   | GEN FREQ Termico(max) | 79   | -410.23    | -0.06        | 3.50         | 3.50           | 1.302          | 2.69         | 1.31E-02        |
| 1              | Spalla 1           | 6   | GEN FREQ Mobili(min)  | 79   | -1105.80   | -1.82        | -13.68       | 13.80          | 1.302          | 10.60        | 1.23E-02        |
| 1              | Spalla 1           | 6   | GEN FREQ Vento(min)   | 79   | -496.45    | -6.55        | -10.11       | 12.05          | 1.302          | 9.25         | 1.26E-02        |
| 1              | Spalla 1           | 6   | GEN FREQ Termico(min) | 79   | -503.09    | -0.19        | -11.33       | 11.33          | 1.302          | 8.70         | 1.25E-02        |
| 1              | Spalla 1           | 7   | GEN FREQ Mobili(max)  | 81   | -415.58    | 1.82         | 3.50         | 3.94           | 1.302          | 3.03         | 1.52E-02        |
| 1              | Spalla 1           | 7   | GEN FREQ Vento(max)   | 81   | -417.06    | 6.55         | 2.28         | 6.94           | 1.302          | 5.33         | 1.30E-02        |
| 1              | Spalla 1           | 7   | GEN FREQ Termico(max) | 81   | -410.23    | 0.19         | 3.50         | 3.51           | 1.302          | 2.69         | 1.31E-02        |
| 1              | Spalla 1           | 7   | GEN FREQ Mobili(min)  | 81   | -1105.80   | -1.72        | -13.68       | 13.79          | 1.302          | 10.59        | 1.23E-02        |
| 1              | Spalla 1           | 7   | GEN FREQ Vento(min)   | 81   | -496.26    | -6.31        | -10.11       | 11.92          | 1.302          | 9.15         | 1.26E-02        |
| 1              | Spalla 1           | 7   | GEN FREQ Termico(min) | 81   | -503.09    | 0.06         | -11.33       | 11.33          | 1.302          | 8.70         | 1.25E-02        |
| 1              | Spalla 1           | 8   | GEN FREQ Mobili(max)  | 83   | -417.98    | 2.28         | 3.42         | 4.11           | 1.302          | 3.16         | 1.57E-02        |
| 1              | Spalla 1           | 8   | GEN FREQ Vento(max)   | 83   | -477.03    | 7.12         | 2.00         | 7.40           | 1.302          | 5.68         | 1.31E-02        |
| 1              | Spalla 1           | 8   | GEN FREQ Termico(max) | 83   | -474.03    | 0.82         | 3.17         | 3.27           | 1.302          | 2.51         | 1.32E-02        |
| 1              | Spalla 1           | 8   | GEN FREQ Mobili(min)  | 83   | -957.29    | -2.10        | -14.37       | 14.52          | 1.302          | 11.15        | 1.23E-02        |
| 1              | Spalla 1           | 8   | GEN FREQ Vento(min)   | 83   | -555.04    | -6.26        | -10.21       | 11.98          | 1.302          | 9.20         | 1.26E-02        |
| 1              | Spalla 1           | 8   | GEN FREQ Termico(min) | 83   | -558.05    | 0.05         | -11.37       | 11.37          | 1.302          | 8.73         | 1.26E-02        |
| 1              | Spalla 2           | 0   | GEN FREQ Mobili(max)  | 78   | -418.02    | 2.10         | 14.37        | 14.52          | 1.302          | 11.15        | 1.23E-02        |
| 1              | Spalla 2           | 0   | GEN FREQ Vento(max)   | 78   | -475.72    | 6.24         | 10.22        | 11.97          | 1.302          | 9.20         | 1.26E-02        |
| 1              | Spalla 2           | 0   | GEN FREQ Termico(max) | 78   | -474.02    | -0.05        | 11.38        | 11.38          | 1.302          | 8.74         | 1.26E-02        |
| 1              | Spalla 2           | 0   | GEN FREQ Mobili(min)  | 78   | -957.23    | -2.28        | -3.41        | 4.10           | 1.302          | 3.15         | 1.57E-02        |
| 1              | Spalla 2           | 0   | GEN FREQ Vento(min)   | 78   | -556.35    | -7.11        | -2.00        | 7.39           | 1.302          | 5.67         | 1.31E-02        |
| 1              | Spalla 2           | 0   | GEN FREQ Termico(min) | 78   | -558.04    | -0.82        | -3.16        | 3.26           | 1.302          | 2.51         | 1.32E-02        |
| 1              | Spalla 2           | 1   | GEN FREQ Mobili(max)  | 80   | -415.61    | 1.72         | 13.69        | 13.80          | 1.302          | 10.60        | 1.23E-02        |
| 1              | Spalla 2           | 1   | GEN FREQ Vento(max)   | 80   | -416.86    | 6.31         | 10.12        | 11.93          | 1.302          | 9.16         | 1.26E-02        |
| 1              | Spalla 2           | 1   | GEN FREQ Termico(max) | 80   | -410.23    | -0.06        | 11.34        | 11.34          | 1.302          | 8.71         | 1.25E-02        |
| 1              | Spalla 2           | 1   | GEN FREQ Mobili(min)  | 80   | -1105.66   | -1.82        | -3.50        | 3.94           | 1.302          | 3.03         | 1.52E-02        |
| 1              | Spalla 2           | 1   | GEN FREQ Vento(min)   | 80   | -496.45    | -6.55        | -2.27        | 6.93           | 1.302          | 5.32         | 1.30E-02        |
| 1              | Spalla 2           | 1   | GEN FREQ Termico(min) | 80   | -503.08    | -0.19        | -3.49        | 3.50           | 1.302          | 2.68         | 1.31E-02        |

CAVALCAVIA ASSE 80 – IMPALCATO – RELAZIONE DI CALCOLO

|   |          |    |   |                       |     |          |        |        |       |       |       |          |
|---|----------|----|---|-----------------------|-----|----------|--------|--------|-------|-------|-------|----------|
| 1 | Spalla 2 | 19 | 2 | GEN FREQ Mobili(max)  | 82  | -415.61  | 1.82   | 13.69  | 13.81 | 1.302 | 10.61 | 1.23E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN FREQ Vento(max)   | 82  | -417.05  | 6.55   | 10.12  | 12.05 | 1.302 | 9.26  | 1.26E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN FREQ Termico(max) | 82  | -410.23  | 0.19   | 11.34  | 11.34 | 1.302 | 8.71  | 1.25E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN FREQ Mobili(min)  | 82  | -1105.66 | -1.72  | -3.50  | 3.90  | 1.302 | 2.99  | 1.52E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN FREQ Vento(min)   | 82  | -496.26  | -6.31  | -2.27  | 6.71  | 1.302 | 5.15  | 1.30E-02 |
| 1 | Spalla 2 | 19 | 2 | GEN FREQ Termico(min) | 82  | -503.08  | 0.06   | -3.49  | 3.49  | 1.302 | 2.68  | 1.31E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN FREQ Mobili(max)  | 84  | -418.02  | 2.28   | 14.37  | 14.55 | 1.302 | 11.17 | 1.24E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN FREQ Vento(max)   | 84  | -477.02  | 7.12   | 10.21  | 12.45 | 1.302 | 9.56  | 1.26E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN FREQ Termico(max) | 84  | -474.02  | 0.82   | 11.38  | 11.41 | 1.302 | 8.76  | 1.26E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN FREQ Mobili(min)  | 84  | -957.23  | -2.10  | -3.41  | 4.00  | 1.302 | 3.08  | 1.56E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN FREQ Vento(min)   | 84  | -555.04  | -6.26  | -1.99  | 6.57  | 1.302 | 5.04  | 1.31E-02 |
| 1 | Spalla 2 | 19 | 3 | GEN FREQ Termico(min) | 84  | -558.04  | 0.05   | -3.16  | 3.16  | 1.302 | 2.43  | 1.32E-02 |
| 2 | Pila 1   | 19 | 5 | GEN FREQ Mobili(max)  | 21  | -1604.68 | 5.31   | 22.88  | 23.49 | 2.855 | 8.23  | 9.66E-04 |
| 2 | Pila 1   | 19 | 5 | GEN FREQ Vento(max)   | 21  | -1654.16 | 17.05  | 17.69  | 24.57 | 2.855 | 8.61  | 2.15E-03 |
| 2 | Pila 1   | 19 | 5 | GEN FREQ Termico(max) | 21  | -1644.63 | 3.04   | 18.69  | 18.94 | 2.855 | 6.63  | 2.14E-03 |
| 2 | Pila 1   | 19 | 5 | GEN FREQ Mobili(min)  | 21  | -2555.82 | -1.16  | -0.03  | 1.16  | 2.855 | 0.41  | 3.59E-03 |
| 2 | Pila 1   | 19 | 5 | GEN FREQ Vento(min)   | 21  | -1945.75 | -14.03 | 7.23   | 15.78 | 2.855 | 5.53  | 2.36E-03 |
| 2 | Pila 1   | 19 | 5 | GEN FREQ Termico(min) | 21  | -1955.27 | -0.02  | 6.23   | 6.23  | 2.855 | 2.18  | 2.38E-03 |
| 2 | Pila 1   | 19 | 6 | GEN FREQ Mobili(max)  | 23  | -1840.29 | 2.82   | 22.19  | 22.37 | 2.855 | 7.84  | 1.07E-03 |
| 2 | Pila 1   | 19 | 6 | GEN FREQ Vento(max)   | 23  | -1838.19 | 15.09  | 17.60  | 23.18 | 2.855 | 8.12  | 2.13E-03 |
| 2 | Pila 1   | 19 | 6 | GEN FREQ Termico(max) | 23  | -1814.37 | 0.87   | 18.63  | 18.65 | 2.855 | 6.53  | 2.11E-03 |
| 2 | Pila 1   | 19 | 6 | GEN FREQ Mobili(min)  | 23  | -2829.06 | -1.60  | 0.86   | 1.82  | 2.855 | 0.64  | 3.38E-03 |
| 2 | Pila 1   | 19 | 6 | GEN FREQ Vento(min)   | 23  | -2101.55 | -14.20 | 7.20   | 15.92 | 2.855 | 5.58  | 2.29E-03 |
| 2 | Pila 1   | 19 | 6 | GEN FREQ Termico(min) | 23  | -2125.37 | 0.01   | 6.18   | 6.18  | 2.855 | 2.16  | 2.31E-03 |
| 2 | Pila 1   | 19 | 7 | GEN FREQ Mobili(max)  | 885 | -1840.29 | 1.60   | 22.19  | 22.25 | 2.855 | 7.79  | 1.12E-03 |
| 2 | Pila 1   | 19 | 7 | GEN FREQ Vento(max)   | 885 | -1837.50 | 14.20  | 17.60  | 22.61 | 2.855 | 7.92  | 2.13E-03 |
| 2 | Pila 1   | 19 | 7 | GEN FREQ Termico(max) | 885 | -1814.37 | -0.01  | 18.63  | 18.63 | 2.855 | 6.53  | 2.11E-03 |
| 2 | Pila 1   | 19 | 7 | GEN FREQ Mobili(min)  | 885 | -2829.06 | -2.82  | 0.86   | 2.95  | 2.855 | 1.03  | 3.36E-03 |
| 2 | Pila 1   | 19 | 7 | GEN FREQ Vento(min)   | 885 | -2102.24 | -15.09 | 7.20   | 16.72 | 2.855 | 5.86  | 2.29E-03 |
| 2 | Pila 1   | 19 | 7 | GEN FREQ Termico(min) | 885 | -2125.37 | -0.87  | 6.18   | 6.24  | 2.855 | 2.19  | 2.30E-03 |
| 2 | Pila 1   | 19 | 8 | GEN FREQ Mobili(max)  | 27  | -1604.68 | 1.16   | 22.88  | 22.91 | 2.855 | 8.03  | 1.30E-03 |
| 2 | Pila 1   | 19 | 8 | GEN FREQ Vento(max)   | 27  | -1655.90 | 14.04  | 17.69  | 22.58 | 2.855 | 7.91  | 2.20E-03 |
| 2 | Pila 1   | 19 | 8 | GEN FREQ Termico(max) | 27  | -1644.63 | 0.02   | 18.69  | 18.69 | 2.855 | 6.55  | 2.19E-03 |
| 2 | Pila 1   | 19 | 8 | GEN FREQ Mobili(min)  | 27  | -2555.82 | -5.31  | -0.03  | 5.31  | 2.855 | 1.86  | 3.48E-03 |
| 2 | Pila 1   | 19 | 8 | GEN FREQ Vento(min)   | 27  | -1944.01 | -17.06 | 7.23   | 18.53 | 2.855 | 6.49  | 2.32E-03 |
| 2 | Pila 1   | 19 | 8 | GEN FREQ Termico(min) | 27  | -1955.27 | -3.04  | 6.23   | 6.93  | 2.855 | 2.43  | 2.33E-03 |
| 2 | Pila 2   | 20 | 0 | GEN FREQ Mobili(max)  | 22  | -1604.69 | 5.31   | 0.03   | 5.31  | 2.855 | 1.86  | 3.48E-03 |
| 2 | Pila 2   | 20 | 0 | GEN FREQ Vento(max)   | 22  | -1654.16 | 17.05  | -7.23  | 18.52 | 2.855 | 6.49  | 2.31E-03 |
| 2 | Pila 2   | 20 | 0 | GEN FREQ Termico(max) | 22  | -1644.64 | 3.04   | -6.23  | 6.93  | 2.855 | 2.43  | 2.33E-03 |
| 2 | Pila 2   | 20 | 0 | GEN FREQ Mobili(min)  | 22  | -2555.85 | -1.17  | -22.89 | 22.92 | 2.855 | 8.03  | 1.30E-03 |
| 2 | Pila 2   | 20 | 0 | GEN FREQ Vento(min)   | 22  | -1945.77 | -14.03 | -17.70 | 22.59 | 2.855 | 7.91  | 2.20E-03 |
| 2 | Pila 2   | 20 | 0 | GEN FREQ Termico(min) | 22  | -1955.28 | -0.02  | -18.70 | 18.70 | 2.855 | 6.55  | 2.19E-03 |
| 2 | Pila 2   | 20 | 1 | GEN FREQ Mobili(max)  | 24  | -1840.30 | 2.82   | -0.86  | 2.95  | 2.855 | 1.03  | 3.36E-03 |

|   |        |    |   |                       |     |          |        |        |       |       |      |          |
|---|--------|----|---|-----------------------|-----|----------|--------|--------|-------|-------|------|----------|
| 2 | Pila 2 | 20 | 1 | GEN FREQ Vento(max)   | 24  | -1838.21 | 15.09  | -7.21  | 16.72 | 2.855 | 5.86 | 2.29E-03 |
| 2 | Pila 2 | 20 | 1 | GEN FREQ Termico(max) | 24  | -1814.39 | 0.87   | -6.18  | 6.24  | 2.855 | 2.19 | 2.30E-03 |
| 2 | Pila 2 | 20 | 1 | GEN FREQ Mobili(min)  | 24  | -2829.09 | -1.61  | -22.20 | 22.26 | 2.855 | 7.80 | 1.12E-03 |
| 2 | Pila 2 | 20 | 1 | GEN FREQ Vento(min)   | 24  | -2101.57 | -14.20 | -17.61 | 22.62 | 2.855 | 7.92 | 2.13E-03 |
| 2 | Pila 2 | 20 | 1 | GEN FREQ Termico(min) | 24  | -2125.39 | 0.01   | -18.64 | 18.64 | 2.855 | 6.53 | 2.11E-03 |
| 2 | Pila 2 | 20 | 2 | GEN FREQ Mobili(max)  | 886 | -1840.30 | 1.61   | -0.86  | 1.83  | 2.855 | 0.64 | 3.38E-03 |
| 2 | Pila 2 | 20 | 2 | GEN FREQ Vento(max)   | 886 | -1837.51 | 14.21  | -7.21  | 15.93 | 2.855 | 5.58 | 2.29E-03 |
| 2 | Pila 2 | 20 | 2 | GEN FREQ Termico(max) | 886 | -1814.39 | -0.01  | -6.18  | 6.18  | 2.855 | 2.16 | 2.31E-03 |
| 2 | Pila 2 | 20 | 2 | GEN FREQ Mobili(min)  | 886 | -2829.09 | -2.82  | -22.20 | 22.38 | 2.855 | 7.84 | 1.07E-03 |
| 2 | Pila 2 | 20 | 2 | GEN FREQ Vento(min)   | 886 | -2102.26 | -15.09 | -17.61 | 23.19 | 2.855 | 8.12 | 2.12E-03 |
| 2 | Pila 2 | 20 | 2 | GEN FREQ Termico(min) | 886 | -2125.39 | -0.87  | -18.64 | 18.66 | 2.855 | 6.54 | 2.11E-03 |
| 2 | Pila 2 | 20 | 3 | GEN FREQ Mobili(max)  | 28  | -1604.69 | 1.17   | 0.03   | 1.17  | 2.855 | 0.41 | 3.59E-03 |
| 2 | Pila 2 | 20 | 3 | GEN FREQ Vento(max)   | 28  | -1655.90 | 14.04  | -7.23  | 15.79 | 2.855 | 5.53 | 2.36E-03 |
| 2 | Pila 2 | 20 | 3 | GEN FREQ Termico(max) | 28  | -1644.64 | 0.02   | -6.23  | 6.23  | 2.855 | 2.18 | 2.38E-03 |
| 2 | Pila 2 | 20 | 3 | GEN FREQ Mobili(min)  | 28  | -2555.85 | -5.31  | -22.89 | 23.50 | 2.855 | 8.23 | 9.65E-04 |
| 2 | Pila 2 | 20 | 3 | GEN FREQ Vento(min)   | 28  | -1944.03 | -17.06 | -17.70 | 24.58 | 2.855 | 8.61 | 2.15E-03 |
| 2 | Pila 2 | 20 | 3 | GEN FREQ Termico(min) | 28  | -1955.28 | -3.04  | -18.70 | 18.95 | 2.855 | 6.64 | 2.13E-03 |

|                        |  |  |  |  | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |          |
|------------------------|--|--|--|--|------------|--------------|--------------|----------------|----------------|--------------|-----------------|----------|
| <b>SFORZI MASSIMI:</b> |  |  |  |  | Spalle     | -410.23      | 7.12         | 14.37          | 14.55          | 1.302        | 11.17           | 1.57E-02 |
|                        |  |  |  |  | Pile       | -1604.68     | 17.05        | 22.88          | 24.58          | 2.855        | 8.61            | 3.59E-03 |
| <b>SFORZI MINIMI:</b>  |  |  |  |  | Spalle     | -1105.80     | -7.11        | -14.37         |                |              |                 |          |
|                        |  |  |  |  | Pile       | -2829.09     | -17.06       | -22.89         |                |              |                 |          |

**STATI LIMITE DI ESERCIZIO - COMBINAZIONI QUASI PERMANENTI**

| Tipo isolatore | Struttura Appoggio | No. | Load            | Node | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|----------------|--------------------|-----|-----------------|------|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| 1              | Spalla 1           | 18  | 5 GEN Q.P.(max) | 77   | -481.03    | -0.11        | 1.96         | 1.96           | 1.302          | 1.51         | 1.31E-02        |
| 1              | Spalla 1           | 18  | 5 GEN Q.P.(min) | 77   | -551.04    | -0.75        | -10.16       | 10.19          | 1.302          | 7.82         | 1.26E-02        |
| 1              | Spalla 1           | 18  | 6 GEN Q.P.(max) | 79   | -417.97    | -0.07        | 2.26         | 2.26           | 1.302          | 1.74         | 1.30E-02        |
| 1              | Spalla 1           | 18  | 6 GEN Q.P.(min) | 79   | -495.35    | -0.17        | -10.10       | 10.10          | 1.302          | 7.76         | 1.26E-02        |
| 1              | Spalla 1           | 18  | 7 GEN Q.P.(max) | 81   | -417.97    | 0.17         | 2.26         | 2.27           | 1.302          | 1.74         | 1.30E-02        |
| 1              | Spalla 1           | 18  | 7 GEN Q.P.(min) | 81   | -495.35    | 0.07         | -10.10       | 10.10          | 1.302          | 7.76         | 1.26E-02        |
| 1              | Spalla 1           | 18  | 8 GEN Q.P.(max) | 83   | -481.03    | 0.75         | 1.96         | 2.10           | 1.302          | 1.61         | 1.31E-02        |
| 1              | Spalla 1           | 18  | 8 GEN Q.P.(min) | 83   | -551.04    | 0.11         | -10.16       | 10.16          | 1.302          | 7.80         | 1.26E-02        |
| 1              | Spalla 2           | 19  | 0 GEN Q.P.(max) | 78   | -481.02    | -0.11        | 10.17        | 10.17          | 1.302          | 7.81         | 1.26E-02        |
| 1              | Spalla 2           | 19  | 0 GEN Q.P.(min) | 78   | -551.04    | -0.75        | -1.95        | 2.09           | 1.302          | 1.60         | 1.31E-02        |
| 1              | Spalla 2           | 19  | 1 GEN Q.P.(max) | 80   | -417.97    | -0.07        | 10.10        | 10.10          | 1.302          | 7.76         | 1.26E-02        |
| 1              | Spalla 2           | 19  | 1 GEN Q.P.(min) | 80   | -495.34    | -0.17        | -2.26        | 2.27           | 1.302          | 1.74         | 1.30E-02        |
| 1              | Spalla 2           | 19  | 2 GEN Q.P.(max) | 82   | -417.97    | 0.17         | 10.10        | 10.10          | 1.302          | 7.76         | 1.26E-02        |
| 1              | Spalla 2           | 19  | 2 GEN Q.P.(min) | 82   | -495.34    | 0.07         | -2.26        | 2.26           | 1.302          | 1.74         | 1.30E-02        |
| 1              | Spalla 2           | 19  | 3 GEN Q.P.(max) | 84   | -481.02    | 0.75         | 10.17        | 10.20          | 1.302          | 7.83         | 1.26E-02        |
| 1              | Spalla 2           | 19  | 3 GEN Q.P.(min) | 84   | -551.04    | 0.11         | -1.95        | 1.95           | 1.302          | 1.50         | 1.31E-02        |







|   |          |         |                    |    |          |        |        |       |       |       |          |
|---|----------|---------|--------------------|----|----------|--------|--------|-------|-------|-------|----------|
| 1 | Spalla 1 | 18<br>7 | GEN SLD Trasv(min) | 81 | -499.55  | -29.09 | -19.13 | 34.82 | 1.302 | 26.74 | 1.25E-02 |
| 1 | Spalla 1 | 18<br>7 | GEN SLD Vert(min)  | 81 | -504.04  | -8.68  | -19.18 | 21.05 | 1.302 | 16.17 | 1.25E-02 |
| 1 | Spalla 1 | 18<br>8 | GEN SLD Long(max)  | 83 | -470.55  | 9.49   | 31.84  | 33.22 | 1.302 | 25.52 | 1.32E-02 |
| 1 | Spalla 1 | 18<br>8 | GEN SLD Trasv(max) | 83 | -461.82  | 29.87  | 11.11  | 31.87 | 1.302 | 24.48 | 1.32E-02 |
| 1 | Spalla 1 | 18<br>8 | GEN SLD Vert(max)  | 83 | -466.95  | 9.49   | 11.08  | 14.59 | 1.302 | 11.20 | 1.32E-02 |
| 1 | Spalla 1 | 18<br>8 | GEN SLD Long(min)  | 83 | -561.53  | -8.63  | -40.05 | 40.97 | 1.302 | 31.46 | 1.26E-02 |
| 1 | Spalla 1 | 18<br>8 | GEN SLD Trasv(min) | 83 | -570.26  | -29.01 | -19.32 | 34.85 | 1.302 | 26.77 | 1.26E-02 |
| 1 | Spalla 1 | 18<br>8 | GEN SLD Vert(min)  | 83 | -565.13  | -8.63  | -19.29 | 21.13 | 1.302 | 16.23 | 1.26E-02 |
| 1 | Spalla 2 | 19<br>0 | GEN SLD Long(max)  | 78 | -470.53  | 8.63   | 40.05  | 40.97 | 1.302 | 31.46 | 1.26E-02 |
| 1 | Spalla 2 | 19<br>0 | GEN SLD Trasv(max) | 78 | -461.81  | 29.00  | 19.32  | 34.85 | 1.302 | 26.76 | 1.26E-02 |
| 1 | Spalla 2 | 19<br>0 | GEN SLD Vert(max)  | 78 | -466.92  | 8.63   | 19.29  | 21.13 | 1.302 | 16.23 | 1.26E-02 |
| 1 | Spalla 2 | 19<br>0 | GEN SLD Long(min)  | 78 | -561.53  | -9.49  | -31.83 | 33.21 | 1.302 | 25.51 | 1.32E-02 |
| 1 | Spalla 2 | 19<br>0 | GEN SLD Trasv(min) | 78 | -570.25  | -29.86 | -11.10 | 31.86 | 1.302 | 24.47 | 1.32E-02 |
| 1 | Spalla 2 | 19<br>0 | GEN SLD Vert(min)  | 78 | -565.15  | -9.49  | -11.07 | 14.58 | 1.302 | 11.20 | 1.32E-02 |
| 1 | Spalla 2 | 19<br>1 | GEN SLD Long(max)  | 80 | -411.70  | 8.68   | 39.95  | 40.88 | 1.302 | 31.40 | 1.25E-02 |
| 1 | Spalla 2 | 19<br>1 | GEN SLD Trasv(max) | 80 | -413.76  | 29.09  | 19.14  | 34.82 | 1.302 | 26.74 | 1.25E-02 |
| 1 | Spalla 2 | 19<br>1 | GEN SLD Vert(max)  | 80 | -409.27  | 8.68   | 19.19  | 21.06 | 1.302 | 16.18 | 1.25E-02 |
| 1 | Spalla 2 | 19<br>1 | GEN SLD Long(min)  | 80 | -501.61  | -8.92  | -32.10 | 33.32 | 1.302 | 25.59 | 1.31E-02 |
| 1 | Spalla 2 | 19<br>1 | GEN SLD Trasv(min) | 80 | -499.55  | -29.33 | -11.29 | 31.43 | 1.302 | 24.14 | 1.31E-02 |
| 1 | Spalla 2 | 19<br>1 | GEN SLD Vert(min)  | 80 | -504.03  | -8.92  | -11.34 | 14.43 | 1.302 | 11.08 | 1.31E-02 |
| 1 | Spalla 2 | 19<br>2 | GEN SLD Long(max)  | 82 | -411.70  | 8.92   | 39.95  | 40.93 | 1.302 | 31.44 | 1.25E-02 |
| 1 | Spalla 2 | 19<br>2 | GEN SLD Trasv(max) | 82 | -413.76  | 29.33  | 19.14  | 35.02 | 1.302 | 26.90 | 1.25E-02 |
| 1 | Spalla 2 | 19<br>2 | GEN SLD Vert(max)  | 82 | -409.28  | 8.92   | 19.19  | 21.16 | 1.302 | 16.25 | 1.25E-02 |
| 1 | Spalla 2 | 19<br>2 | GEN SLD Long(min)  | 82 | -501.61  | -8.68  | -32.10 | 33.25 | 1.302 | 25.54 | 1.31E-02 |
| 1 | Spalla 2 | 19<br>2 | GEN SLD Trasv(min) | 82 | -499.55  | -29.09 | -11.29 | 31.20 | 1.302 | 23.96 | 1.31E-02 |
| 1 | Spalla 2 | 19<br>2 | GEN SLD Vert(min)  | 82 | -504.03  | -8.68  | -11.34 | 14.28 | 1.302 | 10.97 | 1.31E-02 |
| 1 | Spalla 2 | 19<br>3 | GEN SLD Long(max)  | 84 | -470.54  | 9.49   | 40.05  | 41.16 | 1.302 | 31.61 | 1.26E-02 |
| 1 | Spalla 2 | 19<br>3 | GEN SLD Trasv(max) | 84 | -461.82  | 29.86  | 19.32  | 35.57 | 1.302 | 27.31 | 1.26E-02 |
| 1 | Spalla 2 | 19<br>3 | GEN SLD Vert(max)  | 84 | -466.94  | 9.49   | 19.29  | 21.50 | 1.302 | 16.51 | 1.26E-02 |
| 1 | Spalla 2 | 19<br>3 | GEN SLD Long(min)  | 84 | -561.52  | -8.63  | -31.83 | 32.98 | 1.302 | 25.33 | 1.32E-02 |
| 1 | Spalla 2 | 19<br>3 | GEN SLD Trasv(min) | 84 | -570.25  | -29.00 | -11.10 | 31.05 | 1.302 | 23.85 | 1.32E-02 |
| 1 | Spalla 2 | 19<br>3 | GEN SLD Vert(min)  | 84 | -565.12  | -8.63  | -11.07 | 14.04 | 1.302 | 10.78 | 1.32E-02 |
| 2 | Pila 1   | 19<br>5 | GEN SLD Long(max)  | 21 | -1643.08 | 22.34  | 82.41  | 85.38 | 2.855 | 29.91 | 2.13E-03 |
| 2 | Pila 1   | 19<br>5 | GEN SLD Trasv(max) | 21 | -1602.04 | 67.94  | 37.27  | 77.49 | 2.855 | 27.15 | 2.14E-03 |
| 2 | Pila 1   | 19<br>5 | GEN SLD Vert(max)  | 21 | -1628.61 | 22.35  | 37.18  | 43.38 | 2.855 | 15.20 | 2.14E-03 |
| 2 | Pila 1   | 19<br>5 | GEN SLD Long(min)  | 21 | -1956.83 | -19.32 | -57.49 | 60.65 | 2.855 | 21.25 | 2.39E-03 |
| 2 | Pila 1   | 19<br>5 | GEN SLD Trasv(min) | 21 | -1997.87 | -64.92 | -12.35 | 66.08 | 2.855 | 23.15 | 2.39E-03 |
| 2 | Pila 1   | 19<br>5 | GEN SLD Vert(min)  | 21 | -1971.29 | -19.33 | -12.26 | 22.89 | 2.855 | 8.02  | 2.38E-03 |
| 2 | Pila 1   | 19<br>6 | GEN SLD Long(max)  | 23 | -1828.63 | 20.38  | 82.32  | 84.81 | 2.855 | 29.71 | 2.10E-03 |
| 2 | Pila 1   | 19<br>6 | GEN SLD Trasv(max) | 23 | -1825.33 | 66.06  | 37.08  | 75.76 | 2.855 | 26.54 | 2.12E-03 |
| 2 | Pila 1   | 19<br>6 | GEN SLD Vert(max)  | 23 | -1813.01 | 20.38  | 37.08  | 42.31 | 2.855 | 14.82 | 2.12E-03 |
| 2 | Pila 1   | 19<br>6 | GEN SLD Long(min)  | 23 | -2111.11 | -19.49 | -57.51 | 60.72 | 2.855 | 21.27 | 2.31E-03 |
| 2 | Pila 1   | 19<br>6 | GEN SLD Trasv(min) | 23 | -2114.41 | -65.17 | -12.27 | 66.32 | 2.855 | 23.23 | 2.30E-03 |

|   |        |    |                    |     |          |        |        |       |       |       |          |  |
|---|--------|----|--------------------|-----|----------|--------|--------|-------|-------|-------|----------|--|
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 6  | GEN SLD Vert(min)  | 23  | -2126.73 | -19.50 | -12.27 | 23.04 | 2.855 | 8.07  | 2.30E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 7  | GEN SLD Long(max)  | 885 | -1828.88 | 19.50  | 82.32  | 84.60 | 2.855 | 29.64 | 2.11E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 7  | GEN SLD Trasv(max) | 885 | -1825.42 | 65.17  | 37.08  | 74.98 | 2.855 | 26.27 | 2.12E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 7  | GEN SLD Vert(max)  | 885 | -1813.12 | 19.50  | 37.08  | 41.89 | 2.855 | 14.68 | 2.12E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 7  | GEN SLD Long(min)  | 885 | -2110.86 | -20.38 | -57.51 | 61.01 | 2.855 | 21.37 | 2.31E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 7  | GEN SLD Trasv(min) | 885 | -2114.31 | -66.06 | -12.27 | 67.19 | 2.855 | 23.54 | 2.30E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 7  | GEN SLD Vert(min)  | 885 | -2126.61 | -20.38 | -12.27 | 23.79 | 2.855 | 8.33  | 2.30E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 8  | GEN SLD Long(max)  | 27  | -1643.70 | 19.32  | 82.41  | 84.64 | 2.855 | 29.65 | 2.18E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 8  | GEN SLD Trasv(max) | 27  | -1602.24 | 64.92  | 37.27  | 74.86 | 2.855 | 26.22 | 2.20E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 8  | GEN SLD Vert(max)  | 27  | -1628.87 | 19.33  | 37.18  | 41.90 | 2.855 | 14.68 | 2.19E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 8  | GEN SLD Long(min)  | 27  | -1956.21 | -22.34 | -57.49 | 61.68 | 2.855 | 21.61 | 2.34E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 8  | GEN SLD Trasv(min) | 27  | -1997.66 | -67.94 | -12.35 | 69.05 | 2.855 | 24.19 | 2.33E-03 |  |
| 2 | Pila 1 | 19 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 8  | GEN SLD Vert(min)  | 27  | -1971.04 | -22.35 | -12.26 | 25.49 | 2.855 | 8.93  | 2.33E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 0  | GEN SLD Long(max)  | 22  | -1643.62 | 22.34  | 57.59  | 61.77 | 2.855 | 21.64 | 2.34E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 0  | GEN SLD Trasv(max) | 22  | -1602.21 | 67.95  | 12.38  | 69.07 | 2.855 | 24.20 | 2.33E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 0  | GEN SLD Vert(max)  | 22  | -1628.76 | 22.35  | 12.29  | 25.51 | 2.855 | 8.94  | 2.33E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 0  | GEN SLD Long(min)  | 22  | -1956.31 | -19.32 | -82.52 | 84.75 | 2.855 | 29.69 | 2.18E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 0  | GEN SLD Trasv(min) | 22  | -1997.72 | -64.92 | -37.31 | 74.88 | 2.855 | 26.23 | 2.20E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 0  | GEN SLD Vert(min)  | 22  | -1971.17 | -19.33 | -37.22 | 41.94 | 2.855 | 14.69 | 2.19E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 1  | GEN SLD Long(max)  | 24  | -1828.85 | 20.38  | 57.61  | 61.11 | 2.855 | 21.41 | 2.31E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 1  | GEN SLD Trasv(max) | 24  | -1825.43 | 66.06  | 12.30  | 67.20 | 2.855 | 23.54 | 2.30E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 1  | GEN SLD Vert(max)  | 24  | -1813.08 | 20.38  | 12.30  | 23.80 | 2.855 | 8.34  | 2.30E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 1  | GEN SLD Long(min)  | 24  | -2110.92 | -19.49 | -82.43 | 84.70 | 2.855 | 29.67 | 2.11E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 1  | GEN SLD Trasv(min) | 24  | -2114.34 | -65.18 | -37.12 | 75.01 | 2.855 | 26.28 | 2.12E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 1  | GEN SLD Vert(min)  | 24  | -2126.69 | -19.50 | -37.12 | 41.93 | 2.855 | 14.69 | 2.12E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 2  | GEN SLD Long(max)  | 886 | -1828.91 | 19.49  | 57.61  | 60.82 | 2.855 | 21.30 | 2.31E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 2  | GEN SLD Trasv(max) | 886 | -1825.44 | 65.18  | 12.30  | 66.33 | 2.855 | 23.24 | 2.30E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 2  | GEN SLD Vert(max)  | 886 | -1813.11 | 19.50  | 12.30  | 23.06 | 2.855 | 8.08  | 2.30E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 2  | GEN SLD Long(min)  | 886 | -2110.86 | -20.38 | -82.43 | 84.91 | 2.855 | 29.75 | 2.10E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 2  | GEN SLD Trasv(min) | 886 | -2114.33 | -66.06 | -37.12 | 75.77 | 2.855 | 26.54 | 2.12E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 2  | GEN SLD Vert(min)  | 886 | -2126.66 | -20.38 | -37.12 | 42.35 | 2.855 | 14.83 | 2.11E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 3  | GEN SLD Long(max)  | 28  | -1643.61 | 19.32  | 57.59  | 60.74 | 2.855 | 21.28 | 2.38E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 3  | GEN SLD Trasv(max) | 28  | -1602.20 | 64.92  | 12.38  | 66.09 | 2.855 | 23.15 | 2.39E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 3  | GEN SLD Vert(max)  | 28  | -1628.76 | 19.33  | 12.29  | 22.91 | 2.855 | 8.02  | 2.37E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 3  | GEN SLD Long(min)  | 28  | -1956.32 | -22.34 | -82.52 | 85.49 | 2.855 | 29.95 | 2.13E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 3  | GEN SLD Trasv(min) | 28  | -1997.73 | -67.95 | -37.31 | 77.52 | 2.855 | 27.16 | 2.13E-03 |  |
| 2 | Pila 2 | 20 |                    |     |          |        |        |       |       |       |          |  |
|   |        | 3  | GEN SLD Vert(min)  | 28  | -1971.17 | -22.35 | -37.22 | 43.41 | 2.855 | 15.21 | 2.14E-03 |  |

|                        |        | Axial (kN) | Shear-y (kN) | Shear-z (kN) | Shear-tot (kN) | Kh eq. (kN/mm) | Deform. (mm) | Rotazione (rad) |
|------------------------|--------|------------|--------------|--------------|----------------|----------------|--------------|-----------------|
| <b>SFORZI MASSIMI:</b> | Spalle | -409.27    | 29.87        | 40.05        | 41.16          | 1.302          | 31.61        | 1.32E-02        |
|                        | Pile   | -1602.04   | 67.95        | 82.41        | 85.49          | 2.855          | 29.95        | 2.39E-03        |
| <b>SFORZI MINIMI:</b>  | Spalle | -570.27    | -29.87       | -40.05       |                |                |              |                 |
|                        | Pile   | -2126.73   | -67.95       | -82.52       |                |                |              |                 |



## 14 GIUNTI DI DILATAZIONE

I giunti di dilatazione si assumono soggetti alle condizioni di carico di progetto con l'eccezione delle dilatazioni termiche che si considerano raddoppiate ( $\pm 30^\circ$ ) per tener conto dell'incertezza sulla posizione del punto fisso.

Segue tabulato degli spostamenti dei nodi di estremità dell'impalcato per le condizioni di carico pertinenti.

| Node | Load                 | DX (mm)   | DY (mm)   | DZ (mm)   | RX ([rad]) | RY ([rad]) | RZ ([rad]) |
|------|----------------------|-----------|-----------|-----------|------------|------------|------------|
| 719  | Vento strutture (Y+) | 0.073954  | 17.555907 | 0.002208  | -0.000048  | -0.000015  | 0.000017   |
| 760  | Vento strutture (Y+) | 0.023434  | 17.552788 | -0.004664 | -0.000025  | -0.000006  | 0.000016   |
| 801  | Vento strutture (Y+) | -0.025099 | 17.553102 | 0.005454  | -0.000027  | 0.000004   | 0.000016   |
| 842  | Vento strutture (Y+) | -0.074278 | 17.558601 | -0.001555 | -0.000060  | 0.000019   | 0.000017   |
| 719  | Vento mobili (Y+)    | 0.038533  | 6.987374  | -0.013644 | -0.000086  | -0.000042  | 0.000009   |
| 760  | Vento mobili (Y+)    | 0.016743  | 6.972525  | -0.011129 | -0.000022  | -0.000005  | 0.000009   |
| 801  | Vento mobili (Y+)    | -0.009328 | 6.970268  | 0.006919  | -0.000014  | 0.000013   | 0.000009   |
| 842  | Vento mobili (Y+)    | -0.038166 | 6.970738  | 0.010130  | -0.000022  | 0.000025   | 0.000010   |
| 719  | Frenamento (X+)      | 19.193654 | -0.000196 | 0.010495  | 0.000002   | 0.000009   | 0.000000   |
| 760  | Frenamento (X+)      | 19.193494 | 0.000000  | 0.010351  | 0.000000   | 0.000008   | 0.000000   |
| 801  | Frenamento (X+)      | 19.193494 | 0.000000  | 0.010351  | 0.000000   | 0.000008   | 0.000000   |
| 842  | Frenamento (X+)      | 19.193654 | 0.000196  | 0.010495  | -0.000002  | 0.000009   | 0.000000   |
| 719  | Termico Unif. +15°   | -9.153158 | 0.028458  | 0.010992  | -0.000383  | -0.000086  | 0.000000   |
| 760  | Termico Unif. +15°   | -9.120941 | -0.000720 | -0.006033 | -0.000058  | 0.000093   | -0.000001  |
| 801  | Termico Unif. +15°   | -9.120941 | 0.000720  | -0.006033 | 0.000058   | 0.000093   | 0.000001   |
| 842  | Termico Unif. +15°   | -9.153158 | -0.028458 | 0.010992  | 0.000383   | -0.000086  | 0.000000   |
| 719  | Termico diff. +5°/H  | -0.129353 | 0.003519  | -0.351476 | -0.000021  | -0.000444  | 0.000000   |
| 760  | Termico diff. +5°/H  | -0.124709 | 0.000776  | -0.348386 | -0.000004  | -0.000429  | 0.000000   |
| 801  | Termico diff. +5°/H  | -0.124709 | -0.000776 | -0.348386 | 0.000004   | -0.000429  | 0.000000   |
| 842  | Termico diff. +5°/H  | -0.129353 | -0.003519 | -0.351476 | 0.000021   | -0.000444  | 0.000000   |
| 719  | SLC Long (X+)(RS)    | 70.875180 | -0.000810 | 0.032673  | 0.000007   | 0.000031   | 0.000000   |
| 760  | SLC Long (X+)(RS)    | 70.874486 | -0.000154 | 0.031984  | 0.000000   | 0.000028   | 0.000000   |
| 801  | SLC Long (X+)(RS)    | 70.874486 | 0.000134  | 0.031984  | 0.000000   | 0.000028   | 0.000000   |
| 842  | SLC Long (X+)(RS)    | 70.875180 | 0.000802  | 0.032674  | -0.000007  | 0.000031   | 0.000000   |
| 719  | SLC Trasv (Y+)(RS)   | 0.320722  | 69.452819 | 0.011780  | -0.000213  | -0.000079  | 0.000073   |
| 760  | SLC Trasv (Y+)(RS)   | 0.105084  | 69.441155 | -0.022311 | -0.000113  | -0.000024  | 0.000070   |
| 801  | SLC Trasv (Y+)(RS)   | -0.105084 | 69.441155 | 0.022311  | -0.000113  | 0.000024   | 0.000070   |
| 842  | SLC Trasv (Y+)(RS)   | -0.320722 | 69.452819 | -0.011781 | -0.000213  | 0.000079   | 0.000073   |
| 719  | SLC Vert (Z+)(RS)    | 0.096951  | 0.003339  | -0.194587 | -0.000012  | -0.000293  | 0.000000   |
| 760  | SLC Vert (Z+)(RS)    | 0.100365  | 0.002087  | -0.188966 | -0.000004  | -0.000280  | 0.000000   |
| 801  | SLC Vert (Z+)(RS)    | 0.100313  | -0.002062 | -0.188968 | 0.000004   | -0.000280  | 0.000000   |
| 842  | SLC Vert (Z+)(RS)    | 0.096786  | -0.003299 | -0.194577 | 0.000012   | -0.000293  | 0.000000   |

Ai valori determinati in direzione longitudinale si sommano 50 mm assunti come massima deformazione ammissibile delle sottostrutture, già calcolata allo SLU.

Si prevedono giunti con le seguenti caratteristiche di deformazione massime:

- escursione longitudinale:  $\Delta X = \pm 165$  mm
- escursione trasversale:  $\Delta Y = \pm 165$  mm

1) Verifica deformazioni longitudinali statiche: considerando il frenamento come carico variabile dominante si ottiene la deformazione massima allo SLU dalla seguente somma:

|                                  | $\Delta X$ | $\gamma_q$ | $\psi_0$ |           |
|----------------------------------|------------|------------|----------|-----------|
| - Frenamento:                    | 19.3       | x 1.35     | x 1      | = 26.1 mm |
| - Termico unif. $\pm 30^\circ$ : | 2 x 9.2    | x 1.2      | x 0.6    | = 13.2 mm |
| - Deform. max sottostrutture:    |            |            |          | = 50 mm   |

- Deformazione totale: = 89.3 mm (< 165 mm:  
verificato)

2) Verifica deformazioni trasversali statiche: considerando il vento come carico variabile dominante si ottiene la deformazione massima allo SLU dalla seguente somma:

|                         | $\Delta Y$ | $\gamma_q$ | $\Psi_0$ |                                     |
|-------------------------|------------|------------|----------|-------------------------------------|
| - Vento strutture:      | 17.6       | x 1.5      | x 1      | = 26.4 mm                           |
| - Vento carichi mobili: | 7.00       | x 1.5      | x 1      | = 10.5 mm                           |
| - Deformazione totale:  |            |            |          | = 36.9 mm (< 165 mm:<br>verificato) |

3) Verifica deformazioni longitudinali sismiche: si ottiene la deformazione massima allo SLC dalla seguente somma:

|                                  | $\Delta X$ | $\gamma_q$ | $\Psi_0$ |                                     |
|----------------------------------|------------|------------|----------|-------------------------------------|
| - SLC longitudinale:             | 70.9       | x 1        | x 1      | = 70.9 mm                           |
| - Termico unif. $\pm 30^\circ$ : | 2 x 9.2    | x 1        | x 0.5    | = 9.2 mm                            |
| - Deform. max sottostrutture:    |            |            |          | = 50 mm                             |
| - Deformazione totale:           |            |            |          | = 130.1 mm (<165 mm:<br>verificato) |

3) Verifica deformazioni trasversali sismiche: si ottiene la deformazione massima allo SLC dalla seguente somma:

|                        | $\Delta Y$ | $\gamma_q$ | $\Psi_0$ |                                     |
|------------------------|------------|------------|----------|-------------------------------------|
| - SLC trasversale:     | 69.4       | x 1        | x 1      | = 69.4 mm                           |
| - Deformazione totale: |            |            |          | = 69.4 mm (< 165 mm:<br>verificato) |

## 15 VERIFICHE TRASVERSALI DELLA SOLETTA STRADALE DI SCORRIMENTO

### 15.1 VERIFICA IN FASE COSTRUTTIVA

In fase costruttiva la sezione resistente è rappresentata dai tralicci inseriti nelle lastre prefabbricate, sollecitati dal peso della soletta e dal carico accidentale previsto da EN 1991-1-6, § 4.11.2.

#### 15.1.1 FASE COSTRUTTIVA - SBALZO LATERALE – PRIMA FASE DI GETTO

Si considera una prima fase di getto in cui la soletta viene gettata per 1 m oltre l'asse trave.



**VERIFICA LASTRE IN C.A. TRALICCIATE PREFABBRICATE**

**LASTRE PREFABBRICATE CAVALCAVIA ASSE 80 - CAMPATA A SBALZO: L= 185 cm - 1° FASE DI GETTO**

**DATI GENERALI LASTRA PREFABBRICATA.**

Schema vincolare in fase costruttiva: **A) Incastro - B) Estremo libero (a sbalzo)**

Luce di calcolo della lastra: **L= 185 cm**

Larghezza di calcolo della lastra: **B<sub>c</sub>= 240 cm**

Altezza calcestruzzo lastra: **H<sub>c</sub>= 6 cm**

Altezza della soletta (getto in opera): **H<sub>s</sub>= 24 cm**

Peso specifico calcestruzzo: **g<sub>c</sub>= 25 kN/m<sup>3</sup>**

Vincolo laterale armature offerto dalla lastra presente nella sezione di verifica: **NO**

**DATI TRALICCI E ARMATURA AGGIUNTIVA LASTRA PREFABBRICATA.**

Numero tralici per lastra: **N<sub>t</sub>= 4**

Altezza nominale del traliccio: **H<sub>t</sub>= 20.5 cm**

Altezza utile del traliccio: **H<sub>u</sub>= 18.8 cm**

Larghezza traliccio: **B<sub>t</sub>= 12.0 cm**

Passo staffe: **P<sub>s</sub>= 20.0 cm**

Ricoprimento tralici e armature aggiuntive (dal bordo inferiore lastra): **R<sub>tr</sub>= 4.0 cm**

Modulo elastico acciaio: **E<sub>s</sub>= 210 000 N/mm<sup>2</sup>**

Coefficiente parziale di sicurezza dell'acciaio armature: **γ<sub>s</sub>= 1.15**

**DATI ARMATURA LASTRA PREFABBRICATA.**

|                                                        | Singolo traliccio        |                 |        | Arm. agg. lastra |
|--------------------------------------------------------|--------------------------|-----------------|--------|------------------|
|                                                        | Corr. Superiore          | Corr. Inferiori | Staffe |                  |
| Tipo di acciaio                                        | B450C                    | B450C           | B450C  | B450C            |
| Resistenza caratteristica acciaio                      | f <sub>yk</sub> = 450.00 | 450.00          | 450.00 | 450.00           |
| Resistenza di calcolo acciaio                          | f <sub>yd</sub> = 391.30 | 391.30          | 391.30 | 391.30           |
| Diametro:                                              | Φ= 16                    | 12              | 10     | 0                |
| Numero di ferri per traliccio (Arm. agg. per lastra):  | N <sub>f</sub> = 1       | 2               | 2      | 0                |
| Area di armatura per lastra:                           | A <sub>f</sub> = 8.042   | 9.048           | 6.283  | 0.000            |
| Inclinazione ferri in direzione longitudinale:         | α= 0.00                  | 0.00            | 70.97  | 0.00             |
| Inclinazione ferri in direzione trasversale:           | γ= 0.00                  | 0.00            | 80.93  | 0.00             |
| Coefficiente di vincolo:                               | β= 1                     | 1               | 1      | 1                |
| Lunghezza di calcolo per instabilità aste compresse    | L= 20.00                 | 20.00           | 20.14  | 40.00            |
| Lunghezza libera d'inflessione (L <sub>0</sub> = β*L): | L <sub>0</sub> = 20.00   | 20.00           | 20.14  | 40.00            |

**DATI DI CARICO**

NOTA: Per le azioni in fase di costruzione durante il getto di calcestruzzo ci si riferisce a: UNI EN 1991-1-6, § 4.11.2.

Coefficiente parziale dei carichi permanenti strutturali (peso lastra): **γ<sub>g1</sub>= 1.35**

Coefficiente parziale dei carichi permanenti portati (getto in opera): **γ<sub>g2</sub>= 1.5**

Coefficiente parziale dei carichi variabili (sovraccarichi): **γ<sub>q</sub>= 1.5**

Estensione del carico permanente (X1=X2 per carico concentrato):

|                                                                          | Da X1 (cm)                    | A X2 (cm) |
|--------------------------------------------------------------------------|-------------------------------|-----------|
| Peso strutturale lastra:                                                 | G <sub>1</sub> = 1.50 kN/mq   | 0 185     |
| Permanente portato 1: getto in opera soletta:                            | G <sub>2,1</sub> = 6.00 kN/mq | 0 100     |
| Permanente portato 2:                                                    | G <sub>2,2</sub> = 0.00 kN/m  | 0 0       |
| Permanente portato 3:                                                    | G <sub>2,3</sub> = 0.00 kN/m  | 0 0       |
| Sovraccarico all'interno dell'area di lavoro di 3 m x 3 m (L se minore): | Q <sub>1</sub> = 0.75 kN/mq   |           |
| Sovraccarico all'esterno dell'area di lavoro di 3 m x 3 m (L se minore): | Q <sub>2</sub> = 0.75 kN/mq   |           |

**SOLLECITAZIONI ALLO S.L.U.** NOTA: Le sollecitazioni si intendono riferite alla larghezza della lastra.

Momento flettente di progetto: **M<sub>sd</sub>= -23.74 KN.m**

Sforzo di taglio di progetto: **V<sub>sd</sub>= 35.59 KN**

**VERIFICA ALLO S.L.U.**

Sforzi assiali nei correnti (N<sub>sd</sub>=M<sub>sd</sub>/H<sub>u</sub>)

Sforzi assiali nelle staffe (N<sub>sd</sub>=V<sub>sd</sub>/(Sen(α)\*Sen(γ)))

Carico elastico critico (N<sub>CR</sub>=P<sup>2</sup>\*E<sub>s</sub>\*J/L<sub>0</sub><sup>2</sup>)

Snellezza adimensionale (λ = (A<sub>f</sub> f<sub>yk</sub>/N<sub>CR</sub>)<sup>0.5</sup>)

Fattore di imperfezione (NTC 2008; tab. 4.2.VI)

Coefficiente di stabilità delle aste compresse:

Fattore di riduzione

Resistenza di calcolo a compressione (instabilità)

Resistenza di calcolo a trazione e compressione

Considerare instabilità delle aste compresse (non incluse nel cis.)

Resistenza di calcolo considerata (ferro singolo)

Sforzo assiale di calcolo (ferro singolo)

Coefficiente di sfruttamento (N<sub>sd</sub>/N<sub>Rd</sub> < 1: verificato)

|                                  | Corr. Superiore (Teso) | Corr. Inferiori (Compresso) | Staffe (Compresso) | Arm. Agg. (Compresso) |
|----------------------------------|------------------------|-----------------------------|--------------------|-----------------------|
| N <sub>CR</sub>                  | 166.690                | 52.742                      | 25.088             | 0.000                 |
| λ                                | 0.737                  | 0.982                       | 1.187              | 0.000                 |
| α                                | 0.490                  | 0.490                       | 0.490              | 0.490                 |
| Φ                                | 0.903                  | 1.174                       | 1.446              | 0.451                 |
| χ                                | 0.702                  | 0.550                       | 0.440              | 1.000                 |
| N <sub>b,Rd</sub>                | -55.217                | -24.352                     | -13.524            | 0.000                 |
| N <sub>c,Rd</sub>                | +78.676                | +44.255                     | +30.733            | +0.000                |
| SI/NO                            | NO                     | SI                          | SI                 | SI                    |
| N <sub>Rd</sub>                  | 78.676                 | -24.352                     | -13.524            | 0.000                 |
| N <sub>sd</sub>                  | 31.565                 | -15.783                     | 4.765              | 0.000                 |
| N <sub>sd</sub> /N <sub>Rd</sub> | 0.401                  | 0.648                       | 0.352              | 0.000                 |

**15.1.2 SBALZO LATERALE - SECONDA FASE DI GETTO**

Si considera una seconda fase di getto in cui la soletta viene gettata per i rimanenti 0.85 m e si dispone la veletta di chiusura, del peso di 2 kN/ml.

**VERIFICA LASTRE IN C.A. TRALICCIATE PREFABBRICATE**

**LASTRE PREFABBRICATE CAVALCAVIA ASSE 80 - CAMPATA A SBALZO: L= 185 cm - 2° FASE DI GETTO**

**DATI GENERALI LASTRA PREFABBRICATA.**

|                                                                                    |                                                   |
|------------------------------------------------------------------------------------|---------------------------------------------------|
| Schema vincolare in fase costruttiva:                                              | <b>A) Incastro - B) Estremo libero (a sbalzo)</b> |
| Luce di calcolo della lastra:                                                      | <b>L= 85 cm</b>                                   |
| Larghezza di calcolo della lastra:                                                 | <b>B<sub>c</sub>= 240 cm</b>                      |
| Altezza calcestruzzo lastra:                                                       | <b>H<sub>c</sub>= 6 cm</b>                        |
| Altezza della soletta (getto in opera):                                            | <b>H<sub>s</sub>= 24 cm</b>                       |
| Peso specifico calcestruzzo:                                                       | <b>g<sub>c</sub>= 25 kN/m<sup>3</sup></b>         |
| Vincolo laterale armature offerto dalla lastra presente nella sezione di verifica: | <b>NO</b>                                         |

**DATI TRALICCI E ARMATURA AGGIUNTIVA LASTRA PREFABBRICATA.**

|                                                                          |                                                |
|--------------------------------------------------------------------------|------------------------------------------------|
| Numero tralici per lastra:                                               | <b>N<sub>T</sub>= 4</b>                        |
| Altezza nominale del traliccio:                                          | <b>H<sub>T</sub>= 20.5 cm</b>                  |
| Altezza utile del traliccio:                                             | <b>H<sub>U</sub>= 18.8 cm</b>                  |
| Larghezza traliccio:                                                     | <b>B<sub>T</sub>= 12.0 cm</b>                  |
| Passo staffe:                                                            | <b>P<sub>ST</sub>= 20.0 cm</b>                 |
| Ricoprimento tralici e armature aggiuntive (dal bordo inferiore lastra): | <b>R<sub>IT</sub>= 4.0 cm</b>                  |
| Modulo elastico acciaio                                                  | <b>E<sub>s</sub>= 210 000 N/mm<sup>2</sup></b> |
| Coefficiente parziale di sicurezza dell'acciaio armature:                | <b>γ<sub>s</sub>= 1.15</b>                     |

**DATI ARMATURA LASTRA PREFABBRICATA.**

|                                                        | Singolo traliccio             |                 | Staffe        | Arm. agg. lastra |                   |
|--------------------------------------------------------|-------------------------------|-----------------|---------------|------------------|-------------------|
|                                                        | Corr. Superiore               | Corr. Inferiori |               | B450C            | B450C             |
| Resistenza caratteristica acciaio                      | <b>f<sub>yk</sub>= 450.00</b> | <b>450.00</b>   | <b>450.00</b> | <b>450.00</b>    | N/mm <sup>2</sup> |
| Resistenza di calcolo acciaio                          | <b>f<sub>yd</sub>= 391.30</b> | <b>391.30</b>   | <b>391.30</b> | <b>391.30</b>    | N/mm <sup>2</sup> |
| Diametro:                                              | <b>Φ= 16</b>                  | <b>12</b>       | <b>10</b>     | <b>0</b>         | mm                |
| Numero di ferri per traliccio (Arm. agg. per lastra):  | <b>N<sub>F</sub>= 1</b>       | <b>2</b>        | <b>2</b>      | <b>0</b>         |                   |
| Area di armatura per lastra:                           | <b>A<sub>s</sub>= 8.042</b>   | <b>9.048</b>    | <b>6.283</b>  | <b>0.000</b>     | cm <sup>2</sup>   |
| Inclinazione ferri in direzione longitudinale:         | <b>α= 0.00</b>                | <b>0.00</b>     | <b>70.97</b>  | <b>0.00</b>      | °                 |
| Inclinazione ferri in direzione trasversale:           | <b>γ= 0.00</b>                | <b>0.00</b>     | <b>80.93</b>  | <b>0.00</b>      | °                 |
| Coefficiente di vincolo:                               | <b>β= 1</b>                   | <b>1</b>        | <b>1</b>      | <b>1</b>         |                   |
| Lunghezza di calcolo per instabilità aste compresse    | <b>L= 20.00</b>               | <b>20.00</b>    | <b>20.14</b>  | <b>40.00</b>     | cm                |
| Lunghezza libera d'inflessione (L <sub>0</sub> = β*L): | <b>L<sub>0</sub>= 20.00</b>   | <b>20.00</b>    | <b>20.14</b>  | <b>40.00</b>     | cm                |

**DATI DI CARICO**

NOTA: Per le azioni in fase di costruzione durante il getto di calcestruzzo ci si riferisce a: UNI EN 1991-1-6, § 4.11.2.

|                                                                         |                             |
|-------------------------------------------------------------------------|-----------------------------|
| Coefficiente parziale dei carichi permanenti strutturali (peso lastra): | <b>γ<sub>g1</sub>= 1.35</b> |
| Coefficiente parziale dei carichi permanenti portati (getto in opera):  | <b>γ<sub>g2</sub>= 1.5</b>  |
| Coefficiente parziale dei carichi variabili (sovraccarichi):            | <b>γ<sub>q</sub>= 1.5</b>   |

|                                                                          |                                                                  |            |           |
|--------------------------------------------------------------------------|------------------------------------------------------------------|------------|-----------|
|                                                                          | Estensione del carico permanente (X1=X2 per carico concentrato): | Da X1 (cm) | A X2 (cm) |
| Peso strutturale lastra:                                                 | <b>G<sub>1</sub>= 1.50 kN/mq</b>                                 | <b>0</b>   | <b>85</b> |
| Permanente portato 1: getto in opera soletta:                            | <b>G<sub>2,1</sub>= 6.00 kN/mq</b>                               | <b>0</b>   | <b>85</b> |
| Permanente portato 2:                                                    | <b>G<sub>2,2</sub>= 2.00 kN/m</b>                                | <b>85</b>  | <b>85</b> |
| Permanente portato 3:                                                    | <b>G<sub>2,3</sub>= 0.00 kN/m</b>                                | <b>0</b>   | <b>0</b>  |
| Sovraccarico all'interno dell'area di lavoro di 3 m x 3 m (L se minore): | <b>Q<sub>1</sub>= 0.75 kN/mq</b>                                 |            |           |
| Sovraccarico all'esterno dell'area di lavoro di 3 m x 3 m (L se minore): | <b>Q<sub>2</sub>= 0.75 kN/mq</b>                                 |            |           |

**SOLLECITAZIONI ALLO S.L.U.** NOTA: Le sollecitazioni si intendono riferite alla larghezza della lastra.

|                                |                                    |
|--------------------------------|------------------------------------|
| Momento flettente di progetto: | <b>M<sub>sd</sub>= -16.65 KN.m</b> |
| Sforzo di taglio di progetto:  | <b>V<sub>sd</sub>= 31.99 KN</b>    |

**VERIFICA ALLO S.L.U.**

Sforzi assiali nei correnti (N<sub>sd</sub>=M<sub>sd</sub>/H<sub>U</sub>)  
Sforzi assiali nelle staffe (N<sub>sd</sub>=V<sub>sd</sub>/(Sen(α)\*Sen(γ)))

Carico elastico critico (N<sub>CR</sub>=ρ<sup>2</sup>\*E<sub>s</sub>\*J/L<sub>0</sub><sup>2</sup>)  
Snellezza adimensionale (λ = (A<sub>T</sub> f<sub>yk</sub>/N<sub>CR</sub>)<sup>0.5</sup>)  
Fattore di imperfezione (NTC 2008; tab. 4.2.VI)  
Coefficiente di stabilità delle aste compresse:  
Fattore di riduzione  
Resistenza di calcolo a compressione (instabilità)  
Resistenza di calcolo a trazione e compressione  
Considerare instabilità delle aste compresse (non incluse nel cls.)  
Resistenza di calcolo considerata (ferro singolo)  
Sforzo assiale di calcolo (ferro singolo)  
Coefficiente di sfruttamento (N<sub>sd</sub>/N<sub>Rd</sub> < 1: verificato)

|                                      | Corr. Superiore (Teso) | Corr. Inferiori (Compresso) | Staffe (Compresso) | Arm. Agg. (Compresso) |
|--------------------------------------|------------------------|-----------------------------|--------------------|-----------------------|
| <b>N<sub>CR</sub></b>                | 166.690                | 52.742                      | 25.088             | 0.000                 |
| <b>λ</b>                             | 0.737                  | 0.982                       | 1.187              | 0.000                 |
| <b>α</b>                             | 0.490                  | 0.490                       | 0.490              | 0.490                 |
| <b>Φ</b>                             | 0.903                  | 1.174                       | 1.446              | 0.451                 |
| <b>χ</b>                             | 0.702                  | 0.550                       | 0.440              | 1.000                 |
| <b>N<sub>b,Rd</sub></b>              | -55.217                | -24.352                     | -13.524            | 0.000                 |
| <b>N<sub>c,Rd</sub></b>              | ±78.676                | ±44.255                     | ±30.733            | ±0.000                |
| <b>SI/NO</b>                         | NO                     | SI                          | SI                 | SI                    |
| <b>N<sub>Rd</sub></b>                | 78.676                 | -24.352                     | -13.524            | 0.000                 |
| <b>N<sub>sd</sub></b>                | 22.146                 | -11.073                     | 4.283              | 0.000                 |
| <b>N<sub>sd</sub>/N<sub>Rd</sub></b> | 0.281                  | 0.455                       | 0.317              | 0.000                 |

**15.1.3 FASE COSTRUTTIVA - CAMPATA TRA LE TRAVI**

La lastra è semplicemente appoggiata agli estremi sulla luce di 280 cm.

**VERIFICA LASTRE IN C.A. TRALICCIATE PREFABBRICATE**

**LASTRE PREFABBRICATE CAVALCAVIA ASSE 80 - TRA TRAVI: L= 280 cm**

**DATI GENERALI LASTRA PREFABBRICATA.**

Schema vincolare in fase costruttiva:  
Luce di calcolo della lastra:  
Larghezza di calcolo della lastra:  
Altezza calcestruzzo lastra:  
Altezza della soletta (getto in opera):  
Peso specifico calcestruzzo:  
Vincolo laterale armature offerto dalla lastra presente nella sezione di verifica:

**A) Appoggio - B) Appoggio**

L= 280 cm  
B<sub>c</sub>= 240 cm  
H<sub>c</sub>= 6 cm  
H<sub>s</sub>= 36 cm  
g<sub>c</sub>= 25 kN/m<sup>3</sup>  
SI

**DATI TRALICCI E ARMATURA AGGIUNTIVA LASTRA PREFABBRICATA.**

Numero tralici per lastra: N<sub>T</sub>= 4  
Altezza nominale del traliccio: H<sub>T</sub>= 20.5 cm  
Altezza utile del traliccio: H<sub>U</sub>= 18.8 cm  
Larghezza traliccio: B<sub>T</sub>= 12.0 cm  
Passo staffe: P<sub>ST</sub>= 20.0 cm  
Ricoprimento tralici e armature aggiuntive (dal bordo inferiore lastra): R<sub>IT</sub>= 4.0 cm  
Modulo elastico acciaio: E<sub>s</sub>= 210 000 N/mm<sup>2</sup>  
Coefficiente parziale di sicurezza dell'acciaio armature: γ<sub>s</sub>= 1.15

**DATI ARMATURA LASTRA PREFABBRICATA.**

Tipo di acciaio  
Resistenza caratteristica acciaio  
Resistenza di calcolo acciaio  
Diametro:  
Numero di ferri per traliccio (Arm. agg. per lastra):  
Area di armatura per lastra:  
Inclinazione ferri in direzione longitudinale:  
Inclinazione ferri in direzione trasversale:  
Coefficiente di vincolo:  
Lunghezza di calcolo per instabilità aste compresse  
Lunghezza libera d'inflexione (L<sub>0</sub> = β\*L):

|                   | Singolo traliccio |                 | Staffe | Arm. agg. lastra |                   |
|-------------------|-------------------|-----------------|--------|------------------|-------------------|
|                   | Corr. Superiore   | Corr. Inferiori |        | B450C            | B450C             |
| f <sub>yk</sub> = | 450.00            | 450.00          | 450.00 | 450.00           | N/mm <sup>2</sup> |
| f <sub>yd</sub> = | 391.30            | 391.30          | 391.30 | 391.30           | N/mm <sup>2</sup> |
| Φ=                | 16                | 12              | 10     | 0                | mm                |
| N <sub>T</sub> =  | 1                 | 2               | 2      | 0                |                   |
| A <sub>s</sub> =  | 8.042             | 9.048           | 6.283  | 0.000            | cm <sup>2</sup>   |
| α=                | 0.00              | 0.00            | 70.97  | 0.00             | °                 |
| γ=                | 0.00              | 0.00            | 80.93  | 0.00             | °                 |
| β=                | 1                 | 1               | 1      | 1                |                   |
| L=                | 20.00             | 20.00           | 20.14  | 40.00            | cm                |
| L <sub>0</sub> =  | 20.00             | 20.00           | 20.14  | 40.00            | cm                |

**DATI DI CARICO**

NOTA: Per le azioni in fase di costruzione durante il getto di calcestruzzo ci si riferisce a: UNI EN 1991-1-6, § 4.11.2.

Coefficiente parziale dei carichi permanenti strutturali (peso lastra): γ<sub>g1</sub>= 1.35  
Coefficiente parziale dei carichi permanenti portati (getto in opera): γ<sub>g2</sub>= 1.5  
Coefficiente parziale dei carichi variabili (sovraccarichi): γ<sub>q</sub>= 1.5

Estensione del carico permanente (X1=X2 per carico concentrato): Da X1 (cm) A X2 (cm)

Peso strutturale lastra: G<sub>1</sub>= 1.50 kN/mq 0 280  
 Permanente portato 1: getto in opera soletta: G<sub>2,1</sub>= 9.00 kN/mq 0 280  
 Permanente portato 2: G<sub>2,2</sub>= 0.00 kN/m 0 0  
 Permanente portato 3: G<sub>2,3</sub>= 0.00 kN/m 0 0  
 Sovraccarico all'interno dell'area di lavoro di 3 m x 3 m (L se minore): Q<sub>1</sub>= 0.9 kN/mq  
 Sovraccarico all'esterno dell'area di lavoro di 3 m x 3 m (L se minore): Q<sub>2</sub>= 0.75 kN/mq

**SOLLECITAZIONI ALLO S.L.U.** NOTA: Le sollecitazioni si intendono riferite alla larghezza della lastra.

Momento flettente di progetto: M<sub>sd</sub>= 39.69 KN.m  
Sforzo di taglio di progetto: V<sub>sd</sub>= 56.70 KN

**VERIFICA ALLO S.L.U.**

Sforzi assiali nei correnti (N<sub>sd</sub>=M<sub>sd</sub>/H<sub>U</sub>)  
Sforzi assiali nelle staffe (N<sub>sd</sub>=V<sub>sd</sub>/(Sen(α)\*Sen(γ)))

Carico elastico critico (N<sub>CR</sub>=p<sup>2</sup>\*E<sub>s</sub>\*J/L<sub>0</sub><sup>2</sup>)  
Snelzza adimensionale (λ = (A<sub>T</sub> f<sub>yk</sub>/N<sub>CR</sub>)<sup>0.5</sup>)  
Fattore di imperfezione (NTC 2008; tab. 4.2.VI)  
Coefficiente di stabilità delle aste compresse:  
Fattore di riduzione  
Resistenza di calcolo a compressione (instabilità)  
Resistenza di calcolo a trazione e compressione  
Considerare instabilità delle aste compresse (non incluse nel cls.)  
Resistenza di calcolo considerata (ferro singolo)  
Sforzo assiale di calcolo (ferro singolo)  
Coefficiente di sfruttamento (N<sub>sd</sub>/N<sub>Rd</sub> < 1: verificato)

|                                    | Corr. Superiore (Compresso) | Corr. Inferiori (Teso) | Staffe (Compresso) | Arm. Agg. (Teso) |
|------------------------------------|-----------------------------|------------------------|--------------------|------------------|
| N <sub>CR</sub> =                  | 166.690                     | 52.742                 | 25.088             | 0.000            |
| λ=                                 | 0.737                       | 0.982                  | 1.187              | 0.000            |
| α=                                 | 0.490                       | 0.490                  | 0.490              | 0.490            |
| Φ=                                 | 0.903                       | 1.174                  | 1.446              | 0.451            |
| χ=                                 | 0.702                       | 0.550                  | 0.440              | 1.000            |
| N <sub>b,Rd</sub> =                | -55.217                     | -24.352                | -13.524            | 0.000            |
| N <sub>c,Rd</sub> =                | ±78.676                     | ±44.255                | ±30.733            | ±0.000           |
| SI/NO                              | SI                          | NO                     | SI                 | NO               |
| N <sub>Rd</sub> =                  | -55.217                     | 44.255                 | -13.524            | 0.000            |
| N <sub>sd</sub> =                  | -52.779                     | 26.390                 | 7.592              | 0.000            |
| N <sub>sd</sub> /N <sub>Rd</sub> = | 0.956                       | 0.596                  | 0.561              | 0.000            |

**15.2 VERIFICA IN ESERCIZIO - SBALZO LATERALE**

La verifica viene svolta per lo sbalzo avente una luce L= 1.85 m dall'asse trave.

La determinazione delle sollecitazioni massime viene svolta con apposito foglio di calcolo considerando i carichi permanenti e tutte le possibili colonne di carico mobile, compatibilmente con lo sbalzo considerato.

Si considerano in alternativa:

- colonne di schemi di carico 1
- schema di carico 2

Inoltre si considera una combinazione eccezionale con i carichi di cui sopra (con coefficienti SLU unitari) e l'effetto di urto sul sicurvia

Le singole ruote dei carichi mobili vengono disposte in modo da ottenere o il momento flettente massimo o lo sforzo di taglio massimo alla sezione di incastro.

Verifica allo stato limite ultimo: si determina l'area di armatura minima necessaria (verifica a flessione) e l'area minima di armatura trasversale (verifica a taglio).

Verifica allo stato limite di esercizio: si effettua la verifica allo stato limite di fessurazione per via indiretta dimensionando l'armatura minima lavorante ad una definita tensione, funzione del diametro massimo delle barre e dell'apertura ammissibile delle fessure; nel presente caso si ha:

- condizioni ambientali: Molto aggressive
- apertura max fessure, combinazioni frequenti:  $w_1 = 0.2 \text{ mm}$
- apertura max fessure, combinazioni quasi permanenti:  $w_1 = 0.2 \text{ mm}$
- diametro massimo delle barre:  $\varnothing = 25 \text{ mm}$

Dalla tabella C4.1.II della Circ. 02/02/2009 n°617 si ottiene:

- Tensione di progetto nell'acciaio:  $\sigma_s = 160 \text{ N/mm}^2$

**Verifica dello sbalzo trasversale: sezione corrente**

**L= 1.850 m**

|                                                                  |                                        |         |            |         |            |  |            |  |                          |
|------------------------------------------------------------------|----------------------------------------|---------|------------|---------|------------|--|------------|--|--------------------------|
| Altezza della lastra prefabbricata                               |                                        |         |            |         |            |  |            |  | 0.060 m                  |
| Altezza totale soletta all'incastro                              |                                        |         |            |         |            |  |            |  | 0.300 m                  |
| Altezza totale soletta all'estremità libera                      |                                        |         |            |         |            |  |            |  | 0.300 m                  |
| Lunghezza di calcolo dello sbalzo                                |                                        |         |            |         |            |  |            |  | 1.850 m                  |
| Larghezza del marciapiede                                        |                                        |         |            |         |            |  |            |  | 1.900 m                  |
| Altezza del marciapiede                                          |                                        |         |            |         |            |  |            |  | 0.150 m                  |
| Peso specifico calcestruzzo armato                               |                                        |         |            |         |            |  |            |  | 25.000 kN/m <sup>3</sup> |
| Altezza della massicciata                                        |                                        |         |            |         |            |  |            |  | 0.100 m                  |
| Peso della massicciata                                           |                                        |         |            |         |            |  |            |  | 2.500 kN/m <sup>2</sup>  |
| Peso del sicurvìa                                                |                                        |         |            |         |            |  |            |  | 0.800 kN/m               |
| Distanza asse sicurvìa da incastro                               |                                        |         |            |         |            |  |            |  | 0.450 m                  |
| Peso del parapetto                                               |                                        |         |            |         |            |  |            |  | 0.500 kN/m               |
| Distanza asse parapetto da incastro                              |                                        |         |            |         |            |  |            |  | 1.750 m                  |
| Peso della veletta                                               |                                        |         |            |         |            |  |            |  | 2.000 kN/m               |
| Distanza asse veletta da incastro                                |                                        |         |            |         |            |  |            |  | 1.900 m                  |
| Carico della folla (di combinazione)                             |                                        |         |            |         |            |  |            |  | 6.000 kN/m               |
| Distanza asse folla da incastro                                  |                                        |         |            |         |            |  |            |  | 1.050 m                  |
| Coefficiente dinamico                                            |                                        |         |            |         |            |  |            |  | 1.000                    |
| Schema di carico 1:                                              | Larghezza delle colonne di carico      |         |            |         |            |  |            |  | 3.000 m                  |
|                                                                  | Interasse ruote in senso trasversale   |         |            |         |            |  |            |  | 2.000 m                  |
|                                                                  | Interasse ruote in senso longitudinale |         |            |         |            |  |            |  | 1.200 m                  |
|                                                                  | Lati dell'impronta ruota: L=           | 0.400 m |            | T=      | 0.400 m    |  |            |  |                          |
|                                                                  |                                        |         | 1° colonna |         | 2° colonna |  | 3° colonna |  |                          |
|                                                                  | Peso singola ruota                     | 150.000 |            | 100.000 |            |  |            |  | 50.000 kN                |
|                                                                  | Carico distribuito                     | 9.000   |            | 2.500   |            |  |            |  | 2.500 kN/m <sup>2</sup>  |
| Schema di carico 2:                                              | Interasse ruote in senso trasversale   |         |            |         |            |  |            |  | 2.000 m                  |
|                                                                  | Interasse ruote in senso longitudinale |         |            |         |            |  |            |  | 0.000 m                  |
|                                                                  | Lati dell'impronta ruota: L=           | 0.350 m |            | T=      | 0.600 m    |  |            |  |                          |
|                                                                  | Peso singola ruota                     |         |            |         |            |  |            |  | 200.000 kN               |
| Angolo di ripartizione verticale dei carichi nello spessore      |                                        |         |            |         |            |  |            |  | 45.000 °                 |
| Angolo di ripartizione orizzontale dei carichi (effetto piastra) |                                        |         |            |         |            |  |            |  | 45.000 °                 |

Tabella dei carichi mobili

|            |                                                    |
|------------|----------------------------------------------------|
| n° ruote = | numero ruote carico considerato                    |
| b=         | braccio del carico in esame (risultante)           |
| lrip =     | larghezza di ripartizione del carico ruote         |
| q=         | carico distribuito sulla larghezza di ripartizione |

|            | n° carico              | n° ruote | Massimo momento flettente |          |           | Massimo sforzo di taglio |          |           |
|------------|------------------------|----------|---------------------------|----------|-----------|--------------------------|----------|-----------|
|            |                        |          | bm (m)                    | lrip (m) | qm (kN/m) | bt (m)                   | lrip (m) | qt (kN/m) |
| 1° colonna | Schema 1 - distribuito |          | 0.000                     | 1.000    | 0.000     | 0.000                    | 1.000    | 0.000     |
|            | Schema 1 - 1° fila     | 1        | 0.000                     | 0.840    | 0.000     | 0.000                    | 0.840    | 0.000     |
|            | Schema 1 - 1° fila     | 2        | 0.000                     | 2.040    | 0.000     | 0.000                    | 2.040    | 0.000     |
|            | Schema 1 - 2° fila     | 2        | 0.000                     | 2.040    | 0.000     | 0.000                    | 2.040    | 0.000     |
| 2° colonna | Schema 1 - distribuito |          | 0.000                     | 1.000    | 0.000     | 0.000                    | 1.000    | 0.000     |
|            | Schema 1 - 1° fila     | 1        | 0.000                     | 0.840    | 0.000     | 0.000                    | 0.840    | 0.000     |
|            | Schema 1 - 1° fila     | 2        | 0.000                     | 2.040    | 0.000     | 0.000                    | 2.040    | 0.000     |
|            | Schema 1 - 2° fila     | 2        | 0.000                     | 2.040    | 0.000     | 0.000                    | 2.040    | 0.000     |
| 3° colonna | Schema 1 - distribuito |          | 0.000                     | 1.000    | 0.000     | 0.000                    | 1.000    | 0.000     |
|            | Schema 1 - 1° fila     | 1        | 0.000                     | 0.840    | 0.000     | 0.000                    | 0.840    | 0.000     |
|            | Schema 1 - 1° fila     | 2        | 0.000                     | 2.040    | 0.000     | 0.000                    | 2.040    | 0.000     |
|            | Schema 1 - 2° fila     | 2        | 0.000                     | 2.040    | 0.000     | 0.000                    | 2.040    | 0.000     |
|            | Schema 2 - 1° fila     | 1        | 0.000                     | 0.000    | 0.000     | 0.000                    | 0.790    | 0.000     |
|            | Schema 2 - 2° fila     | 1        | 0.000                     | 0.000    | 0.000     | 0.000                    | 0.790    | 0.000     |

|                                                  |            |
|--------------------------------------------------|------------|
| Effetto d'urto sul sicurvìa (azione eccezionale) | 100.000 kN |
| Altezza della forza d'urto da estradosso soletta | 1.150 m    |
| Larghezza di ripartizione urto                   | 1.940 m    |

**Sollecitazioni all'incastro; sezione corrente**

| Tipo di carico            | Tagli (kN/m) | Momenti (kNm/m) | Sf.norm (kN/m) |
|---------------------------|--------------|-----------------|----------------|
| Peso proprio (rettangolo) | 13.875       | -12.834         | 0.000          |
| Peso proprio (triangolo)  | 0.000        | 0.000           | 0.000          |
| Marciapiede               | 7.125        | -6.413          | 0.000          |
| Massicciata               | -0.125       | -0.003          | 0.000          |
| Sicurvia                  | 0.800        | -0.360          | 0.000          |
| Parapetto                 | 0.500        | -0.875          | 0.000          |
| Veletta                   | 2.000        | -3.800          | 0.000          |
| Folla                     | 6.000        | -6.300          | 0.000          |
| Schema 1 - distribuito    | 0.000        | 0.000           | 0.000          |
| Schema 1 (tandem max)     | 0.000        | 0.000           | 0.000          |
| Schema 2 (max)            | 0.000        | 0.000           | 0.000          |
| Urto                      | 0.000        | -67.010         | 51.546         |

| Coefficienti parziali ( $\gamma \times \psi$ ) | SLU   | SLU ECC: | SLE (rare) | SLE (frequenti) | SLE (quasi perm.) |
|------------------------------------------------|-------|----------|------------|-----------------|-------------------|
| Carichi permanenti                             | 1.350 | 1.000    | 1.000      | 1.000           | 1.000             |
| Carichi mobili - Tandem                        | 1.350 | 1.000    | 1.000      | 0.750           | 0.000             |
| Carichi mobili - Distribuiti                   | 1.350 | 1.000    | 1.000      | 0.400           | 0.000             |

**Sollecitazioni complessive all'incastro; sezione corrente**

| Tipo di carico                         | Tagli (kN/m) | Momenti (kNm/m) | Sf.norm (kN/m) |
|----------------------------------------|--------------|-----------------|----------------|
| SLU Totali perm.+ mobili               | 40.736       | -41.290         | 0.000          |
| SLU ECC. Totali perm.+ mobili + urto   | 24.175       | -91.295         | 51.546         |
| SLE (rare) Totali perm.+ mobili        | 30.175       | -30.585         | 0.000          |
| SLE (frequenti) Totali perm.+ mobili   | 26.575       | -26.805         | 0.000          |
| SLE (quasi perm.) Totali perm.+ mobili | 24.175       | -24.285         | 0.000          |

**Dimensionamento armature minime allo SLU**

|                                                     |            |                          |
|-----------------------------------------------------|------------|--------------------------|
| Resistenza di calcolo calcestruzzo:                 | fcd=       | 18.13 N/mm <sup>2</sup>  |
| Resistenza di calcolo armatura:                     | fsd=       | 391.30 N/mm <sup>2</sup> |
| Modulo di elasticità armatura:                      | Es=        | 210000 N/mm <sup>2</sup> |
| Ricoprimento armature:                              | Ry=        | 50 mm                    |
| Inclinazione delle staffe (45° <= α <= 90°)         | α=         | 90.00 °                  |
| Inclinazione di calcolo dei puntoni di calcestruzzo | θ =        | 21.80 °                  |
| Altezza utile della sezione di incastro             | d=         | 25.00 cm                 |
| Altezza minima sezione reagente:                    | X=         | 2.47 cm                  |
| Area di armatura tesa minima:                       | As(sup)=   | 10.49 cm <sup>2</sup> /m |
| Area di armatura compressa minima:                  | As(inf)=   | 0.00 cm <sup>2</sup> /m  |
| Verifica resistenza al taglio, solo cls.            | Vrd,c=     | 129.06 kN                |
| Coefficiente di sicurezza, solo cls.                | Vrd,c/Vsd= | 3.168 >1: Verificato     |
| Area di armatura trasversale necessaria per taglio: | Ast=       | N.D. cm <sup>2</sup> /m  |

**Dimensionamento armature minime allo SLE**

|                                    |          | SLE (frequenti) | SLE (quasi perm.)       |
|------------------------------------|----------|-----------------|-------------------------|
| Valore limite di apertura fessure: | wd=      | 0.2             | 0.2 mm                  |
| Tensione di calcolo armatura:      | Sa=      | 160             | 160 N/mm <sup>2</sup>   |
| Tensione nel calcestruzzo          | Sc=      | 3.67            | 3.46 N/mm <sup>2</sup>  |
| Area di armatura tesa minima:      | As(sup)= | 7.33            | 6.61 cm <sup>2</sup> /m |



### 15.3 VERIFICA IN ESERCIZIO - SOLETTA INTERNA TRA LE TRAVI

La soletta tra le travi si schematizza come trave semi-incastata di luce 3.20 m per una larghezza di calcolo unitaria.

|                                                                            |                                                                              |            |            |                          |
|----------------------------------------------------------------------------|------------------------------------------------------------------------------|------------|------------|--------------------------|
| <b>Verifica soletta tra le travi(o su travi a canaletta)</b>               |                                                                              | <b>L =</b> |            | <b>3.200 m</b>           |
| Altezza della lastra prefabbricata                                         |                                                                              |            |            | 0.060 m                  |
| Altezza della soletta                                                      |                                                                              |            |            | 0.360 m                  |
| Luce di calcolo                                                            |                                                                              |            |            | 3.200 m                  |
| Peso specifico calcestruzzo armato                                         |                                                                              |            |            | 25.000 kN/m <sup>3</sup> |
| Altezza della massicciata                                                  |                                                                              |            |            | 0.100 m                  |
| Peso della massicciata                                                     |                                                                              |            |            | 2.500 kN/m <sup>2</sup>  |
| Coefficiente dinamico                                                      |                                                                              |            |            | 1.000                    |
| Schema di carico 1:                                                        | Larghezza delle colonne di carico                                            |            |            | 3.000 m                  |
|                                                                            | Interasse ruote in senso trasversale                                         |            |            | 2.000 m                  |
|                                                                            | Interasse ruote in senso longitudinale                                       |            |            | 1.200 m                  |
|                                                                            | Lati dell'impronta ruota: L=                                                 | 0.400 m    | T=         | 0.400 m                  |
|                                                                            |                                                                              | 1° colonna | 2° colonna | 3° colonna               |
|                                                                            | Peso singola ruota                                                           | 150.000    | 100.000    | 50.000 kN                |
|                                                                            | Carico distribuito                                                           | 9.000      | 2.500      | 2.500 kN/m <sup>2</sup>  |
| Schema di carico 2:                                                        | Interasse ruote in senso trasversale                                         |            |            | 2.000 m                  |
|                                                                            | Interasse ruote in senso longitudinale                                       |            |            | 0.000 m                  |
|                                                                            | Lati dell'impronta ruota: L=                                                 | 0.350 m    | T=         | 0.600 m                  |
|                                                                            | Peso singola ruota                                                           |            |            | 200.000 kN               |
| Angolo di ripartizione verticale dei carichi nello spessore                |                                                                              |            |            | 45.000 °                 |
| Angolo di ripartizione orizzontale dei carichi (effetto piastra)           |                                                                              |            |            | 45.000 °                 |
| Efficienza minima dei vncoli di incastro (0=appoggio; 1=incastro perfetto) |                                                                              |            |            | 0.750                    |
| Tabella dei carichi mobili                                                 |                                                                              |            |            |                          |
| n° ruote =                                                                 | numero ruote carico considerato                                              |            |            |                          |
| xm =                                                                       | distanza dall'incastro del carico in esame (disposizione di momento massimo) |            |            |                          |
| xt =                                                                       | distanza dall'incastro del carico in esame (disposizione di taglio massimo)  |            |            |                          |
| lrip =                                                                     | larghezza di ripartizione del carico ruote                                   |            |            |                          |
| q=                                                                         | carico distribuito sulla larghezza di ripartizione                           |            |            |                          |

|            | n° carico              | n° ruote | Massimo momento flettente |          |           | Massimo sforzo di taglio |          |           |
|------------|------------------------|----------|---------------------------|----------|-----------|--------------------------|----------|-----------|
|            |                        |          | xm (m)                    | lrip (m) | qm (kN/m) | xt (m)                   | lrip (m) | qt (kN/m) |
| 1° colonna | Schema 1 - distribuito |          | 2.150                     | 1.000    | 18.900    | 1.620                    | 1.000    | 27.000    |
|            | Schema 1 - 1° fila     | 1        | 1.600                     | 2.500    | 60.000    | 0.620                    | 2.500    | 60.000    |
|            | Schema 1 - 1° fila     | 2        | 1.600                     | 3.700    | 81.081    | 0.620                    | 3.700    | 81.081    |
|            | Schema 1 - 2° fila     | 2        | 0.000                     | 0.000    | 0.000     | 2.620                    | 3.700    | 81.081    |
| 2° colonna | Schema 1 - distribuito |          | 1.500                     | 1.000    | 7.500     | 2.950                    | 1.000    | 1.250     |
|            | Schema 1 - 1° fila     | 1        | 0.600                     | 2.500    | 40.000    | 3.200                    | 2.500    | 40.000    |
|            | Schema 1 - 1° fila     | 2        | 0.600                     | 3.700    | 54.054    | 3.200                    | 3.700    | 54.054    |
|            | Schema 1 - 2° fila     | 2        | 0.000                     | 0.000    | 0.000     | 0.000                    | 0.000    | 0.000     |
| 3° colonna | Schema 1 - distribuito |          | 0.000                     | 1.000    | 0.000     | 0.000                    | 1.000    | 0.000     |
|            | Schema 1 - 1° fila     | 1        | 0.000                     | 0.000    | 0.000     | 0.000                    | 0.000    | 0.000     |
|            | Schema 1 - 1° fila     | 2        | 0.000                     | 0.000    | 0.000     | 0.000                    | 0.000    | 0.000     |
|            | Schema 1 - 2° fila     | 2        | 0.000                     | 0.000    | 0.000     | 0.000                    | 0.000    | 0.000     |
|            | Schema 2 - 1° fila     | 1        | 1.600                     | 2.450    | 81.633    | 0.620                    | 2.450    | 81.633    |
|            | Schema 2 - 2° fila     | 1        | 0.000                     | 0.000    | 0.000     | 2.620                    | 2.450    | 81.633    |

#### Sollecitazioni massime/minime

| Tipo di carico         | Taglio max (kN/m) | Momento max (kNm/m) | Momento min (kNm/m) |
|------------------------|-------------------|---------------------|---------------------|
| Peso soletta           | 14.400            | 5.760               | -7.680              |
| Massicciata            | 4.000             | 1.600               | -2.133              |
| Schema 1 - distribuito | 14.901            | 6.204               | -8.885              |
| Schema 1 (max/min)     | 134.952           | 38.822              | -51.349             |
| Schema 2 (max/min)     | 82.505            | 30.556              | -37.450             |

|                              |                  |            |                   |                        |                          |
|------------------------------|------------------|------------|-------------------|------------------------|--------------------------|
| <b>Coefficienti parziali</b> | <b>( γ X ψ )</b> | <b>SLU</b> | <b>SLE (rare)</b> | <b>SLE (frequenti)</b> | <b>SLE (quasi perm.)</b> |
| Carichi permanenti           |                  | 1.350      | 1.000             | 1.000                  | 1.000                    |
| Carichi mobili - Tandem      |                  | 1.350      | 1.000             | 0.750                  | 0.000                    |
| Carichi mobili - Distribuiti |                  | 1.350      | 1.000             | 0.400                  | 0.000                    |

**Sollecitazioni compressive massime/minime**

| Tipo di carico                         | Taglio max (kN/m) | Momento max (kNm/m) | Momento min (kNm/m) |
|----------------------------------------|-------------------|---------------------|---------------------|
| SLU Totali perm.+ mobili               | 227.142           | 70.722              | -94.564             |
| SLE (rare) Totali perm.+ mobili        | 168.253           | 52.387              | -70.047             |
| SLE (frequenti) Totali perm.+ mobili   | 125.575           | 38.959              | -51.879             |
| SLE (quasi perm.) Totali perm.+ mobili | 18.400            | 7.360               | -9.813              |

**Dimensionamento armature minime allo SLU**

|                                                     |          |                             |
|-----------------------------------------------------|----------|-----------------------------|
| Resistenza di calcolo calcestruzzo:                 | fcd=     | 19.83 N/mm <sup>2</sup>     |
| Resistenza di calcolo armatura:                     | fsd=     | 391.30 N/mm <sup>2</sup>    |
| Modulo di elasticità armatura:                      | Es=      | 210000.00 N/mm <sup>2</sup> |
| Ricoprimento armature:                              | Ry(sup)= | 50 mm                       |
|                                                     | Ry(inf)= | 70 mm                       |
| Inclinazione delle staffe (45° <= α <= 90°)         | α=       | 90.00 °                     |
| Inclinazione di calcolo dei puntoni di calcestruzzo | θ =      | 21.80 °                     |

|                                                |            | Momento max | Momento min               |
|------------------------------------------------|------------|-------------|---------------------------|
| Altezza utile della sezione                    | d=         | 29.00       | 31.00 cm                  |
| Altezza sezione reagente:                      | X=         | 1.57        | 1.97 cm                   |
| Area di armatura superiore minima:             | As(sup)=   | 0.00        | 8.00 cm <sup>2</sup> /m   |
| Area di armatura inferiore minima:             | As(inf)=   | 6.37        | 0.00 cm <sup>2</sup> /m   |
| Verifica resistenza al taglio, solo cls.       | Vrd,c=     |             | 155.43 kN                 |
| Coefficiente di sicurezza, solo cls.           | Vrd,c/Vsd= |             | 0.684 < 1: NON Verificato |
| Area di armatura trasversale necessaria per ta | Ast=       |             | 8.32 cm <sup>2</sup> /mq  |

**Dimensionamento armature minime allo SLE (frequenti)**

|                                    |          | Momento max | Momento min              |
|------------------------------------|----------|-------------|--------------------------|
| Valore limite di apertura fessure: | wd=      | 0.2         | 0.2 mm                   |
| Tensione di calcolo armatura:      | Sa=      | 160         | 160 N/mm <sup>2</sup>    |
| Tensione nel calcestruzzo          | Sc=      | 3.84        | 4.21 N/mm <sup>2</sup>   |
| Area di armatura superiore minima: | As(sup)= | 0.00        | 11.55 cm <sup>2</sup> /m |
| Area di armatura inferiore minima: | As(inf)= | 9.21        | 0.00 cm <sup>2</sup> /m  |

## 15.4 VERIFICA CORDOLI

Si effettua la verifica della resistenza a taglio delle armature di collegamento tra la soletta e il cordolo sotto l'effetto dell'urto sul sicurvia; si ha:

- effetto di urto sul sicurvia:  $H = 100 \text{ kN}$
- larghezza di applicazione:  $b = 0.50 \text{ m}$
- area armature:  $1 + 1 \text{ } \varnothing 16 / 20: A_1 = 2 \times 201 \times 50 / 20 = 1005 \text{ mm}^2$   
 $1 \text{ } \varnothing 12 / 20 / 40: A_2 = 4 \times 113 \times 50 / 20 = 1130 \text{ mm}^2$   
 $A = 1005 + 283 = 2135 \text{ mm}^2$
- resistenza tangenziale di calcolo:  $t_d = \frac{450}{1.15\sqrt{3}} = 226 \text{ N/mm}^2$
- resistenza di calcolo:  $R = 226 \times 2.135 = 482.5 \text{ kN}$  (Verificato:  $R > H$ )

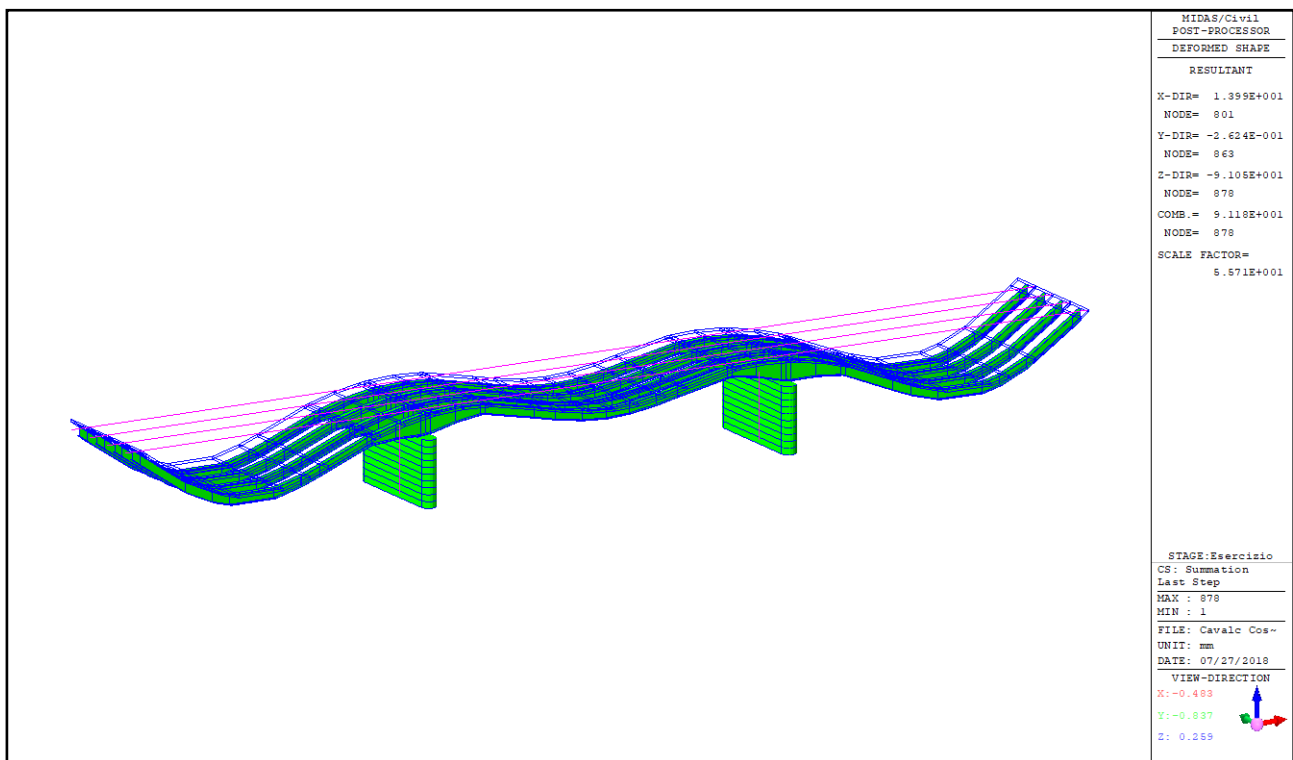
## 16 VERIFICHE DI DEFORMAZIONE TRAVI

### 16.1 DEFORMAZIONE CARICHI PERMANENTI

Si verifica che la deformata minima dovuta ai carichi permanenti sia minore di:

- campate laterali:  $\Delta Z_{lim,L} = L / 300 = 31000 / 300 = 103.3 \text{ mm.}$
- campata centrale:  $\Delta Z_{lim,C} = L / 300 = 38000 / 300 = 126.7 \text{ mm.}$

Segue diagramma delle deformazioni minime per la combinazione considerata.



DEFORMATA MINIMA CARICHI PERMANENTI

Il valore massimo (in modulo) della deformazione vale:

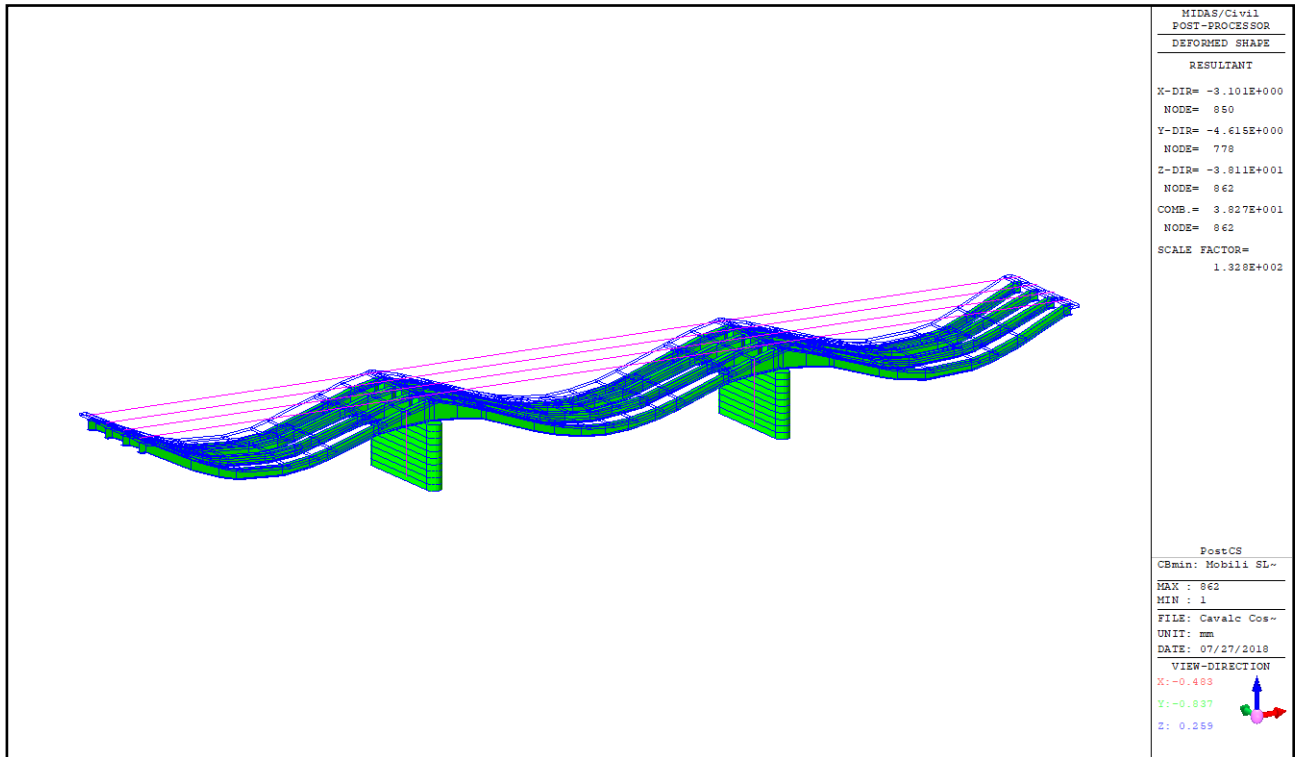
- campate laterali:  $\Delta Z = 91.1 \text{ mm, minore del valore limite } \Delta Z_{lim,L} = 103.3 \text{ mm.}$
- campata centrale:  $\Delta Z = 38.8 \text{ mm, minore del valore limite } \Delta Z_{lim,L} = 126.7 \text{ mm.}$

### 16.2 DEFORMAZIONE CARICHI VARIABILI

Si verifica che la deformata minima dei carichi variabili di progetto, nella combinazione caratteristica (rara) sia minore di:

- campate laterali:  $\Delta Z_{lim,L} = L / 700 = 31000 / 700 = 44.3 \text{ mm.}$
- campata centrale:  $\Delta Z_{lim,C} = L / 700 = 38000 / 700 = 54.3 \text{ mm.}$

Segue diagramma delle deformazioni minime per la combinazione considerata; si nota che il diagramma risulta dall'involuppo di diverse condizioni di carico mobile, quindi i valori sono corretti ma la "forma" della deformata non è significativa.



DEFORMATA MINIMA CARICHI MOBILI (INVILUPPO MINIMI)

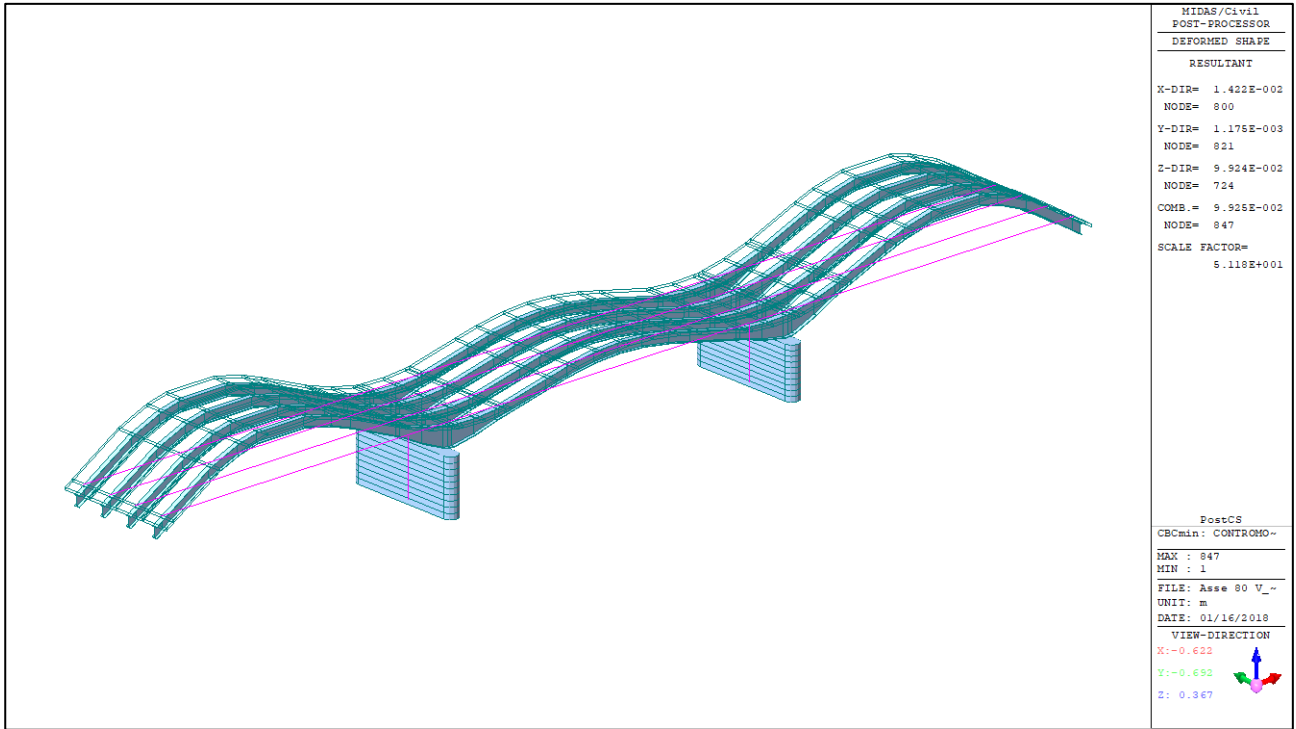
Il valore massimo (in modulo) della deformazione vale:

- campate laterali:  $\Delta Z = 33.1$  mm, minore del valore limite  $\Delta Z_{lim,L} = 44.3$  mm.
- campata centrale:  $\Delta Z = 38.1$  mm, minore del valore limite  $\Delta Z_{lim,L} = 54.3$  mm.

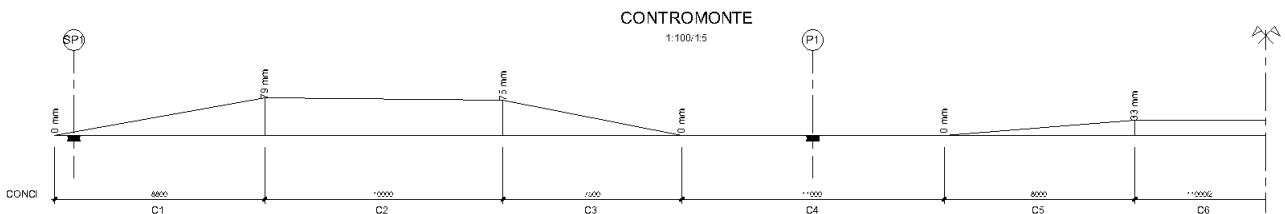
## 17 CONTROMONTE TRAVI

Le travature vengono realizzate con una controfreccia pari all'inverso delle deformazioni permanenti più il 25% dei carichi mobili, nella combinazione caratteristica; l'andamento dei singoli conci è rettilineo.

Nel diagramma a pagina seguente sono evidenziate le deformazioni da ottenere.



DEFORMATA - CONTROMONTE DI PROGETTO



CONTROMONTE EFFETTIVE (1/2 IMPALCATO)

## 18 VERIFICA DEI CONTENUTI DI CUI AL PAR. 10.2 DELLE N.T.C. 2008

- Tipo di analisi svolta
- Analisi strutturale condotta di tipo statico e dinamico lineare
- Metodo adottato per la risoluzione del problema strutturale: Metodo agli elementi finiti con software Midas/Civil 2015 v.1.2.
- Metodologie seguite per la verifica o per il progetto-verifica delle sezioni. Metodo semiprobabilistico agli Stati Limite
- Combinazioni di carico adottate: Le combinazioni di carico adottate sono riportate nel paragrafo 5 "Combinazioni di carico" e sono state scelte in modo da massimizzare tutte le sollecitazioni sulla struttura. L'impiego delle combinazioni adottate è esaustivo delle configurazioni studiate per la struttura in esame.
  - Origine e Caratteristiche dei Codici di Calcolo
- Vedi paragrafo 2 "Modellazione strutturale".
  - Affidabilità dei codici utilizzati
- Vedi paragrafo 2 "Modellazione strutturale".
  - Validazione dei codici.
- Nel caso in cui si rendesse necessaria una validazione indipendente del calcolo strutturale, i calcoli potranno essere eseguiti nuovamente da soggetto diverso da quello originario mediante programmi di calcolo diversi da quelli usati originariamente e ciò al fine di eseguire un effettivo controllo incrociato sui risultati delle elaborazioni. Al fondo della presente relazione si riporta il tabulato di input del modello strutturale con tutte le informazioni sufficienti a rendere ripercorribili tutti i calcoli effettuati.
  - Modalità di presentazione dei risultati.
- Il percorso che ha condotto ai risultati è stato: modellazione della struttura, analisi dei carichi e disposizione degli stessi sul modello; calcolo delle sollecitazioni; verifica degli elementi ritenuti significativi. La quantità di informazioni che ha accompagnato l'utilizzo del software in input e in output è cospicua. Per non appesantire eccessivamente la relazione di calcolo, si è operata la scelta di fornire soltanto una sintesi completa ed efficace dei risultati privilegiando schemi grafici ai tabulati. E' comunque disponibile su supporto informatico l'intero sviluppo dei tabulati di output con tutte le informazioni necessarie alla eventuale riproduzione del calcolo automatico.
  - Informazioni generali sull'elaborazione.
- Al termine della elaborazione sono stati svolti estesi controlli per l'esame dei risultati e per una valutazione complessiva dell'elaborazione dal punto di vista del corretto comportamento del modello.
  - Giudizio motivato di accettabilità dei risultati.
- I risultati della elaborazione sono stati analizzati criticamente mediante confronto con calcoli di massima eseguiti manualmente; tali controlli sommari hanno portato a confermare la validità dei risultati. I risultati delle elaborazioni sono quindi stati sottoposti a controlli che ne hanno comprovato l'attendibilità. In particolare si è svolto il controllo di equilibrio tra reazioni vincolari e carichi applicati.

## 19 ALLEGATO – DATI PROGRAMMA DI CALCOLO

A seguire l'allegato con i dati del programma di calcolo.



**20 SOTTOSCRIZIONE DELL'ELABORATO DA PARTE DEL R.T.P.**

**STUDIO CORONA S.r.l.**

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**ECOPLAN S.r.l.**

---

**I.T. S.r.l.**

---

**E&G S.r.l.**

---

**CONSORZIO UNING**

---

**ARKE' INGEGNERIA S.r.l.**

---

**SETAC S.r.l.**

---

**ING. RENATO DEL PRETE**

---

**DOTT. DANILO GALLO**

---

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

```

;-----
; MIDAS/Civil Text (MCT) File.
; Date : 2017/12/16
;-----

```

\*VERSION  
8.3.5

\*UNIT ; Unit System  
KN , M, J, C

\*PROJINFO ; Project Information  
PROJECT=CASALPUSTERLENGO  
REVISION=2  
USER=Renato Vaira  
ADDRESS=Studio Corona  
TITLE=CAVALCAVIA STRADA VIC. DELLE COSTE


\*STRUCTYPE ; Structure Type  
0, 1, 1, NO, YES, 9.806, 0, NO, NO, NO

\*REBAR-MATL-CODE ; Rebar Material Code  
UNI(RC), B450C, UNI(RC), B450C

\*NODE ; Nodes


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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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PROJECT TITLE : CASALPUSTERLENGO

|  | Company      | Client          |
|-----------------------------------------------------------------------------------|--------------|-----------------|
|                                                                                   | Author       | File Name       |
|                                                                                   | Renato Vaira | Asse 80 V_1.mct |

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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 371, 47.4, -1.6, -1.06  
 372, 52.6, -1.6, -1.06

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

373, 57.8, -1.6, -1.06  
374, 63, -1.6, -1.06  
375, 75, -1.6, -1.06  
376, 80, -1.6, -1.06  
377, 85, -1.6, -1.06  
378, 90, -1.6, -1.06  
379, 95, -1.6, -1.06  
380, 100, -1.6, -1.06  
381, 0, 0, -1.06  
382, 5, 0, -1.06  
383, 10, 0, -1.06  
384, 15, 0, -1.06  
385, 20, 0, -1.06  
386, 25, 0, -1.06  
387, 37, 0, -1.06  
388, 42.2, 0, -1.06  
389, 47.4, 0, -1.06  
390, 52.6, 0, -1.06  
391, 57.8, 0, -1.06  
392, 63, 0, -1.06  
393, 75, 0, -1.06  
394, 80, 0, -1.06  
395, 85, 0, -1.06  
396, 90, 0, -1.06  
397, 95, 0, -1.06  
398, 100, 0, -1.06  
399, 0, 1.6, -1.06  
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401, 10, 1.6, -1.06  
402, 15, 1.6, -1.06  
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404, 25, 1.6, -1.06  
405, 37, 1.6, -1.06  
406, 42.2, 1.6, -1.06  
407, 47.4, 1.6, -1.06  
408, 52.6, 1.6, -1.06  
409, 57.8, 1.6, -1.06  
410, 63, 1.6, -1.06  
411, 75, 1.6, -1.06  
412, 80, 1.6, -1.06  
413, 85, 1.6, -1.06  
414, 90, 1.6, -1.06  
415, 95, 1.6, -1.06  
416, 100, 1.6, -1.06  
417, 0, 3.2, -1.06  
418, 5, 3.2, -1.06  
419, 10, 3.2, -1.06  
420, 15, 3.2, -1.06  
421, 20, 3.2, -1.06  
422, 25, 3.2, -1.06  
423, 37, 3.2, -1.06  
424, 42.2, 3.2, -1.06  
425, 47.4, 3.2, -1.06  
426, 52.6, 3.2, -1.06  
427, 57.8, 3.2, -1.06  
428, 63, 3.2, -1.06  
429, 75, 3.2, -1.06  
430, 80, 3.2, -1.06  
431, 85, 3.2, -1.06  
432, 90, 3.2, -1.06  
433, 95, 3.2, -1.06  
434, 100, 3.2, -1.06  
435, 0, 4.8, -1.06  
436, 5, 4.8, -1.06  
437, 10, 4.8, -1.06  
438, 15, 4.8, -1.06  
439, 20, 4.8, -1.06  
440, 25, 4.8, -1.06  
441, 37, 4.8, -1.06  
442, 42.2, 4.8, -1.06  
443, 47.4, 4.8, -1.06  
444, 52.6, 4.8, -1.06  
445, 57.8, 4.8, -1.06  
446, 63, 4.8, -1.06  
447, 75, 4.8, -1.06  
448, 80, 4.8, -1.06  
449, 85, 4.8, -1.06  
450, 90, 4.8, -1.06



PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

451, 95, 4.8, -1.06  
 452, 100, 4.8, -1.06  
 489, 0, -4.8, -0.46  
 490, 5, -4.8, -0.46  
 491, 10, -4.8, -0.46  
 492, 15, -4.8, -0.46  
 493, 20, -4.8, -0.46  
 494, 25, -4.8, -0.46  
 495, 28, -4.8, -0.46  
 496, 31, -4.8, -0.46  
 497, 34, -4.8, -0.46  
 498, 37, -4.8, -0.46  
 499, 42.2, -4.8, -0.46  
 500, 47.4, -4.8, -0.46  
 501, 52.6, -4.8, -0.46  
 502, 57.8, -4.8, -0.46  
 503, 63, -4.8, -0.46  
 504, 66, -4.8, -0.46  
 505, 69, -4.8, -0.46  
 506, 72, -4.8, -0.46  
 507, 75, -4.8, -0.46  
 508, 80, -4.8, -0.46  
 509, 85, -4.8, -0.46  
 510, 90, -4.8, -0.46  
 511, 95, -4.8, -0.46  
 512, 100, -4.8, -0.46  
 513, 0, -1.6, -0.46  
 514, 5, -1.6, -0.46  
 515, 10, -1.6, -0.46  
 516, 15, -1.6, -0.46  
 517, 20, -1.6, -0.46  
 518, 25, -1.6, -0.46  
 519, 28, -1.6, -0.46  
 520, 31, -1.6, -0.46  
 521, 34, -1.6, -0.46  
 522, 37, -1.6, -0.46  
 523, 42.2, -1.6, -0.46  
 524, 47.4, -1.6, -0.46  
 525, 52.6, -1.6, -0.46  
 526, 57.8, -1.6, -0.46  
 527, 63, -1.6, -0.46  
 528, 66, -1.6, -0.46  
 529, 69, -1.6, -0.46  
 530, 72, -1.6, -0.46  
 531, 75, -1.6, -0.46  
 532, 80, -1.6, -0.46  
 533, 85, -1.6, -0.46  
 534, 90, -1.6, -0.46  
 535, 95, -1.6, -0.46  
 536, 100, -1.6, -0.46  
 537, 0, 1.6, -0.46  
 538, 5, 1.6, -0.46  
 539, 10, 1.6, -0.46  
 540, 15, 1.6, -0.46  
 541, 20, 1.6, -0.46  
 542, 25, 1.6, -0.46  
 543, 28, 1.6, -0.46  
 544, 31, 1.6, -0.46  
 545, 34, 1.6, -0.46  
 546, 37, 1.6, -0.46  
 547, 42.2, 1.6, -0.46  
 548, 47.4, 1.6, -0.46  
 549, 52.6, 1.6, -0.46  
 550, 57.8, 1.6, -0.46  
 551, 63, 1.6, -0.46  
 552, 66, 1.6, -0.46  
 553, 69, 1.6, -0.46  
 554, 72, 1.6, -0.46  
 555, 75, 1.6, -0.46  
 556, 80, 1.6, -0.46  
 557, 85, 1.6, -0.46  
 558, 90, 1.6, -0.46  
 559, 95, 1.6, -0.46  
 560, 100, 1.6, -0.46  
 561, 0, 4.8, -0.46  
 562, 5, 4.8, -0.46  
 563, 10, 4.8, -0.46  
 564, 15, 4.8, -0.46

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

565, 20, 4.8, -0.46  
 566, 25, 4.8, -0.46  
 567, 28, 4.8, -0.46  
 568, 31, 4.8, -0.46  
 569, 34, 4.8, -0.46  
 570, 37, 4.8, -0.46  
 571, 42.2, 4.8, -0.46  
 572, 47.4, 4.8, -0.46  
 573, 52.6, 4.8, -0.46  
 574, 57.8, 4.8, -0.46  
 575, 63, 4.8, -0.46  
 576, 66, 4.8, -0.46  
 577, 69, 4.8, -0.46  
 578, 72, 4.8, -0.46  
 579, 75, 4.8, -0.46  
 580, 80, 4.8, -0.46  
 581, 85, 4.8, -0.46  
 582, 90, 4.8, -0.46  
 583, 95, 4.8, -0.46  
 584, 100, 4.8, -0.46  
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 610, 5, -4.8, -0.43  
 611, 10, -4.8, -0.43  
 612, 15, -4.8, -0.43  
 613, 20, -4.8, -0.43  
 614, 25, -4.8, -0.43  
 615, 28, -4.8, -0.43  
 616, 34, -4.8, -0.43  
 617, 37, -4.8, -0.43  
 618, 42.2, -4.8, -0.43  
 619, 47.4, -4.8, -0.43  
 620, 52.6, -4.8, -0.43  
 621, 57.8, -4.8, -0.43  
 622, 63, -4.8, -0.43  
 623, 66, -4.8, -0.43  
 624, 72, -4.8, -0.43  
 625, 75, -4.8, -0.43  
 626, 80, -4.8, -0.43  
 627, 85, -4.8, -0.43  
 628, 90, -4.8, -0.43  
 629, 95, -4.8, -0.43  
 630, 100, -4.8, -0.43  
 631, 0, -1.6, -0.43  
 632, 5, -1.6, -0.43  
 633, 10, -1.6, -0.43  
 634, 15, -1.6, -0.43  
 635, 20, -1.6, -0.43  
 636, 25, -1.6, -0.43  
 637, 28, -1.6, -0.43  
 638, 34, -1.6, -0.43  
 639, 37, -1.6, -0.43  
 640, 42.2, -1.6, -0.43  
 641, 47.4, -1.6, -0.43  
 642, 52.6, -1.6, -0.43  
 643, 57.8, -1.6, -0.43  
 644, 63, -1.6, -0.43  
 645, 66, -1.6, -0.43  
 646, 72, -1.6, -0.43  
 647, 75, -1.6, -0.43  
 648, 80, -1.6, -0.43  
 649, 85, -1.6, -0.43  
 650, 90, -1.6, -0.43  
 651, 95, -1.6, -0.43  
 652, 100, -1.6, -0.43  
 653, 0, 1.6, -0.43  
 654, 5, 1.6, -0.43  
 655, 10, 1.6, -0.43  
 656, 15, 1.6, -0.43  
 657, 20, 1.6, -0.43  
 658, 25, 1.6, -0.43  
 659, 28, 1.6, -0.43  
 660, 34, 1.6, -0.43  
 661, 37, 1.6, -0.43  
 662, 42.2, 1.6, -0.43  
 663, 47.4, 1.6, -0.43  
 664, 52.6, 1.6, -0.43  
 665, 57.8, 1.6, -0.43  
 666, 63, 1.6, -0.43

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

667, 66, 1.6, -0.43  
 668, 72, 1.6, -0.43  
 669, 75, 1.6, -0.43  
 670, 80, 1.6, -0.43  
 671, 85, 1.6, -0.43  
 672, 90, 1.6, -0.43  
 673, 95, 1.6, -0.43  
 674, 100, 1.6, -0.43  
 675, 0, 4.8, -0.43  
 676, 5, 4.8, -0.43  
 677, 10, 4.8, -0.43  
 678, 15, 4.8, -0.43  
 679, 20, 4.8, -0.43  
 680, 25, 4.8, -0.43  
 681, 28, 4.8, -0.43  
 682, 34, 4.8, -0.43  
 683, 37, 4.8, -0.43  
 684, 42.2, 4.8, -0.43  
 685, 47.4, 4.8, -0.43  
 686, 52.6, 4.8, -0.43  
 687, 57.8, 4.8, -0.43  
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 689, 66, 4.8, -0.43  
 690, 72, 4.8, -0.43  
 691, 75, 4.8, -0.43  
 692, 80, 4.8, -0.43  
 693, 85, 4.8, -0.43  
 694, 90, 4.8, -0.43  
 695, 95, 4.8, -0.43  
 696, 100, 4.8, -0.43  
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 721, 5, -4.8, 0  
 722, 8, -4.8, 0  
 723, 10, -4.8, 0  
 724, 15, -4.8, 0  
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 756, 92, -4.8, 0  
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 762, 5, -1.6, 0  
 763, 8, -1.6, 0  
 764, 10, -1.6, 0  
 765, 15, -1.6, 0  
 766, 18, -1.6, 0

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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767, 20, -1.6, 0
768, 25, -1.6, 0
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772, 31, -1.6, 0
773, 31.5, -1.6, 0
774, 34, -1.6, 0
775, 36.5, -1.6, 0
776, 37, -1.6, 0
777, 42.2, -1.6, 0
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783, 57.8, -1.6, 0
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785, 63.5, -1.6, 0
786, 66, -1.6, 0
787, 68.5, -1.6, 0
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790, 72, -1.6, 0
791, 74.5, -1.6, 0
792, 75, -1.6, 0
793, 80, -1.6, 0
794, 82, -1.6, 0
795, 85, -1.6, 0
796, 90, -1.6, 0
797, 92, -1.6, 0
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804, 8, 1.6, 0
805, 10, 1.6, 0
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816, 36.5, 1.6, 0
817, 37, 1.6, 0
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819, 44.5, 1.6, 0
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825, 63, 1.6, 0
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837, 90, 1.6, 0
838, 92, 1.6, 0
839, 95, 1.6, 0
840, 100, 1.6, 0
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843, 0, 4.8, 0
844, 5, 4.8, 0

```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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845, 8, 4.8, 0
846, 10, 4.8, 0
847, 15, 4.8, 0
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852, 28, 4.8, 0
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856, 34, 4.8, 0
857, 36.5, 4.8, 0
858, 37, 4.8, 0
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860, 44.5, 4.8, 0
861, 47.4, 4.8, 0
862, 50, 4.8, 0
863, 52.6, 4.8, 0
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865, 57.8, 4.8, 0
866, 63, 4.8, 0
867, 63.5, 4.8, 0
868, 66, 4.8, 0
869, 68.5, 4.8, 0
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871, 69.5, 4.8, 0
872, 72, 4.8, 0
873, 74.5, 4.8, 0
874, 75, 4.8, 0
875, 80, 4.8, 0
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878, 90, 4.8, 0
879, 92, 4.8, 0
880, 95, 4.8, 0
881, 100, 4.8, 0
882, 100.8, 4.8, 0
883, 0, 0, -1.7
884, 100, 0, -1.7
885, 31, 1.6, -2.7
886, 69, 1.6, -2.7
905, 31, 0, -7.74
906, 31, 0, -7.18
907, 31, 0, -6.62
908, 31, 0, -6.06
909, 31, 0, -5.5
910, 31, 0, -4.94
911, 31, 0, -4.38
912, 31, 0, -3.82
913, 31, 0, -3.26
914, 69, 0, -7.29
915, 69, 0, -6.78
916, 69, 0, -6.27
917, 69, 0, -5.76
918, 69, 0, -5.25
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920, 69, 0, -4.23
921, 69, 0, -3.72
922, 69, 0, -3.21
    
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\*ELEMENT ; Elements

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101, BEAM , 5, 3, 719, 720, 0
102, BEAM , 5, 3, 720, 721, 0
103, BEAM , 5, 3, 721, 722, 0
104, BEAM , 5, 4, 722, 723, 0
105, BEAM , 5, 4, 723, 724, 0
106, BEAM , 5, 4, 724, 725, 0
107, BEAM , 5, 14, 725, 726, 0
108, BEAM , 5, 14, 726, 727, 0
109, BEAM , 5, 14, 727, 728, 0
110, BEAM , 5, 9, 728, 729, 0
111, BEAM , 5, 9, 729, 730, 0
112, BEAM , 5, 21, 730, 731, 0
113, BEAM , 5, 21, 731, 732, 0
114, BEAM , 5, 10, 732, 733, 0
115, BEAM , 5, 10, 733, 734, 0
116, BEAM , 5, 12, 734, 735, 0
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

|           |   |    |     |      |      |   |
|-----------|---|----|-----|------|------|---|
| 117, BEAM | , | 5, | 12, | 735, | 736, | 0 |
| 118, BEAM | , | 5, | 12, | 736, | 737, | 0 |
| 119, BEAM | , | 5, | 13, | 737, | 738, | 0 |
| 120, BEAM | , | 5, | 13, | 738, | 739, | 0 |
| 121, BEAM | , | 5, | 13, | 739, | 740, | 0 |
| 122, BEAM | , | 5, | 13, | 740, | 741, | 0 |
| 123, BEAM | , | 5, | 12, | 741, | 742, | 0 |
| 124, BEAM | , | 5, | 12, | 742, | 743, | 0 |
| 125, BEAM | , | 5, | 12, | 743, | 744, | 0 |
| 126, BEAM | , | 5, | 9,  | 744, | 745, | 0 |
| 127, BEAM | , | 5, | 9,  | 745, | 746, | 0 |
| 128, BEAM | , | 5, | 21, | 746, | 747, | 0 |
| 129, BEAM | , | 5, | 21, | 747, | 748, | 0 |
| 130, BEAM | , | 5, | 10, | 748, | 749, | 0 |
| 131, BEAM | , | 5, | 10, | 749, | 750, | 0 |
| 132, BEAM | , | 5, | 14, | 750, | 751, | 0 |
| 133, BEAM | , | 5, | 14, | 751, | 752, | 0 |
| 134, BEAM | , | 5, | 14, | 752, | 753, | 0 |
| 135, BEAM | , | 5, | 4,  | 753, | 754, | 0 |
| 136, BEAM | , | 5, | 4,  | 754, | 755, | 0 |
| 137, BEAM | , | 5, | 4,  | 755, | 756, | 0 |
| 138, BEAM | , | 5, | 3,  | 756, | 757, | 0 |
| 139, BEAM | , | 5, | 3,  | 757, | 758, | 0 |
| 140, BEAM | , | 5, | 3,  | 758, | 759, | 0 |
| 201, BEAM | , | 5, | 1,  | 760, | 761, | 0 |
| 202, BEAM | , | 5, | 1,  | 761, | 762, | 0 |
| 203, BEAM | , | 5, | 1,  | 762, | 763, | 0 |
| 204, BEAM | , | 5, | 2,  | 763, | 764, | 0 |
| 205, BEAM | , | 5, | 2,  | 764, | 765, | 0 |
| 206, BEAM | , | 5, | 2,  | 765, | 766, | 0 |
| 207, BEAM | , | 5, | 11, | 766, | 767, | 0 |
| 208, BEAM | , | 5, | 11, | 767, | 768, | 0 |
| 209, BEAM | , | 5, | 11, | 768, | 769, | 0 |
| 210, BEAM | , | 5, | 5,  | 769, | 770, | 0 |
| 211, BEAM | , | 5, | 5,  | 770, | 771, | 0 |
| 212, BEAM | , | 5, | 16, | 771, | 772, | 0 |
| 213, BEAM | , | 5, | 16, | 772, | 773, | 0 |
| 214, BEAM | , | 5, | 6,  | 773, | 774, | 0 |
| 215, BEAM | , | 5, | 6,  | 774, | 775, | 0 |
| 216, BEAM | , | 5, | 7,  | 775, | 776, | 0 |
| 217, BEAM | , | 5, | 7,  | 776, | 777, | 0 |
| 218, BEAM | , | 5, | 7,  | 777, | 778, | 0 |
| 219, BEAM | , | 5, | 8,  | 778, | 779, | 0 |
| 220, BEAM | , | 5, | 8,  | 779, | 780, | 0 |
| 221, BEAM | , | 5, | 8,  | 780, | 781, | 0 |
| 222, BEAM | , | 5, | 8,  | 781, | 782, | 0 |
| 223, BEAM | , | 5, | 7,  | 782, | 783, | 0 |
| 224, BEAM | , | 5, | 7,  | 783, | 784, | 0 |
| 225, BEAM | , | 5, | 7,  | 784, | 785, | 0 |
| 226, BEAM | , | 5, | 5,  | 785, | 786, | 0 |
| 227, BEAM | , | 5, | 5,  | 786, | 787, | 0 |
| 228, BEAM | , | 5, | 16, | 787, | 788, | 0 |
| 229, BEAM | , | 5, | 16, | 788, | 789, | 0 |
| 230, BEAM | , | 5, | 6,  | 789, | 790, | 0 |
| 231, BEAM | , | 5, | 6,  | 790, | 791, | 0 |
| 232, BEAM | , | 5, | 11, | 791, | 792, | 0 |
| 233, BEAM | , | 5, | 11, | 792, | 793, | 0 |
| 234, BEAM | , | 5, | 11, | 793, | 794, | 0 |
| 235, BEAM | , | 5, | 2,  | 794, | 795, | 0 |
| 236, BEAM | , | 5, | 2,  | 795, | 796, | 0 |
| 237, BEAM | , | 5, | 2,  | 796, | 797, | 0 |
| 238, BEAM | , | 5, | 1,  | 797, | 798, | 0 |
| 239, BEAM | , | 5, | 1,  | 798, | 799, | 0 |
| 240, BEAM | , | 5, | 1,  | 799, | 800, | 0 |
| 301, BEAM | , | 5, | 1,  | 801, | 802, | 0 |
| 302, BEAM | , | 5, | 1,  | 802, | 803, | 0 |
| 303, BEAM | , | 5, | 1,  | 803, | 804, | 0 |
| 304, BEAM | , | 5, | 2,  | 804, | 805, | 0 |
| 305, BEAM | , | 5, | 2,  | 805, | 806, | 0 |
| 306, BEAM | , | 5, | 2,  | 806, | 807, | 0 |
| 307, BEAM | , | 5, | 11, | 807, | 808, | 0 |
| 308, BEAM | , | 5, | 11, | 808, | 809, | 0 |
| 309, BEAM | , | 5, | 11, | 809, | 810, | 0 |
| 310, BEAM | , | 5, | 5,  | 810, | 811, | 0 |
| 311, BEAM | , | 5, | 5,  | 811, | 812, | 0 |
| 312, BEAM | , | 5, | 16, | 812, | 813, | 0 |
| 313, BEAM | , | 5, | 16, | 813, | 814, | 0 |
| 314, BEAM | , | 5, | 6,  | 814, | 815, | 0 |


PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

|            |   |    |     |      |      |     |
|------------|---|----|-----|------|------|-----|
| 315, BEAM  | , | 5, | 6,  | 815, | 816, | 0   |
| 316, BEAM  | , | 5, | 7,  | 816, | 817, | 0   |
| 317, BEAM  | , | 5, | 7,  | 817, | 818, | 0   |
| 318, BEAM  | , | 5, | 7,  | 818, | 819, | 0   |
| 319, BEAM  | , | 5, | 8,  | 819, | 820, | 0   |
| 320, BEAM  | , | 5, | 8,  | 820, | 821, | 0   |
| 321, BEAM  | , | 5, | 8,  | 821, | 822, | 0   |
| 322, BEAM  | , | 5, | 8,  | 822, | 823, | 0   |
| 323, BEAM  | , | 5, | 7,  | 823, | 824, | 0   |
| 324, BEAM  | , | 5, | 7,  | 824, | 825, | 0   |
| 325, BEAM  | , | 5, | 7,  | 825, | 826, | 0   |
| 326, BEAM  | , | 5, | 5,  | 826, | 827, | 0   |
| 327, BEAM  | , | 5, | 5,  | 827, | 828, | 0   |
| 328, BEAM  | , | 5, | 16, | 828, | 829, | 0   |
| 329, BEAM  | , | 5, | 16, | 829, | 830, | 0   |
| 330, BEAM  | , | 5, | 6,  | 830, | 831, | 0   |
| 331, BEAM  | , | 5, | 6,  | 831, | 832, | 0   |
| 332, BEAM  | , | 5, | 11, | 832, | 833, | 0   |
| 333, BEAM  | , | 5, | 11, | 833, | 834, | 0   |
| 334, BEAM  | , | 5, | 11, | 834, | 835, | 0   |
| 335, BEAM  | , | 5, | 2,  | 835, | 836, | 0   |
| 336, BEAM  | , | 5, | 2,  | 836, | 837, | 0   |
| 337, BEAM  | , | 5, | 2,  | 837, | 838, | 0   |
| 338, BEAM  | , | 5, | 1,  | 838, | 839, | 0   |
| 339, BEAM  | , | 5, | 1,  | 839, | 840, | 0   |
| 340, BEAM  | , | 5, | 1,  | 840, | 841, | 0   |
| 401, BEAM  | , | 5, | 3,  | 842, | 843, | 0   |
| 402, BEAM  | , | 5, | 3,  | 843, | 844, | 0   |
| 403, BEAM  | , | 5, | 3,  | 844, | 845, | 0   |
| 404, BEAM  | , | 5, | 4,  | 845, | 846, | 0   |
| 405, BEAM  | , | 5, | 4,  | 846, | 847, | 0   |
| 406, BEAM  | , | 5, | 4,  | 847, | 848, | 0   |
| 407, BEAM  | , | 5, | 14, | 848, | 849, | 0   |
| 408, BEAM  | , | 5, | 14, | 849, | 850, | 0   |
| 409, BEAM  | , | 5, | 14, | 850, | 851, | 0   |
| 410, BEAM  | , | 5, | 9,  | 851, | 852, | 0   |
| 411, BEAM  | , | 5, | 9,  | 852, | 853, | 0   |
| 412, BEAM  | , | 5, | 21, | 853, | 854, | 0   |
| 413, BEAM  | , | 5, | 21, | 854, | 855, | 0   |
| 414, BEAM  | , | 5, | 10, | 855, | 856, | 0   |
| 415, BEAM  | , | 5, | 10, | 856, | 857, | 0   |
| 416, BEAM  | , | 5, | 12, | 857, | 858, | 0   |
| 417, BEAM  | , | 5, | 12, | 858, | 859, | 0   |
| 418, BEAM  | , | 5, | 12, | 859, | 860, | 0   |
| 419, BEAM  | , | 5, | 13, | 860, | 861, | 0   |
| 420, BEAM  | , | 5, | 13, | 861, | 862, | 0   |
| 421, BEAM  | , | 5, | 13, | 862, | 863, | 0   |
| 422, BEAM  | , | 5, | 13, | 863, | 864, | 0   |
| 423, BEAM  | , | 5, | 12, | 864, | 865, | 0   |
| 424, BEAM  | , | 5, | 12, | 865, | 866, | 0   |
| 425, BEAM  | , | 5, | 12, | 866, | 867, | 0   |
| 426, BEAM  | , | 5, | 9,  | 867, | 868, | 0   |
| 427, BEAM  | , | 5, | 9,  | 868, | 869, | 0   |
| 428, BEAM  | , | 5, | 21, | 869, | 870, | 0   |
| 429, BEAM  | , | 5, | 21, | 870, | 871, | 0   |
| 430, BEAM  | , | 5, | 10, | 871, | 872, | 0   |
| 431, BEAM  | , | 5, | 10, | 872, | 873, | 0   |
| 432, BEAM  | , | 5, | 14, | 873, | 874, | 0   |
| 433, BEAM  | , | 5, | 14, | 874, | 875, | 0   |
| 434, BEAM  | , | 5, | 14, | 875, | 876, | 0   |
| 435, BEAM  | , | 5, | 4,  | 876, | 877, | 0   |
| 436, BEAM  | , | 5, | 4,  | 877, | 878, | 0   |
| 437, BEAM  | , | 5, | 4,  | 878, | 879, | 0   |
| 438, BEAM  | , | 5, | 3,  | 879, | 880, | 0   |
| 439, BEAM  | , | 5, | 3,  | 880, | 881, | 0   |
| 440, BEAM  | , | 5, | 3,  | 881, | 882, | 0   |
| 1014, BEAM | , | 1, | 18, | 107, | 111, | 180 |
| 1015, BEAM | , | 1, | 18, | 111, | 115, | 180 |
| 1016, BEAM | , | 1, | 18, | 115, | 119, | 180 |
| 1017, BEAM | , | 1, | 18, | 119, | 123, | 180 |
| 1018, BEAM | , | 1, | 18, | 123, | 127, | 180 |
| 1019, BEAM | , | 1, | 18, | 127, | 131, | 180 |
| 1022, BEAM | , | 1, | 18, | 108, | 112, | 180 |
| 1023, BEAM | , | 1, | 18, | 112, | 116, | 180 |
| 1024, BEAM | , | 1, | 18, | 116, | 120, | 180 |
| 1025, BEAM | , | 1, | 18, | 120, | 124, | 180 |
| 1026, BEAM | , | 1, | 18, | 124, | 128, | 180 |
| 1027, BEAM | , | 1, | 18, | 128, | 132, | 180 |



PROJECT TITLE : CASALPUSTERLENGO

|  | Company |              |  | Client          |
|-----------------------------------------------------------------------------------|---------|--------------|--|-----------------|
|                                                                                   | Author  | Renato Vaira |  | File Name       |
|                                                                                   |         |              |  | Asse 80 v_1.mct |

|                                   |
|-----------------------------------|
| 1030, BEAM , 1, 18, 109, 113, 180 |
| 1031, BEAM , 1, 18, 113, 117, 180 |
| 1032, BEAM , 1, 18, 117, 121, 180 |
| 1033, BEAM , 1, 18, 121, 125, 180 |
| 1034, BEAM , 1, 18, 125, 129, 180 |
| 1035, BEAM , 1, 18, 129, 133, 180 |
| 1038, BEAM , 1, 18, 110, 114, 180 |
| 1039, BEAM , 1, 18, 114, 118, 180 |
| 1040, BEAM , 1, 18, 118, 122, 180 |
| 1041, BEAM , 1, 18, 122, 126, 180 |
| 1042, BEAM , 1, 18, 126, 130, 180 |
| 1043, BEAM , 1, 18, 130, 134, 180 |
| 1110, BEAM , 1, 18, 327, 345, 180 |
| 1111, BEAM , 1, 18, 345, 363, 180 |
| 1112, BEAM , 1, 18, 363, 381, 180 |
| 1113, BEAM , 1, 18, 381, 399, 180 |
| 1114, BEAM , 1, 18, 399, 417, 180 |
| 1115, BEAM , 1, 18, 417, 435, 180 |
| 1118, BEAM , 1, 18, 328, 346, 180 |
| 1119, BEAM , 1, 18, 346, 364, 180 |
| 1120, BEAM , 1, 18, 364, 382, 180 |
| 1121, BEAM , 1, 18, 382, 400, 180 |
| 1122, BEAM , 1, 18, 400, 418, 180 |
| 1123, BEAM , 1, 18, 418, 436, 180 |
| 1126, BEAM , 1, 18, 329, 347, 180 |
| 1127, BEAM , 1, 18, 347, 365, 180 |
| 1128, BEAM , 1, 18, 365, 383, 180 |
| 1129, BEAM , 1, 18, 383, 401, 180 |
| 1130, BEAM , 1, 18, 401, 419, 180 |
| 1131, BEAM , 1, 18, 419, 437, 180 |
| 1134, BEAM , 1, 18, 330, 348, 180 |
| 1135, BEAM , 1, 18, 348, 366, 180 |
| 1136, BEAM , 1, 18, 366, 384, 180 |
| 1137, BEAM , 1, 18, 384, 402, 180 |
| 1138, BEAM , 1, 18, 402, 420, 180 |
| 1139, BEAM , 1, 18, 420, 438, 180 |
| 1142, BEAM , 1, 18, 331, 349, 180 |
| 1143, BEAM , 1, 18, 349, 367, 180 |
| 1144, BEAM , 1, 18, 367, 385, 180 |
| 1145, BEAM , 1, 18, 385, 403, 180 |
| 1146, BEAM , 1, 18, 403, 421, 180 |
| 1147, BEAM , 1, 18, 421, 439, 180 |
| 1150, BEAM , 1, 18, 332, 350, 180 |
| 1151, BEAM , 1, 18, 350, 368, 180 |
| 1152, BEAM , 1, 18, 368, 386, 180 |
| 1153, BEAM , 1, 18, 386, 404, 180 |
| 1154, BEAM , 1, 18, 404, 422, 180 |
| 1155, BEAM , 1, 18, 422, 440, 180 |
| 1158, BEAM , 1, 18, 333, 351, 180 |
| 1159, BEAM , 1, 18, 351, 369, 180 |
| 1160, BEAM , 1, 18, 369, 387, 180 |
| 1161, BEAM , 1, 18, 387, 405, 180 |
| 1162, BEAM , 1, 18, 405, 423, 180 |
| 1163, BEAM , 1, 18, 423, 441, 180 |
| 1166, BEAM , 1, 18, 334, 352, 180 |
| 1167, BEAM , 1, 18, 352, 370, 180 |
| 1168, BEAM , 1, 18, 370, 388, 180 |
| 1169, BEAM , 1, 18, 388, 406, 180 |
| 1170, BEAM , 1, 18, 406, 424, 180 |
| 1171, BEAM , 1, 18, 424, 442, 180 |
| 1174, BEAM , 1, 18, 335, 353, 180 |
| 1175, BEAM , 1, 18, 353, 371, 180 |
| 1176, BEAM , 1, 18, 371, 389, 180 |
| 1177, BEAM , 1, 18, 389, 407, 180 |
| 1178, BEAM , 1, 18, 407, 425, 180 |
| 1179, BEAM , 1, 18, 425, 443, 180 |
| 1182, BEAM , 1, 18, 336, 354, 180 |
| 1183, BEAM , 1, 18, 354, 372, 180 |
| 1184, BEAM , 1, 18, 372, 390, 180 |
| 1185, BEAM , 1, 18, 390, 408, 180 |
| 1186, BEAM , 1, 18, 408, 426, 180 |
| 1187, BEAM , 1, 18, 426, 444, 180 |
| 1190, BEAM , 1, 18, 337, 355, 180 |
| 1191, BEAM , 1, 18, 355, 373, 180 |
| 1192, BEAM , 1, 18, 373, 391, 180 |
| 1193, BEAM , 1, 18, 391, 409, 180 |
| 1194, BEAM , 1, 18, 409, 427, 180 |
| 1195, BEAM , 1, 18, 427, 445, 180 |

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

|                                   |
|-----------------------------------|
| 1198, BEAM , 1, 18, 338, 356, 180 |
| 1199, BEAM , 1, 18, 356, 374, 180 |
| 1200, BEAM , 1, 18, 374, 392, 180 |
| 1201, BEAM , 1, 18, 392, 410, 180 |
| 1202, BEAM , 1, 18, 410, 428, 180 |
| 1203, BEAM , 1, 18, 428, 446, 180 |
| 1206, BEAM , 1, 18, 339, 357, 180 |
| 1207, BEAM , 1, 18, 357, 375, 180 |
| 1208, BEAM , 1, 18, 375, 393, 180 |
| 1209, BEAM , 1, 18, 393, 411, 180 |
| 1210, BEAM , 1, 18, 411, 429, 180 |
| 1211, BEAM , 1, 18, 429, 447, 180 |
| 1214, BEAM , 1, 18, 340, 358, 180 |
| 1215, BEAM , 1, 18, 358, 376, 180 |
| 1216, BEAM , 1, 18, 376, 394, 180 |
| 1217, BEAM , 1, 18, 394, 412, 180 |
| 1218, BEAM , 1, 18, 412, 430, 180 |
| 1219, BEAM , 1, 18, 430, 448, 180 |
| 1222, BEAM , 1, 18, 341, 359, 180 |
| 1223, BEAM , 1, 18, 359, 377, 180 |
| 1224, BEAM , 1, 18, 377, 395, 180 |
| 1225, BEAM , 1, 18, 395, 413, 180 |
| 1226, BEAM , 1, 18, 413, 431, 180 |
| 1227, BEAM , 1, 18, 431, 449, 180 |
| 1230, BEAM , 1, 18, 342, 360, 180 |
| 1231, BEAM , 1, 18, 360, 378, 180 |
| 1232, BEAM , 1, 18, 378, 396, 180 |
| 1233, BEAM , 1, 18, 396, 414, 180 |
| 1234, BEAM , 1, 18, 414, 432, 180 |
| 1235, BEAM , 1, 18, 432, 450, 180 |
| 1238, BEAM , 1, 18, 343, 361, 180 |
| 1239, BEAM , 1, 18, 361, 379, 180 |
| 1240, BEAM , 1, 18, 379, 397, 180 |
| 1241, BEAM , 1, 18, 397, 415, 180 |
| 1242, BEAM , 1, 18, 415, 433, 180 |
| 1243, BEAM , 1, 18, 433, 451, 180 |
| 1246, BEAM , 1, 18, 344, 362, 180 |
| 1247, BEAM , 1, 18, 362, 380, 180 |
| 1248, BEAM , 1, 18, 380, 398, 180 |
| 1249, BEAM , 1, 18, 398, 416, 180 |
| 1250, BEAM , 1, 18, 416, 434, 180 |
| 1251, BEAM , 1, 18, 434, 452, 180 |
| 1254, BEAM , 1, 64, 495, 111, 180 |
| 1255, BEAM , 1, 64, 111, 519, 180 |
| 1256, BEAM , 1, 64, 519, 119, 180 |
| 1257, BEAM , 1, 64, 119, 543, 180 |
| 1258, BEAM , 1, 64, 543, 127, 180 |
| 1259, BEAM , 1, 64, 127, 567, 180 |
| 1262, BEAM , 1, 64, 497, 112, 180 |
| 1263, BEAM , 1, 64, 112, 521, 180 |
| 1264, BEAM , 1, 64, 521, 120, 180 |
| 1265, BEAM , 1, 64, 120, 545, 180 |
| 1266, BEAM , 1, 64, 545, 128, 180 |
| 1267, BEAM , 1, 64, 128, 569, 180 |
| 1270, BEAM , 1, 64, 504, 113, 180 |
| 1271, BEAM , 1, 64, 113, 528, 180 |
| 1272, BEAM , 1, 64, 528, 121, 180 |
| 1273, BEAM , 1, 64, 121, 552, 180 |
| 1274, BEAM , 1, 64, 552, 129, 180 |
| 1275, BEAM , 1, 64, 129, 576, 180 |
| 1278, BEAM , 1, 64, 506, 114, 180 |
| 1279, BEAM , 1, 64, 114, 530, 180 |
| 1280, BEAM , 1, 64, 530, 122, 180 |
| 1281, BEAM , 1, 64, 122, 554, 180 |
| 1282, BEAM , 1, 64, 554, 130, 180 |
| 1283, BEAM , 1, 64, 130, 578, 180 |
| 1286, BEAM , 1, 64, 489, 345, 180 |
| 1287, BEAM , 1, 64, 345, 513, 180 |
| 1288, BEAM , 1, 64, 513, 381, 180 |
| 1289, BEAM , 1, 64, 381, 537, 180 |
| 1290, BEAM , 1, 64, 537, 417, 180 |
| 1291, BEAM , 1, 64, 417, 561, 180 |
| 1294, BEAM , 1, 64, 490, 346, 180 |
| 1295, BEAM , 1, 64, 346, 514, 180 |
| 1296, BEAM , 1, 64, 514, 382, 180 |
| 1297, BEAM , 1, 64, 382, 538, 180 |
| 1298, BEAM , 1, 64, 538, 418, 180 |
| 1299, BEAM , 1, 64, 418, 562, 180 |

PROJECT TITLE : CASALPUSTERLENGO

|  | Company      | Client          |
|-----------------------------------------------------------------------------------|--------------|-----------------|
|                                                                                   | Author       | File Name       |
|                                                                                   | Renato Vaira | Asse 80 v_1.mct |

|                                   |
|-----------------------------------|
| 1302, BEAM , 1, 64, 491, 347, 180 |
| 1303, BEAM , 1, 64, 347, 515, 180 |
| 1304, BEAM , 1, 64, 515, 383, 180 |
| 1305, BEAM , 1, 64, 383, 539, 180 |
| 1306, BEAM , 1, 64, 539, 419, 180 |
| 1307, BEAM , 1, 64, 419, 563, 180 |
| 1310, BEAM , 1, 64, 492, 348, 180 |
| 1311, BEAM , 1, 64, 348, 516, 180 |
| 1312, BEAM , 1, 64, 516, 384, 180 |
| 1313, BEAM , 1, 64, 384, 540, 180 |
| 1314, BEAM , 1, 64, 540, 420, 180 |
| 1315, BEAM , 1, 64, 420, 564, 180 |
| 1318, BEAM , 1, 64, 493, 349, 180 |
| 1319, BEAM , 1, 64, 349, 517, 180 |
| 1320, BEAM , 1, 64, 517, 385, 180 |
| 1321, BEAM , 1, 64, 385, 541, 180 |
| 1322, BEAM , 1, 64, 541, 421, 180 |
| 1323, BEAM , 1, 64, 421, 565, 180 |
| 1326, BEAM , 1, 64, 494, 350, 180 |
| 1327, BEAM , 1, 64, 350, 518, 180 |
| 1328, BEAM , 1, 64, 518, 386, 180 |
| 1329, BEAM , 1, 64, 386, 542, 180 |
| 1330, BEAM , 1, 64, 542, 422, 180 |
| 1331, BEAM , 1, 64, 422, 566, 180 |
| 1334, BEAM , 1, 64, 498, 351, 180 |
| 1335, BEAM , 1, 64, 351, 522, 180 |
| 1336, BEAM , 1, 64, 522, 387, 180 |
| 1337, BEAM , 1, 64, 387, 546, 180 |
| 1338, BEAM , 1, 64, 546, 423, 180 |
| 1339, BEAM , 1, 64, 423, 570, 180 |
| 1342, BEAM , 1, 64, 499, 352, 180 |
| 1343, BEAM , 1, 64, 352, 523, 180 |
| 1344, BEAM , 1, 64, 523, 388, 180 |
| 1345, BEAM , 1, 64, 388, 547, 180 |
| 1346, BEAM , 1, 64, 547, 424, 180 |
| 1347, BEAM , 1, 64, 424, 571, 180 |
| 1350, BEAM , 1, 64, 500, 353, 180 |
| 1351, BEAM , 1, 64, 353, 524, 180 |
| 1352, BEAM , 1, 64, 524, 389, 180 |
| 1353, BEAM , 1, 64, 389, 548, 180 |
| 1354, BEAM , 1, 64, 548, 425, 180 |
| 1355, BEAM , 1, 64, 425, 572, 180 |
| 1358, BEAM , 1, 64, 501, 354, 180 |
| 1359, BEAM , 1, 64, 354, 525, 180 |
| 1360, BEAM , 1, 64, 525, 390, 180 |
| 1361, BEAM , 1, 64, 390, 549, 180 |
| 1362, BEAM , 1, 64, 549, 426, 180 |
| 1363, BEAM , 1, 64, 426, 573, 180 |
| 1366, BEAM , 1, 64, 502, 355, 180 |
| 1367, BEAM , 1, 64, 355, 526, 180 |
| 1368, BEAM , 1, 64, 526, 391, 180 |
| 1369, BEAM , 1, 64, 391, 550, 180 |
| 1370, BEAM , 1, 64, 550, 427, 180 |
| 1371, BEAM , 1, 64, 427, 574, 180 |
| 1374, BEAM , 1, 64, 503, 356, 180 |
| 1375, BEAM , 1, 64, 356, 527, 180 |
| 1376, BEAM , 1, 64, 527, 392, 180 |
| 1377, BEAM , 1, 64, 392, 551, 180 |
| 1378, BEAM , 1, 64, 551, 428, 180 |
| 1379, BEAM , 1, 64, 428, 575, 180 |
| 1382, BEAM , 1, 64, 507, 357, 180 |
| 1383, BEAM , 1, 64, 357, 531, 180 |
| 1384, BEAM , 1, 64, 531, 393, 180 |
| 1385, BEAM , 1, 64, 393, 555, 180 |
| 1386, BEAM , 1, 64, 555, 429, 180 |
| 1387, BEAM , 1, 64, 429, 579, 180 |
| 1390, BEAM , 1, 64, 508, 358, 180 |
| 1391, BEAM , 1, 64, 358, 532, 180 |
| 1392, BEAM , 1, 64, 532, 394, 180 |
| 1393, BEAM , 1, 64, 394, 556, 180 |
| 1394, BEAM , 1, 64, 556, 430, 180 |
| 1395, BEAM , 1, 64, 430, 580, 180 |
| 1398, BEAM , 1, 64, 509, 359, 180 |
| 1399, BEAM , 1, 64, 359, 533, 180 |
| 1400, BEAM , 1, 64, 533, 395, 180 |
| 1401, BEAM , 1, 64, 395, 557, 180 |
| 1402, BEAM , 1, 64, 557, 431, 180 |
| 1403, BEAM , 1, 64, 431, 581, 180 |

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|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

|            |   |    |     |      |      |     |
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| 1406, BEAM | , | 1, | 64, | 510, | 360, | 180 |
| 1407, BEAM | , | 1, | 64, | 360, | 534, | 180 |
| 1408, BEAM | , | 1, | 64, | 534, | 396, | 180 |
| 1409, BEAM | , | 1, | 64, | 396, | 558, | 180 |
| 1410, BEAM | , | 1, | 64, | 558, | 432, | 180 |
| 1411, BEAM | , | 1, | 64, | 432, | 582, | 180 |
| 1414, BEAM | , | 1, | 64, | 511, | 361, | 180 |
| 1415, BEAM | , | 1, | 64, | 361, | 535, | 180 |
| 1416, BEAM | , | 1, | 64, | 535, | 397, | 180 |
| 1417, BEAM | , | 1, | 64, | 397, | 559, | 180 |
| 1418, BEAM | , | 1, | 64, | 559, | 433, | 180 |
| 1419, BEAM | , | 1, | 64, | 433, | 583, | 180 |
| 1422, BEAM | , | 1, | 64, | 512, | 362, | 180 |
| 1423, BEAM | , | 1, | 64, | 362, | 536, | 180 |
| 1424, BEAM | , | 1, | 64, | 536, | 398, | 180 |
| 1425, BEAM | , | 1, | 64, | 398, | 560, | 180 |
| 1426, BEAM | , | 1, | 64, | 560, | 434, | 180 |
| 1427, BEAM | , | 1, | 64, | 434, | 584, | 180 |
| 1430, BEAM | , | 1, | 17, | 609, | 631, | 0   |
| 1431, BEAM | , | 1, | 17, | 631, | 653, | 0   |
| 1432, BEAM | , | 1, | 17, | 653, | 675, | 0   |
| 1434, BEAM | , | 1, | 17, | 610, | 632, | 0   |
| 1435, BEAM | , | 1, | 17, | 632, | 654, | 0   |
| 1436, BEAM | , | 1, | 17, | 654, | 676, | 0   |
| 1438, BEAM | , | 1, | 17, | 611, | 633, | 0   |
| 1439, BEAM | , | 1, | 17, | 633, | 655, | 0   |
| 1440, BEAM | , | 1, | 17, | 655, | 677, | 0   |
| 1442, BEAM | , | 1, | 17, | 612, | 634, | 0   |
| 1443, BEAM | , | 1, | 17, | 634, | 656, | 0   |
| 1444, BEAM | , | 1, | 17, | 656, | 678, | 0   |
| 1446, BEAM | , | 1, | 17, | 613, | 635, | 0   |
| 1447, BEAM | , | 1, | 17, | 635, | 657, | 0   |
| 1448, BEAM | , | 1, | 17, | 657, | 679, | 0   |
| 1450, BEAM | , | 1, | 17, | 614, | 636, | 0   |
| 1451, BEAM | , | 1, | 17, | 636, | 658, | 0   |
| 1452, BEAM | , | 1, | 17, | 658, | 680, | 0   |
| 1454, BEAM | , | 1, | 17, | 615, | 637, | 0   |
| 1455, BEAM | , | 1, | 17, | 637, | 659, | 0   |
| 1456, BEAM | , | 1, | 17, | 659, | 681, | 0   |
| 1458, BEAM | , | 1, | 17, | 616, | 638, | 0   |
| 1459, BEAM | , | 1, | 17, | 638, | 660, | 0   |
| 1460, BEAM | , | 1, | 17, | 660, | 682, | 0   |
| 1462, BEAM | , | 1, | 17, | 617, | 639, | 0   |
| 1463, BEAM | , | 1, | 17, | 639, | 661, | 0   |
| 1464, BEAM | , | 1, | 17, | 661, | 683, | 0   |
| 1466, BEAM | , | 1, | 17, | 618, | 640, | 0   |
| 1467, BEAM | , | 1, | 17, | 640, | 662, | 0   |
| 1468, BEAM | , | 1, | 17, | 662, | 684, | 0   |
| 1470, BEAM | , | 1, | 17, | 619, | 641, | 0   |
| 1471, BEAM | , | 1, | 17, | 641, | 663, | 0   |
| 1472, BEAM | , | 1, | 17, | 663, | 685, | 0   |
| 1474, BEAM | , | 1, | 17, | 620, | 642, | 0   |
| 1475, BEAM | , | 1, | 17, | 642, | 664, | 0   |
| 1476, BEAM | , | 1, | 17, | 664, | 686, | 0   |
| 1478, BEAM | , | 1, | 17, | 621, | 643, | 0   |
| 1479, BEAM | , | 1, | 17, | 643, | 665, | 0   |
| 1480, BEAM | , | 1, | 17, | 665, | 687, | 0   |
| 1482, BEAM | , | 1, | 17, | 622, | 644, | 0   |
| 1483, BEAM | , | 1, | 17, | 644, | 666, | 0   |
| 1484, BEAM | , | 1, | 17, | 666, | 688, | 0   |
| 1486, BEAM | , | 1, | 17, | 623, | 645, | 0   |
| 1487, BEAM | , | 1, | 17, | 645, | 667, | 0   |
| 1488, BEAM | , | 1, | 17, | 667, | 689, | 0   |
| 1490, BEAM | , | 1, | 17, | 624, | 646, | 0   |
| 1491, BEAM | , | 1, | 17, | 646, | 668, | 0   |
| 1492, BEAM | , | 1, | 17, | 668, | 690, | 0   |
| 1494, BEAM | , | 1, | 17, | 625, | 647, | 0   |
| 1495, BEAM | , | 1, | 17, | 647, | 669, | 0   |
| 1496, BEAM | , | 1, | 17, | 669, | 691, | 0   |
| 1498, BEAM | , | 1, | 17, | 626, | 648, | 0   |
| 1499, BEAM | , | 1, | 17, | 648, | 670, | 0   |
| 1500, BEAM | , | 1, | 17, | 670, | 692, | 0   |
| 1502, BEAM | , | 1, | 17, | 627, | 649, | 0   |
| 1503, BEAM | , | 1, | 17, | 649, | 671, | 0   |
| 1504, BEAM | , | 1, | 17, | 671, | 693, | 0   |
| 1506, BEAM | , | 1, | 17, | 628, | 650, | 0   |
| 1507, BEAM | , | 1, | 17, | 650, | 672, | 0   |
| 1508, BEAM | , | 1, | 17, | 672, | 694, | 0   |

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|                                   |
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| 1510, BEAM , 1, 17, 629, 651, 0   |
| 1511, BEAM , 1, 17, 651, 673, 0   |
| 1512, BEAM , 1, 17, 673, 695, 0   |
| 1514, BEAM , 1, 17, 630, 652, 0   |
| 1515, BEAM , 1, 17, 652, 674, 0   |
| 1516, BEAM , 1, 17, 674, 696, 0   |
| 2001, BEAM , 1, 65, 51, 53, 180   |
| 2002, BEAM , 1, 65, 53, 55, 180   |
| 2003, BEAM , 1, 65, 55, 57, 180   |
| 2005, BEAM , 1, 65, 52, 54, 180   |
| 2006, BEAM , 1, 65, 54, 56, 180   |
| 2007, BEAM , 1, 65, 56, 58, 180   |
| 2009, BEAM , 1, 20, 51, 169, 180  |
| 2010, BEAM , 1, 20, 169, 53, 180  |
| 2011, BEAM , 1, 20, 53, 171, 180  |
| 2012, BEAM , 1, 20, 171, 55, 180  |
| 2013, BEAM , 1, 20, 55, 173, 180  |
| 2014, BEAM , 1, 20, 173, 57, 180  |
| 2017, BEAM , 1, 20, 52, 170, 180  |
| 2018, BEAM , 1, 20, 170, 54, 180  |
| 2019, BEAM , 1, 20, 54, 172, 180  |
| 2020, BEAM , 1, 20, 172, 56, 180  |
| 2021, BEAM , 1, 20, 56, 174, 180  |
| 2022, BEAM , 1, 20, 174, 58, 180  |
| 2025, BEAM , 1, 20, 496, 169, 180 |
| 2026, BEAM , 1, 20, 169, 520, 180 |
| 2027, BEAM , 1, 20, 520, 171, 180 |
| 2028, BEAM , 1, 20, 171, 544, 180 |
| 2029, BEAM , 1, 20, 544, 173, 180 |
| 2030, BEAM , 1, 20, 173, 568, 180 |
| 2033, BEAM , 1, 20, 505, 170, 180 |
| 2034, BEAM , 1, 20, 170, 529, 180 |
| 2035, BEAM , 1, 20, 529, 172, 180 |
| 2036, BEAM , 1, 20, 172, 553, 180 |
| 2037, BEAM , 1, 20, 553, 174, 180 |
| 2038, BEAM , 1, 20, 174, 577, 180 |
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| 2042, BEAM , 1, 19, 520, 544, 0   |
| 2043, BEAM , 1, 19, 544, 568, 0   |
| 2045, BEAM , 1, 19, 505, 529, 0   |
| 2046, BEAM , 1, 19, 529, 553, 0   |
| 2047, BEAM , 1, 19, 553, 577, 0   |
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| 3002, BEAM , 1, 15, 195, 210, 90  |
| 3003, BEAM , 1, 15, 210, 228, 90  |
| 3004, BEAM , 1, 15, 228, 243, 90  |
| 3005, BEAM , 1, 15, 243, 261, 90  |
| 3006, BEAM , 1, 15, 261, 276, 90  |
| 3009, BEAM , 1, 15, 178, 195, 90  |
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| 3012, BEAM , 1, 15, 228, 244, 90  |
| 3013, BEAM , 1, 15, 244, 261, 90  |
| 3014, BEAM , 1, 15, 261, 277, 90  |
| 3017, BEAM , 1, 15, 178, 196, 90  |
| 3018, BEAM , 1, 15, 196, 211, 90  |
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| 3022, BEAM , 1, 15, 262, 277, 90  |
| 3025, BEAM , 1, 15, 179, 196, 90  |
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| 3029, BEAM , 1, 15, 245, 262, 90  |
| 3030, BEAM , 1, 15, 262, 278, 90  |
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| 3037, BEAM , 1, 15, 245, 263, 90  |
| 3038, BEAM , 1, 15, 263, 278, 90  |
| 3041, BEAM , 1, 15, 180, 197, 90  |
| 3042, BEAM , 1, 15, 197, 213, 90  |
| 3043, BEAM , 1, 15, 213, 230, 90  |
| 3044, BEAM , 1, 15, 230, 246, 90  |
| 3045, BEAM , 1, 15, 246, 263, 90  |
| 3046, BEAM , 1, 15, 263, 279, 90  |

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| 3049, BEAM , 1, 15, 180, 198, 90 |
| 3050, BEAM , 1, 15, 198, 213, 90 |
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| 3052, BEAM , 1, 15, 231, 246, 90 |
| 3053, BEAM , 1, 15, 246, 264, 90 |
| 3054, BEAM , 1, 15, 264, 279, 90 |
| 3057, BEAM , 1, 15, 181, 198, 90 |
| 3058, BEAM , 1, 15, 198, 214, 90 |
| 3059, BEAM , 1, 15, 214, 231, 90 |
| 3060, BEAM , 1, 15, 231, 247, 90 |
| 3061, BEAM , 1, 15, 247, 264, 90 |
| 3062, BEAM , 1, 15, 264, 280, 90 |
| 3065, BEAM , 1, 15, 181, 199, 90 |
| 3066, BEAM , 1, 15, 199, 214, 90 |
| 3067, BEAM , 1, 15, 214, 232, 90 |
| 3068, BEAM , 1, 15, 232, 247, 90 |
| 3069, BEAM , 1, 15, 247, 265, 90 |
| 3070, BEAM , 1, 15, 265, 280, 90 |
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| 3074, BEAM , 1, 15, 199, 215, 90 |
| 3075, BEAM , 1, 15, 215, 232, 90 |
| 3076, BEAM , 1, 15, 232, 248, 90 |
| 3077, BEAM , 1, 15, 248, 265, 90 |
| 3078, BEAM , 1, 15, 265, 281, 90 |
| 3081, BEAM , 1, 15, 182, 143, 90 |
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| 3083, BEAM , 1, 15, 215, 147, 90 |
| 3084, BEAM , 1, 15, 147, 248, 90 |
| 3085, BEAM , 1, 15, 248, 151, 90 |
| 3086, BEAM , 1, 15, 151, 281, 90 |
| 3089, BEAM , 1, 15, 87, 143, 90  |
| 3090, BEAM , 1, 15, 143, 91, 90  |
| 3091, BEAM , 1, 15, 91, 147, 90  |
| 3092, BEAM , 1, 15, 147, 95, 90  |
| 3093, BEAM , 1, 15, 95, 151, 90  |
| 3094, BEAM , 1, 15, 151, 99, 90  |
| 3097, BEAM , 1, 15, 87, 61, 90   |
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| 3099, BEAM , 1, 15, 91, 65, 90   |
| 3100, BEAM , 1, 15, 65, 95, 90   |
| 3101, BEAM , 1, 15, 95, 69, 90   |
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| 3105, BEAM , 1, 15, 41, 61, 90   |
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| 3107, BEAM , 1, 15, 43, 65, 90   |
| 3108, BEAM , 1, 15, 65, 45, 90   |
| 3109, BEAM , 1, 15, 45, 69, 90   |
| 3110, BEAM , 1, 15, 69, 47, 90   |
| 3113, BEAM , 1, 15, 41, 62, 90   |
| 3114, BEAM , 1, 15, 62, 43, 90   |
| 3115, BEAM , 1, 15, 43, 66, 90   |
| 3116, BEAM , 1, 15, 66, 45, 90   |
| 3117, BEAM , 1, 15, 45, 70, 90   |
| 3118, BEAM , 1, 15, 70, 47, 90   |
| 3121, BEAM , 1, 15, 88, 62, 90   |
| 3122, BEAM , 1, 15, 62, 92, 90   |
| 3123, BEAM , 1, 15, 92, 66, 90   |
| 3124, BEAM , 1, 15, 66, 96, 90   |
| 3125, BEAM , 1, 15, 96, 70, 90   |
| 3126, BEAM , 1, 15, 70, 100, 90  |
| 3129, BEAM , 1, 15, 88, 144, 90  |
| 3130, BEAM , 1, 15, 144, 92, 90  |
| 3131, BEAM , 1, 15, 92, 148, 90  |
| 3132, BEAM , 1, 15, 148, 96, 90  |
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| 3134, BEAM , 1, 15, 152, 100, 90 |
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| 3138, BEAM , 1, 15, 144, 216, 90 |
| 3139, BEAM , 1, 15, 216, 148, 90 |
| 3140, BEAM , 1, 15, 148, 249, 90 |
| 3141, BEAM , 1, 15, 249, 152, 90 |
| 3142, BEAM , 1, 15, 152, 282, 90 |
| 3145, BEAM , 1, 15, 183, 200, 90 |
| 3146, BEAM , 1, 15, 200, 216, 90 |
| 3147, BEAM , 1, 15, 216, 233, 90 |
| 3148, BEAM , 1, 15, 233, 249, 90 |
| 3149, BEAM , 1, 15, 249, 266, 90 |
| 3150, BEAM , 1, 15, 266, 282, 90 |


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| 3153, BEAM , 1, 15, 184, 200, 90 |
| 3154, BEAM , 1, 15, 200, 217, 90 |
| 3155, BEAM , 1, 15, 217, 233, 90 |
| 3156, BEAM , 1, 15, 233, 250, 90 |
| 3157, BEAM , 1, 15, 250, 266, 90 |
| 3158, BEAM , 1, 15, 266, 283, 90 |
| 3161, BEAM , 1, 15, 184, 201, 90 |
| 3162, BEAM , 1, 15, 201, 217, 90 |
| 3163, BEAM , 1, 15, 217, 234, 90 |
| 3164, BEAM , 1, 15, 234, 250, 90 |
| 3165, BEAM , 1, 15, 250, 267, 90 |
| 3166, BEAM , 1, 15, 267, 283, 90 |
| 3169, BEAM , 1, 15, 185, 201, 90 |
| 3170, BEAM , 1, 15, 201, 218, 90 |
| 3171, BEAM , 1, 15, 218, 234, 90 |
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| 3173, BEAM , 1, 15, 251, 267, 90 |
| 3174, BEAM , 1, 15, 267, 284, 90 |
| 3177, BEAM , 1, 15, 185, 202, 90 |
| 3178, BEAM , 1, 15, 202, 218, 90 |
| 3179, BEAM , 1, 15, 218, 235, 90 |
| 3180, BEAM , 1, 15, 235, 251, 90 |
| 3181, BEAM , 1, 15, 251, 268, 90 |
| 3182, BEAM , 1, 15, 268, 284, 90 |
| 3185, BEAM , 1, 15, 186, 202, 90 |
| 3186, BEAM , 1, 15, 202, 219, 90 |
| 3187, BEAM , 1, 15, 219, 235, 90 |
| 3188, BEAM , 1, 15, 235, 252, 90 |
| 3189, BEAM , 1, 15, 252, 268, 90 |
| 3190, BEAM , 1, 15, 268, 285, 90 |
| 3193, BEAM , 1, 15, 186, 203, 90 |
| 3194, BEAM , 1, 15, 203, 219, 90 |
| 3195, BEAM , 1, 15, 219, 236, 90 |
| 3196, BEAM , 1, 15, 236, 252, 90 |
| 3197, BEAM , 1, 15, 252, 269, 90 |
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| 3201, BEAM , 1, 15, 187, 203, 90 |
| 3202, BEAM , 1, 15, 203, 220, 90 |
| 3203, BEAM , 1, 15, 220, 236, 90 |
| 3204, BEAM , 1, 15, 236, 253, 90 |
| 3205, BEAM , 1, 15, 253, 269, 90 |
| 3206, BEAM , 1, 15, 269, 286, 90 |
| 3209, BEAM , 1, 15, 187, 204, 90 |
| 3210, BEAM , 1, 15, 204, 220, 90 |
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| 3212, BEAM , 1, 15, 237, 253, 90 |
| 3213, BEAM , 1, 15, 253, 270, 90 |
| 3214, BEAM , 1, 15, 270, 286, 90 |
| 3217, BEAM , 1, 15, 188, 204, 90 |
| 3218, BEAM , 1, 15, 204, 221, 90 |
| 3219, BEAM , 1, 15, 221, 237, 90 |
| 3220, BEAM , 1, 15, 237, 254, 90 |
| 3221, BEAM , 1, 15, 254, 270, 90 |
| 3222, BEAM , 1, 15, 270, 287, 90 |
| 3225, BEAM , 1, 15, 188, 145, 90 |
| 3226, BEAM , 1, 15, 145, 221, 90 |
| 3227, BEAM , 1, 15, 221, 149, 90 |
| 3228, BEAM , 1, 15, 149, 254, 90 |
| 3229, BEAM , 1, 15, 254, 153, 90 |
| 3230, BEAM , 1, 15, 153, 287, 90 |
| 3233, BEAM , 1, 15, 89, 145, 90  |
| 3234, BEAM , 1, 15, 145, 93, 90  |
| 3235, BEAM , 1, 15, 93, 149, 90  |
| 3236, BEAM , 1, 15, 149, 97, 90  |
| 3237, BEAM , 1, 15, 97, 153, 90  |
| 3238, BEAM , 1, 15, 153, 101, 90 |
| 3241, BEAM , 1, 15, 89, 63, 90   |
| 3242, BEAM , 1, 15, 63, 93, 90   |
| 3243, BEAM , 1, 15, 93, 67, 90   |
| 3244, BEAM , 1, 15, 67, 97, 90   |
| 3245, BEAM , 1, 15, 97, 71, 90   |
| 3246, BEAM , 1, 15, 71, 101, 90  |
| 3249, BEAM , 1, 15, 42, 63, 90   |
| 3250, BEAM , 1, 15, 63, 44, 90   |
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| 3252, BEAM , 1, 15, 67, 46, 90   |
| 3253, BEAM , 1, 15, 46, 71, 90   |
| 3254, BEAM , 1, 15, 71, 48, 90   |



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| 3257, BEAM , 1, 15, 42, 64, 90   |
| 3258, BEAM , 1, 15, 64, 44, 90   |
| 3259, BEAM , 1, 15, 44, 68, 90   |
| 3260, BEAM , 1, 15, 68, 46, 90   |
| 3261, BEAM , 1, 15, 46, 72, 90   |
| 3262, BEAM , 1, 15, 72, 48, 90   |
| 3265, BEAM , 1, 15, 90, 64, 90   |
| 3266, BEAM , 1, 15, 64, 94, 90   |
| 3267, BEAM , 1, 15, 94, 68, 90   |
| 3268, BEAM , 1, 15, 68, 98, 90   |
| 3269, BEAM , 1, 15, 98, 72, 90   |
| 3270, BEAM , 1, 15, 72, 102, 90  |
| 3273, BEAM , 1, 15, 90, 146, 90  |
| 3274, BEAM , 1, 15, 146, 94, 90  |
| 3275, BEAM , 1, 15, 94, 150, 90  |
| 3276, BEAM , 1, 15, 150, 98, 90  |
| 3277, BEAM , 1, 15, 98, 154, 90  |
| 3278, BEAM , 1, 15, 154, 102, 90 |
| 3281, BEAM , 1, 15, 189, 146, 90 |
| 3282, BEAM , 1, 15, 146, 222, 90 |
| 3283, BEAM , 1, 15, 222, 150, 90 |
| 3284, BEAM , 1, 15, 150, 255, 90 |
| 3285, BEAM , 1, 15, 255, 154, 90 |
| 3286, BEAM , 1, 15, 154, 288, 90 |
| 3289, BEAM , 1, 15, 189, 205, 90 |
| 3290, BEAM , 1, 15, 205, 222, 90 |
| 3291, BEAM , 1, 15, 222, 238, 90 |
| 3292, BEAM , 1, 15, 238, 255, 90 |
| 3293, BEAM , 1, 15, 255, 271, 90 |
| 3294, BEAM , 1, 15, 271, 288, 90 |
| 3297, BEAM , 1, 15, 190, 205, 90 |
| 3298, BEAM , 1, 15, 205, 223, 90 |
| 3299, BEAM , 1, 15, 223, 238, 90 |
| 3300, BEAM , 1, 15, 238, 256, 90 |
| 3301, BEAM , 1, 15, 256, 271, 90 |
| 3302, BEAM , 1, 15, 271, 289, 90 |
| 3305, BEAM , 1, 15, 190, 206, 90 |
| 3306, BEAM , 1, 15, 206, 223, 90 |
| 3307, BEAM , 1, 15, 223, 239, 90 |
| 3308, BEAM , 1, 15, 239, 256, 90 |
| 3309, BEAM , 1, 15, 256, 272, 90 |
| 3310, BEAM , 1, 15, 272, 289, 90 |
| 3313, BEAM , 1, 15, 191, 206, 90 |
| 3314, BEAM , 1, 15, 206, 224, 90 |
| 3315, BEAM , 1, 15, 224, 239, 90 |
| 3316, BEAM , 1, 15, 239, 257, 90 |
| 3317, BEAM , 1, 15, 257, 272, 90 |
| 3318, BEAM , 1, 15, 272, 290, 90 |
| 3321, BEAM , 1, 15, 191, 207, 90 |
| 3322, BEAM , 1, 15, 207, 224, 90 |
| 3323, BEAM , 1, 15, 224, 240, 90 |
| 3324, BEAM , 1, 15, 240, 257, 90 |
| 3325, BEAM , 1, 15, 257, 273, 90 |
| 3326, BEAM , 1, 15, 273, 290, 90 |
| 3329, BEAM , 1, 15, 192, 207, 90 |
| 3330, BEAM , 1, 15, 207, 225, 90 |
| 3331, BEAM , 1, 15, 225, 240, 90 |
| 3332, BEAM , 1, 15, 240, 258, 90 |
| 3333, BEAM , 1, 15, 258, 273, 90 |
| 3334, BEAM , 1, 15, 273, 291, 90 |
| 3337, BEAM , 1, 15, 192, 208, 90 |
| 3338, BEAM , 1, 15, 208, 225, 90 |
| 3339, BEAM , 1, 15, 225, 241, 90 |
| 3340, BEAM , 1, 15, 241, 258, 90 |
| 3341, BEAM , 1, 15, 258, 274, 90 |
| 3342, BEAM , 1, 15, 274, 291, 90 |
| 3345, BEAM , 1, 15, 193, 208, 90 |
| 3346, BEAM , 1, 15, 208, 226, 90 |
| 3347, BEAM , 1, 15, 226, 241, 90 |
| 3348, BEAM , 1, 15, 241, 259, 90 |
| 3349, BEAM , 1, 15, 259, 274, 90 |
| 3350, BEAM , 1, 15, 274, 292, 90 |
| 3353, BEAM , 1, 15, 193, 209, 90 |
| 3354, BEAM , 1, 15, 209, 226, 90 |
| 3355, BEAM , 1, 15, 226, 242, 90 |
| 3356, BEAM , 1, 15, 242, 259, 90 |
| 3357, BEAM , 1, 15, 259, 275, 90 |
| 3358, BEAM , 1, 15, 275, 292, 90 |

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|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
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|                                  |
|----------------------------------|
| 3361, BEAM , 1, 15, 194, 209, 90 |
| 3362, BEAM , 1, 15, 209, 227, 90 |
| 3363, BEAM , 1, 15, 227, 242, 90 |
| 3364, BEAM , 1, 15, 242, 260, 90 |
| 3365, BEAM , 1, 15, 260, 275, 90 |
| 3366, BEAM , 1, 15, 275, 293, 90 |
| 4001, BEAM , 3, 46, 1, 905, 0    |
| 4002, BEAM , 3, 46, 905, 906, 0  |
| 4003, BEAM , 3, 46, 906, 907, 0  |
| 4004, BEAM , 3, 46, 907, 908, 0  |
| 4005, BEAM , 3, 46, 908, 909, 0  |
| 4006, BEAM , 3, 46, 909, 910, 0  |
| 4007, BEAM , 3, 46, 910, 911, 0  |
| 4008, BEAM , 3, 46, 911, 912, 0  |
| 4009, BEAM , 3, 46, 912, 913, 0  |
| 4010, BEAM , 3, 46, 913, 25, 0   |
| 4011, BEAM , 3, 46, 2, 914, 0    |
| 4012, BEAM , 3, 46, 914, 915, 0  |
| 4013, BEAM , 3, 46, 915, 916, 0  |
| 4014, BEAM , 3, 46, 916, 917, 0  |
| 4015, BEAM , 3, 46, 917, 918, 0  |
| 4016, BEAM , 3, 46, 918, 919, 0  |
| 4017, BEAM , 3, 46, 919, 920, 0  |
| 4018, BEAM , 3, 46, 920, 921, 0  |
| 4019, BEAM , 3, 46, 921, 922, 0  |
| 4020, BEAM , 3, 46, 922, 26, 0   |
| 5009, BEAM , 2, 22, 720, 761, 0  |
| 5010, BEAM , 2, 22, 761, 802, 0  |
| 5011, BEAM , 2, 22, 802, 843, 0  |
| 5013, BEAM , 2, 23, 721, 762, 0  |
| 5014, BEAM , 2, 23, 762, 803, 0  |
| 5015, BEAM , 2, 23, 803, 844, 0  |
| 5017, BEAM , 2, 23, 723, 764, 0  |
| 5018, BEAM , 2, 23, 764, 805, 0  |
| 5019, BEAM , 2, 23, 805, 846, 0  |
| 5021, BEAM , 2, 23, 724, 765, 0  |
| 5022, BEAM , 2, 23, 765, 806, 0  |
| 5023, BEAM , 2, 23, 806, 847, 0  |
| 5025, BEAM , 2, 23, 726, 767, 0  |
| 5026, BEAM , 2, 23, 767, 808, 0  |
| 5027, BEAM , 2, 23, 808, 849, 0  |
| 5029, BEAM , 2, 24, 727, 768, 0  |
| 5030, BEAM , 2, 24, 768, 809, 0  |
| 5031, BEAM , 2, 24, 809, 850, 0  |
| 5033, BEAM , 2, 25, 729, 770, 0  |
| 5034, BEAM , 2, 25, 770, 811, 0  |
| 5035, BEAM , 2, 25, 811, 852, 0  |
| 5037, BEAM , 2, 25, 731, 772, 0  |
| 5038, BEAM , 2, 25, 772, 813, 0  |
| 5039, BEAM , 2, 25, 813, 854, 0  |
| 5041, BEAM , 2, 25, 733, 774, 0  |
| 5042, BEAM , 2, 25, 774, 815, 0  |
| 5043, BEAM , 2, 25, 815, 856, 0  |
| 5045, BEAM , 2, 26, 735, 776, 0  |
| 5046, BEAM , 2, 26, 776, 817, 0  |
| 5047, BEAM , 2, 26, 817, 858, 0  |
| 5049, BEAM , 2, 27, 736, 777, 0  |
| 5050, BEAM , 2, 27, 777, 818, 0  |
| 5051, BEAM , 2, 27, 818, 859, 0  |
| 5053, BEAM , 2, 27, 738, 779, 0  |
| 5054, BEAM , 2, 27, 779, 820, 0  |
| 5055, BEAM , 2, 27, 820, 861, 0  |
| 5070, BEAM , 2, 27, 863, 822, 0  |
| 5071, BEAM , 2, 27, 822, 781, 0  |
| 5072, BEAM , 2, 27, 781, 740, 0  |
| 5074, BEAM , 2, 27, 865, 824, 0  |
| 5075, BEAM , 2, 27, 824, 783, 0  |
| 5076, BEAM , 2, 27, 783, 742, 0  |
| 5078, BEAM , 2, 26, 866, 825, 0  |
| 5079, BEAM , 2, 26, 825, 784, 0  |
| 5080, BEAM , 2, 26, 784, 743, 0  |
| 5082, BEAM , 2, 25, 868, 827, 0  |
| 5083, BEAM , 2, 25, 827, 786, 0  |
| 5084, BEAM , 2, 25, 786, 745, 0  |
| 5086, BEAM , 2, 25, 870, 829, 0  |
| 5087, BEAM , 2, 25, 829, 788, 0  |
| 5088, BEAM , 2, 25, 788, 747, 0  |
| 5090, BEAM , 2, 25, 872, 831, 0  |

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|                                 |
|---------------------------------|
| 5091, BEAM , 2, 25, 831, 790, 0 |
| 5092, BEAM , 2, 25, 790, 749, 0 |
| 5094, BEAM , 2, 24, 874, 833, 0 |
| 5095, BEAM , 2, 24, 833, 792, 0 |
| 5096, BEAM , 2, 24, 792, 751, 0 |
| 5098, BEAM , 2, 23, 875, 834, 0 |
| 5099, BEAM , 2, 23, 834, 793, 0 |
| 5100, BEAM , 2, 23, 793, 752, 0 |
| 5102, BEAM , 2, 23, 877, 836, 0 |
| 5103, BEAM , 2, 23, 836, 795, 0 |
| 5104, BEAM , 2, 23, 795, 754, 0 |
| 5110, BEAM , 2, 23, 878, 837, 0 |
| 5111, BEAM , 2, 23, 837, 796, 0 |
| 5112, BEAM , 2, 23, 796, 755, 0 |
| 5114, BEAM , 2, 23, 880, 839, 0 |
| 5115, BEAM , 2, 23, 839, 798, 0 |
| 5116, BEAM , 2, 23, 798, 757, 0 |
| 5118, BEAM , 2, 22, 881, 840, 0 |
| 5119, BEAM , 2, 22, 840, 799, 0 |
| 5120, BEAM , 2, 22, 799, 758, 0 |

```
*GROUP ; Group
TRAVI PRINCIPALI, 719to882, 101to140 201to240 301to340 401to440, 0
TRASVERSI INTERMEDI, 107to134 327to452 489to495 497to504 506to519 521to528 \
  530to543 545to552 554to567 569to576 578to584 609to696, 1014to1019 \
  1022to1027 1030to1035 1038to1043 1110to1422by8 1111to1423by8 \
  1112to1424by8 1113to1425by8 1114to1426by8 1115to1427by8 1430to1514by4 \
  1431to1515by4 1432to1516by4, 0
TRASVERSI PILE, 51to58 169to174 496to568by24 505to577by24, 2001to2003 \
  2005to2007 2009to2014 2017to2022 2025to2030 2033to2038 2041to2043 \
  2045to2047, 0
PILE , 1 2 25 26, 4001to4020, 0
NODI , 1 2 21to28 31to38 41to48 51to58 61to72 77to84 87to102 107to134 \
  143to154 159to166 169to174 177to293 327to452 489to584 609to696 719to886, \
  , 0
CONTROVENTI, 41to48 61to72 87to102 143to154 177to293, 3001to3361by8 \
  3002to3362by8 3003to3363by8 3004to3364by8 3005to3365by8 3006to3366by8, 0
SOLETTA FITTIZIA, , 5009to5053by4 5010to5054by4 5011to5055by4 5070to5102by4 \
  5071to5103by4 5072to5104by4 5110to5112 5114to5116 5118to5120, 0
```

```
*BNDR-GROUP ; Boundary Group
RIGID LINK, 0
APPOGGI SPALLE, 0
SUPPORTI, 0
CONTR.FITTIZI SOLETTA, 0
APPOGGI PILE, 6492008
```

```
*LOAD-GROUP ; Load Group
PESO SOLETTA
PESO FINITURE
PESO ACCIAIO E PILE
```

```
*MATERIAL ; Material
1, STEEL, S355 (g x1.15) , 0, 0, , C, YES, 0.05, 2, 2.1000e+008, 0.3, 1.2000e-005, 88.53, 9.025
2, CONC , C32/40 SOL.TRASV , 0, 0, , C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e-005, 0, 0
3, CONC , C28/35 PILE , 0, 0, , C, NO, 0.05, 1, NTC08(RC) , , C28/35
4, CONC , C32/40 SOLETTA , 0, 0, , C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e-005, 0, 0
5, SRC , COMPOSITO , 0, 0, , C, YES, 0.05, 2, 2.1000e+008, 0.3, 1.2000e-005, 88.53, 9.02, \
  2, 3.3345e+007, 0.2, 1.0000e-005, 0, 0
```

```
*MATL-COLOR
1, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
2, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
3, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
4, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
5, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
```


```
*TDM-TYPE ; Time Dependent Material
NAME=SOLETTA, European, 32000, 70, 0.3, Class N, 1, 1, NO
```

```
*TDM-ELAST ; Time Dependent Material (Comp. Strength)
NAME=SOLETTA, CODE, EUROPEAN, 40000, 2
```

```
*TDM-LINK ; Time Dependent Material Link
4, SOLETTA, SOLETTA
```

```
*SECTION ; Section
```

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```

1, COMPOSITE , C1 Interno      , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2, COMPOSITE , C2 Interno      , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3, COMPOSITE , C1 Esterno      , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
4, COMPOSITE , C2 esterno      , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
5, TAPERED   , C4 Interno Sx    , CT, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0
6, TAPERED   , C4 Interno Dx    , CT, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0
7, COMPOSITE , C5 Interno      , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
8, COMPOSITE , C6 Interno      , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
9, TAPERED   , C4 Esterno Sx    , CT, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0

```

PROJECT TITLE : CASALPUSTERLENGO

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|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
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```

0
0

10, TAPERED , C4 Esterno Dx , CT, 0, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

11, COMPOSITE , C3 Interno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
12, COMPOSITE , C5 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
13, COMPOSITE , C6 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
14, COMPOSITE , C3 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
15, DBUSER , CONTROV 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0,
0, 0, 0, 0
16, COMPOSITE , C4 Interno Centr , CT, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
17, DBUSER , HSUP_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
18, DBUSER , HINF_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
19, DBUSER , HSUP_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,
0, 0, 0, 0
20, DBUSER , DIAG_PILE 2L80x8d20, CC, 0, 1, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.02, 0
, 0, 0, 0, 0
21, COMPOSITE , C4 Esterno Centr , CT, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
22, DBUSER , 330x30 , RT, 0, 0, 1, 0.8, 0, 0, NO, SB , 2, 0.3, 3.3, 0, 0, 0, 0, 0, 0, 0
23, DBUSER , 500x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5, 0, 0, 0, 0, 0, 0, 0
24, DBUSER , 400x30 , LT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0
25, DBUSER , 300x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 3, 0, 0, 0, 0, 0, 0, 0
26, DBUSER , 400x30 dx , RT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0
27, DBUSER , 520x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5.2, 0, 0, 0, 0, 0, 0, 0
46, DBUSER , PILA , CC, 0, 0, 0, 0, 0, 0, YES, STRK, 2, 1.5, 12, 0, 0, 0, 0, 0, 0, 0
64, DBUSER , DIAG_CORR 2L80x8d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.015,
0, 0, 0, 0, 0
65, DBUSER , HINF_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,

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0, 0, 0, 0

\*SECT-COLOR

```

1, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
2, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
3, 3, 111, 0, 93, 255, 87, 3, 111, 0, NO, 0.5
4, 70, 70, 0, 192, 255, 0, 70, 70, 0, NO, 0.5
5, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
6, 0, 13, 13, 0, 128, 128, 0, 13, 13, NO, 0.5
7, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
8, 123, 40, 0, 255, 192, 160, 123, 40, 0, NO, 0.5
9, 3, 0, 108, 163, 160, 255, 3, 0, 108, NO, 0.5
10, 3, 111, 66, 163, 255, 160, 3, 111, 66, NO, 0.5
11, 255, 0, 0, 0, 255, 0, 0, 0, 255, NO, 0.5
12, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
13, 0, 72, 72, 0, 192, 192, 0, 72, 72, NO, 0.5
14, 102, 69, 0, 192, 128, 0, 102, 69, 0, NO, 0.5
15, 65, 165, 65, 192, 192, 192, 65, 165, 65, NO, 0.5
16, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
17, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
18, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
19, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
20, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
21, 87, 87, 0, 192, 192, 0, 87, 87, 0, NO, 0.5
22, 70, 70, 0, 192, 255, 0, 70, 70, 0, NO, 0.5
23, 121, 0, 91, 255, 0, 192, 121, 0, 91, NO, 0.5
24, 89, 0, 162, 212, 160, 255, 89, 0, 162, NO, 0.5
25, 0, 43, 19, 0, 128, 57, 0, 43, 19, NO, 0.5
26, 115, 0, 0, 255, 87, 87, 115, 0, 0, NO, 0.5
27, 34, 77, 0, 85, 192, 0, 34, 77, 0, NO, 0.5
46, 85, 32, 0, 192, 72, 0, 85, 32, 0, NO, 0.5
64, 85, 32, 0, 192, 72, 0, 85, 32, 0, NO, 0.5
65, 85, 32, 0, 192, 72, 0, 85, 32, 0, NO, 0.5
    
```

\*COMP-GEN-SECT-PSC-DESIGN ; Composite Section for PSC Design

```

1, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
2, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
3, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
4, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
5, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
6, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
7, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
8, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
9, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
10, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
11, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
12, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
13, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
14, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
16, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
21, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
    
```

\*DGN-SECT

```

1, COMPOSITE , C1 Interno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2, COMPOSITE , C2 Interno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3, COMPOSITE , C1 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.94, 0.015, 0.5, 0.03, 0.6, 0.03
0, 0, 0, 0, 0, 0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
4, COMPOSITE , C2 esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.015, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
5, TAPERED , C4 Interno Sx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

6, TAPERED , C4 Interno Dx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
3.2, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
3.2, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

7, COMPOSITE , C5 Interno , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
8, COMPOSITE , C6 Interno , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
9, TAPERED , C4 Esterno Sx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

10, TAPERED , C4 Esterno Dx , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, CP_I, 1, 1, CMP-I
1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
2.95, 1.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
2.95, 0.92, 0.02, 0.6, , 0.04, 0.7, , 0.04
0, 0, 0, 0, 0, 0
0
0
0
0, 0, 0, 0, 0, 0
0
0
0

11, COMPOSITE , C3 Interno , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
12, COMPOSITE , C5 Esterno , CT, 0, 0, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04

```



PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
13, COMPOSITE , C6 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.012, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
14, COMPOSITE , C3 Esterno , CT, 0, 0, 0, 0, 0, 0, YES, I
0.93, 0.02, 0.5, 0.03, 0.6, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
15, DBUSER , CONTROV 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0,
0, 0, 0, 0
16, COMPOSITE , C4 Interno Centr , CT, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.2, 1, 0, 3.2, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
17, DBUSER , HSUP_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
18, DBUSER , HINF_CORR 2L80x10d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.01, 0.01, 0.015, 0
, 0, 0, 0, 0
19, DBUSER , HSUP_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,
0, 0, 0, 0
20, DBUSER , DIAG_PILE 2L80x8d20, CC, 0, 1, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.02, 0
, 0, 0, 0, 0
21, COMPOSITE , C4 Esterno Centr , CT, 0, 0, 0, 0, 0, 0, YES, I
1.92, 0.02, 0.6, 0.04, 0.7, 0.04
0, 0, 0, 0, 0, 0
0
0
0
2.95, 1, 0, 2.95, 0.24, 0.06, 6.2978, 0, 0.3, 0.2, NO, ,
22, DBUSER , 330x30 , RT, 0, 0, 1, 0.8, 0, 0, NO, SB , 2, 0.3, 3.3, 0, 0, 0, 0, 0, 0, 0, 0
23, DBUSER , 500x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5, 0, 0, 0, 0, 0, 0, 0, 0
24, DBUSER , 400x30 , LT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0, 0
25, DBUSER , 300x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 3, 0, 0, 0, 0, 0, 0, 0, 0
26, DBUSER , 400x30 dx , RT, 0, 1, 1, 1.5, 0, 0, NO, SB , 2, 0.3, 4, 0, 0, 0, 0, 0, 0, 0, 0
27, DBUSER , 520x30 , CT, 0, 0, 0, 0, 0, 0, NO, SB , 2, 0.3, 5.2, 0, 0, 0, 0, 0, 0, 0, 0
46, DBUSER , PILA , CC, 0, 0, 0, 0, 0, 0, YES, STRK, 2, 1.5, 12, 0, 0, 0, 0, 0, 0, 0
64, DBUSER , DIAG_CORR 2L80x8d15, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.08, 0.08, 0.008, 0.008, 0.015,
0, 0, 0, 0, 0
65, DBUSER , HINF_PILE 2L100x10d20, CC, 0, 0, 0, 0, 0, 0, YES, 2L , 2, 0.1, 0.1, 0.01, 0.01, 0.02, 0,
0, 0, 0, 0

```

```

*REBAR-PSC ; Reinforcement of Section
SECT=1, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=2, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=3, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=4, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2

```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |


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I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
SECT=5, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
SECT=6, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
SECT=7, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
SECT=8, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=9, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
SECT=10, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
SECT=11, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=12, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=13, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
SECT=14, YES, NO, NO
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=16, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
SECT=21, YES, NO, YES
NO, , , NO, , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2

```

\*TS-GROUP ; Tapered Section Group  
T3 SX, 310 311, LINEAR, , , LINEAR, , , 0

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

```
T3 DX, 314 315, LINEAR, , , , LINEAR, , , , 0
T1 SX, 110 111, LINEAR, , , , LINEAR, , , , 0
T1 DX, 114 115, LINEAR, , , , LINEAR, , , , 0
T2 SX, 210 211, LINEAR, , , , LINEAR, , , , 0
T2 DX, 214 215, LINEAR, , , , LINEAR, , , , 0
T4 SX, 410 411, LINEAR, , , , LINEAR, , , , 0
T4 DX, 414 415, LINEAR, , , , LINEAR, , , , 0
T1 SX 2, 126 127, LINEAR, , , , LINEAR, , , , 0
T1 DX 2, 130 131, LINEAR, , , , LINEAR, , , , 0
T2 SX 2, 226 227, LINEAR, , , , LINEAR, , , , 0
T2 DX 2, 230 231, LINEAR, , , , LINEAR, , , , 0
T3 SX 2, 326 327, LINEAR, , , , LINEAR, , , , 0
T3 DX 2, 330 331, LINEAR, , , , LINEAR, , , , 0
T4 SX 2, 426 427, LINEAR, , , , LINEAR, , , , 0
T4 DX 2, 430 431, LINEAR, , , , LINEAR, , , , 0
```

\*STLDCASE ; Static Load Cases

```
Peso acciaio e pile, D ,
Peso soletta, D ,
Finiture, DW,
Vento strutture (Y+), W ,
Vento mobili (Y+), WL,
Frenamento (X+), BRK,
Termico Unif. +15°, T ,
Termico diff. +5°/H, TPG,
```

\*DGN-CTRL ; General Design Data

```
NO, NO, YES, , , , , 0, 3D, YES, NO, NO, 0, 1, NO, 2
```

\*DGN-STEEL ; Steel Design Code

```
CODE=Eurocode3-2:05, NO, 0
1.05, 1.1, 1.25, 0, 0
```

\*CONSTRAINT ; Supports

```
1 2 883 884, 111111, SUPPORTI
```

\*ELASTICLINK

```
185, 77, 159, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
186, 79, 161, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
187, 81, 163, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
188, 83, 165, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
190, 78, 160, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
191, 80, 162, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
192, 82, 164, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
193, 84, 166, GEN , 0, 767676, 1302, 1302, 1, 1, 1, NO, 0.5, 0.5, APPOGGI SPALLE
195, 21, 31, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
196, 23, 33, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
197, 885, 35, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
198, 27, 37, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
200, 22, 32, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
201, 24, 34, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
202, 886, 36, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
203, 28, 38, GEN , 0, 1.79446e+006, 2855, 2855, 1, 1, 1, NO, 0.5, 0.5, APPOGGI PILE
```

\*MEMBERTYPE ; Modify Member Type

```
1014to1019 1022to1027 1030to1035 1038to1043 1110to1422by8 , BRACE, 0
1111to1423by8 1112to1424by8 1113to1425by8 1114to1426by8 , BRACE, 0
1115to1427by8 1430to1514by4 1431to1515by4 1432to1516by4 2001to2003 , BRACE, 0
2005to2007 2009to2014 2017to2022 2025to2030 2033to2038 2041to2043 , BRACE, 0
2045to2047 3001to3361by8 3002to3362by8 3003to3363by8 3004to3364by8 , BRACE, 0
3005to3365by8 3006to3366by8, BRACE, 0
```

\*RIGIDLINK ; Rigid Link

```
25, 111111, 21 23 27 885, RIGID LINK
26, 111111, 22 24 28 886, RIGID LINK
720, 111111, 159 177 327 489 609, RIGID LINK
721, 111111, 178 328 490 610, RIGID LINK
723, 111111, 179 329 491 611, RIGID LINK
724, 111111, 180 330 492 612, RIGID LINK
726, 111111, 181 331 493 613, RIGID LINK
727, 111111, 182 332 494 614, RIGID LINK
729, 111111, 87 107 495 615, RIGID LINK
731, 111111, 31 41 51 496, RIGID LINK
733, 111111, 88 108 497 616, RIGID LINK
735, 111111, 183 333 498 617, RIGID LINK
736, 111111, 184 334 499 618, RIGID LINK
738, 111111, 185 335 500 619, RIGID LINK
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

740, 111111, 186 336 501 620, RIGID LINK  
 742, 111111, 187 337 502 621, RIGID LINK  
 743, 111111, 188 338 503 622, RIGID LINK  
 745, 111111, 89 109 504 623, RIGID LINK  
 747, 111111, 32 42 52 505, RIGID LINK  
 749, 111111, 90 110 506 624, RIGID LINK  
 751, 111111, 189 339 507 625, RIGID LINK  
 752, 111111, 190 340 508 626, RIGID LINK  
 754, 111111, 191 341 509 627, RIGID LINK  
 755, 111111, 192 342 510 628, RIGID LINK  
 757, 111111, 193 343 511 629, RIGID LINK  
 758, 111111, 160 194 344 512 630, RIGID LINK  
 761, 111111, 161 210 363 513 631, RIGID LINK  
 762, 111111, 211 364 514 632, RIGID LINK  
 764, 111111, 212 365 515 633, RIGID LINK  
 765, 111111, 213 366 516 634, RIGID LINK  
 767, 111111, 214 367 517 635, RIGID LINK  
 768, 111111, 215 368 518 636, RIGID LINK  
 770, 111111, 91 115 519 637, RIGID LINK  
 772, 111111, 33 43 53 520, RIGID LINK  
 774, 111111, 92 116 521 638, RIGID LINK  
 776, 111111, 216 369 522 639, RIGID LINK  
 777, 111111, 217 370 523 640, RIGID LINK  
 779, 111111, 218 371 524 641, RIGID LINK  
 781, 111111, 219 372 525 642, RIGID LINK  
 783, 111111, 220 373 526 643, RIGID LINK  
 784, 111111, 221 374 527 644, RIGID LINK  
 786, 111111, 93 117 528 645, RIGID LINK  
 788, 111111, 34 44 54 529, RIGID LINK  
 790, 111111, 94 118 530 646, RIGID LINK  
 792, 111111, 222 375 531 647, RIGID LINK  
 793, 111111, 223 376 532 648, RIGID LINK  
 795, 111111, 224 377 533 649, RIGID LINK  
 796, 111111, 225 378 534 650, RIGID LINK  
 798, 111111, 226 379 535 651, RIGID LINK  
 799, 111111, 162 227 380 536 652, RIGID LINK  
 802, 111111, 163 243 399 537 653, RIGID LINK  
 803, 111111, 244 400 538 654, RIGID LINK  
 805, 111111, 245 401 539 655, RIGID LINK  
 806, 111111, 246 402 540 656, RIGID LINK  
 808, 111111, 247 403 541 657, RIGID LINK  
 809, 111111, 248 404 542 658, RIGID LINK  
 811, 111111, 95 123 543 659, RIGID LINK  
 813, 111111, 35 45 55 544, RIGID LINK  
 815, 111111, 96 124 545 660, RIGID LINK  
 817, 111111, 249 405 546 661, RIGID LINK  
 818, 111111, 250 406 547 662, RIGID LINK  
 820, 111111, 251 407 548 663, RIGID LINK  
 822, 111111, 252 408 549 664, RIGID LINK  
 824, 111111, 253 409 550 665, RIGID LINK  
 825, 111111, 254 410 551 666, RIGID LINK  
 827, 111111, 97 125 552 667, RIGID LINK  
 829, 111111, 36 46 56 553, RIGID LINK  
 831, 111111, 98 126 554 668, RIGID LINK  
 833, 111111, 255 411 555 669, RIGID LINK  
 834, 111111, 256 412 556 670, RIGID LINK  
 836, 111111, 257 413 557 671, RIGID LINK  
 837, 111111, 258 414 558 672, RIGID LINK  
 839, 111111, 259 415 559 673, RIGID LINK  
 840, 111111, 164 260 416 560 674, RIGID LINK  
 843, 111111, 165 276 435 561 675, RIGID LINK  
 844, 111111, 277 436 562 676, RIGID LINK  
 846, 111111, 278 437 563 677, RIGID LINK  
 847, 111111, 279 438 564 678, RIGID LINK  
 849, 111111, 280 439 565 679, RIGID LINK  
 850, 111111, 281 440 566 680, RIGID LINK  
 852, 111111, 99 131 567 681, RIGID LINK  
 854, 111111, 37 47 57 568, RIGID LINK  
 856, 111111, 100 132 569 682, RIGID LINK  
 858, 111111, 282 441 570 683, RIGID LINK  
 859, 111111, 283 442 571 684, RIGID LINK  
 861, 111111, 284 443 572 685, RIGID LINK  
 863, 111111, 285 444 573 686, RIGID LINK  
 865, 111111, 286 445 574 687, RIGID LINK  
 866, 111111, 287 446 575 688, RIGID LINK  
 868, 111111, 101 133 576 689, RIGID LINK  
 870, 111111, 38 48 58 577, RIGID LINK  
 872, 111111, 102 134 578 690, RIGID LINK

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

874, 111111, 288 447 579 691, RIGID LINK  
 875, 111111, 289 448 580 692, RIGID LINK  
 877, 111111, 290 449 581 693, RIGID LINK  
 878, 111111, 291 450 582 694, RIGID LINK  
 880, 111111, 292 451 583 695, RIGID LINK  
 881, 111111, 166 293 452 584 696, RIGID LINK  
 883, 111111, 77to83by2, RIGID LINK  
 884, 111111, 78to84by2, RIGID LINK

\*LOADTOMASS, XYZ, YES, YES, YES, YES, 9.806  
 Peso soletta, 1, Finiture, 1

\*USE-STLD, Peso acciaio e pile

\*SELEWEIGHT, 0, 0, -1, PESO ACCIAIO E PILE

; End of data for load case [Peso acciaio e pile] -----

\*USE-STLD, Peso soletta

\*BEAMLOAD ; Element Beam Loads

101, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 102, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 103, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 104, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 105, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 106, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 107, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 108, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 109, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 110, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 111, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 112, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 113, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 114, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 115, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 116, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 117, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 118, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 119, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 120, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 121, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 122, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 123, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 124, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 125, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 126, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 127, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 128, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0  
 , NO  
 129, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0









PROJECT TITLE : CASALPUSTERLENGO

| MIDAS | Company |              | Client    |                 |
|-------|---------|--------------|-----------|-----------------|
|       | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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, NO
427, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
428, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
429, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
430, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
431, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
432, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
433, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
434, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
435, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
436, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
437, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
438, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
439, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO
440, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -27.6, 1, -27.6, 0, 0, 0, 0, PESO SOLETTA, NO, 0, 0
, NO

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; End of data for load case [Peso soletta] -----

\*USE-STLD, Finiture

\*BEAMLOAD ; Element Beam Loads

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101, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
102, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
103, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
104, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
105, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
106, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
107, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
108, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
109, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
110, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
111, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
112, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
113, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
114, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
115, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
116, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
117, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
118, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
119, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
120, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
121, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI
TURE, NO, 0, 0, NO
122, BEAM , UNILOAD, GZ, NO , YES, 1, LY, -0.57, -0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINI

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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RE, NO, 0, 0, NO
421, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
422, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
423, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
424, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
425, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
426, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
427, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
428, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
429, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
430, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
431, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
432, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
433, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
434, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
435, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
436, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
437, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
438, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
439, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO
440, BEAM , UNILoad, GZ, NO , YES, 1, LY, 0.57, 0.57, NO, 0, -14.31, 1, -14.31, 0, 0, 0, 0, PESO FINITU
RE, NO, 0, 0, NO

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; End of data for load case [Finiture] -----

\*USE-STLD, Vento strutture (Y+)

\*BEAMLOAD ; Element Beam Loads

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101, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
102, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
103, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
104, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
105, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
106, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
107, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
108, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
109, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N
O
110, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.51, YES, 0, 3.03, 1, 3.71, 0, 0, 0, 0, , NO, 0, 0,
NO
111, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.51, -0.76, YES, 0, 3.71, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0,
NO
112, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.76, NO, 0, 4.38, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0, N
O
113, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.76, NO, 0, 4.38, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0, N
O
114, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.51, YES, 0, 4.38, 1, 3.71, 0, 0, 0, 0, , NO, 0, 0,
NO
115, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.51, -0.25, YES, 0, 3.71, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0,
NO
116, BEAM , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N

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PROJECT TITLE : CASALPUSTERLENGO

| MIDAS | Company |      |                                                                                                |  |  |  |  |  |  |  |  |  | Client    |                 |
|-------|---------|------|------------------------------------------------------------------------------------------------|--|--|--|--|--|--|--|--|--|-----------|-----------------|
|       | Author  |      | Renato Vaira                                                                                   |  |  |  |  |  |  |  |  |  | File Name | Asse 80 V_1.mct |
| O     | 117     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 118     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 119     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 120     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 121     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 122     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 123     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 124     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 125     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| NO    | 126     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.51, YES, 0, 3.03, 1, 3.71, 0, 0, 0, 0, , NO, 0, 0, 0 |  |  |  |  |  |  |  |  |  |           |                 |
| NO    | 127     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.51, -0.76, YES, 0, 3.71, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0, 0 |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 128     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.76, NO, 0, 4.38, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 129     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.76, NO, 0, 4.38, 1, 4.38, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| NO    | 130     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.76, -0.51, YES, 0, 4.38, 1, 3.71, 0, 0, 0, 0, , NO, 0, 0, 0 |  |  |  |  |  |  |  |  |  |           |                 |
| NO    | 131     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.51, -0.25, YES, 0, 3.71, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, 0 |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 132     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 133     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 134     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 135     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 136     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 137     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 138     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 139     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 140     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, -0.25, -0.25, NO, 0, 3.03, 1, 3.03, 0, 0, 0, 0, , NO, 0, 0, N  |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 401     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 402     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 403     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 404     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 405     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 406     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 407     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 408     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 409     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 410     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.2, YES, 0, 2.69, 1, 2.35, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 411     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.2, 0.29, YES, 0, 2.35, 1, 2.01, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 412     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.29, 0.29, NO, 0, 2.01, 1, 2.01, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 413     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.29, 0.29, NO, 0, 2.01, 1, 2.01, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 414     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.29, 0.2, YES, 0, 2.01, 1, 2.35, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 415     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.2, 0.11, YES, 0, 2.35, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 416     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 417     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 418     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 419     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 420     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 421     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 422     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 423     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 424     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 425     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 426     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.11, 0.2, YES, 0, 2.69, 1, 2.35, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 427     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.2, 0.29, YES, 0, 2.35, 1, 2.01, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 428     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.29, 0.29, NO, 0, 2.01, 1, 2.01, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |
| O     | 429     | BEAM | , UNILoad, GY, NO , YES, 1, LZ, 0.29, 0.29, NO, 0, 2.01, 1, 2.01, 0, 0, 0, 0, , NO, 0, 0, NO   |  |  |  |  |  |  |  |  |  |           |                 |

PROJECT TITLE : CASALPUSTERLENGO

| MIDAS      |                                                                                              | Company |              |  |  |  |  |  |  |  |  |  | Client    |                 |  |  |  |  |  |  |  |  |  |
|------------|----------------------------------------------------------------------------------------------|---------|--------------|--|--|--|--|--|--|--|--|--|-----------|-----------------|--|--|--|--|--|--|--|--|--|
|            |                                                                                              | Author  | Renato Vaira |  |  |  |  |  |  |  |  |  | File Name | Asse 80 V_1.mct |  |  |  |  |  |  |  |  |  |
| 430, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.29, 0.2, YES, 0, 2.01, 1, 2.35, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 431, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.2, 0.11, YES, 0, 2.35, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 432, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 433, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 434, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 435, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 436, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 437, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 438, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 439, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 440, BEAM  | , UNILoad, GY, NO, YES, 1, LZ, 0.11, 0.11, NO, 0, 2.69, 1, 2.69, 0, 0, 0, 0, , NO, 0, 0, NO  |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4001, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4002, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4003, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 7.93016e-016, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, N |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4004, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 1.98254e-015, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, N |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4005, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4006, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4007, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 7.93016e-016, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, N |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4008, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4009, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 3.96508e-016, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, N |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4010, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4011, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4012, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 1.08845e-015, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, N |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4013, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4014, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 8.70763e-016, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, N |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4015, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4016, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4017, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 8.70763e-016, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, N |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4018, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4019, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |
| 4020, BEAM | , UNILoad, GY, NO, NO, aDir[1], , , , 0, 1.74, 1, 1.74, 0, 0, 0, 0, , NO, 0, 0, NO           |         |              |  |  |  |  |  |  |  |  |  |           |                 |  |  |  |  |  |  |  |  |  |

; End of data for load case [Vento strutture (Y+)] -----

\*USE-STLD, Vento mobili (Y+)

\*BEAMLOAD ; Element Beam Loads

|           |                                                                                       |
|-----------|---------------------------------------------------------------------------------------|
| 101, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 102, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 103, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 104, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 105, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 106, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 107, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 108, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 109, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 110, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 111, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 112, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 113, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 114, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 115, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 116, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 117, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 118, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 119, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 120, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 121, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 122, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 123, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 124, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 125, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 126, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 127, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 128, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 129, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 130, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 131, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 132, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 133, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |
| 134, BEAM | , UNILoad, GY, NO, YES, 1, LZ, 2, 2, NO, 0, 2.32, 1, 2.32, 0, 0, 0, 0, , NO, 0, 0, NO |







PROJECT TITLE : CASALPUSTERLENGO

| MIDAS | Company |              |           | Client          |
|-------|---------|--------------|-----------|-----------------|
|       | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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425, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
426, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
427, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
428, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
429, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
430, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
431, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
432, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
433, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
434, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
435, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
436, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
437, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
438, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
439, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
440, BEAM , UNILoad, GX, NO , YES, 1, LZ, 0, 0, NO, 0, 1.56, 1, 1.56, 0, 0, 0, 0, , NO, 0, 0, NO
    
```

; End of data for load case [Frenamento (X+)] -----

\*USE-STLD, Termico Unif. +15°

\*ELTEMPER ; Element Temperatures

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101, 15,
102, 15,
103, 15,
104, 15,
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109, 15,
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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |


- 217, 15,
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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

- 1354, 15,
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- 1358, 15,
- 1359, 15,
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- 1362, 15,
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- 1456, 15,
- 1458, 15,

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

- 1459, 15,
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- 2002, 15,
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- 2012, 15,
- 2013, 15,
- 2014, 15,
- 2017, 15,
- 2018, 15,
- 2019, 15,
- 2020, 15,
- 2021, 15,
- 2022, 15,
- 2025, 15,
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- 2041, 15,
- 2042, 15,
- 2043, 15,
- 2045, 15,

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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2047, 15,  
3001, 15,  
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3003, 15,  
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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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 3203, 15,  
 3204, 15,

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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3301, 15,  
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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

- 3309, 15,
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- 3364, 15,
- 3365, 15,
- 3366, 15,

; End of data for load case [Termico Unif. +15°] -----

\*USE-STLD, Termico diff. +5°/H

\*THERGRAD ; Temperature Gradient

- 101, 1, 5, YES, 0, 0, YES, 0,
- 102, 1, 5, YES, 0, 0, YES, 0,
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- 127, 1, 5, YES, 0, 0, YES, 0,
- 128, 1, 5, YES, 0, 0, YES, 0,

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

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 326, 1, 5, YES, 0, 0, YES, 0,



PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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327, 1, 5, YES, 0, 0, YES, 0,
328, 1, 5, YES, 0, 0, YES, 0,
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439, 1, 5, YES, 0, 0, YES, 0,
440, 1, 5, YES, 0, 0, YES, 0,

```

; End of data for load case [Termico diff. +5°/H] -----

\*STAGE ; Define Construction Stage

```

NAME=Costruzione, 30, YES, NO
STEP=3, 10
AELEM=TRAVI PRINCIPALI, 1, TRASVERSI INTERMEDI, 1, TRASVERSI PILE, 1, PILE, 1, NODI, 1
      CONTROVENTI, 1
ABNDR=RIGID LINK, DEFORMED, APPOGGI SPALLE, DEFORMED, SUPPORTI, DEFORMED
      APPOGGI PILE, DEFORMED
ALOAD=PESO SOLETTA, LAST, PESO ACCIAIO E PILE, FIRST
NAME=Esercizio, 10000, YES, NO
STEP=3, 10, 32, 100, 316, 1000, 3162
AELEM=SOLETTA FITTIZIA, 1
ALOAD=PESO FINITURE, FIRST

```

\*SFUNCTION ; Spectrum Function

```

FUNC=SLD Orizz, 1, 0, 1, 9.806, 0.05, , 1.000000
USER
      0.000000,      0.0694,      0.140770,      0.17635
      0.422320,      0.17635,      0.487220,      0.15287
      0.552110,      0.1349,      0.617000,      0.12071
      0.681890,      0.10922,      0.746790,      0.09973
      0.811680,      0.09176,      0.876570,      0.08497

```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

```

0.941460,      0.07911,      1.006350,      0.07401
1.071250,      0.06953,      1.136140,      0.06555
1.201030,      0.06201,      1.213600,      0.06137
1.214600,      0.05007,      1.265920,      0.04804
1.330820,      0.0457,       1.395710,      0.04357
1.460600,      0.04163,      1.525490,      0.03986
1.590380,      0.03824,      1.655280,      0.03674
1.720170,      0.03535,      1.785060,      0.03407
1.890530,      0.03037,      1.996010,      0.02725
2.101480,      0.02458,      2.206950,      0.02229
2.312430,      0.0203,       2.417900,      0.01857
2.523370,      0.01705,      2.628850,      0.01571
2.734320,      0.01452,      2.839790,      0.01346
2.945270,      0.01251,      3.050740,      0.01166
3.156210,      0.0109,       3.261690,      0.0102
3.367160,      0.00957,      3.472630,      0.009
3.578110,      0.00848,      3.683580,      0.008
3.789050,      0.00756,      3.894530,      0.00716
4.000000,      0.00678

```

FUNC=SLD Vert, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000,      0.01343,      0.050000,      0.03414
0.150000,      0.03414,      0.235000,      0.02179
0.320000,      0.016,       0.405000,      0.01264
0.490000,      0.01045,      0.575000,      0.00891
0.660000,      0.00776,      0.745000,      0.00687
0.830000,      0.00617,      0.915000,      0.0056
1.000000,      0.00512,      1.094000,      0.00428
1.188000,      0.00363,      1.281000,      0.00312
1.375000,      0.00271,      1.469000,      0.00237
1.563000,      0.0021,      1.656000,      0.00187
1.750000,      0.00167,      1.844000,      0.00151
1.938000,      0.00136,      2.031000,      0.00124
2.125000,      0.00113,      2.219000,      0.00104
2.313000,      0.00096,      2.406000,      0.00088
2.500000,      0.00082,      2.594000,      0.00076
2.688000,      0.00071,      2.781000,      0.00066
2.875000,      0.00062,      2.969000,      0.00058
3.063000,      0.00055,      3.156000,      0.00051
3.250000,      0.00048,      3.344000,      0.00046
3.438000,      0.00043,      3.531000,      0.00041
3.625000,      0.00039,      3.719000,      0.00037
3.813000,      0.00035,      3.906000,      0.00034
4.000000,      0.00032

```

FUNC=SLV Orizz, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000,      0.1516,       0.155370,      0.38525
0.466100,      0.38525,      0.539350,      0.33293
0.612600,      0.29312,      0.685840,      0.26182
0.759090,      0.23655,      0.832330,      0.21574
0.905580,      0.19829,      0.978820,      0.18345
1.052070,      0.17068,      1.125320,      0.15957
1.198560,      0.14982,      1.213600,      0.14796
1.214600,      0.12071,      1.271810,      0.11528
1.345050,      0.109,        1.418300,      0.10337
1.491540,      0.0983,       1.564790,      0.0937
1.638040,      0.08951,      1.711280,      0.08567
1.784530,      0.08216,      1.857770,      0.07892
1.931020,      0.07593,      2.004260,      0.07315
2.099300,      0.06668,      2.194330,      0.06103
2.289370,      0.05607,      2.384400,      0.05169
2.479440,      0.0478,       2.574470,      0.04434
2.669510,      0.04124,      2.764540,      0.03845
2.859580,      0.03594,      2.954610,      0.03366
3.049650,      0.0316,       3.144680,      0.02972
3.239720,      0.028,        3.334750,      0.02642
3.429790,      0.02498,      3.524820,      0.02365
3.619860,      0.02243,      3.714890,      0.02129
3.809930,      0.02024,      3.904960,      0.01927
4.000000,      0.01837

```

FUNC=SLV Vert, 1, 0, 1, 9.806, 0.05, , 1.000000

USER

```

0.000000,      0.04338,      0.050000,      0.11023
0.150000,      0.11023,      0.235000,      0.07036
0.320000,      0.05167,      0.405000,      0.04082
0.490000,      0.03374,      0.575000,      0.02875
0.660000,      0.02505,      0.745000,      0.02219
0.830000,      0.01992,      0.915000,      0.01807

```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

```

1.000000, 0.01653, 1.094000, 0.01382
1.188000, 0.01172, 1.281000, 0.01007
1.375000, 0.00875, 1.469000, 0.00766
1.563000, 0.00677, 1.656000, 0.00603
1.750000, 0.0054, 1.844000, 0.00486
1.938000, 0.0044, 2.031000, 0.00401
2.125000, 0.00366, 2.219000, 0.00336
2.313000, 0.00309, 2.406000, 0.00286
2.500000, 0.00265, 2.594000, 0.00246
2.688000, 0.00229, 2.781000, 0.00214
2.875000, 0.002, 2.969000, 0.00188
3.063000, 0.00176, 3.156000, 0.00166
3.250000, 0.00157, 3.344000, 0.00148
3.438000, 0.0014, 3.531000, 0.00133
3.625000, 0.00126, 3.719000, 0.0012
3.813000, 0.00114, 3.906000, 0.00108
4.000000, 0.00103

```

FUNC=SLC Orizz, 1, 0, 1, 9.806, 0.05, , 1.000000  
 USER

```

0.000000, 0.19241, 0.158030, 0.48639
0.474100, 0.48639, 0.552150, 0.41764
0.630200, 0.36592, 0.708240, 0.32559
0.786290, 0.29327, 0.864340, 0.26679
0.942390, 0.2447, 1.020440, 0.22598
1.098480, 0.20992, 1.176530, 0.196
1.213600, 0.19001, 1.214600, 0.15502
1.254580, 0.15008, 1.332630, 0.14129
1.410670, 0.13347, 1.488720, 0.12647
1.566770, 0.12017, 1.644820, 0.11447
1.722870, 0.10928, 1.800910, 0.10455
1.878960, 0.10021, 1.957010, 0.09621
2.035060, 0.09252, 2.113100, 0.0891
2.202960, 0.08198, 2.292810, 0.07568
2.382660, 0.07008, 2.472510, 0.06508
2.562360, 0.0606, 2.652220, 0.05656
2.742070, 0.05291, 2.831920, 0.04961
2.921770, 0.04661, 3.011630, 0.04387
3.101480, 0.04136, 3.191330, 0.03907
3.281180, 0.03695, 3.371030, 0.03501
3.460890, 0.03322, 3.550740, 0.03156
3.640590, 0.03002, 3.730440, 0.02859
3.820300, 0.02726, 3.910150, 0.02602
4.000000, 0.02487

```

FUNC=SLC Vert, 1, 0, 1, 9.806, 0.05, , 1.000000  
 USER

```

0.000000, 0.06202, 0.050000, 0.15678
0.150000, 0.15678, 0.235000, 0.10007
0.320000, 0.07349, 0.405000, 0.05807
0.490000, 0.04799, 0.575000, 0.0409
0.660000, 0.03563, 0.745000, 0.03157
0.830000, 0.02833, 0.915000, 0.0257
1.000000, 0.02352, 1.094000, 0.01966
1.188000, 0.01668, 1.281000, 0.01433
1.375000, 0.01244, 1.469000, 0.0109
1.563000, 0.00963, 1.656000, 0.00857
1.750000, 0.00768, 1.844000, 0.00692
1.938000, 0.00626, 2.031000, 0.0057
2.125000, 0.00521, 2.219000, 0.00478
2.313000, 0.0044, 2.406000, 0.00406
2.500000, 0.00376, 2.594000, 0.0035
2.688000, 0.00326, 2.781000, 0.00304
2.875000, 0.00285, 2.969000, 0.00267
3.063000, 0.00251, 3.156000, 0.00236
3.250000, 0.00223, 3.344000, 0.0021
3.438000, 0.00199, 3.531000, 0.00189
3.625000, 0.00179, 3.719000, 0.0017
3.813000, 0.00162, 3.906000, 0.00154
4.000000, 0.00147

```

```

*SPLDCASE ; Spectrum Load Cases
NAME=SLD Long (X+), XY, 0, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLD Orizz
NAME=SLD Trasv (Y+), XY, 90, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLD Orizz
NAME=SLD Vert (Z+), Z, 0, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO

```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 v_1.mct |

```

SLD Vert
NAME=SLV Long (X+), XY, 0, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLV Orizz
NAME=SLV Trasv (Y+), XY, 90, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLV Orizz
NAME=SLV Vert (Z+), Z, 0, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLV Vert
NAME=SLC Long (X+), XY, 0, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLC Orizz
NAME=SLC Trasv (Y+), XY, 90, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLC Orizz
NAME=SLC Vert (Z+), Z, 0, 1, 1, NO, NO, LOG,
CQC, YES, 0, NO
SLC Vert
    
```

\*MVLDCODE ; Moving Load Code  
CODE=EUROCODE

\*LINELANE ; Traffic Line Lanes

```

NAME=C. LENTA DX, LANE, , 0, 0, BOTH, 3, 2
201, 0.15, 0, NO, 0, 202, 0.15, 0, NO, 0, 203, 0.15, 0, NO, 0
204, 0.15, 0, NO, 0, 205, 0.15, 0, NO, 0, 206, 0.15, 0, NO, 0
207, 0.15, 0, NO, 0, 208, 0.15, 0, NO, 0, 209, 0.15, 0, NO, 0
210, 0.15, 0, NO, 0, 211, 0.15, 0, NO, 0, 212, 0.15, 0, NO, 0
213, 0.15, 0, NO, 0, 214, 0.15, 0, NO, 0, 215, 0.15, 0, NO, 0
216, 0.15, 0, NO, 0, 217, 0.15, 0, NO, 0, 218, 0.15, 0, NO, 0
219, 0.15, 0, NO, 0, 220, 0.15, 0, NO, 0, 221, 0.15, 0, NO, 0
222, 0.15, 0, NO, 0, 223, 0.15, 0, NO, 0, 224, 0.15, 0, NO, 0
225, 0.15, 0, NO, 0, 226, 0.15, 0, NO, 0, 227, 0.15, 0, NO, 0
228, 0.15, 0, NO, 0, 229, 0.15, 0, NO, 0, 230, 0.15, 0, NO, 0
231, 0.15, 0, NO, 0, 232, 0.15, 0, NO, 0, 233, 0.15, 0, NO, 0
234, 0.15, 0, NO, 0, 235, 0.15, 0, NO, 0, 236, 0.15, 0, NO, 0
237, 0.15, 0, NO, 0, 238, 0.15, 0, NO, 0, 239, 0.15, 0, NO, 0
240, 0.15, 0, NO, 0
NAME=C. LENTA SX, LANE, , 0, 0, BOTH, 3, 2
301, -0.15, 0, NO, 0, 302, -0.15, 0, NO, 0, 303, -0.15, 0, NO, 0
304, -0.15, 0, NO, 0, 305, -0.15, 0, NO, 0, 306, -0.15, 0, NO, 0
307, -0.15, 0, NO, 0, 308, -0.15, 0, NO, 0, 309, -0.15, 0, NO, 0
310, -0.15, 0, NO, 0, 311, -0.15, 0, NO, 0, 312, -0.15, 0, NO, 0
313, -0.15, 0, NO, 0, 314, -0.15, 0, NO, 0, 315, -0.15, 0, NO, 0
316, -0.15, 0, NO, 0, 317, -0.15, 0, NO, 0, 318, -0.15, 0, NO, 0
319, -0.15, 0, NO, 0, 320, -0.15, 0, NO, 0, 321, -0.15, 0, NO, 0
322, -0.15, 0, NO, 0, 323, -0.15, 0, NO, 0, 324, -0.15, 0, NO, 0
325, -0.15, 0, NO, 0, 326, -0.15, 0, NO, 0, 327, -0.15, 0, NO, 0
328, -0.15, 0, NO, 0, 329, -0.15, 0, NO, 0, 330, -0.15, 0, NO, 0
331, -0.15, 0, NO, 0, 332, -0.15, 0, NO, 0, 333, -0.15, 0, NO, 0
334, -0.15, 0, NO, 0, 335, -0.15, 0, NO, 0, 336, -0.15, 0, NO, 0
337, -0.15, 0, NO, 0, 338, -0.15, 0, NO, 0, 339, -0.15, 0, NO, 0
340, -0.15, 0, NO, 0
NAME=DX 1, LANE, , 0, 0, BOTH, 3, 2
101, -1.55, 0, NO, 0, 102, -1.55, 0, NO, 0, 103, -1.55, 0, NO, 0
104, -1.55, 0, NO, 0, 105, -1.55, 0, NO, 0, 106, -1.55, 0, NO, 0
107, -1.55, 0, NO, 0, 108, -1.55, 0, NO, 0, 109, -1.55, 0, NO, 0
110, -1.55, 0, NO, 0, 111, -1.55, 0, NO, 0, 112, -1.55, 0, NO, 0
113, -1.55, 0, NO, 0, 114, -1.55, 0, NO, 0, 115, -1.55, 0, NO, 0
116, -1.55, 0, NO, 0, 117, -1.55, 0, NO, 0, 118, -1.55, 0, NO, 0
119, -1.55, 0, NO, 0, 120, -1.55, 0, NO, 0, 121, -1.55, 0, NO, 0
122, -1.55, 0, NO, 0, 123, -1.55, 0, NO, 0, 124, -1.55, 0, NO, 0
125, -1.55, 0, NO, 0, 126, -1.55, 0, NO, 0, 127, -1.55, 0, NO, 0
128, -1.55, 0, NO, 0, 129, -1.55, 0, NO, 0, 130, -1.55, 0, NO, 0
131, -1.55, 0, NO, 0, 132, -1.55, 0, NO, 0, 133, -1.55, 0, NO, 0
134, -1.55, 0, NO, 0, 135, -1.55, 0, NO, 0, 136, -1.55, 0, NO, 0
137, -1.55, 0, NO, 0, 138, -1.55, 0, NO, 0, 139, -1.55, 0, NO, 0
140, -1.55, 0, NO, 0
NAME=DX 2, LANE, , 0, 0, BOTH, 3, 2
201, -1.35, 0, NO, 0, 202, -1.35, 0, NO, 0, 203, -1.35, 0, NO, 0
204, -1.35, 0, NO, 0, 205, -1.35, 0, NO, 0, 206, -1.35, 0, NO, 0
207, -1.35, 0, NO, 0, 208, -1.35, 0, NO, 0, 209, -1.35, 0, NO, 0
210, -1.35, 0, NO, 0, 211, -1.35, 0, NO, 0, 212, -1.35, 0, NO, 0
213, -1.35, 0, NO, 0, 214, -1.35, 0, NO, 0, 215, -1.35, 0, NO, 0
216, -1.35, 0, NO, 0, 217, -1.35, 0, NO, 0, 218, -1.35, 0, NO, 0
219, -1.35, 0, NO, 0, 220, -1.35, 0, NO, 0, 221, -1.35, 0, NO, 0
222, -1.35, 0, NO, 0, 223, -1.35, 0, NO, 0, 224, -1.35, 0, NO, 0
    
```

PROJECT TITLE : CASALPUSTERLENGO

| MIDAS | Company |              | Client    |                 |
|-------|---------|--------------|-----------|-----------------|
|       | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

```

225, -1.35, 0, NO, 0, 226, -1.35, 0, NO, 0, 227, -1.35, 0, NO, 0
228, -1.35, 0, NO, 0, 229, -1.35, 0, NO, 0, 230, -1.35, 0, NO, 0
231, -1.35, 0, NO, 0, 232, -1.35, 0, NO, 0, 233, -1.35, 0, NO, 0
234, -1.35, 0, NO, 0, 235, -1.35, 0, NO, 0, 236, -1.35, 0, NO, 0
237, -1.35, 0, NO, 0, 238, -1.35, 0, NO, 0, 239, -1.35, 0, NO, 0
240, -1.35, 0, NO, 0
NAME=DX 3, LANE, , 0, 0, BOTH, 3, 2
301, -1.15, 0, NO, 0, 302, -1.15, 0, NO, 0, 303, -1.15, 0, NO, 0
304, -1.15, 0, NO, 0, 305, -1.15, 0, NO, 0, 306, -1.15, 0, NO, 0
307, -1.15, 0, NO, 0, 308, -1.15, 0, NO, 0, 309, -1.15, 0, NO, 0
310, -1.15, 0, NO, 0, 311, -1.15, 0, NO, 0, 312, -1.15, 0, NO, 0
313, -1.15, 0, NO, 0, 314, -1.15, 0, NO, 0, 315, -1.15, 0, NO, 0
316, -1.15, 0, NO, 0, 317, -1.15, 0, NO, 0, 318, -1.15, 0, NO, 0
319, -1.15, 0, NO, 0, 320, -1.15, 0, NO, 0, 321, -1.15, 0, NO, 0
322, -1.15, 0, NO, 0, 323, -1.15, 0, NO, 0, 324, -1.15, 0, NO, 0
325, -1.15, 0, NO, 0, 326, -1.15, 0, NO, 0, 327, -1.15, 0, NO, 0
328, -1.15, 0, NO, 0, 329, -1.15, 0, NO, 0, 330, -1.15, 0, NO, 0
331, -1.15, 0, NO, 0, 332, -1.15, 0, NO, 0, 333, -1.15, 0, NO, 0
334, -1.15, 0, NO, 0, 335, -1.15, 0, NO, 0, 336, -1.15, 0, NO, 0
337, -1.15, 0, NO, 0, 338, -1.15, 0, NO, 0, 339, -1.15, 0, NO, 0
340, -1.15, 0, NO, 0
NAME=DX RIM, LANE, , 0, 0, BOTH, 0.5, 0
401, 0.3, 0, NO, 0, 402, 0.3, 0, NO, 0, 403, 0.3, 0, NO, 0
404, 0.3, 0, NO, 0, 405, 0.3, 0, NO, 0, 406, 0.3, 0, NO, 0
407, 0.3, 0, NO, 0, 408, 0.3, 0, NO, 0, 409, 0.3, 0, NO, 0
410, 0.3, 0, NO, 0, 411, 0.3, 0, NO, 0, 412, 0.3, 0, NO, 0
413, 0.3, 0, NO, 0, 414, 0.3, 0, NO, 0, 415, 0.3, 0, NO, 0
416, 0.3, 0, NO, 0, 417, 0.3, 0, NO, 0, 418, 0.3, 0, NO, 0
419, 0.3, 0, NO, 0, 420, 0.3, 0, NO, 0, 421, 0.3, 0, NO, 0
422, 0.3, 0, NO, 0, 423, 0.3, 0, NO, 0, 424, 0.3, 0, NO, 0
425, 0.3, 0, NO, 0, 426, 0.3, 0, NO, 0, 427, 0.3, 0, NO, 0
428, 0.3, 0, NO, 0, 429, 0.3, 0, NO, 0, 430, 0.3, 0, NO, 0
431, 0.3, 0, NO, 0, 432, 0.3, 0, NO, 0, 433, 0.3, 0, NO, 0
434, 0.3, 0, NO, 0, 435, 0.3, 0, NO, 0, 436, 0.3, 0, NO, 0
437, 0.3, 0, NO, 0, 438, 0.3, 0, NO, 0, 439, 0.3, 0, NO, 0
440, 0.3, 0, NO, 0
NAME=MARC DX, LANE, , 0, 0, BOTH, 1.2, 0
101, 1.05, 0, NO, 0, 102, 1.05, 0, NO, 0, 103, 1.05, 0, NO, 0
104, 1.05, 0, NO, 0, 105, 1.05, 0, NO, 0, 106, 1.05, 0, NO, 0
107, 1.05, 0, NO, 0, 108, 1.05, 0, NO, 0, 109, 1.05, 0, NO, 0
110, 1.05, 0, NO, 0, 111, 1.05, 0, NO, 0, 112, 1.05, 0, NO, 0
113, 1.05, 0, NO, 0, 114, 1.05, 0, NO, 0, 115, 1.05, 0, NO, 0
116, 1.05, 0, NO, 0, 117, 1.05, 0, NO, 0, 118, 1.05, 0, NO, 0
119, 1.05, 0, NO, 0, 120, 1.05, 0, NO, 0, 121, 1.05, 0, NO, 0
122, 1.05, 0, NO, 0, 123, 1.05, 0, NO, 0, 124, 1.05, 0, NO, 0
125, 1.05, 0, NO, 0, 126, 1.05, 0, NO, 0, 127, 1.05, 0, NO, 0
128, 1.05, 0, NO, 0, 129, 1.05, 0, NO, 0, 130, 1.05, 0, NO, 0
131, 1.05, 0, NO, 0, 132, 1.05, 0, NO, 0, 133, 1.05, 0, NO, 0
134, 1.05, 0, NO, 0, 135, 1.05, 0, NO, 0, 136, 1.05, 0, NO, 0
137, 1.05, 0, NO, 0, 138, 1.05, 0, NO, 0, 139, 1.05, 0, NO, 0
140, 1.05, 0, NO, 0
NAME=MARC SX, LANE, , 0, 0, BOTH, 1.2, 0
401, -1.05, 0, NO, 0, 402, -1.05, 0, NO, 0, 403, -1.05, 0, NO, 0
404, -1.05, 0, NO, 0, 405, -1.05, 0, NO, 0, 406, -1.05, 0, NO, 0
407, -1.05, 0, NO, 0, 408, -1.05, 0, NO, 0, 409, -1.05, 0, NO, 0
410, -1.05, 0, NO, 0, 411, -1.05, 0, NO, 0, 412, -1.05, 0, NO, 0
413, -1.05, 0, NO, 0, 414, -1.05, 0, NO, 0, 415, -1.05, 0, NO, 0
416, -1.05, 0, NO, 0, 417, -1.05, 0, NO, 0, 418, -1.05, 0, NO, 0
419, -1.05, 0, NO, 0, 420, -1.05, 0, NO, 0, 421, -1.05, 0, NO, 0
422, -1.05, 0, NO, 0, 423, -1.05, 0, NO, 0, 424, -1.05, 0, NO, 0
425, -1.05, 0, NO, 0, 426, -1.05, 0, NO, 0, 427, -1.05, 0, NO, 0
428, -1.05, 0, NO, 0, 429, -1.05, 0, NO, 0, 430, -1.05, 0, NO, 0
431, -1.05, 0, NO, 0, 432, -1.05, 0, NO, 0, 433, -1.05, 0, NO, 0
434, -1.05, 0, NO, 0, 435, -1.05, 0, NO, 0, 436, -1.05, 0, NO, 0
437, -1.05, 0, NO, 0, 438, -1.05, 0, NO, 0, 439, -1.05, 0, NO, 0
440, -1.05, 0, NO, 0
NAME=SX 1, LANE, , 0, 0, BOTH, 3, 2
401, 1.55, 0, NO, 0, 402, 1.55, 0, NO, 0, 403, 1.55, 0, NO, 0
404, 1.55, 0, NO, 0, 405, 1.55, 0, NO, 0, 406, 1.55, 0, NO, 0
407, 1.55, 0, NO, 0, 408, 1.55, 0, NO, 0, 409, 1.55, 0, NO, 0
410, 1.55, 0, NO, 0, 411, 1.55, 0, NO, 0, 412, 1.55, 0, NO, 0
413, 1.55, 0, NO, 0, 414, 1.55, 0, NO, 0, 415, 1.55, 0, NO, 0
416, 1.55, 0, NO, 0, 417, 1.55, 0, NO, 0, 418, 1.55, 0, NO, 0
419, 1.55, 0, NO, 0, 420, 1.55, 0, NO, 0, 421, 1.55, 0, NO, 0
422, 1.55, 0, NO, 0, 423, 1.55, 0, NO, 0, 424, 1.55, 0, NO, 0
425, 1.55, 0, NO, 0, 426, 1.55, 0, NO, 0, 427, 1.55, 0, NO, 0
428, 1.55, 0, NO, 0, 429, 1.55, 0, NO, 0, 430, 1.55, 0, NO, 0
431, 1.55, 0, NO, 0, 432, 1.55, 0, NO, 0, 433, 1.55, 0, NO, 0

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PROJECT TITLE : CASALPUSTERLENGO

| MIDAS | Company |              | Client    |                 |
|-------|---------|--------------|-----------|-----------------|
|       | Author  | Renato Vaira | File Name | Asse 80 v_1.mct |

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434, 1.55, 0, NO, 0, 435, 1.55, 0, NO, 0, 436, 1.55, 0, NO, 0
437, 1.55, 0, NO, 0, 438, 1.55, 0, NO, 0, 439, 1.55, 0, NO, 0
440, 1.55, 0, NO, 0
NAME=SX 2, LANE, , 0, 0, BOTH, 3, 2
301, 1.35, 0, NO, 0, 302, 1.35, 0, NO, 0, 303, 1.35, 0, NO, 0
304, 1.35, 0, NO, 0, 305, 1.35, 0, NO, 0, 306, 1.35, 0, NO, 0
307, 1.35, 0, NO, 0, 308, 1.35, 0, NO, 0, 309, 1.35, 0, NO, 0
310, 1.35, 0, NO, 0, 311, 1.35, 0, NO, 0, 312, 1.35, 0, NO, 0
313, 1.35, 0, NO, 0, 314, 1.35, 0, NO, 0, 315, 1.35, 0, NO, 0
316, 1.35, 0, NO, 0, 317, 1.35, 0, NO, 0, 318, 1.35, 0, NO, 0
319, 1.35, 0, NO, 0, 320, 1.35, 0, NO, 0, 321, 1.35, 0, NO, 0
322, 1.35, 0, NO, 0, 323, 1.35, 0, NO, 0, 324, 1.35, 0, NO, 0
325, 1.35, 0, NO, 0, 326, 1.35, 0, NO, 0, 327, 1.35, 0, NO, 0
328, 1.35, 0, NO, 0, 329, 1.35, 0, NO, 0, 330, 1.35, 0, NO, 0
331, 1.35, 0, NO, 0, 332, 1.35, 0, NO, 0, 333, 1.35, 0, NO, 0
334, 1.35, 0, NO, 0, 335, 1.35, 0, NO, 0, 336, 1.35, 0, NO, 0
337, 1.35, 0, NO, 0, 338, 1.35, 0, NO, 0, 339, 1.35, 0, NO, 0
340, 1.35, 0, NO, 0
NAME=SX 3, LANE, , 0, 0, BOTH, 3, 2
201, 1.15, 0, NO, 0, 202, 1.15, 0, NO, 0, 203, 1.15, 0, NO, 0
204, 1.15, 0, NO, 0, 205, 1.15, 0, NO, 0, 206, 1.15, 0, NO, 0
207, 1.15, 0, NO, 0, 208, 1.15, 0, NO, 0, 209, 1.15, 0, NO, 0
210, 1.15, 0, NO, 0, 211, 1.15, 0, NO, 0, 212, 1.15, 0, NO, 0
213, 1.15, 0, NO, 0, 214, 1.15, 0, NO, 0, 215, 1.15, 0, NO, 0
216, 1.15, 0, NO, 0, 217, 1.15, 0, NO, 0, 218, 1.15, 0, NO, 0
219, 1.15, 0, NO, 0, 220, 1.15, 0, NO, 0, 221, 1.15, 0, NO, 0
222, 1.15, 0, NO, 0, 223, 1.15, 0, NO, 0, 224, 1.15, 0, NO, 0
225, 1.15, 0, NO, 0, 226, 1.15, 0, NO, 0, 227, 1.15, 0, NO, 0
228, 1.15, 0, NO, 0, 229, 1.15, 0, NO, 0, 230, 1.15, 0, NO, 0
231, 1.15, 0, NO, 0, 232, 1.15, 0, NO, 0, 233, 1.15, 0, NO, 0
234, 1.15, 0, NO, 0, 235, 1.15, 0, NO, 0, 236, 1.15, 0, NO, 0
237, 1.15, 0, NO, 0, 238, 1.15, 0, NO, 0, 239, 1.15, 0, NO, 0
240, 1.15, 0, NO, 0
NAME=SX RIM, LANE, , 0, 0, BOTH, 0.5, 0
101, -0.3, 0, NO, 0, 102, -0.3, 0, NO, 0, 103, -0.3, 0, NO, 0
104, -0.3, 0, NO, 0, 105, -0.3, 0, NO, 0, 106, -0.3, 0, NO, 0
107, -0.3, 0, NO, 0, 108, -0.3, 0, NO, 0, 109, -0.3, 0, NO, 0
110, -0.3, 0, NO, 0, 111, -0.3, 0, NO, 0, 112, -0.3, 0, NO, 0
113, -0.3, 0, NO, 0, 114, -0.3, 0, NO, 0, 115, -0.3, 0, NO, 0
116, -0.3, 0, NO, 0, 117, -0.3, 0, NO, 0, 118, -0.3, 0, NO, 0
119, -0.3, 0, NO, 0, 120, -0.3, 0, NO, 0, 121, -0.3, 0, NO, 0
122, -0.3, 0, NO, 0, 123, -0.3, 0, NO, 0, 124, -0.3, 0, NO, 0
125, -0.3, 0, NO, 0, 126, -0.3, 0, NO, 0, 127, -0.3, 0, NO, 0
128, -0.3, 0, NO, 0, 129, -0.3, 0, NO, 0, 130, -0.3, 0, NO, 0
131, -0.3, 0, NO, 0, 132, -0.3, 0, NO, 0, 133, -0.3, 0, NO, 0
134, -0.3, 0, NO, 0, 135, -0.3, 0, NO, 0, 136, -0.3, 0, NO, 0
137, -0.3, 0, NO, 0, 138, -0.3, 0, NO, 0, 139, -0.3, 0, NO, 0
140, -0.3, 0, NO, 0

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\*VEHICLE ; Vehicles

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NAME=Fatica 2-A, 1, 3, Fatigue Load Model 2 (280), , , 1, , ,
NAME=Fatica 2-B, 1, 3, Fatigue Load Model 2 (360), , , 1, , ,
NAME=Fatica 2-C, 1, 3, Fatigue Load Model 2 (630), , , 1, , ,
NAME=Fatica 2-D, 1, 3, Fatigue Load Model 2 (560), , , 1, , ,
NAME=Fatica 2-E, 1, 3, Fatigue Load Model 2 (610), , , 1, , ,
NAME=Fatica 3 (1 veicolo), 1, 3, Fatigue Load Model 3 (One Vehicle), , , 1, , ,
NAME=Schema 1, 1, 1, Load Model 1, 0.75, 0.4, , 1, 1, 1, 1, 1, 1
NAME=Schema 5, 1, 2, Uniform load (Road bridge footway), , , 0.4, 2.5,

```

\*MVLDCASE (EURO) ; Moving Load Cases

```

NAME=Mobili SLU DX, 1, , Schema 1, Schema 5, YES
1, DX 1, DX 2, DX 3
2, DX RIM
3, MARC DX, MARC SX
NAME=Mobili SLU SX, 1, , Schema 1, Schema 5, YES
1, SX 1, SX 2, SX 3
2, SX RIM
3, MARC DX, MARC SX
NAME=Mobili SLE DX, 1, , Schema 1, Schema 5, NO
1, DX 1, DX 2, DX 3
2, DX RIM
3, MARC DX, MARC SX
NAME=Mobili SLE SX, 1, , Schema 1, Schema 5, NO
1, SX 1, SX 2, SX 3
2, SX RIM
3, MARC DX, MARC SX
NAME=Fatica V.ILL., 2, , 1, YES
Fatica 2-A, 1, 0, 1, C. LENTA DX, C. LENTA SX

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
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Fatica 2-B, 1, 0, 1, C. LENTA DX, C.LENTA SX  
 Fatica 2-C, 1, 0, 1, C. LENTA DX, C.LENTA SX  
 Fatica 2-D, 1, 0, 1, C. LENTA DX, C.LENTA SX  
 Fatica 2-E, 1, 0, 1, C. LENTA DX, C.LENTA SX  
 NAME=Fatica DANN.ACC., 2, , 1, YES  
 Fatica 3 (1 veicolo), 1, 0, 1, C. LENTA DX, C.LENTA SX

\*CPOSECT4CS ; Composite Section for Construction Stage

SEC=5, Costruzione, NORMAL, YES  
 1, MATL, 1, , 0, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=11, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=1, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=2, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=7, Costruzione, NORMAL, NO  
 1, MATL, 1, , 0, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=16, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=3, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=4, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=6, Costruzione, NORMAL, YES  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=8, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=9, Costruzione, NORMAL, YES  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=10, Costruzione, NORMAL, YES  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=12, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=13, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=14, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3  
 SEC=21, Costruzione, NORMAL, NO  
 1, MATL, 1, , 1, 1, 1, 1, 1, 1, 1, 0  
 2, MATL, 4, Esercizio, 1, 1, 1, 1, 1, 1, 1, 0.3

\*STAGE-COLOR ; Diagram Color for Construction Stage

Costruzione, 0, 157, 192  
 Esercizio, 255, 255, 255

\*STAGE-CTRL ; Construction Stage Analysis Control Data

YES, , INTERNAL, NO, YES, YES, NO, NO, NO, NO, NO, , , DW, W, DW, Erection Load 1, Erection Load 2, Erection Load 3, YES, NO, , NO, NO  
 NO  
 NO  
 YES, YES, BOTH, 5, 0.01, YES, NO, YES, YES, 0, YES  
 NO, NO, 2, YES, 2, 5, 7, 10, 20  
 NO, , , 1, NO  
 Finiture

\*LOADCOMB ; Combinations

NAME=Vento Strutture (Y-), GEN, INACTIVE, 0, 0, , 0, 0  
 ST, Vento strutture (Y+), -1  
 NAME=Vento Mobili (Y-), GEN, INACTIVE, 0, 0, , 0, 0  
 ST, Vento mobili (Y+), -1  
 NAME=Frenamento (X-), GEN, INACTIVE, 0, 0, , 0, 0  
 ST, Frenamento (X+), -1



PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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NAME=Termico unif. -15°, GEN, INACTIVE, 0, 0, , 0, 0
  ST, Termico Unif. +15°, -1
NAME=Termico diff. -5°/H, GEN, INACTIVE, 0, 0, , 0, 0
  ST, Termico diff. +5°/H, -1
NAME=SLD Long (X-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLD Long (X+), -1
NAME=SLD Trasv (Y-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLD Trasv (Y+), -1
NAME=SLD Vert (Z-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLD Vert (Z+), -1
NAME=SLV Long (X-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLV Long (X+), -1
NAME=SLV Trasv (Y-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLV Trasv (Y+), -1
NAME=SLV Vert (Z-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLV Vert (Z+), -1
NAME=SLC Long (X-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLC Long (X+), -1
NAME=SLC Trasv (Y-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLC Trasv (Y+), -1
NAME=SLC Vert (Z-), GEN, INACTIVE, 0, 0, , 0, 0
  RS, SLC Vert (Z+), -1
NAME=Vento Strutture INV, GEN, INACTIVE, 0, 1, , 0, 0
  ST, Vento strutture (Y+), 1, CB, Vento Strutture (Y-), 1
NAME=Vento Mobili INV, GEN, INACTIVE, 0, 1, , 0, 0
  ST, Vento mobili (Y+), 1, CB, Vento Mobili (Y-), 1
NAME=Frenamento INV, GEN, ACTIVE, 0, 1, , 0, 0
  ST, Frenamento (X+), 1, CB, Frenamento (X-), 1
NAME=Termico unif. INV, GEN, ACTIVE, 0, 1, , 0, 0
  ST, Termico Unif. +15°, 1, CB, Termico unif. -15°, 1
NAME=Termico diff. INV, GEN, ACTIVE, 0, 1, , 0, 0
  ST, Termico diff. +5°/H, 1, CB, Termico diff. -5°/H, 1
NAME=SLD Long INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLD Long (X+), 1, CB, SLD Long (X-), 1
NAME=SLD Trasv INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLD Trasv (Y+), 1, CB, SLD Trasv (Y-), 1
NAME=SLD Vert INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLD Vert (Z+), 1, CB, SLD Vert (Z-), 1
NAME=SLV Long INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLV Long (X+), 1, CB, SLV Long (X-), 1
NAME=SLV Trasv INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLV Trasv (Y+), 1, CB, SLV Trasv (Y-), 1
NAME=SLV Vert INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLV Vert (Z+), 1, CB, SLV Vert (Z-), 1
NAME=SLC Long INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLC Long (X+), 1, CB, SLC Long (X-), 1
NAME=SLC Trasv INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLC Trasv (Y+), 1, CB, SLC Trasv (Y-), 1
NAME=SLC Vert INV, GEN, INACTIVE, 0, 1, , 0, 0
  RS, SLC Vert (Z+), 1, CB, SLC Vert (Z-), 1
NAME=Mobili SLU INV, GEN, ACTIVE, 0, 1, , 0, 0
  MV, Mobili SLU DX, 1, MV, Mobili SLU SX, 1
NAME=Mobili SLE INV, GEN, ACTIVE, 0, 1, , 0, 0
  MV, Mobili SLE DX, 1, MV, Mobili SLE SX, 1
NAME=GEN SLU Mobili, GEN, ACTIVE, 0, 0, , 0, 0
  CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2
  CS, Shrinkage Secondary, 1.2, CB, Vento Strutture INV, 0.9
  CB, Vento Mobili INV, 0.9, CB, Termico unif. INV, 0.72
  CB, Termico diff. INV, 0.72, CB, Mobili SLU INV, 1.35
NAME=GEN SLU Vento, GEN, ACTIVE, 0, 0, , 0, 0
  CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2
  CS, Shrinkage Secondary, 1.2, CB, Vento Strutture INV, 1.5
  CB, Vento Mobili INV, 1.5, CB, Termico unif. INV, 0.72
  CB, Termico diff. INV, 0.72, CB, Mobili SLE INV, 1.35
NAME=GEN SLU Frenamento, GEN, ACTIVE, 0, 0, , 0, 0
  CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2
  CS, Shrinkage Secondary, 1.2, CB, Frenamento INV, 1.35
  CB, Termico unif. INV, 0.72, CB, Termico diff. INV, 0.72
  CB, Mobili SLE INV, 1.35
NAME=GEN SLU Termico, GEN, ACTIVE, 0, 0, , 0, 0
  CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2
  CS, Shrinkage Secondary, 1.2, CB, Vento Strutture INV, 0.9
  CB, Vento Mobili INV, 0.9, CB, Termico unif. INV, 1.2
  CB, Termico diff. INV, 1.2, CB, Mobili SLE INV, 1.35
NAME=GEN SLD Long, GEN, ACTIVE, 0, 0, , 0, 0
  CS, Summation, 1, CB, Termico unif. INV, 0.5
  CB, Termico diff. INV, 0.5, CB, SLD Long INV, 1, CB, SLD Trasv INV, 0.3
  CB, SLD Vert INV, 0.3

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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NAME=GEN SLD Trasv, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLD Long INV, 0.3, CB, SLD Trasv INV, 1
    CB, SLD Vert INV, 0.3
NAME=GEN SLD Vert, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLD Long INV, 0.3
    CB, SLD Trasv INV, 0.3, CB, SLD Vert INV, 1
NAME=GEN SLV Long, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLV Long INV, 1, CB, SLV Trasv INV, 0.3
    CB, SLV Vert INV, 0.3
NAME=GEN SLV Trasv, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLV Long INV, 0.3, CB, SLV Trasv INV, 1
    CB, SLV Vert INV, 0.3
NAME=GEN SLV Vert, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLV Long INV, 0.3
    CB, SLV Trasv INV, 0.3, CB, SLV Vert INV, 1
NAME=GEN SLC Long, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLC Long INV, 1, CB, SLC Trasv INV, 0.3
    CB, SLC Vert INV, 0.3
NAME=GEN SLC Trasv, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLC Long INV, 0.3, CB, SLC Trasv INV, 1
    CB, SLC Vert INV, 0.3
NAME=GEN SLC Vert, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, SLC Long INV, 0.3
    CB, SLC Trasv INV, 0.3, CB, SLC Vert INV, 1
NAME=GEN RARA Mobili, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Vento Strutture INV, 0.6
    CB, Vento Mobili INV, 0.6, CB, Termico unif. INV, 0.6
    CB, Termico diff. INV, 0.6, CB, Mobili SLU INV, 1
NAME=GEN RARA Vento, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Vento Strutture INV, 1, CB, Vento Mobili INV, 1
    CB, Termico unif. INV, 0.6, CB, Termico diff. INV, 0.6
    CB, Mobili SLE INV, 1
NAME=GEN RARA Frenam, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Frenamento INV, 1, CB, Termico unif. INV, 0.6
    CB, Termico diff. INV, 0.6, CB, Mobili SLE INV, 1
NAME=GEN RARA Termico, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Vento Strutture INV, 0.6
    CB, Vento Mobili INV, 0.6, CB, Termico unif. INV, 1
    CB, Termico diff. INV, 1, CB, Mobili SLE INV, 1
NAME=GEN FREQ Mobili, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5, CB, Mobili SLE INV, 1
NAME=GEN FREQ Vento, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Vento Strutture INV, 0.2
    CB, Vento Mobili INV, 0.2, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5
NAME=GEN FREQ Termico, GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.6
    CB, Termico diff. INV, 0.6
NAME=GEN Q.P., GEN, ACTIVE, 0, 0, , 0, 0
    CS, Summation, 1, CB, Termico unif. INV, 0.5
    CB, Termico diff. INV, 0.5
NAME=GEN SLU INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, GEN SLU Mobili, 1, CB, GEN SLU Vento, 1, CB, GEN SLU Frenamento, 1
    CB, GEN SLU Termico, 1
NAME=GEN SLD INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, GEN SLD Long, 1, CB, GEN SLD Trasv, 1, CB, GEN SLD Vert, 1
NAME=GEN SLV INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, GEN SLV Long, 1, CB, GEN SLV Trasv, 1, CB, GEN SLV Vert, 1
NAME=GEN SLC INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, GEN SLC Long, 1, CB, GEN SLC Trasv, 1, CB, GEN SLC Vert, 1
NAME=GEN RARA INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, GEN RARA Mobili, 1, CB, GEN RARA Vento, 1, CB, GEN RARA Frenam, 1
    CB, GEN RARA Termico, 1
NAME=GEN FREQ INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, GEN FREQ Mobili, 1, CB, GEN FREQ Vento, 1, CB, GEN FREQ Termico, 1
NAME=VENTO TOT INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, Vento Strutture INV, 1, CB, Vento Mobili INV, 1
NAME=GEN SLU/SLV INV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, GEN SLU Mobili, 1, CB, GEN SLU Vento, 1, CB, GEN SLU Frenamento, 1
    
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PROJECT TITLE : CASALPUSTERLENGO

| MIDAS | Company      | Client          |
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|       | Author       | File Name       |
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CB, GEN SLU Termico, 1, CB, GEN SLV Long, 1, CB, GEN SLV Trasv, 1  
 CB, GEN SLV Vert, 1  
 NAME=Vento Strutture (Y-), STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Vento strutture (Y+), -1  
 NAME=Vento Mobili (Y-), STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Vento mobili (Y+), -1  
 NAME=Frenamento (X-), STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Frenamento (X+), -1  
 NAME=Termico unif. -15°, STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Termico Unif. +15°, -1  
 NAME=Termico diff. -5°/H, STEEL, INACTIVE, 0, 0, , 0, 0  
 ST, Termico diff. +5°/H, -1  
 NAME=SLV Long (X-), STEEL, INACTIVE, 0, 0, , 0, 0  
 RS, SLV Long (X+), -1  
 NAME=SLV Trasv (Y-), STEEL, INACTIVE, 0, 0, , 0, 0  
 RS, SLV Trasv (Y+), -1  
 NAME=SLV Vert (Z-), STEEL, INACTIVE, 0, 0, , 0, 0  
 RS, SLV Vert (Z+), -1  
 NAME=Vento Strutture INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Vento strutture (Y+), 1, CBS, Vento Strutture (Y-), 1  
 NAME=Vento Mobili INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Vento mobili (Y+), 1, CBS, Vento Mobili (Y-), 1  
 NAME=Frenamento INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Frenamento (X+), 1, CBS, Frenamento (X-), 1  
 NAME=Termico unif. INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Termico Unif. +15°, 1, CBS, Termico unif. -15°, 1  
 NAME=Termico diff. INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 ST, Termico diff. +5°/H, 1, CBS, Termico diff. -5°/H, 1  
 NAME=SLV Long INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 RS, SLV Long (X+), 1, CBS, SLV Long (X-), 1  
 NAME=SLV Trasv INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 RS, SLV Trasv (Y+), 1, CBS, SLV Trasv (Y-), 1  
 NAME=SLV Vert INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 RS, SLV Vert (Z+), 1, CBS, SLV Vert (Z-), 1  
 NAME=Mobili SLU INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 MV, Mobili SLU DX, 1, MV, Mobili SLU SX, 1  
 NAME=Mobili SLE INV, STEEL, INACTIVE, 0, 1, , 0, 0  
 MV, Mobili SLE DX, 1, MV, Mobili SLE SX, 1  
 NAME=ST SLU Mobili, STEEL, STRENGTH, 0, 0, , 0, 0  
 CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
 CS, Shrinkage Secondary, 1.2, CBS, Vento Strutture INV, 0.9  
 CBS, Vento Mobili INV, 0.9, CBS, Termico unif. INV, 0.72  
 CBS, Termico diff. INV, 0.72, CBS, Mobili SLU INV, 1.35  
 NAME=ST SLU Vento, STEEL, STRENGTH, 0, 0, , 0, 0  
 CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
 CS, Shrinkage Secondary, 1.2, CBS, Vento Strutture INV, 1.5  
 CBS, Vento Mobili INV, 1.5, CBS, Termico unif. INV, 0.72  
 CBS, Termico diff. INV, 0.72, CBS, Mobili SLE INV, 1.35  
 NAME=ST SLU Frenamento, STEEL, STRENGTH, 0, 0, , 0, 0  
 CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
 CS, Shrinkage Secondary, 1.2, CBS, Frenamento INV, 1.35  
 CBS, Termico unif. INV, 0.72, CBS, Termico diff. INV, 0.72  
 CBS, Mobili SLE INV, 1.35  
 NAME=ST SLU Termico, STEEL, STRENGTH, 0, 0, , 0, 0  
 CS, Dead Load, 1.35, CS, Erection Load 1, 1.5, CS, Creep Secondary, 1.2  
 CS, Shrinkage Secondary, 1.2, CBS, Vento Strutture INV, 0.9  
 CBS, Vento Mobili INV, 0.9, CBS, Termico unif. INV, 1.2  
 CBS, Termico diff. INV, 1.2, CBS, Mobili SLE INV, 1.35  
 NAME=ST SLV Long, STEEL, STRENGTH, 0, 0, , 0, 0  
 CS, Summation, 1, CBS, Termico unif. INV, 0.5  
 CBS, Termico diff. INV, 0.5, CBS, SLV Long INV, 1  
 CBS, SLV Trasv INV, 0.3, CBS, SLV Vert INV, 0.3  
 NAME=ST SLV Trasv, STEEL, STRENGTH, 0, 0, , 0, 0  
 CS, Summation, 1, CBS, Termico unif. INV, 0.5  
 CBS, Termico diff. INV, 0.5, CBS, SLV Long INV, 0.3  
 CBS, SLV Trasv INV, 1, CBS, SLV Vert INV, 0.3  
 NAME=ST SLV Vert, STEEL, STRENGTH, 0, 0, , 0, 0  
 CS, Summation, 1, CBS, Termico unif. INV, 0.5  
 CBS, Termico diff. INV, 0.5, CBS, SLV Long INV, 0.3  
 CBS, SLV Trasv INV, 0.3, CBS, SLV Vert INV, 1  
 NAME=ST FATICA VITA ILL., STEEL, INACTIVE, 0, 0, , 0, 0  
 MV, Fatica V.ILL., 1, CS, Summation, 1  
 NAME=ST FATICA DANN.ACC., STEEL, INACTIVE, 0, 0, , 0, 0  
 MV, Fatica DANN.ACC., 1, CS, Summation, 1  
 NAME=ST RARA Mobili, STEEL, SERVICE, 0, 0, , 3, 0  
 CS, Summation, 1, CBS, Vento Strutture INV, 0.6  
 CBS, Vento Mobili INV, 0.6, CBS, Termico unif. INV, 0.6  
 CBS, Termico diff. INV, 0.6, CBS, Mobili SLU INV, 1

PROJECT TITLE : CASALPUSTERLENGO

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|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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NAME=ST RARA Vento, STEEL, SERVICE, 0, 0, , 3, 0
  CS, Summation, 1, CBS, Vento Strutture INV, 1, CBS, Vento Mobili INV, 1
  CBS, Termico unif. INV, 0.6, CBS, Termico diff. INV, 0.6
  CBS, Mobili SLE INV, 1
NAME=ST RARA Frenamento, STEEL, SERVICE, 0, 0, , 3, 0
  CS, Summation, 1, CBS, Frenamento INV, 1, CBS, Termico unif. INV, 0.6
  CBS, Termico diff. INV, 0.6, CBS, Mobili SLE INV, 1
NAME=ST RARA Termico, STEEL, SERVICE, 0, 0, , 3, 0
  CS, Summation, 1, CBS, Vento Strutture INV, 0.6
  CBS, Vento Mobili INV, 0.6, CBS, Termico unif. INV, 1
  CBS, Termico diff. INV, 1, CBS, Mobili SLE INV, 1
NAME=ST FREQ Mobili, STEEL, SERVICE, 0, 0, , 2, 0
  CS, Summation, 1, CBS, Termico unif. INV, 0.5
  CBS, Termico diff. INV, 0.5, CBS, Mobili SLE INV, 1
NAME=ST FREQ Vento, STEEL, SERVICE, 0, 0, , 2, 0
  CS, Summation, 1, CBS, Vento Strutture INV, 0.2
  CBS, Vento Mobili INV, 0.2, CBS, Termico unif. INV, 0.5
  CBS, Termico diff. INV, 0.5
NAME=ST FREQ Termico, STEEL, SERVICE, 0, 0, , 2, 0
  CS, Summation, 1, CBS, Termico unif. INV, 0.6
  CBS, Termico diff. INV, 0.6
NAME=ST Q.P., STEEL, SERVICE, 0, 0, , 1, 0
  CS, Summation, 1, CBS, Termico unif. INV, 0.5
  CBS, Termico diff. INV, 0.5
NAME=Mobili SLU INV, CONC, INACTIVE, 0, 1, , 0, 0
  MV, Mobili SLU DX, 1, MV, Mobili SLU SX, 1
NAME=CONTROMONTE, CONC, STRENGTH, 0, 0, , 0, 0
  CS, Summation, -1, CBC, Mobili SLU INV, -0.25
    
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*LC-COLOR ; Diagram Color for Load Case
ST, Peso acciaio e pile, 0, 192, 128, 255, 0, 192, 192, 192, 0
ST, Peso soletta, 0, 128, 255, 255, 255, 87, 192, 0, 128
ST, Finiture, 255, 128, 0, 192, 192, 192, 192, 192, 192
ST, Vento strutture (Y+), 255, 0, 128, 0, 128, 57, 192, 0, 192
ST, Vento mobili (Y+), 0, 192, 128, 255, 0, 192, 212, 160, 255
ST, Frenamento (X+), 255, 128, 0, 85, 0, 192, 160, 255, 255
ST, Termico Unif. +15°, 192, 0, 192, 0, 128, 57, 255, 87, 128
MV, Mobili SLU DX, 85, 192, 0, 163, 255, 160, 192, 192, 0
CS, Dead Load, 192, 128, 0, 255, 255, 255, 255, 160, 255
CS, Tendon Secondary, 163, 160, 255, 148, 87, 255, 255, 0, 192
CS, Creep Primary, 192, 0, 192, 163, 160, 255, 255, 0, 192
CS, Creep Secondary, 78, 0, 255, 255, 0, 192, 160, 192, 255
CS, Shrinkage Primary, 0, 192, 192, 192, 0, 192, 160, 192, 255
CS, Shrinkage Secondary, 0, 192, 192, 255, 255, 255, 210, 210, 210
CS, Summation, 0, 192, 128, 160, 255, 255, 192, 192, 0
RS, SLD Long (X+), 0, 128, 57, 255, 0, 192, 255, 87, 128
RS, SLD Trasv (Y+), 255, 0, 192, 192, 192, 192, 255, 255, 255
RS, SLD Vert (Z+), 192, 0, 128, 255, 192, 87, 0, 128, 57
RS, SLV Long (X+), 85, 0, 192, 192, 192, 0, 255, 192, 87
RS, SLV Trasv (Y+), 192, 72, 0, 192, 192, 0, 255, 0, 192
RS, SLV Vert (Z+), 192, 192, 192, 255, 192, 87, 210, 210, 210
RS, SLC Long (X+), 0, 128, 128, 85, 0, 192, 192, 192, 0
RS, SLC Trasv (Y+), 192, 0, 128, 255, 0, 192, 255, 0, 128
RS, SLC Vert (Z+), 255, 0, 128, 0, 157, 192, 192, 72, 0
ST, Termico diff. +5°/H, 0, 128, 192, 78, 0, 255, 0, 192, 128
MV, Mobili SLU SX, 160, 255, 255, 192, 72, 0, 192, 72, 0
MV, Mobili SLE DX, 78, 0, 255, 160, 255, 255, 255, 87, 87
CB, Vento Strutture (Y-), 255, 87, 87, 0, 192, 128, 0, 157, 192
CB, Vento Mobili (Y-), 192, 0, 192, 146, 0, 255, 192, 0, 128
MV, Mobili SLE SX, 85, 192, 0, 163, 160, 255, 0, 192, 192
CB, Frenamento (X-), 78, 0, 255, 85, 0, 192, 85, 192, 0
CB, Termico unif. -15°, 255, 255, 255, 128, 192, 0, 128, 192, 0
CB, Termico diff. -5°/H, 163, 160, 255, 255, 0, 192, 255, 0, 192
CB, SLD Long (X-), 210, 210, 210, 0, 128, 255, 0, 128, 128
CB, SLD Trasv (Y-), 255, 255, 87, 78, 0, 255, 85, 0, 192
CB, SLD Vert (Z-), 192, 0, 128, 128, 192, 0, 255, 0, 128
CB, SLV Long (X-), 0, 128, 192, 192, 0, 128, 0, 128, 192
CB, SLV Trasv (Y-), 148, 87, 255, 255, 0, 128, 192, 72, 0
CB, SLV Vert (Z-), 192, 72, 0, 0, 128, 192, 192, 0, 128
CB, SLC Long (X-), 212, 160, 255, 212, 160, 255, 85, 0, 192
CB, SLC Trasv (Y-), 255, 87, 87, 192, 128, 0, 0, 192, 128
CB, SLC Vert (Z-), 0, 157, 192, 146, 0, 255, 255, 128, 0
CB, Vento Strutture INV, 0, 192, 128, 255, 0, 128, 255, 255, 87
CB, Vento Mobili INV, 255, 87, 128, 192, 0, 128, 128, 192, 0
CB, Frenamento INV, 0, 128, 192, 192, 0, 192, 212, 160, 255
CB, Termico unif. INV, 255, 87, 128, 78, 0, 255, 192, 72, 0
CB, Termico diff. INV, 85, 0, 192, 255, 255, 255, 0, 128, 128
CB, SLD Long INV, 210, 210, 210, 255, 87, 87, 192, 192, 192
    
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PROJECT TITLE : CASALPUSTERLENGO

| MIDAS | Company |              | Client    |                 |
|-------|---------|--------------|-----------|-----------------|
|       | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

CB, SLD Trasv INV, 93, 255, 87, 0, 192, 128, 0, 128, 192  
 CB, SLD Vert INV, 192, 72, 0, 163, 160, 255, 192, 0, 192  
 CB, SLV Long INV, 192, 192, 0, 255, 128, 0, 255, 0, 192  
 CB, SLV Trasv INV, 0, 128, 192, 210, 210, 210, 93, 255, 87  
 CB, SLV Vert INV, 128, 192, 0, 255, 255, 255, 255, 192, 87  
 CB, SLC Long INV, 0, 128, 128, 255, 192, 160, 212, 160, 255  
 CB, SLC Trasv INV, 255, 192, 160, 255, 87, 87, 0, 192, 192  
 MV, Fatica V.ILL., 255, 87, 87, 255, 87, 87, 255, 255, 87  
 MV, Fatica DANN.ACC., 0, 157, 192, 148, 87, 255, 85, 0, 192  
 CB, SLC Vert INV, 163, 255, 160, 255, 87, 128, 255, 87, 87  
 CB, Mobili SLU INV, 160, 192, 255, 146, 0, 255, 255, 160, 255  
 CB, Mobili SLE INV, 255, 87, 128, 192, 0, 192, 93, 255, 87  
 CB, GEN SLU Mobili, 210, 210, 210, 255, 128, 0, 163, 255, 160  
 CB, GEN SLU Vento, 160, 255, 255, 128, 192, 0, 192, 0, 192  
 CB, GEN SLU Frenamento, 255, 128, 0, 0, 192, 128, 255, 192, 87  
 CB, GEN SLU Termico, 192, 192, 0, 0, 128, 128, 85, 0, 192  
 CB, GEN SLD Long, 0, 128, 192, 148, 87, 255, 85, 0, 192  
 CB, GEN SLD Trasv, 192, 0, 192, 93, 255, 87, 255, 160, 255  
 CB, GEN SLD Vert, 163, 255, 160, 78, 0, 255, 85, 192, 0  
 CB, GEN SLV Long, 255, 255, 87, 192, 192, 192, 192, 72, 0  
 CB, GEN SLV Trasv, 255, 0, 192, 212, 160, 255, 85, 0, 192  
 CB, GEN SLV Vert, 210, 210, 210, 160, 255, 255, 85, 192, 0  
 CB, GEN SLC Long, 0, 128, 128, 160, 255, 255, 78, 0, 255  
 CB, GEN SLC Trasv, 0, 192, 128, 255, 0, 128, 163, 255, 160  
 CB, GEN SLC Vert, 93, 255, 87, 85, 0, 192, 78, 0, 255  
 CB, GEN RARA Mobili, 255, 87, 128, 0, 192, 192, 255, 255, 255  
 CB, GEN RARA Vento, 212, 160, 255, 255, 192, 160, 255, 87, 87  
 CB, GEN RARA Frenam, 255, 87, 87, 255, 0, 128, 163, 160, 255  
 CB, GEN RARA Termico, 146, 0, 255, 255, 192, 160, 192, 128, 0  
 CB, GEN FREQ Mobili, 210, 210, 210, 93, 255, 87, 212, 160, 255  
 CB, GEN FREQ Vento, 85, 192, 0, 78, 0, 255, 93, 255, 87  
 CB, GEN FREQ Termico, 160, 255, 255, 85, 192, 0, 146, 0, 255  
 CB, GEN Q.P., 192, 0, 128, 255, 255, 255, 163, 255, 160  
 CBS, Vento Strutture (Y-), 255, 255, 255, 192, 192, 192, 192, 192, 192  
 CBS, Vento Mobili (Y-), 163, 255, 160, 255, 87, 128, 255, 0, 128  
 CBS, Frenamento (X-), 255, 0, 128, 0, 192, 192, 85, 192, 0  
 CBS, Termico unif. -15°, 192, 192, 0, 163, 255, 160, 0, 128, 128  
 CBS, Termico diff. -5°/H, 192, 128, 0, 255, 0, 192, 255, 87, 87  
 CBS, SLV Long (X-), 85, 192, 0, 255, 87, 87, 192, 192, 0  
 CBS, SLV Trasv (Y-), 85, 192, 0, 146, 0, 255, 0, 128, 128  
 CBS, SLV Vert (Z-), 255, 192, 87, 255, 87, 128, 0, 192, 128  
 CBS, Vento Strutture INV, 146, 0, 255, 78, 0, 255, 255, 160, 255  
 CBS, Vento Mobili INV, 192, 128, 0, 192, 72, 0, 255, 255, 87  
 CBS, Frenamento INV, 255, 87, 87, 0, 192, 128, 163, 160, 255  
 CBS, Termico unif. INV, 192, 72, 0, 212, 160, 255, 0, 128, 192  
 CBS, Termico diff. INV, 255, 87, 128, 210, 210, 210, 160, 192, 255  
 CBS, SLV Long INV, 192, 0, 128, 255, 255, 87, 85, 0, 192  
 CBS, SLV Trasv INV, 0, 192, 128, 255, 0, 192, 163, 255, 160  
 CBS, SLV Vert INV, 0, 192, 128, 255, 255, 87, 128, 192, 0  
 CBS, Mobili SLU INV, 255, 255, 255, 0, 128, 255, 163, 160, 255  
 CBS, Mobili SLE INV, 255, 255, 87, 0, 128, 255, 163, 160, 255  
 CBS, ST SLU Mobili, 78, 0, 255, 255, 255, 255, 255, 192, 87  
 CBS, ST SLU Vento, 192, 0, 192, 212, 160, 255, 255, 255, 255  
 CBS, ST SLU Frenamento, 192, 192, 192, 0, 128, 57, 255, 160, 255  
 CBS, ST SLU Termico, 160, 192, 255, 192, 128, 0, 146, 0, 255  
 CBS, ST SLV Long, 255, 128, 0, 210, 210, 210, 192, 128, 0  
 CBS, ST SLV Trasv, 255, 0, 128, 255, 192, 87, 160, 192, 255  
 CBS, ST SLV Vert, 0, 192, 128, 0, 128, 192, 0, 128, 128  
 CBS, ST FATICA VITA ILL., 0, 192, 128, 192, 192, 192, 0, 128, 57  
 CBS, ST FATICA DANN.ACC., 210, 210, 210, 192, 192, 192, 192, 192, 0  
 CBS, ST RARA Mobili, 255, 0, 128, 146, 0, 255, 255, 0, 192  
 CBS, ST RARA Vento, 78, 0, 255, 160, 192, 255, 255, 192, 160  
 CBC, Mobili SLU INV, 128, 192, 0, 192, 72, 0, 0, 128, 128  
 CB, GEN SLU INV, 255, 255, 255, 163, 160, 255, 255, 255, 255  
 CB, GEN SLD INV, 255, 255, 255, 255, 255, 87, 0, 192, 192  
 CB, GEN SLV INV, 0, 192, 192, 192, 0, 192, 0, 128, 255  
 CB, GEN SLC INV, 160, 192, 255, 210, 210, 210, 210, 210, 210  
 CB, GEN RARA INV, 0, 128, 57, 0, 157, 192, 210, 210, 210  
 CB, GEN FREQ INV, 0, 192, 128, 128, 192, 0, 255, 87, 87  
 CBS, ST RARA Frenamento, 78, 0, 255, 0, 157, 192, 192, 0, 128  
 CBS, ST RARA Termico, 160, 192, 255, 93, 255, 87, 85, 0, 192  
 CBS, ST FREQ Mobili, 163, 255, 160, 192, 192, 0, 0, 128, 128  
 CBS, ST FREQ Vento, 192, 192, 192, 85, 192, 0, 0, 128, 128  
 CBS, ST FREQ Termico, 192, 0, 128, 255, 87, 128, 146, 0, 255  
 CBS, ST Q.P., 255, 255, 87, 146, 0, 255, 160, 192, 255  
 CB, VENTO TOT INV, 128, 192, 0, 0, 128, 128, 85, 0, 192  
 CB, GEN SLU/SLV INV, 163, 160, 255, 255, 255, 255, 0, 192, 128  
 CBC, CONTROMONTE, 160, 192, 255, 93, 255, 87, 255, 160, 255



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|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
SECT=7, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
SECT=8, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P16, 16, 0, 0, 0, 0.05, 0.2, 2
I, P16, 16, 0, 0, 0, 0.22, 0.2, 2
J, P16, 16, 0, 0, 0, 0.05, 0.2, 2
J, P16, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=9, YES, NO, YES
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
SECT=10, YES, NO, YES
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
SECT=11, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 16, 0, 0, 0, 0.05, 0.2, 2
I, P20, 16, 0, 0, 0, 0.22, 0.2, 2
J, P20, 16, 0, 0, 0, 0.05, 0.2, 2
J, P20, 16, 0, 0, 0, 0.22, 0.2, 2
SECT=12, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=13, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P16, 15, 0, 0, 0, 0.05, 0.2, 2
I, P16, 15, 0, 0, 0, 0.22, 0.2, 2
J, P16, 15, 0, 0, 0, 0.05, 0.2, 2
J, P16, 15, 0, 0, 0, 0.22, 0.2, 2
SECT=14, YES, NO, NO
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 15, 0, 0, 0, 0.22, 0.2, 2
I, P20, 15, 0, 0, 0, 0.05, 0.2, 2
J, P20, 15, 0, 0, 0, 0.22, 0.2, 2
J, P20, 15, 0, 0, 0, 0.05, 0.2, 2
SECT=16, YES, NO, YES
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 32, 0, 0, 0, 0.05, 0.1, 2
I, P20, 32, 0, 0, 0, 0.22, 0.1, 2
J, P20, 32, 0, 0, 0, 0.05, 0.1, 2
J, P20, 32, 0, 0, 0, 0.22, 0.1, 2
SECT=21, YES, NO, YES
NO, , , NO, , , , NO, , , NO, , , NO, ,
I, P20, 29, 0, 0, 0, 0.22, 0.1, 2
I, P20, 29, 0, 0, 0, 0.05, 0.1, 2
J, P20, 29, 0, 0, 0, 0.22, 0.1, 2
J, P20, 29, 0, 0, 0, 0.05, 0.1, 2

```

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*DGN-TRANS_STIFFENER ; Transverse Stiffener of Element
ELEM=101, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=102, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=103, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=104, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=105, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=106, YES
I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0

```



PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

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ELEM=107, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=108, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=109, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=110, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=111, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=112, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=113, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=114, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=115, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=116, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=117, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=118, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=119, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=120, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=121, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=122, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=123, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=124, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=125, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=126, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=127, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=128, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=129, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=130, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=131, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=132, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=133, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=134, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=135, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=136, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=137, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=138, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=139, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=140, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=201, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=202, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=203, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=204, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=205, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

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ELEM=206, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=207, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=208, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=209, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=210, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=211, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=212, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=213, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=214, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=215, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=216, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=217, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=218, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=219, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=220, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=221, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=222, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=223, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=224, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=225, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=226, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=227, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=228, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=229, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=230, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=231, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=232, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=233, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=234, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=235, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=236, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=237, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=238, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=239, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=240, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=301, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=302, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=303, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=304, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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ELEM=305, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=306, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=307, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=308, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=309, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=310, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=311, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=312, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=313, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=314, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=315, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=316, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=317, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=318, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=319, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=320, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=321, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=322, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=323, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=324, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5.2, 0, 0
ELEM=325, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=326, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=327, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=328, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=329, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=330, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=331, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 3, 0, 0
ELEM=332, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=333, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=334, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=335, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=336, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=337, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=338, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=339, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=340, YES
  I, YES, NO, NO, 1, 0.2, 0.015, 5, 0, 0
ELEM=401, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=402, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=403, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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ELEM=404, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=405, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=406, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=407, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=408, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=409, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=410, YES
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ELEM=411, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=412, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=413, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=414, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=415, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=416, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=417, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=418, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=419, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=420, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=421, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=422, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=423, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=424, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5.2, 0, 0
ELEM=425, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=426, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=427, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=428, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=429, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=430, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=431, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 3, 0, 0
ELEM=432, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=433, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=434, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=435, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=436, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=437, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=438, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=439, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
ELEM=440, YES
  I, YES, NO, NO, 0, 0.2, 0.015, 5, 0, 0
    
```

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*DGN-SHEAR_CONNECTOR ; Shear Connector of Element
ELEM=101, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

```

ELEM=102, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=103, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=104, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=105, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=106, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=107, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=108, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=109, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=110, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=111, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=112, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=113, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=114, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=115, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=116, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=117, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=118, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=119, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=120, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=121, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=122, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=123, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=124, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=125, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=126, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=127, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=128, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=129, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=130, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=131, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=132, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=133, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=134, YES
  I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2
ELEM=135, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=136, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=137, YES
  I, YES, 3, 450000, 0.0222, 0.175, 0.15, 0.2
ELEM=138, YES
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ELEM=139, YES
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```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

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ELEM=201, YES
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```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 v_1.mct |

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```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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ELEM=339, YES
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```



PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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 ELEM=440, YES  
 I, YES, 4, 450000, 0.0222, 0.175, 0.1, 0.2

\*DGN-POSITION ; Design Position of Element  
 101to140 201to240 301to340 401to440, 2

\*DGN-DESIGN OUTPUT ; Composite Position for Design Output  
 104, 1

\*DGN-PLATEGIRDER ; Composite Plate Girder Design  
 1901, YES, YES, YES, YES, YES, NO, YES, YES, \  
 1.5, 1.15, 1.05, 1.1, 1.25, 1, 1.15, 1, 50, 1, 0.6, 0.45, 0.8, 0.6, 0.95, 0.8, 0.9, 1, 1, 0.85, 1, 1.25,  
 1.25, 1.25, 1.25, 1.25, 1, 0, YES, YES, YES, YES, YES, YES, YES, YES, 0, NO, YES, NO, NO

\*DGN-PLATE-LT-BUCKLING ; Composite Girder Lateral-Torsional Buckling

ELEM=101, YES  
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 ELEM=102, YES  
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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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ELEM=131, YES
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ELEM=229, YES
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=230, YES  
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 ELEM=319, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=320, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=321, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=322, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=323, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=324, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=325, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=326, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=327, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=328, YES

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=329, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=330, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=331, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=332, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=333, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=334, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=335, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=336, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=337, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=338, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=339, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=340, YES  
 I, YES, 0.1, 0.01, 3, 3.2  
 ELEM=401, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=402, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=403, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=404, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=405, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=406, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=407, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=408, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=409, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=410, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=411, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=412, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=413, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=414, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=415, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=416, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=417, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=418, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=419, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=420, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=421, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=422, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=423, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=424, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=425, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=426, YES  
 I, YES, 0.1, 0.01, 2, 3.2  
 ELEM=427, YES

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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I, YES, 0.1, 0.01, 2, 3.2
ELEM=428, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=429, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=430, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=431, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=432, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=433, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=434, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=435, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=436, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=437, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=438, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=439, YES
I, YES, 0.1, 0.01, 2, 3.2
ELEM=440, YES
I, YES, 0.1, 0.01, 2, 3.2
    
```

\*DGN-PLATE-FTG ; Composite Girder Fatigue Resistance

```

ELEM=101, YES
I, YES, 1.1, 1
ELEM=102, YES
I, YES, 1.1, 1
ELEM=103, YES
I, YES, 1.1, 1
ELEM=104, YES
I, YES, 1.1, 1
ELEM=105, YES
I, YES, 1.1, 1
ELEM=106, YES
I, YES, 1.1, 1
ELEM=107, YES
I, YES, 1.1, 1
ELEM=108, YES
I, YES, 1.1, 1
ELEM=109, YES
I, YES, 1.1, 1
ELEM=110, YES
I, YES, 1.1, 1
ELEM=111, YES
I, YES, 1.1, 1
ELEM=112, YES
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ELEM=113, YES
I, YES, 1.1, 1
ELEM=114, YES
I, YES, 1.1, 1
ELEM=115, YES
I, YES, 1.1, 1
ELEM=116, YES
I, YES, 1.1, 1
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I, YES, 1.1, 1
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I, YES, 1.1, 1
ELEM=120, YES
I, YES, 1.1, 1
ELEM=121, YES
I, YES, 1.1, 1
ELEM=122, YES
I, YES, 1.1, 1
ELEM=123, YES
I, YES, 1.1, 1
ELEM=124, YES
I, YES, 1.1, 1
ELEM=125, YES
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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I, YES, 1.1, 1
ELEM=126, YES
I, YES, 1.1, 1
ELEM=127, YES
I, YES, 1.1, 1
ELEM=128, YES
I, YES, 1.1, 1
ELEM=129, YES
I, YES, 1.1, 1
ELEM=130, YES
I, YES, 1.1, 1
ELEM=131, YES
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I, YES, 1.1, 1
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I, YES, 1.1, 1
ELEM=138, YES
I, YES, 1.1, 1
ELEM=139, YES
I, YES, 1.1, 1
ELEM=140, YES
I, YES, 1.1, 1
ELEM=201, YES
I, YES, 1.1, 1
ELEM=202, YES
I, YES, 1.1, 1
ELEM=203, YES
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I, YES, 1.1, 1
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I, YES, 1.1, 1
ELEM=208, YES
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I, YES, 1.1, 1
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I, YES, 1.1, 1
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I, YES, 1.1, 1
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I, YES, 1.1, 1
ELEM=214, YES
I, YES, 1.1, 1
ELEM=215, YES
I, YES, 1.1, 1
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I, YES, 1.1, 1
ELEM=217, YES
I, YES, 1.1, 1
ELEM=218, YES
I, YES, 1.1, 1
ELEM=219, YES
I, YES, 1.1, 1
ELEM=220, YES
I, YES, 1.1, 1
ELEM=221, YES
I, YES, 1.1, 1
ELEM=222, YES
I, YES, 1.1, 1
ELEM=223, YES
I, YES, 1.1, 1
ELEM=224, YES
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

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I, YES, 1.1, 1
ELEM=225, YES
I, YES, 1.1, 1
ELEM=226, YES
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I, YES, 1.1, 1
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I, YES, 1.1, 1
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I, YES, 1.1, 1
ELEM=230, YES
I, YES, 1.1, 1
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ELEM=232, YES
I, YES, 1.1, 1
ELEM=233, YES
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ELEM=308, YES
I, YES, 1.1, 1
ELEM=309, YES
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ELEM=310, YES
I, YES, 1.1, 1
ELEM=311, YES
I, YES, 1.1, 1
ELEM=312, YES
I, YES, 1.1, 1
ELEM=313, YES
I, YES, 1.1, 1
ELEM=314, YES
I, YES, 1.1, 1
ELEM=315, YES
I, YES, 1.1, 1
ELEM=316, YES
I, YES, 1.1, 1
ELEM=317, YES
I, YES, 1.1, 1
ELEM=318, YES
I, YES, 1.1, 1
ELEM=319, YES
I, YES, 1.1, 1
ELEM=320, YES
I, YES, 1.1, 1
ELEM=321, YES
I, YES, 1.1, 1
ELEM=322, YES
I, YES, 1.1, 1
ELEM=323, YES
    
```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

```


I, YES, 1.1, 1
ELEM=324, YES
I, YES, 1.1, 1
ELEM=325, YES
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ELEM=326, YES
I, YES, 1.1, 1
ELEM=327, YES
I, YES, 1.1, 1
ELEM=328, YES
I, YES, 1.1, 1
ELEM=329, YES
I, YES, 1.1, 1
ELEM=330, YES
I, YES, 1.1, 1
ELEM=331, YES
I, YES, 1.1, 1
ELEM=332, YES
I, YES, 1.1, 1
ELEM=333, YES
I, YES, 1.1, 1
ELEM=334, YES
I, YES, 1.1, 1
ELEM=335, YES
I, YES, 1.1, 1
ELEM=336, YES
I, YES, 1.1, 1
ELEM=337, YES
I, YES, 1.1, 1
ELEM=338, YES
I, YES, 1.1, 1
ELEM=339, YES
I, YES, 1.1, 1
ELEM=340, YES
I, YES, 1.1, 1
ELEM=401, YES
I, YES, 1.1, 1
ELEM=402, YES
I, YES, 1.1, 1
ELEM=403, YES
I, YES, 1.1, 1
ELEM=404, YES
I, YES, 1.1, 1
ELEM=405, YES
I, YES, 1.1, 1
ELEM=406, YES
I, YES, 1.1, 1
ELEM=407, YES
I, YES, 1.1, 1
ELEM=408, YES
I, YES, 1.1, 1
ELEM=409, YES
I, YES, 1.1, 1
ELEM=410, YES
I, YES, 1.1, 1
ELEM=411, YES
I, YES, 1.1, 1
ELEM=412, YES
I, YES, 1.1, 1
ELEM=413, YES
I, YES, 1.1, 1
ELEM=414, YES
I, YES, 1.1, 1
ELEM=415, YES
I, YES, 1.1, 1
ELEM=416, YES
I, YES, 1.1, 1
ELEM=417, YES
I, YES, 1.1, 1
ELEM=418, YES
I, YES, 1.1, 1
ELEM=419, YES
I, YES, 1.1, 1
ELEM=420, YES
I, YES, 1.1, 1
ELEM=421, YES
I, YES, 1.1, 1
ELEM=422, YES
    
```







PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

```

-1.5, 0.0980688, 0, 0, 1.5, 0.0980688, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.268069, 0, 0, 1.5, 0.268069, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.0980688, 0, 0, 1.5, 0.0980688, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=2, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.299742, 0, 6.16954e-007, 1.5, 0.299742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.129742, 0, 6.16954e-007, 1.5, 0.129742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.299742, 0, 6.16954e-007, 1.5, 0.299742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.129742, 0, 6.16954e-007, 1.5, 0.129742, 0
1, 1.61, 0.990523
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=3, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.109898, 0, 6.16954e-007, 1.4, 0.109898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.279898, 0, 6.16954e-007, 1.4, 0.279898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.109898, 0, 6.16954e-007, 1.4, 0.109898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.279898, 0, 6.16954e-007, 1.4, 0.279898, 0
1, 1.61, 0.990523
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=4, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.312978, 0, 6.16954e-007, 1.4, 0.312978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.142978, 0, 6.16954e-007, 1.4, 0.142978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.312978, 0, 6.16954e-007, 1.4, 0.312978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.142978, 0, 6.16954e-007, 1.4, 0.142978, 0
1, 1.61, 0.990523
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=5, YES, YES, YES
2, 2
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.328865, 0, 0, 1.55, 0.328865, 0
1, 0, 0
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.158865, 0, 0, 1.55, 0.158865, 0
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1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0.261942, 1.55, 0.590807, 0
1, 0, 0.261942
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0.261942, 1.55, 0.420807, 0
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NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=6, YES, YES, YES
2, 2
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0

```

PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
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1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
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1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
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NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=7, YES, YES, NO
2, 2
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1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
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1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
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1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
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NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=8, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=9, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, 0, 1.4, 0.172355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, 0, 1.4, 0.342355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0.272916, 1.4, 0.445271, 0
1, 0, 0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0.272916, 1.4, 0.615271, 0
1, 0, 0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=10, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, -0.272916, 1.4, 0.342355, 0
1, 0, -0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, -0.272916, 1.4, 0.172355, 0
1, 0, -0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=11, YES, YES, NO
2, 2

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 v_1.mct |

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1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=12, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=13, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0.05, 0.2
-1.4, 0.305564, 0, -6.65975e-007, 1.4, 0.305564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0.22, 0.2
-1.4, 0.135564, 0, -6.65975e-007, 1.4, 0.135564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0.05, 0.2
-1.4, 0.305564, 0, -6.65975e-007, 1.4, 0.305564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0.22, 0.2
-1.4, 0.135564, 0, -6.65975e-007, 1.4, 0.135564, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=14, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=16, YES, YES, YES
2, 2
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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SECT=21, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
    
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\*SECTION MANAGER-REBAR DESIGN ; Section Manager - Reinforcement Design

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SECT=1, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.268069, 0, 0, 1.5, 0.268069, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.0980688, 0, 0, 1.5, 0.0980688, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.268069, 0, 0, 1.5, 0.268069, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.0980688, 0, 0, 1.5, 0.0980688, 0
1, 0, 0
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=2, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.299742, 0, 6.16954e-007, 1.5, 0.299742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.129742, 0, 6.16954e-007, 1.5, 0.129742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.299742, 0, 6.16954e-007, 1.5, 0.299742, 0
1, 1.61, 0.990523
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.129742, 0, 6.16954e-007, 1.5, 0.129742, 0
1, 1.61, 0.990523
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=3, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.109898, 0, 6.16954e-007, 1.4, 0.109898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.279898, 0, 6.16954e-007, 1.4, 0.279898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.109898, 0, 6.16954e-007, 1.4, 0.109898, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.279898, 0, 6.16954e-007, 1.4, 0.279898, 0
1, 1.61, 0.990523
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=4, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.312978, 0, 6.16954e-007, 1.4, 0.312978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.142978, 0, 6.16954e-007, 1.4, 0.142978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.312978, 0, 6.16954e-007, 1.4, 0.312978, 0
1, 1.61, 0.990523
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
    
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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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-1.4, 0.142978, 0, 6.16954e-007, 1.4, 0.142978, 0
1, 1.61, 0.990523
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=5, YES, YES, YES
2, 2
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.328865, 0, 0, 1.55, 0.328865, 0
1, 0, 0
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.158865, 0, 0, 1.55, 0.158865, 0
1, 0, 0
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0.261942, 1.55, 0.590807, 0
1, 0, 0.261942
1, 1, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0.261942, 1.55, 0.420807, 0
1, 0, 0.261942
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=6, YES, YES, YES
2, 2
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.420807, 0, 0, 1.55, 0.420807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.590807, 0, 0, 1.55, 0.590807, 0
1, 0, 0
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.55, 0.158865, 0, -0.261942, 1.55, 0.158865, 0
1, 0, -0.261942
1, 0, 0, 0, 32, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.55, 0.328865, 0, -0.261942, 1.55, 0.328865, 0
1, 0, -0.261942
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=7, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, 0, 1.5, 0.141273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, 0, 1.5, 0.311273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, 0, 1.5, 0.141273, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, 0, 1.5, 0.311273, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=8, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.292524, 0, 0, 1.5, 0.292524, 0
1, 0, 0
1, 0, 0, 0, 16, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.122524, 0, 0, 1.5, 0.122524, 0
1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, 0, NO, 0, NO
SECT=9, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, 0, 1.4, 0.172355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, 0, 1.4, 0.342355, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0.272916, 1.4, 0.445271, 0
    
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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
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|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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1, 0, 0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0.272916, 1.4, 0.615271, 0
1, 0, 0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=10, YES, YES, YES
2, 2
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.615271, 0, 0, 1.4, 0.615271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.445271, 0, 0, 1.4, 0.445271, 0
1, 0, 0
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.1
-1.4, 0.342355, 0, -0.272916, 1.4, 0.342355, 0
1, 0, -0.272916
1, 0, 0, 0, 29, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.1
-1.4, 0.172355, 0, -0.272916, 1.4, 0.172355, 0
1, 0, -0.272916
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=11, YES, YES, NO
2, 2
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.5, 0.311273, 0, -6.65975e-007, 1.5, 0.311273, 0
1, 1.61, 2.02956
1, 0, 0, 0, 16, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.5, 0.141273, 0, -6.65975e-007, 1.5, 0.141273, 0
1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=12, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.154793, 0, -6.65975e-007, 1.4, 0.154793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
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1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
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1, 1.61, 2.02956
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=13, YES, YES, NO
2, 2
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.305564, 0, -6.65975e-007, 1.4, 0.305564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.22, 0.2
-1.4, 0.135564, 0, -6.65975e-007, 1.4, 0.135564, 0
1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P16, 0, 2, 0, 0, 0, 0, 0.05, 0.2
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NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
SECT=14, YES, YES, NO
2, 2
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1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956

```



PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |         |              |           |                 |
|-----------------------------------------------------------------------------------|---------|--------------|-----------|-----------------|
|  | Company |              | Client    |                 |
|                                                                                   | Author  | Renato Vaira | File Name | Asse 80 V_1.mct |

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1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.22, 0.2
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1, 1.61, 2.02956
1, 0, 0, 0, 15, 0, YES, P20, 0, 2, 0, 0, 0, 0, 0.05, 0.2
-1.4, 0.324793, 0, -6.65975e-007, 1.4, 0.324793, 0
1, 1.61, 2.02956
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NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
  SECT=16, YES, YES, YES
2, 2
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NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
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1, 0, 0
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO
NO, 0, 0, 0, NO, 0, 90, 0, 0, 1, NO, 0, 0, 0, NO, 0, 0, NO, 0, NO

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*SECTION MANAGER-GROUP & PART ; Section Manager - Group & Part
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  SECT=3, YES
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  SECT=4, YES
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  SECT=7, YES
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  SECT=10, YES
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  SECT=11, YES
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  SECT=14, YES
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  SECT=16, YES
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  SECT=21, YES
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*SECTION MANAGER-STIFFENER ; Section Manager - Stiffener
  SECT=1, YES
0, 0

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PROJECT TITLE : CASALPUSTERLENGO

|                                                                                   |                |              |                  |                 |
|-----------------------------------------------------------------------------------|----------------|--------------|------------------|-----------------|
|  | <b>Company</b> |              | <b>Client</b>    |                 |
|                                                                                   | <b>Author</b>  | Renato Vaira | <b>File Name</b> | Asse 80 V_1.mct |

SECT=2, YES  
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SECT=3, YES  
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SECT=4, YES  
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\*ENDDATA