

**ITINERARIO RAGUSA-CATANIA**

Collegamento viario compreso tra lo Svincolo della S.S. 514 "di Chiaramonte"  
con la S.S. 115 e lo Svincolo della S.S. 194 "Ragusana"  
LOTTO 1 - Dallo svincolo n. 1 sulla S.S. 115 (compreso) allo svincolo n. 3 sulla S.P. 5 (escluso)

**PROGETTO ESECUTIVO**

COD. **PA895**

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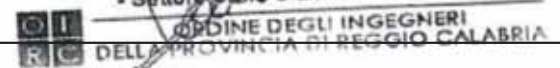
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**CAVALCAVIA  
CAVALCAVIA AL KM 12+325  
Relazione di calcolo spalle**

CODICE PROGETTO

PROGETTO      LIV. PROG.      N. PROG.

**L0408Z**    **E**    **2101**

NOME FILE

T01CV05STRRE01B

CODICE ELAB.

**T01CV05STRRE01**

REVISIONE

**B**

SCALA:

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REV.	DESCRIZIONE	DATA	REDATTO	VERIFICATO	APPROVATO
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C					
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LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

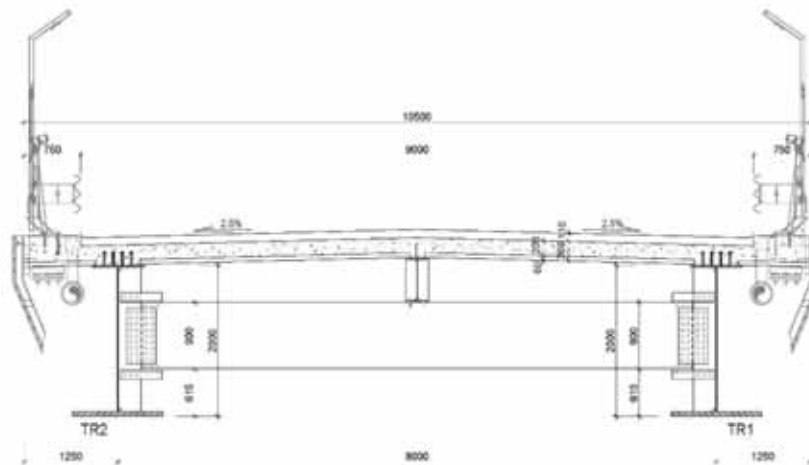
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## 1 DESCRIZIONE DELLE OPERE

La presente relazione di calcolo riguarda l'impalcato posizionato alla progressiva 12+325 del Lotto 1. L'impalcato è realizzato in struttura mista acciaio-calcestruzzo e presenta una larghezza complessiva di 10,50 metri e luce di 42,50 metri, con cordoli laterali da 75cm. La soletta ha uno spessore medio di 25cm. Le travi in acciaio sono poste ad interasse pari a 4,00 metri e presentano un'altezza totale di 2000mm. Le piattabande hanno larghezze e spessori variabili. I traversi sono costituiti da profili a doppio T saldati di altezza pari a 880mm e posti ad interasse 4100mm. L'impalcato viene realizzato a mezzo di 3 conci diversi (Concio C1, C2 e C3) che vengono saldati in opera, collegati agli elementi trasversali e poi il tutto viene sollevato da autogru di grossa portata posizionate in corrispondenza dei rilevati delle spalle. Per dettagli circa le geometrie delle travi fare riferimento agli elaborati specifici, comunque nel proseguo vengono riportate delle immagini rappresentative delle opere in oggetto

### SEZIONE TIPO IN CAMPATA

Scala 1:50



### SEZIONE TIPO AGLI APPOGGI

Scala 1:50

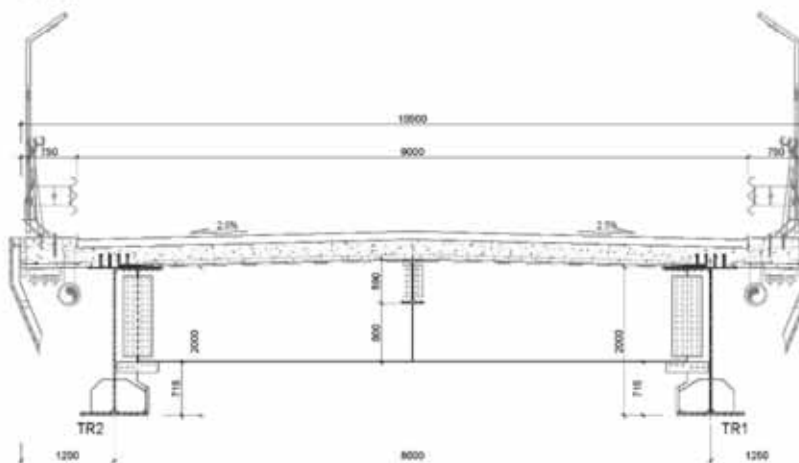


Figura 1 - Sezioni trasversali dell'impalcato

LOTTO 1 - CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

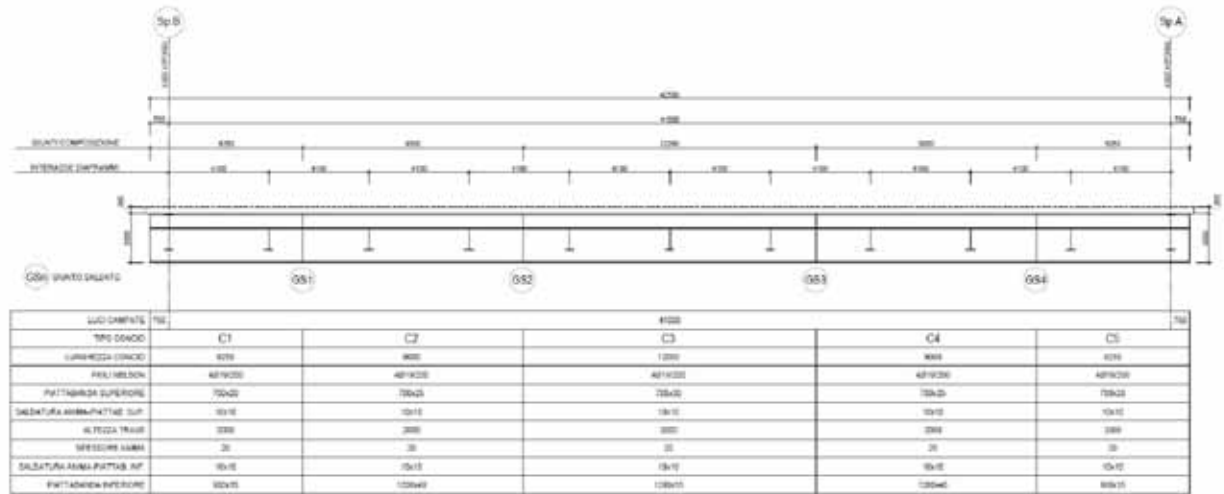


Figura 2 - Schema Conci

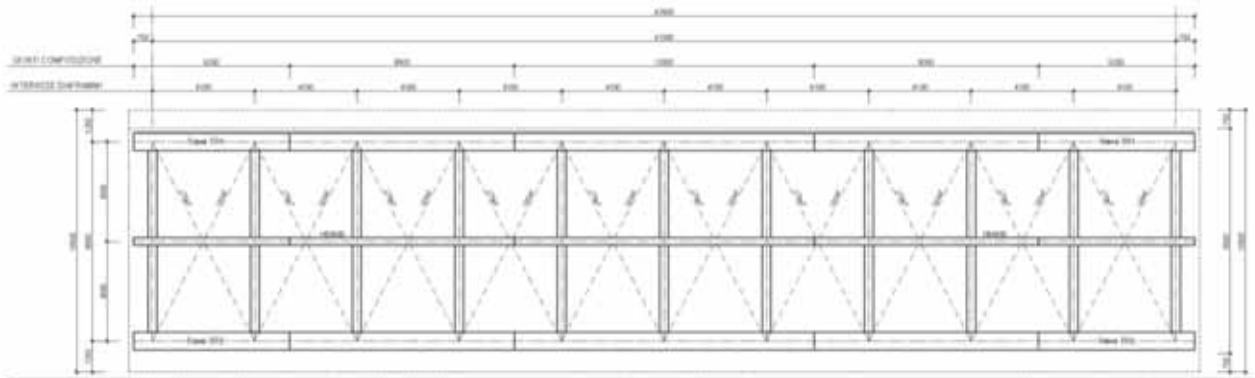


Figura 3 - Pianta conci

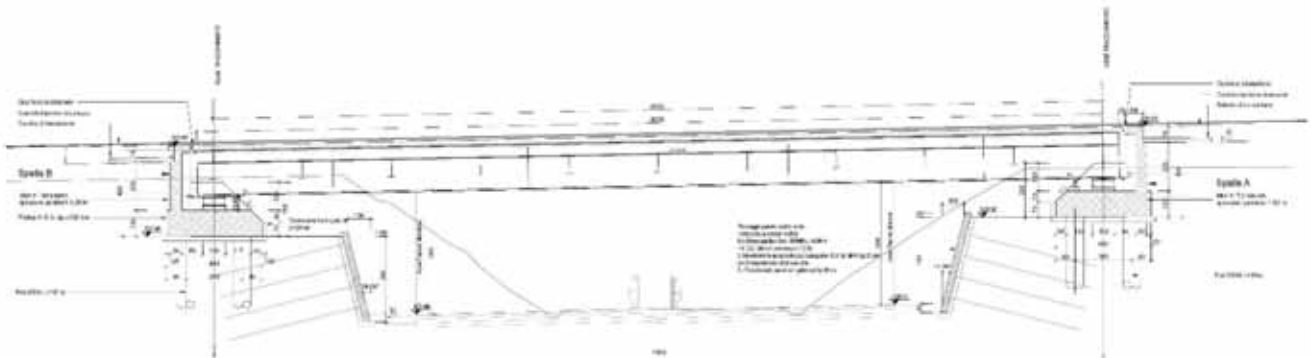


Figura 4 - Sezione prospettico

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Lo schema di vincolo prevede l'utilizzo di 4 dispositivi di appoggio tipo isolatori elastomerici con nucleo di piombo che consentono uno spostamento di  $\pm 300\text{mm}$  e di conseguenza un giunto di dilatazione di spalla che permetta il corrispondente spostamento. Si riporta di seguito le caratteristiche che deve avere il dispositivo di appoggio.

V	Fzd	Ke	$\xi_e$	Kv	Dg	te	h	H	Z
kN	kN	kN/mm	%	kN/mm	mm	mm	mm	mm	mm
15080	33560	3,83	21	3737	1000	180	316	396	1050
<b>V</b>	Carico verticale massimo agente in presenza del sisma allo SLC								
<b>Fzd</b>	Carico verticale massimo in assenza di SISMA e spostamento 10mm								
<b>Ke</b>	Rigidità orizzontale equivalente								
<b><math>\xi_e</math></b>	Coefficiente di smorzamento viscoso equivalente								
<b>Kv</b>	Rigidità verticale								
<b>Dg</b>	Diametro elastomero								
<b>te</b>	Spessore totale gomma								
<b>h</b>	Altezza escluse piastre di ancoraggio								
<b>H</b>	Altezza totale incluse piastre di ancoraggio								
<b>Z</b>	Lato piastre di ancoraggio								

SCHEMA APPOGGI



Figura 5 - Schema di vincolo

## 2 NORMATIVA DI RIFERIMENTO E RIFERIMENTI TECNICI

Le strutture sono state verificate con il criterio degli stati limite (SL). I calcoli sono stati eseguiti in osservanza alle seguenti disposizioni normative e regole tecniche:

### 2.1 NORME TECNICHE

- Ministero delle infrastrutture - D.M. 14/01/2008. Norme tecniche per le costruzioni.
- Consiglio superiore dei lavori pubblici. Istruzioni per l'applicazione delle "Norme tecniche per le costruzioni" di cui al D.M. 14/01/2008.

### 2.2 RIFERIMENTI TECNICI

#### 2.2.1 CNR e UNI

- Norma UNI EN 11104:2004
- Calcestruzzo – Specificazione, prestazione, produzione e conformità – Istruzioni complementari per l'applicazione della EN 206-1.
- Norma UNI EN 13369:2013
- Regole comuni per i prodotti prefabbricati di calcestruzzo.
- Norma UNI EN 15050:2012
- Prodotti prefabbricati di calcestruzzo. Elementi da ponte.

#### 2.2.2 EUROCODICI

- UNI EN 1993-1-1: Eurocodice 3 – Progettazione delle strutture in acciaio. Parte 1-1: Regole generali e regole per gli edifici.
- UNI EN 1993-1-5: Eurocodice 3 – Progettazione delle strutture in acciaio. Parte 1-5: Regole generali – Regole supplementari per lastre ortotrope in assenza di carichi trasversali.
- UNI EN 1993-1-9: Eurocodice 3 - Progettazione delle strutture in acciaio. Parte 1-9: Fatica.
- UNI EN 1993-1-10: Eurocodice 3 - Progettazione delle strutture in acciaio. Parte 1-10: Resilienza del materiale e proprietà attraverso lo spessore.
- UNI EN 1993-2: Eurocodice 3 – Progettazione delle strutture in acciaio. Parte 2: Ponti in acciaio.
- UNI EN 1998-2: Eurocodice 8 - Indicazioni progettuali per la resistenza sismica delle strutture. Parte 2: Ponti.



### 3 METODO DI CALCOLO

Lo studio delle strutture è stato condotto secondo i metodi della scienza delle costruzioni supponendo i materiali elastici, omogenei ed isotropi.

La ricerca dei parametri di sollecitazione è stata fatta secondo le disposizioni di carico più gravose avvalendosi di codici di calcolo automatico per l'analisi strutturale.

Le verifiche di resistenza delle sezioni sono state eseguite secondo il metodo degli stati limite.

#### 3.1 CODICI DI CALCOLO

Tutti i codici di calcolo automatico utilizzati per il calcolo e la verifica delle strutture e la redazione della presente relazione di calcolo sono di sicura ed accertata validità e sono stati impiegati conformemente alle loro caratteristiche. Tale affermazione è suffragata dai seguenti elementi:

- grande diffusione del codice di calcolo sul mercato;
- storia consolidata del codice di calcolo (svariati anni di utilizzo);
- utilizzo delle versioni più aggiornate (dopo test);
- pratica d'uso frequente in studio.

In considerazione dei casi-studio, caratterizzati da piccoli spostamenti e tensioni inferiori ai limiti elastici dei materiali, si è ritenuto sufficiente adottare una schematizzazione della geometria e dei materiali di tipo lineare con leggi elastiche e isotrope ed omogenee.

##### 3.1.1 Programmi di Verifica

Per la verifica delle sezioni in acciaio e composte è stato utilizzato un foglio di excel opportunamente testato.

## 4 MATERIALI

### 4.1 CONGLOMERATO CEMENTIZIO

#### 4.1.1 Calcestruzzo - solette

Classe di resistenza	C32/40	$f_{ck}/R_{ck} = 32/40$ MPa
Resistenza di calcolo (SLU)	$f_{cd} = \frac{\alpha_{cc} \cdot f_{ck}}{\gamma_c} = 18.81$ MPa	
Resistenza a compressione media	$f_{cm} = f_{ck} + 8 = 41.20$ MPa	
Resistenza a trazione semplice	$f_{ctm} = 0,3 \cdot f_{ck}^{2/3} = 3.10$ MPa	
Modulo elastico	$E_{cm} = 22000 \cdot [f_{cm} / 10]^{0.3} = 33642.78$ MPa	

#### 4.1.2 Calcestruzzo – pile e spalle

Classe di resistenza	C32/40	$f_{ck}/R_{ck} = 32/40$ MPa
Resistenza di calcolo (SLU)	$f_{cd} = \frac{\alpha_{cc} \cdot f_{ck}}{\gamma_c} = 18.81$ MPa	
Resistenza a compressione media	$f_{cm} = f_{ck} + 8 = 41.20$ MPa	
Resistenza a trazione semplice	$f_{ctm} = 0,3 \cdot f_{ck}^{2/3} = 3.10$ MPa	
Modulo elastico	$E_{cm} = 22000 \cdot [f_{cm} / 10]^{0.3} = 33642.78$ MPa	

### 4.2 ACCIAIO

#### 4.2.1 Acciaio per cemento armato tipo B450C

Tensione caratteristica di snervamento	$f_{yk} \geq 450$ MPa
Tensione caratteristica di rottura	$f_{tk} \geq 540$ MPa
Resistenza di calcolo	$f_{yd} = \frac{f_{yk}}{1,15} = 391$ MPa

#### 4.2.2 Acciaio per carpenteria tipo S355

Tensione caratteristica di rottura	$f_{tk} = 470$ MPa (per $s > 40$ mm) $f_{tk} = 510$ MPa (per $s < 40$ mm)
Resistenza caratteristica di snervamento	$f_{yk} = 335$ MPa (per $s > 40$ mm) $f_{yk} = 355$ MPa (per $s < 40$ mm)
Coefficiente di sicurezza	$\gamma_{M0} = 1,05$

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Coefficiente di sicurezza per verifiche a fatica  $\gamma_{M,F} = 1,35$

**4.2.3 Acciaio in reti e tralicci elettrosaldati ad aderenza migliorata per c.a.**

Tipo di acciaio: B450C ad aderenza migliorata, controllato in stabilimento

Tensione caratteristica di snervamento:  $f_{yk} \geq 450$  MPa

Tensione caratteristica di rottura:  $f_{tk} \geq 540$  MPa

Allungamento percentuale:  $A_{gtk} \geq 7.5\%$

**4.3 BULLONI AD ALTA RESISTENZA – CLASSE 10.9**

Vite: classe 10.9 (UNI EN 14399:2005)

Tensione di rottura a trazione  $f_{tk} \geq 1000$  MPa

Tensione di snervamento  $f_{yk} \geq 900$  MPa

**4.4 PIOLI CON TESTA**

Acciaio: S235J2G3+C450

Tensione di rottura a trazione  $f_{tk} \geq 450$  MPa

Tensione di snervamento  $f_{yk} \geq 370$  MPa

Allungamento  $A_5 \geq 15\%$

**4.5 SALDATURE DI TESTA O A T A COMPLETA PENETRAZIONE**

Giunto di prima classe:  $f_{d,S355} = 338$  MPa

Giunto di seconda classe:  $0.85 f_{d,S355} = 287.38$  MPa

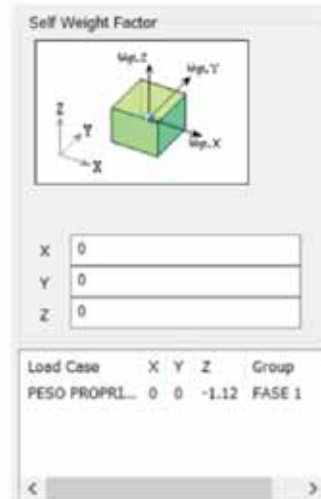
**4.6 SALDATURE A CORDONE D'ANGOLO**

Per S355:  $|\sigma_{\perp}|, |\tau_{\perp}|, |\tau_{\parallel}| \leq 0.70 f_{d,S355} = 236.6$  MPa

## 5 ANALISI GENERALE DEI CARICHI

### 5.1 PESI PROPRI STRUTTURALI

I pesi strutturali delle opere in acciaio comprensive di travi principali, trave di spina, traversi e controventi, vengono considerati in automatico nel programma di calcolo. Per considerare l'effetto del "piastrame", è stato scelto di aumentare il peso proprio del 12%, come riportato nell'immagine che segue.



### 5.2 PESI PROPRI PORTATI

I pesi proprio portati sono stati presi in conto nel modello di calcolo utilizzato, come carichi lineari direttamente applicati alle singole travi portanti. Si riporta di seguito la definizione dei carichi applicati.

#### 5.2.1 PESO PROPRIO SOLETTA E CORDOLI

Per la determinazione delle azioni da applicare alle singole travi è stata schematizzata una trave su più appoggi a cui è stata applicato il carico distribuito della soletta e dei cordoli laterali. Le reazioni vincolari risultanti sono poi state applicate alle travi del modello.

PESO SOLETTA		
Hsoletta =	0,26	m
$\gamma$ =	25	kN/mc
peso =	6,5	kN/mq
Carico trave SX =	19,783	
Carico trave Centrale =	28,685	
Carico trave DX =	19,783	
CORDOLI		
Hcordoli =	0,15	m
$\gamma$ =	25	kN/mc
peso =	3,75	kN/mq
Carico trave SX =	3,733	
Carico trave Centrale =	-1,841	
Carico trave DX =	3,733	

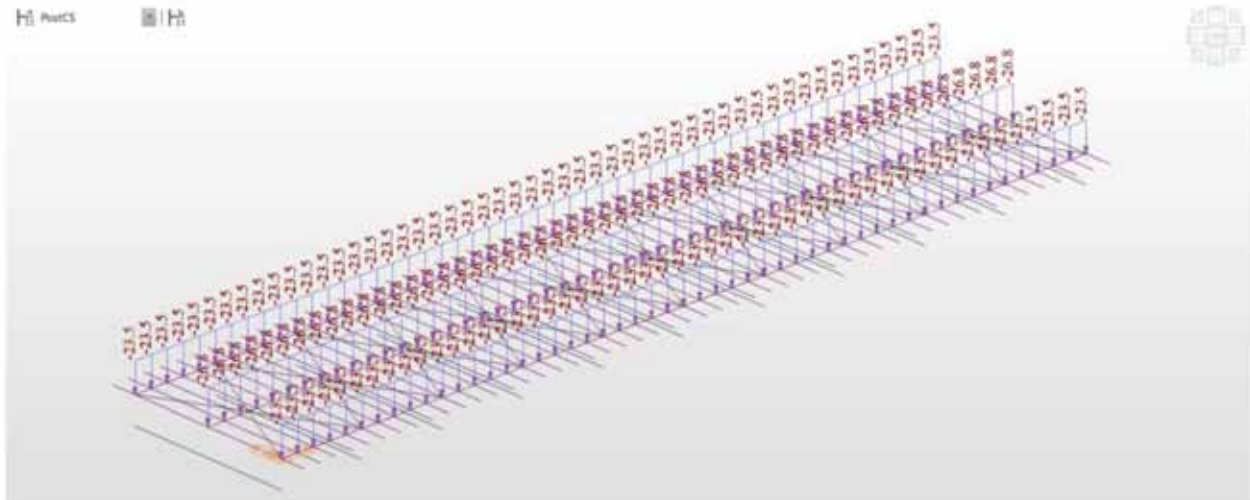


Figura 6 – Pesì propri soletta e cordoli

## 5.2.2 PESO PERMANENTE PORTATO

Per la determinazione delle azioni da applicare alle singole travi è stata schematizzata una trave su più appoggi a cui è stata applicato il carico del pacchetto stradale e delle barriere bordo ponte. Le reazioni vincolari risultanti sono poi state applicate alle travi del modello.

### PERMANENTE PORTATO

Hsoletta =	<b>0,11</b>	cm
$\gamma$ =	24	kN/mc
peso =	2,64	kN/mq
Carico trave SX =	<b>5,407</b>	
Carico trave Centrale =	<b>12,946</b>	
Carico trave DX =	<b>5,407</b>	

### BARRIERE+VELETTE E TUBAZIONI

Barriere =	1,5	kN/ml
Tubazioni =	0,1	kN/ml
Veletta =	0,4	kN/ml
Totale	<b>2,000</b>	kN/ml
Carico trave SX =	<b>2,374</b>	
Carico trave Centrale =	<b>-0,748</b>	
Carico trave DX =	<b>2,374</b>	

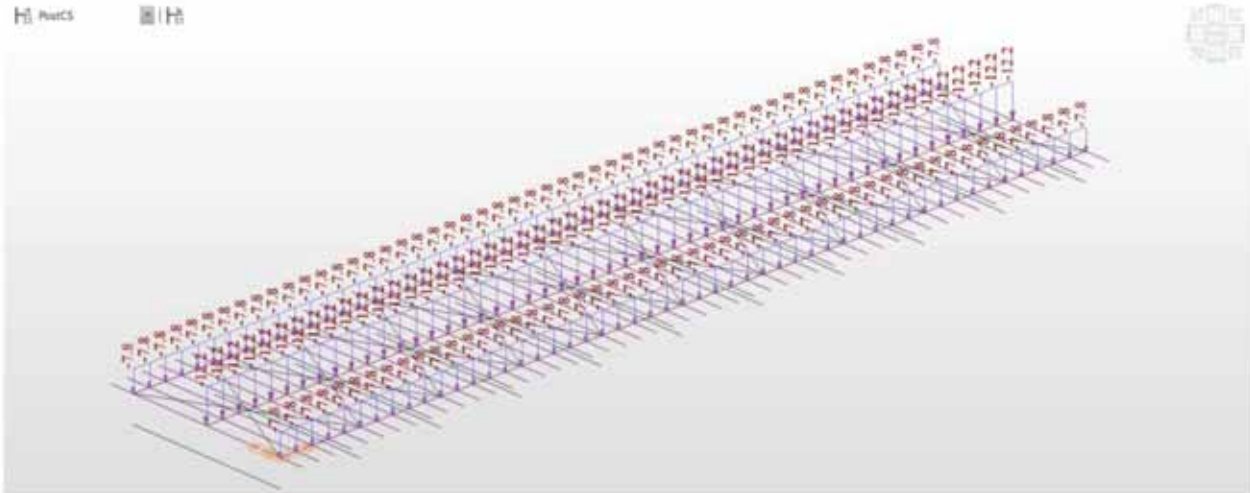


Figura 7 - Peso permanente portato

### 5.3 AZIONI ACCIDENTALI

#### 5.3.1 Carichi mobili

Le azioni accidentali considerate nei calcoli sono quelle previste dall'attuale D.M. 14.1.2008 per i ponti classificati di prima categoria e calcolate in relazione alla larghezza dell'impalcato.

La carreggiata tra i cordoli ha larghezza di 9,00 m, pertanto sono state considerate ai fini del calcolo 3 corsie da 3,00 m.

Si riporta lo schema di carico 1:

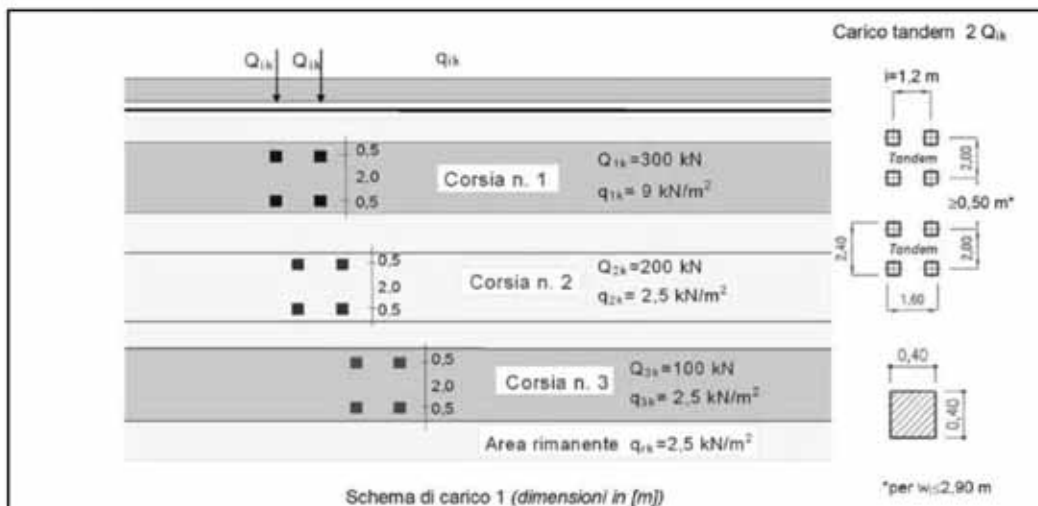


Figura 8 - Schema di carico 1

L'intensità dei carichi da applicare è riportata nella seguente tabella:

**Tabella 5.1.II - Intensità dei carichi  $Q_{ik}$  e  $q_{ik}$  per le diverse corsie**

Posizione	Carico asse $Q_{ik}$ [kN]	$q_{ik}$ [kN/m <sup>2</sup> ]
Corsia Numero 1	300	9,00
Corsia Numero 2	200	2,50
Corsia Numero 3	100	2,50
Altre corsie	0,00	2,50

Le suddette colonne di carico sono state disposte secondo lo schema longitudinale che produce le azioni accidentali più gravose per la struttura in esame.

Si riportano di seguito alcune immagini dei carichi considerati.

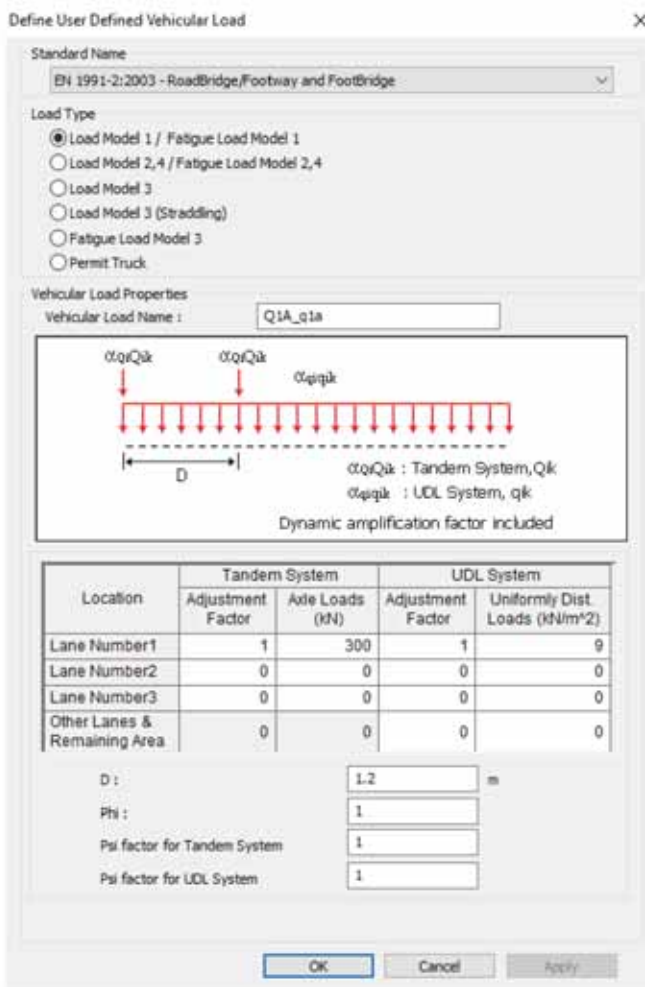


Figura 9 - Colonna di carico 1

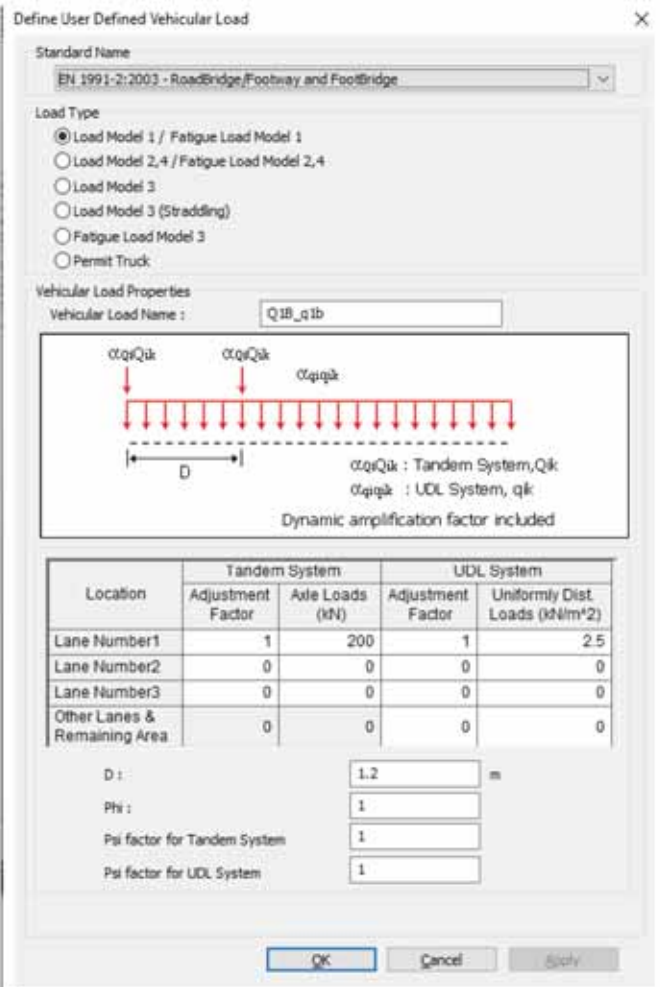


Figura 10 - Colonna di carico 2

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

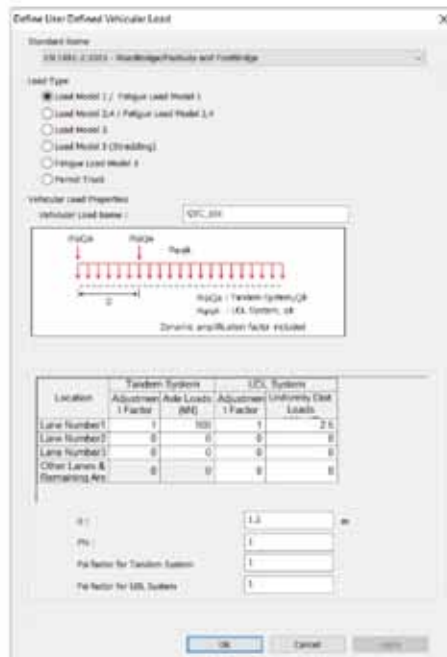


Figura 11 - Colonna di carico 3

**5.3.2 Frenamento**

FRENATURA		
Si considera un'azione longitudinale funzione del carico verticale agente sulla corsia convenzionale n°1		
$Q1k = 0.6 \times ( 2 \times Q1k ) + 0.10 \times q1k \times W1 \times L$		
<b>Q1k =</b>	<b>474,750</b>	<b>kN</b>
<b>q1k =</b>	<b>3,724</b>	<b>kN</b>

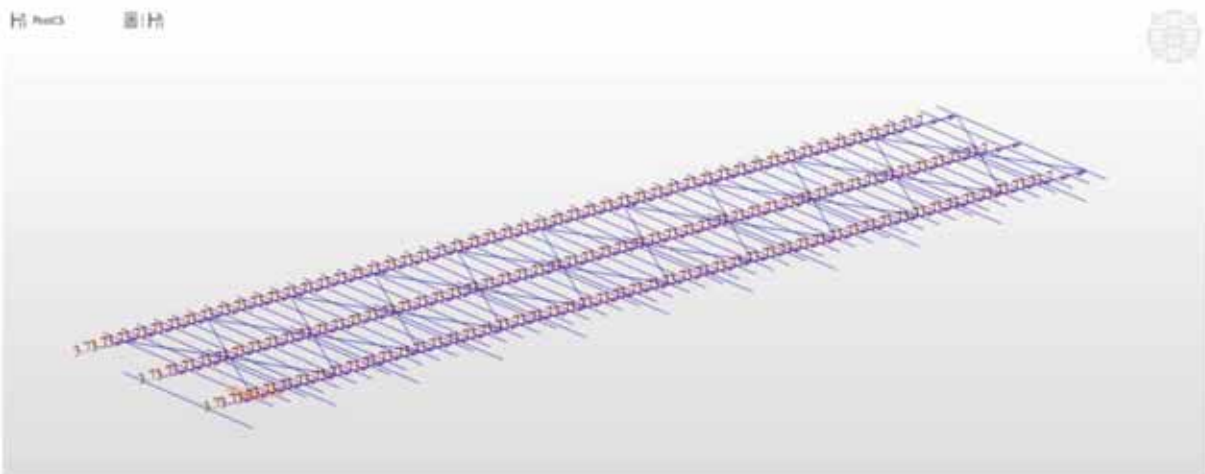
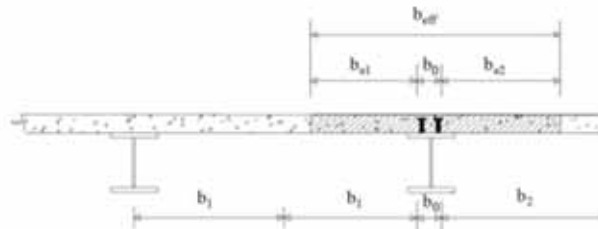


Figura 12 - Frenamento



## 5.4 CALCOLO SEZIONE COLLABORANTE

La distribuzione delle tensioni normali negli elementi composti è stata determinata mediante un modello che tiene conto della diffusione degli sforzi nelle ali della trave metallica e nella soletta in calcestruzzo. La larghezza efficace,  $b_{eff}$ , della soletta in calcestruzzo è stata determinata, in conformità a quanto prescritto dalla normativa, mediante l'espressione:  $b_{eff} = b_0 + be_1 + be_2$ , in cui  $b_0$  è la distanza tra gli assi dei connettori esterni e  $be_i = \min (L_e/8, b_i)$  è il valore della larghezza collaborante da ciascun lato della sezione composta.



$L_e$ , nelle travate in semplice appoggio, indica la luce della trave.

Per il caso in esame si ha:

$L_e =$	41,00 m		
$L_e/8 =$	5,125 m		
$b_0 =$	0,382 m		
$b_{i2} =$	1,059 m	SBALZO	
$b_{i1} =$	1,902 m	DISTANZA	
$be_2 =$	1,059		
$be_1 =$	1,902		
	<b>3,343</b> m	<b>TRAVE PRINCIPALE</b>	
$b_{eff} =$	<b>3,804</b> m	<b>TRAVE DI SPINA</b>	

## 5.5 AZIONI METEORICHE

### 5.5.1 Azioni dovute alla neve

Il carico dovuto alla neve, non risulta dimensionante per il tipo di struttura

### 5.5.2 Azioni dovute al vento

Viene di seguito riportata una tabella di calcolo per la determinazione della pressione del vento da applicare alla struttura

<b>AZIONE DEL VENTO</b>				
<i>(Inserire i dati necessari nelle celle campite)</i>	<i>simbolo</i>	<i>valore</i>	<i>unità</i>	<i>formula</i>
Altitudine sul livello del mare	$a_s =$	630	[m]	
Regione		Sicilia e Provincia di Reggio Calabria		
	Zona =	4		
Parametri tabella 3.3.1	$v_{b0} =$	28	[m/s]	
Parametri tabella 3.3.1	$a_0 =$	500	[m]	
Parametri tabella 3.3.1	$k_s =$	0.360	[-]	
Coefficiente di altitudine	$c_a =$	1.094	[-]	
Velocità di riferimento	$v_b =$	30.6	[m/s]	
Periodo di ritorno	$T_R =$	50	[anni]	
Velocità di riferimento associata a $T_R = 50$	$v_b =$	30.6	[m/s]	
Velocità di riferimento	$v_r =$	30.6	[m/s]	$v_r = v_b \times c_r$
<b>CALCOLO PRESSIONE DEL VENTO</b>				
Densità dell'aria	$\rho =$	1.25	[kg/m <sup>3</sup> ]	
Pressione cinetica di riferimento	$q_r =$	586.9	[N/m <sup>2</sup> ]	$1/2 \times \rho v_r^2$
Categoria di esposizione del terreno		II		
Parametri per la definizione del coefficiente di esposizione	$k_r =$	0.19	[-]	
	$z_0 =$	0.05	[m]	
	$z_{min} =$	4.00	[m]	
Quota struttura	$z =$	9	[m]	
Coefficiente di topografia	$c_t =$	1		
Classe di rugosità		D		
Coefficiente di esposizione	$c_e =$	2.29		
Coefficiente di forma	$c_p =$	1.40		
Coefficiente dinamico	$c_d =$	1		
<b>Pressione del vento</b>	<b><math>p =</math></b>	<b>1.88</b>	<b>[kN/m<sup>2</sup>]</b>	<b><math>p = q_b \times c_e \times c_p \times c_d</math></b>

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Sulla base delle caratteristiche geometriche dell'impalcato, sono state determinate le azioni da applicare alle strutture in caso di ponte scarico e ponte carico.

VENTO A PONTE SCARICO		
Fh =	4,507	kN/ml
Pv =	0,592	kN/ml
VENTO A PONTE CARICO		
Fh =	10,142	kN/ml
Pv =	0,304	kN/ml

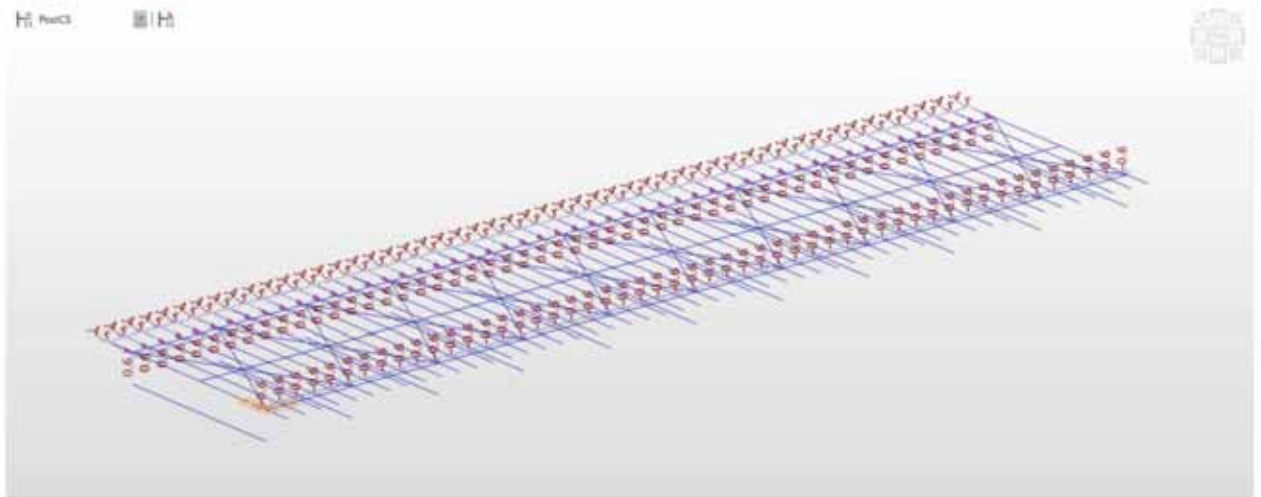


Figura 13 - Vento a ponte scarico

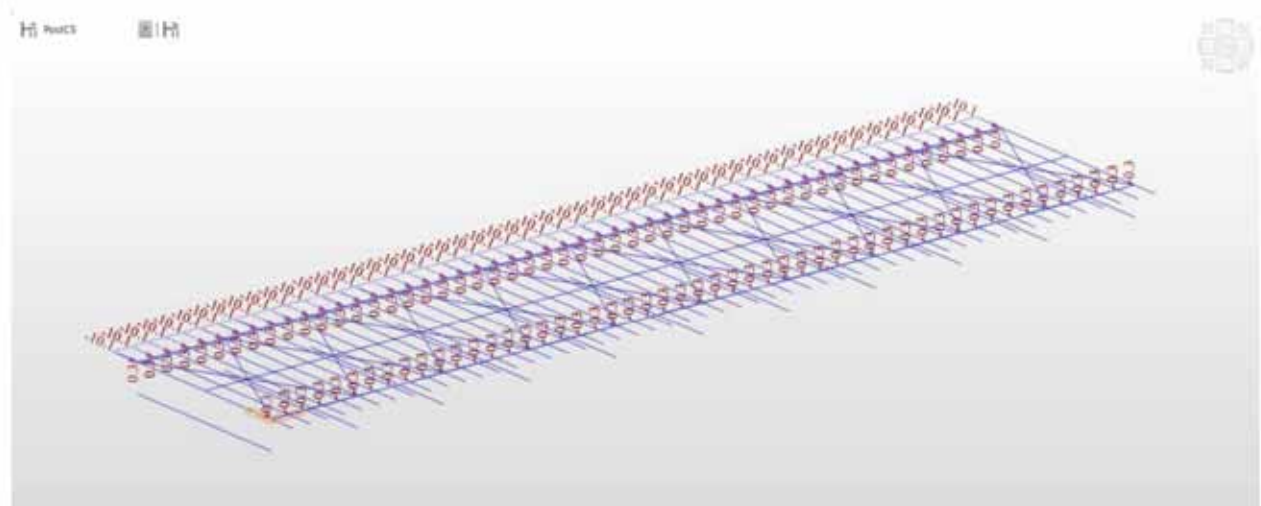


Figura 14 - Vento a ponte carico

### 5.5.3 Azioni dovute alla temperatura

L'azione termica, sia uniforme che a forma di gradiente, è stata applicata in base a quanto previsto dalla normativa vigente.

AZIONE DELLA TEMPERATURA				
<i>Inserire i dati necessari nelle celle campite)</i>				
	simbolo	valore	unità	formula
Altitudine di riferimento	$a_s =$	630	[m]	
Individuazione della zona :		Zona IV		
Calabria, Sicilia				
Temperatura massima estiva dell'aria	$T_{max} =$	45	[°C]	Valore raccomandato in assenza di studi specifici
Temperatura minima invernale dell'aria	$T_{min} =$	-15	[°C]	Valore raccomandato in assenza di studi specifici
<i>I valori di <math>T_{max}</math> e <math>T_{min}</math> sono riferiti ad un periodo di ritorno di 50 anni</i>				
<i>Per la valutazione della temperatura uniforme minima/massima del ponte, si considera il grafico riportato sull'eurocodice nella sezione Ponti</i>				
Tipologia di impalcato	Tipo 2			
	Impalcato a struttura mista			
	$T_0 =$	15	[°C]	Atteso Nazionale A.1(3)
Legenda				
1 - Tipo 1 $T_{c,max} = T_{max} + 16$				
2 - Tipo 2 $T_{c,max} = T_{max} + 4$				
3 - Tipo 3 $T_{c,max} = T_{max} + 2$				
4 - Tipo 1 $T_{c,min} = T_{min} - 3$				
5 - Tipo 2 $T_{c,min} = T_{min} + 4$				
6 - Tipo 3 $T_{c,min} = T_{min} + 8$				
<b>Calcolo variazione termica uniforme</b>				
Temperatura uniforme del ponte massima	$T_{c,max} =$	49	[°C]	
Temperatura uniforme del ponte minima	$T_{c,min} =$	-11	[°C]	
Variazione termica di espansione	$\Delta T_{N,exp} =$	34	[°C]	
Variazione termica di contrazione	$\Delta T_{N,cont} =$	-26	[°C]	
<b>Calcolo variazione termica lineare</b>				
Spessore della superficie		100	[mm]	
Coefficiente riduttivo heat	$K_{sur,heat} =$	1.00	[-]	
Coefficiente riduttivo cool	$K_{sur,cool} =$	1.00	[-]	
Variazione termica lineare (heat)	$\Delta T_{M,heat} =$	15	[°C]	
Variazione termica lineare (cool)	$\Delta T_{M,cool} =$	-18	[°C]	

## 5.6 AZIONE SISMICA

Con riferimento al DM 14.01.2008, sono stati presi in considerazione i seguenti parametri, per il sito in esame:

Vita nominale della costruzione –  $VN = 50$  anni

Coefficiente d'uso della costruzione –  $cu = 2.0$

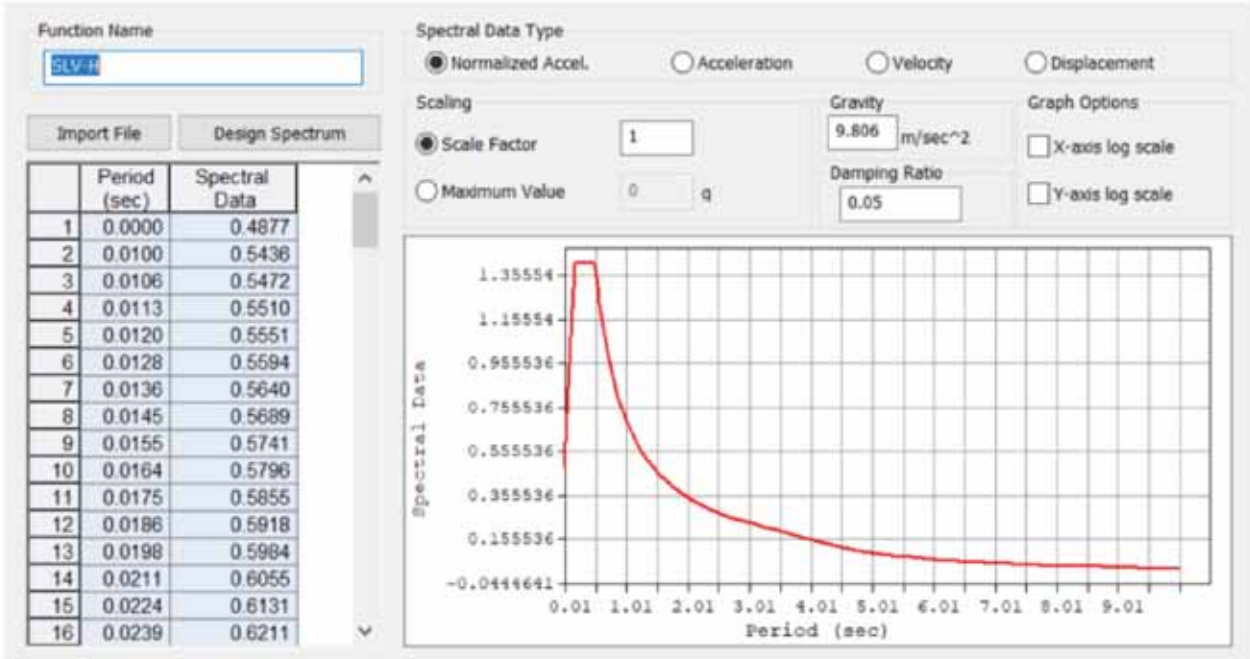
Ne deriva un periodo di riferimento per la costruzione  $VR=100$  anni.

Chilometrica	Categoria di sottosuolo NTC	Categoria topografica	$S_T$	$S_s$	ag	F0	Tc*	Latitudine	Longitudine
1+459	RSL	T1							
3+005	B	T1	1,00	1,116	0,301	2,361	0,454	36,959319	14,669724
5+204	B	T1	1,00	1,105	0,312	2,36	0,455	36,975768	14,67698
12+235	RSL	T1							
15+818	RSL	T1							
0+553	RSL	T1							
1+259	B	T1	1,00	1,087	0,332	2,355	0,456	37,091473	14,63782
2+065	B	T1	1,00	1,086	0,333	2,356	0,457	37,098526	14,640441
2+781	B	T1	1,00	1,085	0,334	2,357	0,457	37,104181	14,643716
4+182	B	T1	1,00	1,081	0,337	2,36	0,458	37,114961	14,650551
8+318	B	T1	1,00	1,075	0,344	2,364	0,461	37,147444	14,671512
8+746	B	T1	1,00	1,075	0,344	2,363	0,462	37,151077	14,673177

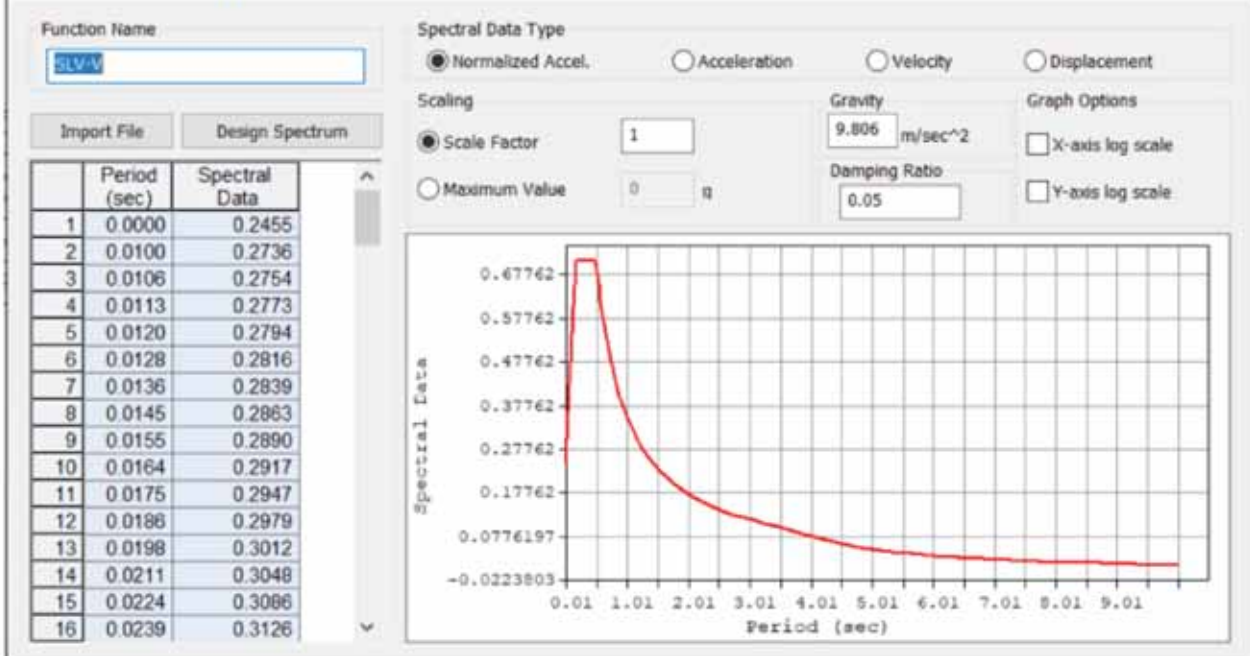
Gli spettri di risposta utilizzati nel programma di calcolo sono rappresentati nelle figure seguenti e sono stati determinati a mezzo di un'analisi sismica locale del sito:

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Add/Modify/Show Response Spectrum Functions

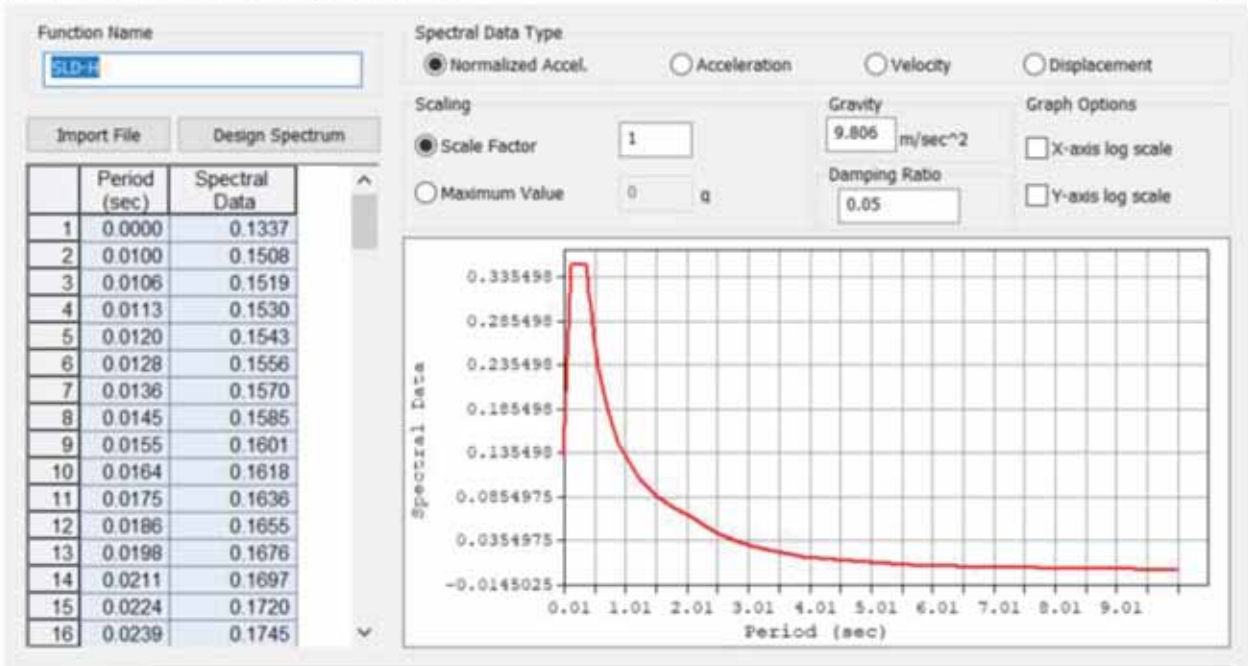


Add/Modify/Show Response Spectrum Functions

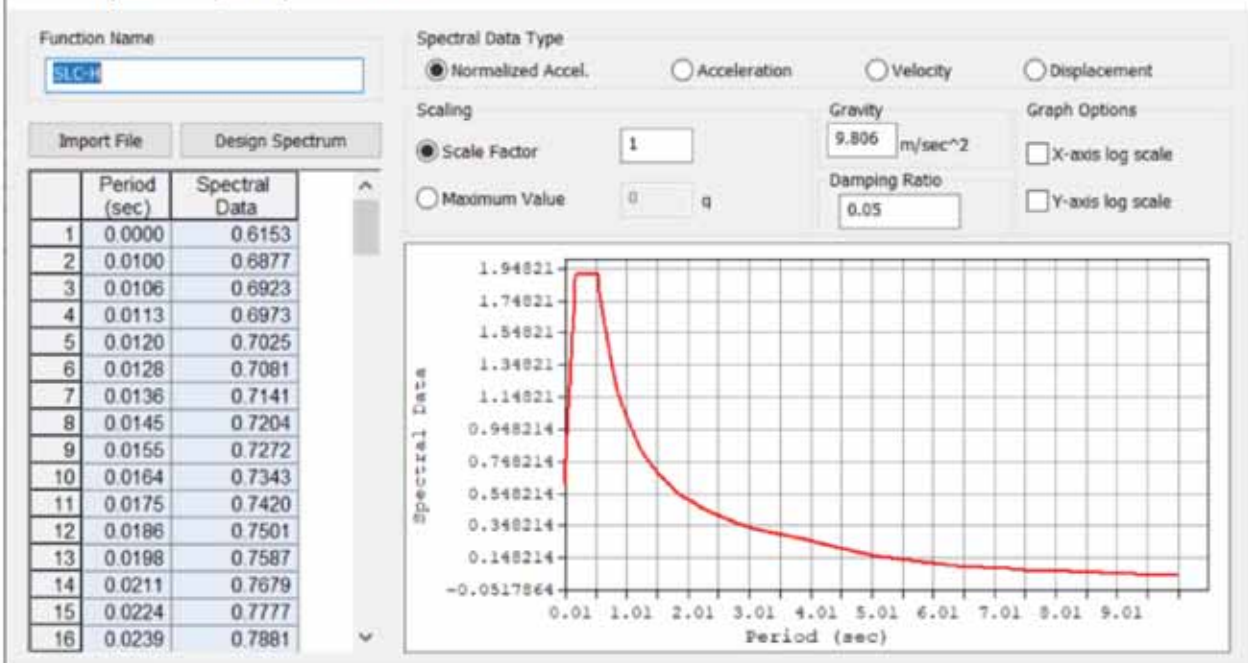


LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Add/Modify/Show Response Spectrum Functions



Add/Modify/Show Response Spectrum Functions



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Node	Mode	UX	UY	UZ	RX	RY	RZ						
<b>EIGENVALUE ANALYSIS</b>													
	Mode No	Frequency		Period	Tolerance								
		(rad/sec)	(cycle/sec)	(sec)									
	1	5.580838	0.888218	1.125850	0.0000e+000								
	2	5.663574	0.901386	1.109403	0.0000e+000								
	3	9.605327	1.528735	0.654135	0.0000e+000								
	4	14.880747	2.365181	0.422804	0.0000e+000								
	5	18.758016	2.965431	0.334960	0.0000e+000								
	6	49.064216	7.808812	0.128090	1.3257e-085								
	7	62.142137	9.850228	0.101110	5.4102e-073								
	8	73.808575	11.747000	0.085128	6.4438e-064								
	9	84.789687	13.494898	0.074103	2.3737e-056								
	10	105.629519	16.811480	0.059483	1.9272e-043								
<b>MODAL PARTICIPATION MASSES PRINTOUT</b>													
	Mode No	TRAN-X		TRAN-Y		TRAN-Z		ROTN-X		ROTN-Y		ROTN-Z	
		MASS(%)	SUM(%)	MASS(%)	SUM(%)	MASS(%)	SUM(%)	MASS(%)	SUM(%)	MASS(%)	SUM(%)	MASS(%)	SUM(%)
	1	0.00	0.00	99.98	99.98	0.00	0.00	0.01	0.01	0.00	0.00	0.00	0.00
	2	100.00	100.00	0.00	99.98	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.00
	3	0.00	100.00	0.00	99.98	0.00	0.00	0.00	0.01	0.00	0.00	100.00	100.00
	4	0.00	100.00	0.00	99.98	81.33	81.33	0.00	0.01	0.00	0.00	0.00	100.00
	5	0.00	100.00	0.01	99.99	0.00	81.33	80.46	80.46	0.00	0.00	0.00	100.00
	6	0.00	100.00	0.00	99.99	0.00	81.33	0.00	80.46	60.70	60.70	0.00	100.00
	7	0.00	100.00	0.00	99.99	0.00	81.33	0.00	80.46	0.00	60.70	0.00	100.00
	8	0.00	100.00	0.01	100.00	0.00	81.33	0.43	80.90	0.00	60.70	0.00	100.00
	9	0.00	100.00	0.00	100.00	7.91	89.24	0.00	80.90	0.00	60.70	0.00	100.00
	10	0.00	100.00	0.00	100.00	0.00	89.24	0.00	80.90	11.80	72.29	0.00	100.00
	Mode No	TRAN-X		TRAN-Y		TRAN-Z		ROTN-X		ROTN-Y		ROTN-Z	
		MASS	SUM	MASS	SUM	MASS	SUM	MASS	SUM	MASS	SUM	MASS	SUM
	1	0.00	0.00	524.38	524.38	0.00	0.00	0.38	0.38	0.00	0.00	0.00	0.00
	2	524.49	524.49	0.00	524.38	0.00	0.00	0.00	0.38	0.57	0.57	0.00	0.00
	3	0.00	524.49	0.00	524.38	0.00	0.00	0.00	0.38	0.00	0.57	78562.64	78562.64
	4	0.00	524.49	0.00	524.38	426.80	426.80	0.00	0.38	0.00	0.57	0.00	78562.64
	5	0.00	524.49	0.05	524.43	0.00	426.80	4701.08	4701.48	0.00	0.57	0.00	78562.64
	6	0.01	524.50	0.00	524.43	0.00	426.80	0.00	4701.48	44586.59	44586.15	0.00	78562.64
	7	0.00	524.50	0.00	524.43	0.00	426.80	0.00	4701.48	0.00	44586.15	0.04	78562.68
	8	0.00	524.50	0.06	524.49	0.00	426.80	25.36	4726.83	0.00	44586.15	0.00	78562.68
	9	0.00	524.50	0.00	524.49	41.47	468.07	0.00	4726.83	0.00	44586.15	0.00	78562.68
	10	0.00	524.50	0.00	524.49	0.00	468.07	0.00	4726.83	8517.91	53104.06	0.00	78562.68

è stata eseguita un'analisi modale a spettro di risposta. Come si nota dall'immagine sopra riportata, avendo introdotto come appoggi degli isolatori, le azioni sismiche orizzontali si dimostrano praticamente disaccoppiate. In altre parole, andando ad analizzare i modi di vibrare che "muovono" maggior quantitativo di masse, si nota che il modo di vibrare numero 1 e 2 muovono massa in direzione rispettivamente Y ed X in modo disaccoppiato.



## 6 MODELLAZIONE STRUTTURALE

La modellazione strutturale è stata effettuata tramite il programma di calcolo Midas-Civil, schematizzando l'opera come un graticcio equivalente.

Trattandosi di un impalcato a sezione mista acciaio-clc, nell'ambito della modellazione, sono state introdotte le fasi di maturazione del clc necessarie per le verifiche degli elementi portanti e nel dettaglio:

- ✓ Fase1 : Presenza della sola struttura in acciaio soggetta al solo peso proprio strutturale (durata per la sola fase di varo)

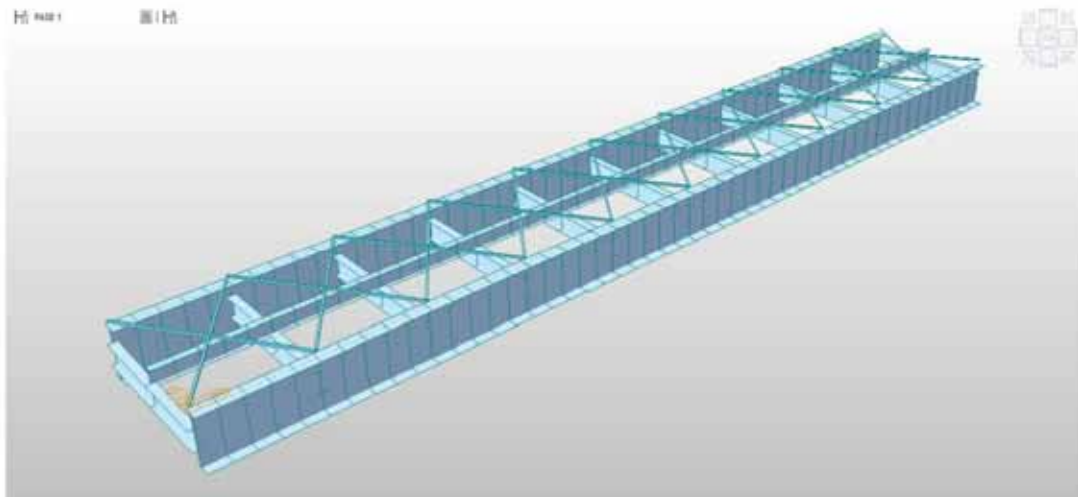


Figura 15 - Schema struttura in Fase 1

- ✓ Fase2: Presenza della struttura in acciaio e della soletta (non ancora collaborante) ma che interviene solo come peso portato (durata della fase 28giorni).
- ✓ Fase 3: Fase in cui hanno corso gli effetti reologici del calcestruzzo e in cui viene posato anche il carico permanente portato (durata giorni – fino alla fine degli effetti reologici).

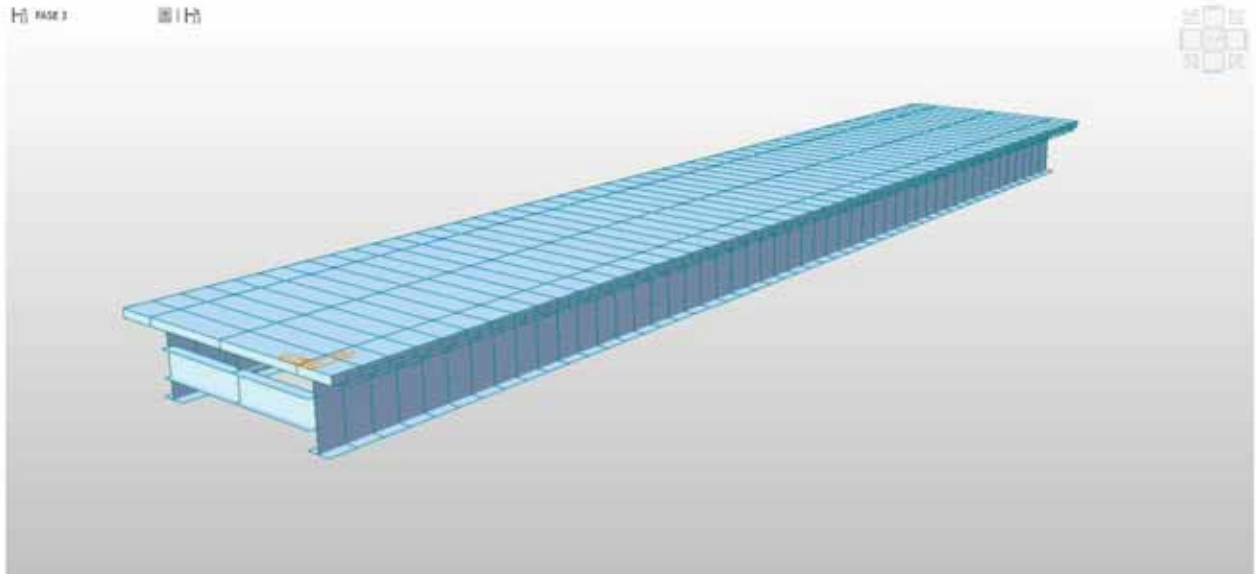


Figura 16 - Schema struttura in Fase 2-3

Inserendo in modo opportuno le varie curve di maturazione e ritiro del calcestruzzo, il programma di calcolo tiene in conto in automatico degli effetti reologici nel tempo. Pertanto, nelle varie fasi di carico, e di maturazione del getto, si instaurano effetti dovuti a ritiro e viscosità che vengono tenuti in conto in automatico dal programma di calcolo nell'ambito delle singole fasi introdotte. La fase 3, che ha una durata effettiva di circa 10000 giorni, ha lo scopo di esaurire tutti gli effetti reologici del calcestruzzo e precede la successiva fase di applicazione dei carichi variabili da traffico e meteorologici.

Construction Stage

Name	Duration	Date	Step	Result
FASE 1	2	2	0	Stage
FASE 2	28	30	0	Stage
FASE 3	10000	10030	0	Stage

Buttons: Add, Insert Prev, Insert Next, Generate, Show, Delete, Close

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Add/Modify Time Dependent Material (Creep / Shrinkage)

Name : C+S Code : European

European

Characteristic compressive cylinder strength of concrete at the age of 28 days (f<sub>ck</sub>) : 32000 kN/m<sup>2</sup>

Relative Humidity of ambient environment (40 - 99) : 70 %

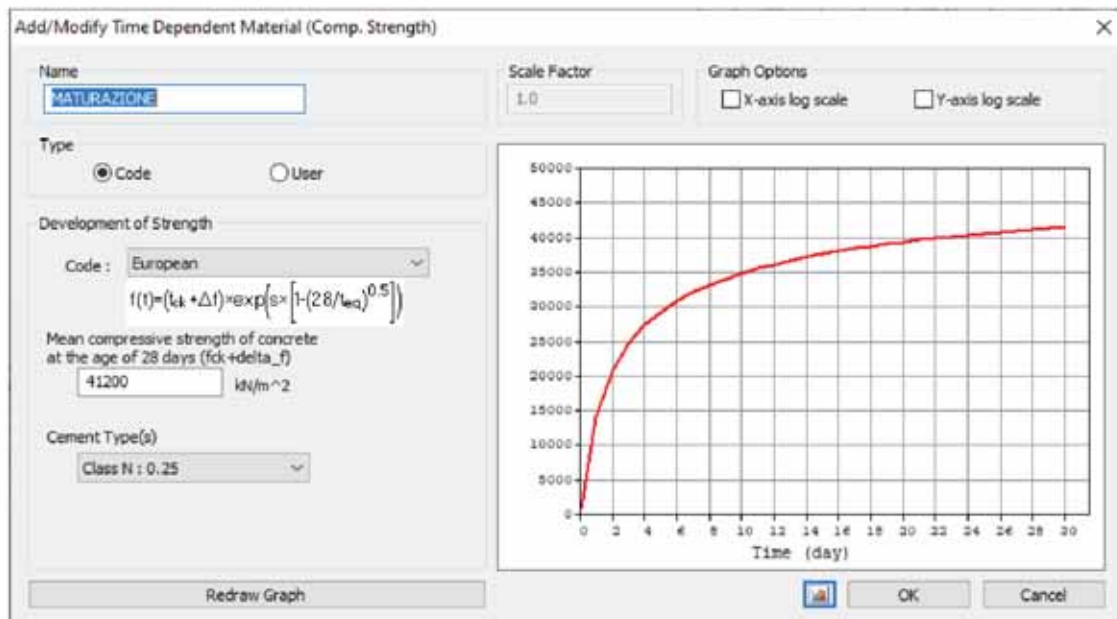
Notional size of member : 0.25 m  
h = 2 \* A<sub>c</sub> / u (A<sub>c</sub> : Section Area, u : Perimeter in contact with atmosphere)

Type of cement  
 Class S  Class N  Class R

Type of code  
 EN 1992-1 (General Structure)  EN 1992-2 (Concrete Bridge)  Use of silica-fume

Age of concrete at the beginning of shrinkage : 3 day

Show Result... OK Cancel Apply



Per quanto riguarda la soletta, la stessa viene modellata con elementi tipo frame di dimensioni pari all'interasse previsto, e viene utilizzata per l'applicazione in senso trasversale dei carichi mobili. In altre parole, i carichi mobili vengono ripartiti lungo gli elementi principali tramite la soletta.

## 6.1 COMBINAZIONI DI CARICO

Le combinazioni di carico considerate sono quelle desunte secondo lo schema seguente.

Gruppo di azioni	Carichi sulla carreggiata					Carichi su marciapiedi e piste ciclabili
	Carichi verticali			Carichi orizzontali		Carichi verticali
	Modello principale (Schema di carico 1, 2, 3, 4, 6)	Veicoli speciali	Folla (Schema di carico 5)	Frenatura $q_3$	Forza centrifuga $q_4$	
1	Valore caratteristico					Carico uniformemente distribuito
2 a	Valore frequente			Valore caratteristico		Schema di carico 5 con valore di combinazione 2,5 kN/m <sup>2</sup>
2 b	Valore frequente				Valore caratteristico	
3 <sup>(*)</sup>						Schema di carico 5 con valore caratteristico 5,0 kN/m <sup>2</sup>
4 <sup>(**)</sup>			Schema di carico 5 con valore caratteristico 5,0 kN/m <sup>2</sup>			Schema di carico 5 con valore caratteristico 5,0 kN/m <sup>2</sup>
5 <sup>(***)</sup>	Da definirsi per il singolo progetto	Valore caratteristico o nominale				

<sup>(\*)</sup> Ponti di 3<sup>a</sup> categoria  
<sup>(\*\*)</sup> Da considerare solo se richiesto dal particolare progetto (ad es. ponti in zona urbana)  
<sup>(\*\*\*)</sup> Da considerare solo se si considerano veicoli speciali

I coefficienti parziali di sicurezza per le combinazioni di carico SLU sono i seguenti.

		Coefficiente	EQU <sup>(1)</sup>	A1 STR	A2 GEO
Carichi permanenti	favorevoli	$\gamma_{G1}$	0,90	1,00	1,00
	sfavorevoli		1,10	1,35	1,00
Carichi permanenti non strutturali <sup>(2)</sup>	favorevoli	$\gamma_{G2}$	0,00	0,00	0,00
	sfavorevoli		1,50	1,50	1,30
Carichi variabili da traffico	favorevoli	$\gamma_Q$	0,00	0,00	0,00
	sfavorevoli		1,35	1,35	1,15
Carichi variabili	favorevoli	$\gamma_{Qi}$	0,00	0,00	0,00
	sfavorevoli		1,50	1,50	1,30
Distorsioni e presollecitazioni di progetto	favorevoli	$\gamma_{e1}$	0,90	1,00	1,00
	sfavorevoli		1,00 <sup>(3)</sup>	1,00 <sup>(4)</sup>	1,00
Ritiro e viscosità, Variazioni termiche, Cedimenti vincolari	favorevoli	$\gamma_{e2}, \gamma_{e3}, \gamma_{e4}$	0,00	0,00	0,00
	sfavorevoli		1,20	1,20	1,00

<sup>(1)</sup> Equilibrio che non coinvolga i parametri di deformabilità e resistenza del terreno; altrimenti si applicano i valori di GEO.  
<sup>(2)</sup> Nel caso in cui i carichi permanenti non strutturali (ad es. carichi permanenti portati) siano compiutamente definiti si potranno adottare gli stessi coefficienti validi per le azioni permanenti.  
<sup>(3)</sup> 1,30 per instabilità in strutture con precompressione esterna  
<sup>(4)</sup> 1,20 per effetti locali

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

I coefficienti  $\psi$  per le azioni variabili per i ponti stradali sono i seguenti.

Azioni	Gruppo di azioni (Tabella 5.1.IV)	Coefficiente $\psi_0$ di combinazione	Coefficiente $\psi_1$ (valori frequenti)	Coefficiente $\psi_2$ (valori quasi permanenti)
Azioni da traffico (Tabella 5.1.IV)	Schema 1 (Carichi tandem)	0,75	0,75	0,0
	Schemi 1, 5 e 6 (Carichi distribuiti)	0,40	0,40	0,0
	Schemi 3 e 4 (carichi concentrati)	0,40	0,40	0,0
	Schema 2	0,0	0,75	0,0
	2	0,0	0,0	0,0
	3	0,0	0,0	0,0
	4 (folia)	----	0,75	0,0
Vento $q_3$	Vento a ponte scarico			
	SLU e SLE	0,6	0,2	0,0
	Esecuzione	0,8	----	0,0
	Vento a ponte carico	0,6		
Neve $q_3$	SLU e SLE	0,0	0,0	0,0
	esecuzione	0,8	0,6	0,5
Temperatura	$T_k$	0,6	0,6	0,5

Si riporta di seguito una tabella raffigurante tutte le combinazioni di carico utilizzate.

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

		PESO PROPRIO ACCIAIO	PESO PROPRIO SOLETTA	PERMANENTE PORTATO	VENTO PONTE SCARICO	VENTO PONTE CARICO	TERMICA UNIFORME +	TERMICA UNIFORME -	TERMICA GRADIENTE +	TERMICA GRADIENTE -	FREINATURA	LMD1	LMD2	LMD3	SISMA X	SISMA Y	SISMA Z
SLU	SLU1	1.35	1.35	1.35	1.5	0	0.9	0	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU2	1.35	1.35	1.35	1.5	0	0	0.9	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU3	1.35	1.35	1.35	1.5	0	0	0	0.9	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU4	1.35	1.35	1.35	1.5	0	0	0	0	0.9	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU5	1.35	1.35	1.35	0	1.5	0.9	0	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU6	1.35	1.35	1.35	0	1.5	0	0.9	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU7	1.35	1.35	1.35	0	1.5	0	0	0.9	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU8	1.35	1.35	1.35	0	1.5	0	0	0	0.9	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU9	1.35	1.35	1.35	0.9	0	0.9	0	0	0	1.35	1.35	0	0	0	0	0
	SLU10	1.35	1.35	1.35	0.9	0	0	0.9	0	0	1.35	1.35	0	0	0	0	0
	SLU11	1.35	1.35	1.35	0.9	0	0	0	0.9	0	1.35	1.35	0	0	0	0	0
	SLU12	1.35	1.35	1.35	0.9	0	0	0	0	0.9	1.35	1.35	0	0	0	0	0
	SLU13	1.35	1.35	1.35	0	0.9	0.9	0	0	0	1.35	1.35	0	0	0	0	0
	SLU14	1.35	1.35	1.35	0	0.9	0	0.9	0	0	1.35	1.35	0	0	0	0	0
	SLU15	1.35	1.35	1.35	0	0.9	0	0	0.9	0	1.35	1.35	0	0	0	0	0
	SLU16	1.35	1.35	1.35	0	0.9	0	0	0	0.9	1.35	1.35	0	0	0	0	0
	SLU17	1.35	1.35	1.35	0.9	0	0.9	0	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU18	1.35	1.35	1.35	0.9	0	0	0.9	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU19	1.35	1.35	1.35	0.9	0	0	0	0.9	0	1.35	1.35	1.35	1.35	0	0	0
	SLU20	1.35	1.35	1.35	0.9	0	0	0	0	0.9	1.35	1.35	1.35	1.35	0	0	0
	SLU21	1.35	1.35	1.35	0	0.9	0.9	0	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU22	1.35	1.35	1.35	0	0.9	0	0.9	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU23	1.35	1.35	1.35	0	0.9	0	0	0.9	0	1.35	1.35	1.35	1.35	0	0	0
	SLU24	1.35	1.35	1.35	0	0.9	0	0	0	0.9	1.35	1.35	1.35	1.35	0	0	0
RARA	RARA_1	1	1	1	1	0	0.6	0	0	0	0.75	0.75	0	0	0	0	0
	RARA_2	1	1	1	1	0	0	0.6	0	0	0.75	0.75	0	0	0	0	0
	RARA_3	1	1	1	1	0	0	0	0.6	0	0.75	0.75	0	0	0	0	0
	RARA_4	1	1	1	1	0	0	0	0	0.6	0.75	0.75	0	0	0	0	0
	RARA_5	1	1	1	0	1	0.6	0	0	0	0.75	0.75	0	0	0	0	0
	RARA_6	1	1	1	0	1	0	0.6	0	0	0.75	0.75	0	0	0	0	0
	RARA_7	1	1	1	0	1	0	0	0.6	0	0.75	0.75	0	0	0	0	0
	RARA_8	1	1	1	0	1	0	0	0	0.6	0.75	0.75	0	0	0	0	0
	RARA_9	1	1	1	0.6	0	0.6	0	0	0	1	1	0	0	0	0	0
	RARA_10	1	1	1	0.6	0	0	0.6	0	0	1	1	0	0	0	0	0
	RARA_11	1	1	1	0.6	0	0	0	0.6	0	1	1	0	0	0	0	0
	RARA_12	1	1	1	0.6	0	0	0	0	0.6	1	1	0	0	0	0	0
	RARA_13	1	1	1	0	0.6	0.6	0	0	0	1	1	0	0	0	0	0
	RARA_14	1	1	1	0	0.6	0	0.6	0	0	1	1	0	0	0	0	0
	RARA_15	1	1	1	0	0.6	0	0	0.6	0	1	1	0	0	0	0	0
	RARA_16	1	1	1	0	0.6	0	0	0	0.6	1	1	0	0	0	0	0
FREQ	FREQ_1	1	1	1	0.2	0	0.5	0	0	0	0.75	0.75	0	0	0	0	0
	FREQ_2	1	1	1	0.2	0	0	0.5	0	0	0.75	0.75	0	0	0	0	0
	FREQ_3	1	1	1	0.2	0	0	0	0.5	0	0.75	0.75	0	0	0	0	0
	FREQ_4	1	1	1	0.2	0	0	0	0	0.5	0.75	0.75	0	0	0	0	0
	FREQ_5	1	1	1	0	0	0.5	0	0	0	0.75	0.75	0	0	0	0	0
	FREQ_6	1	1	1	0	0	0	0.5	0	0	0.75	0.75	0	0	0	0	0
	FREQ_7	1	1	1	0	0	0	0	0.5	0	0.75	0.75	0	0	0	0	0
	FREQ_8	1	1	1	0	0	0	0	0	0.5	0.75	0.75	0	0	0	0	0
	FREQ_9	1	1	1	0.2	0	0.5	0	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_10	1	1	1	0.2	0	0	0.5	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_11	1	1	1	0.2	0	0	0.5	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_12	1	1	1	0.2	0	0	0	0	0.5	0.75	0.75	0.75	0.75	0	0	0
	FREQ_13	1	1	1	0	0	0.5	0	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_14	1	1	1	0	0	0	0.5	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_15	1	1	1	0	0	0	0	0	0.5	0.75	0.75	0.75	0.75	0	0	0
	FREQ_16	1	1	1	0	0	0	0	0	0.5	0.75	0.75	0.75	0.75	0	0	0

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

QP	QP_1	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0	0	0
	QP_2	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0	0	0
	QP_3	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0	0	0
	QP_4	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	0	0	0
SISMA SLV	SLV1	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	1	0.3	0
	SLV2	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-1	0.3	0
	SLV3	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	1	-0.3	0
	SLV4	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-1	-0.3	0
	SLV5	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0.3	1	0
	SLV6	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0.3	-1	0
	SLV7	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-0.3	1	0
	SLV8	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-0.3	-1	0
	SLV9	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	1	0.3	0
	SLV10	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-1	0.3	0
	SLV11	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	1	-0.3	0
	SLV12	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-1	-0.3	0
	SLV13	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0.3	1	0
	SLV14	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0.3	-1	0
	SLV15	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-0.3	1	0
	SLV16	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-0.3	-1	0
	SLV17	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	1	0.3	0
	SLV18	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-1	0.3	0
	SLV19	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	1	-0.3	0
	SLV20	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-1	-0.3	0
	SLV21	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0.3	1	0
	SLV22	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0.3	-1	0
	SLV23	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-0.3	1	0
	SLV24	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-0.3	-1	0
	SLV25	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	1	0.3	0
	SLV26	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	-1	0.3	0
	SLV27	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	1	-0.3	0
	SLV28	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	-1	-0.3	0
	SLV29	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	0.3	1	0
	SLV30	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	0.3	-1	0
	SLV31	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	-0.3	1	0
	SLV32	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	-0.3	-1	0
	SLV33	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0.3	0.3	1
	SLV34	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-0.3	0.3	1
	SLV35	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0.3	-0.3	1
	SLV36	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-0.3	-0.3	1
	SLV37	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0.3	0.3	1
	SLV38	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-0.3	0.3	1
	SLV39	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0.3	-0.3	1
	SLV40	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-0.3	-0.3	1
	SLV41	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0.3	0.3	1
	SLV42	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-0.3	0.3	1
	SLV43	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0.3	-0.3	1
	SLV44	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-0.3	-0.3	1
	SLV45	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0.3	0.3	-1
	SLV46	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-0.3	0.3	-1
	SLV47	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0.3	-0.3	-1
	SLV48	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	-0.3	-0.3	-1
	SLV49	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0.3	0.3	-1
	SLV50	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-0.3	0.3	-1
	SLV51	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0.3	-0.3	-1
	SLV52	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	-0.3	-0.3	-1
	SLV53	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0.3	0.3	-1
	SLV54	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-0.3	0.3	-1
	SLV55	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0.3	-0.3	-1
	SLV56	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	-0.3	-0.3	-1
SISMA SLD	SLD1	1	1	1	0	0	0.5	0	0	0	0	0	0	0	1	0	0	
	SLD2	1	1	1	0	0	0.5	0	0	0	0	0	0	0	-1	0	0	
	SLD3	1	1	1	0	0	0.5	0	0	0	0	0	0	0	1	0	0	
	SLD4	1	1	1	0	0	0.5	0	0	0	0	0	0	0	-1	0	0	

## 6.2 VERIFICHE STRUTTURALI

### 6.2.1 VERIFICA ELEMENTI DI IMPALCATO

Si riportano di seguito alcune immagini del modello 3D effettuato con la suddivisione in conci delle travi principali.

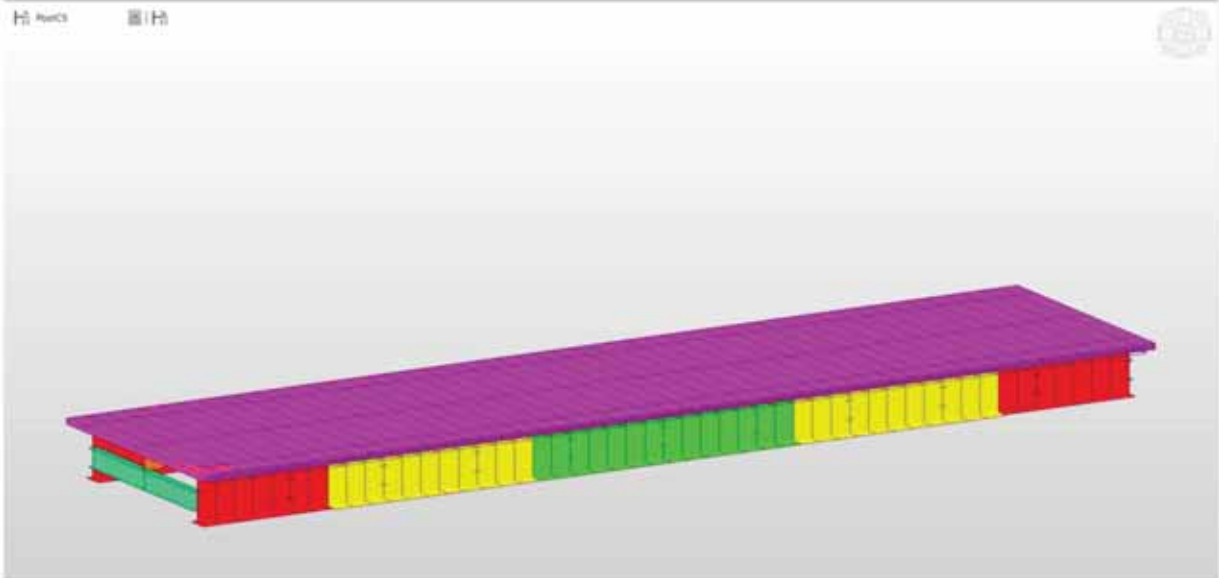


Figura 17 - Immagine modello 3D

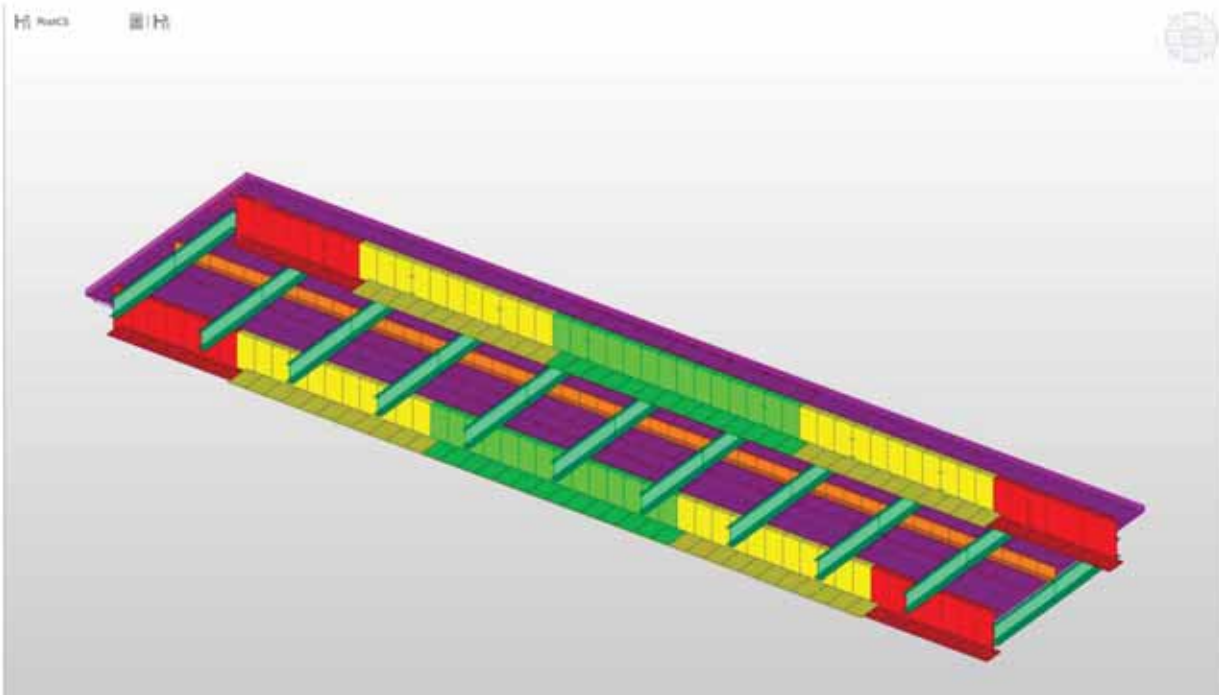


Figura 18 - Immagine modello 3D



Si riportano inoltre i diagrammi delle sollecitazioni M, N e T nelle varie fasi operative.

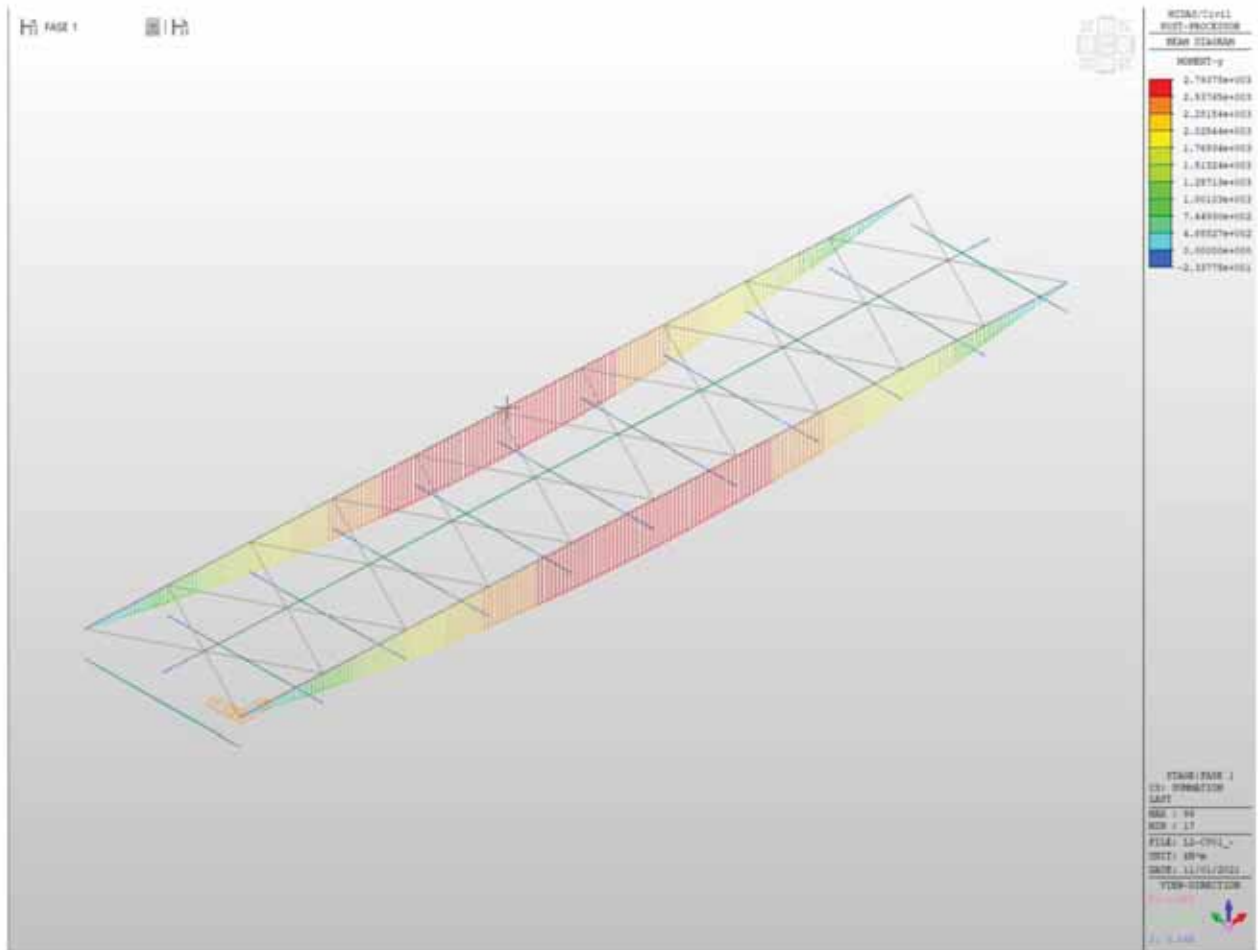


Figura 19 - Diagramma  $M_y$  - Fase 1

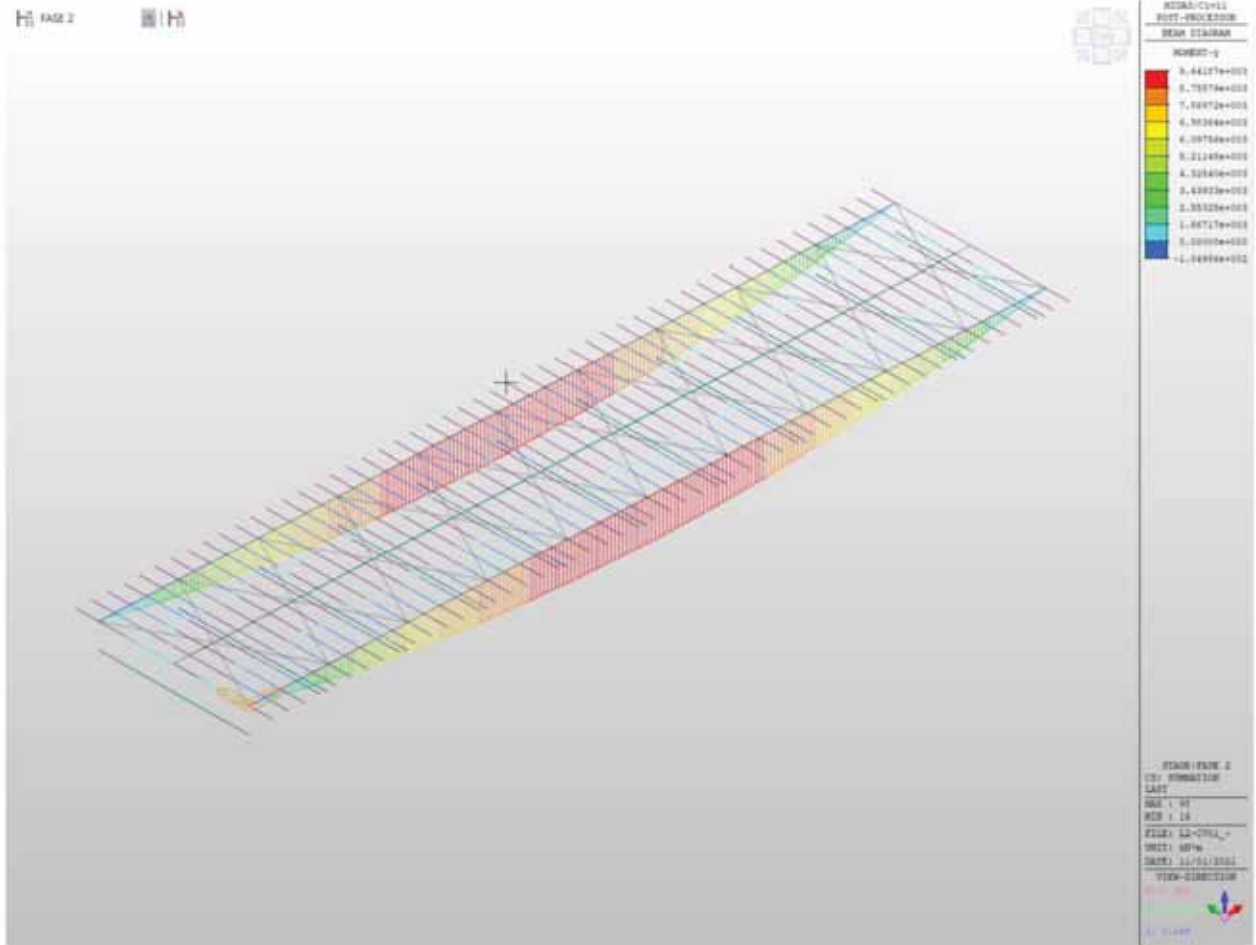


Figura 20 - Diagramma My - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

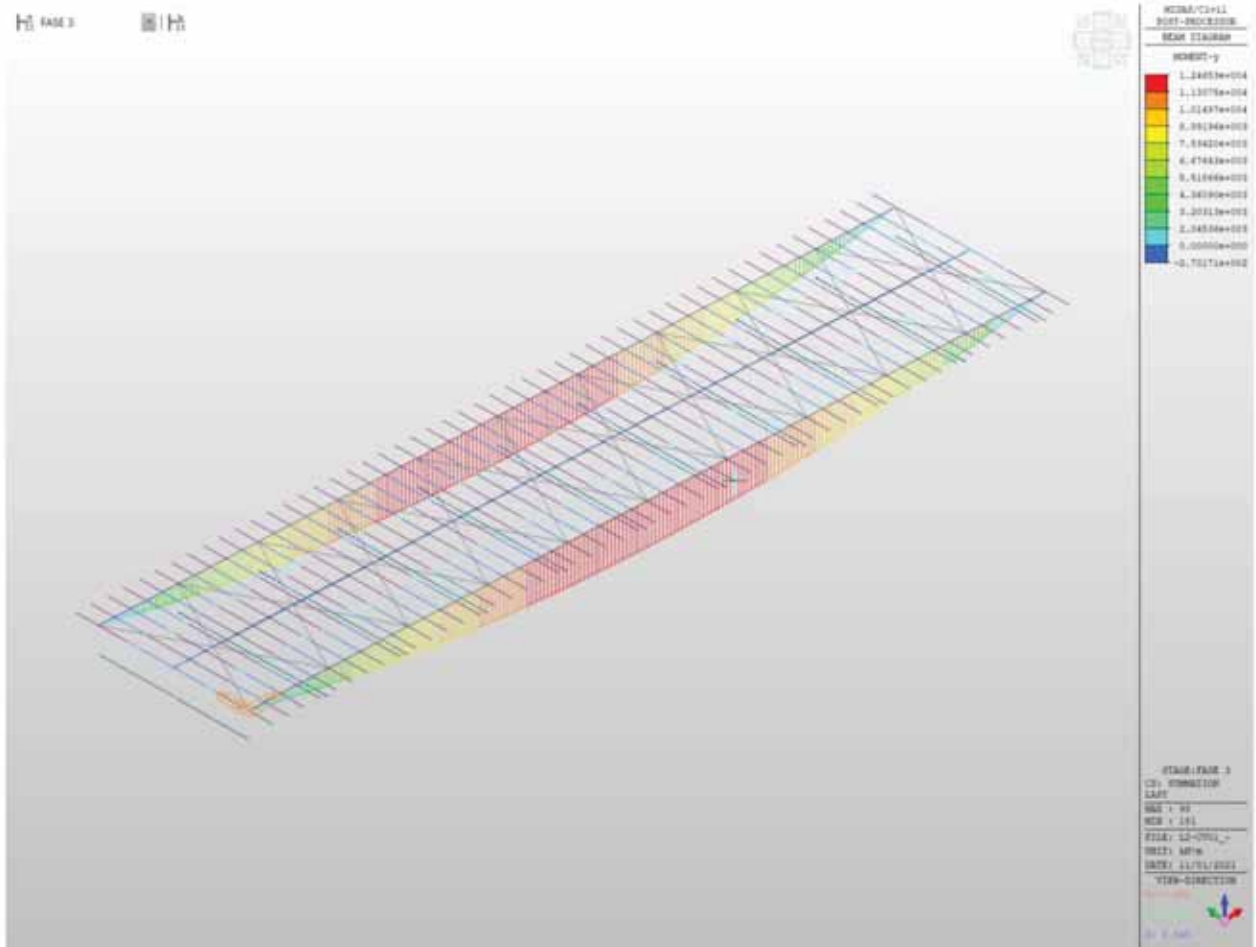


Figura 21 - Diagramma My - Fase 3



**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

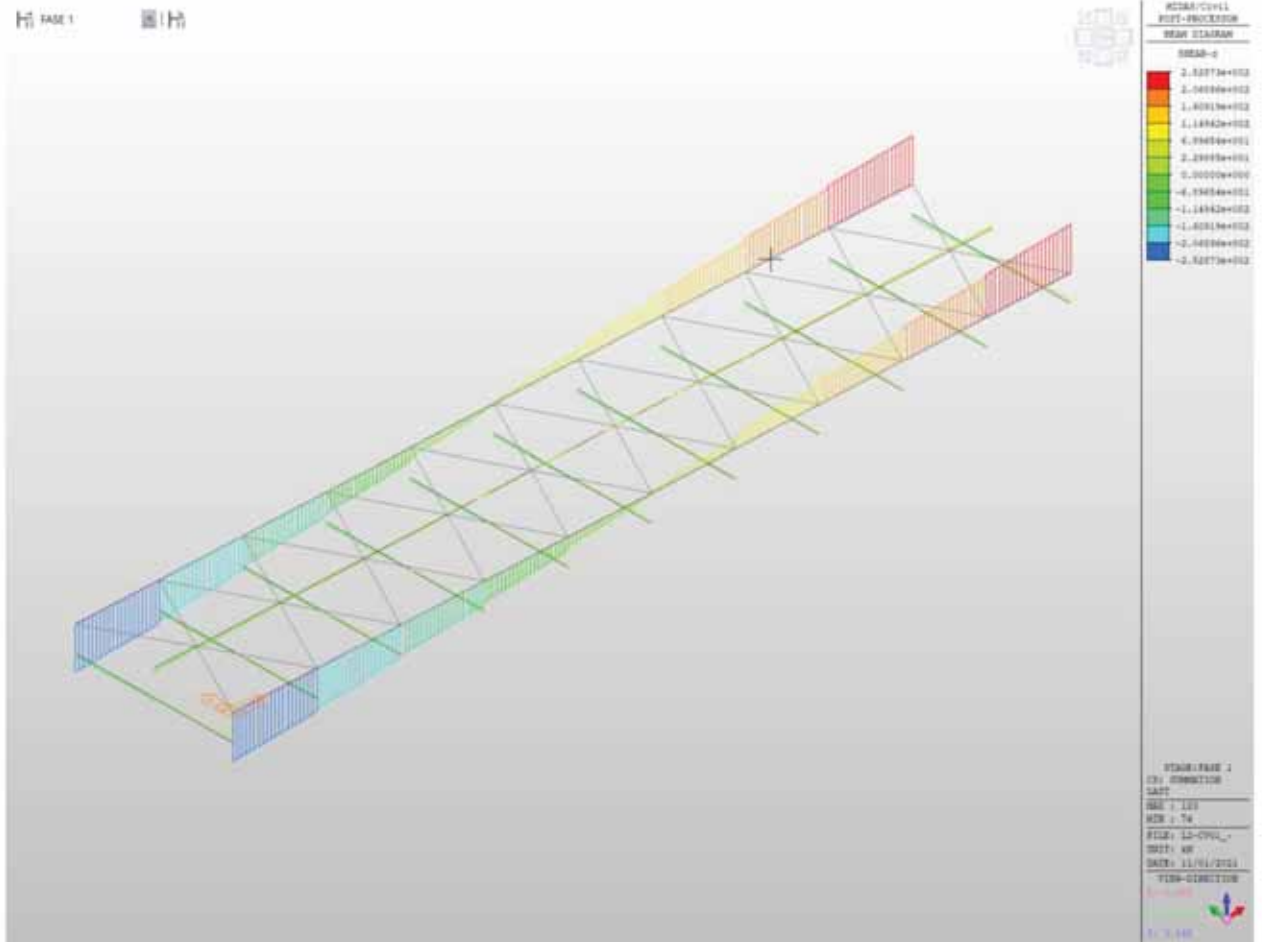


Figura 23 - Diagramma Tz - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

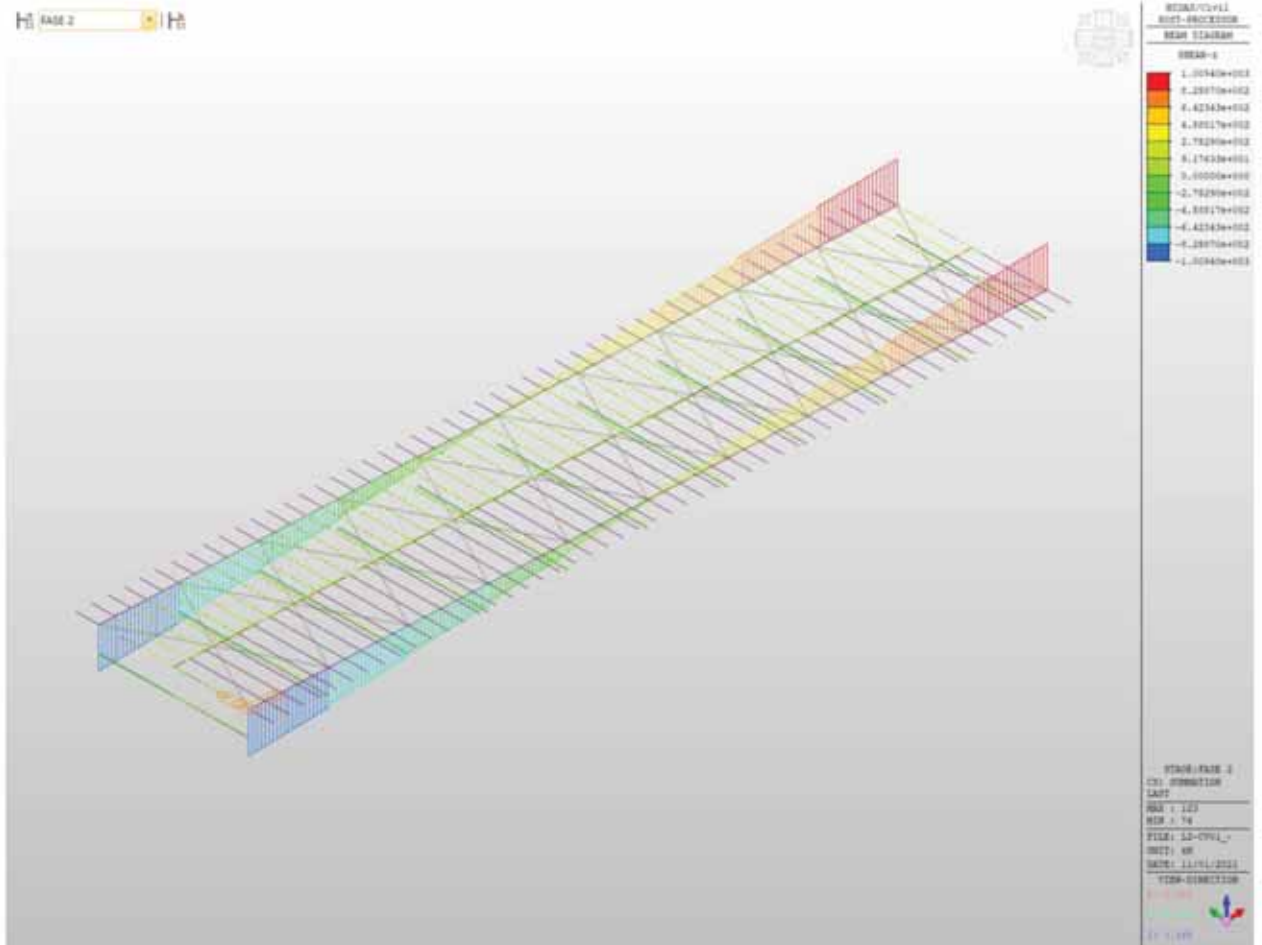


Figura 24 - Diagramma Tz - Fase 2

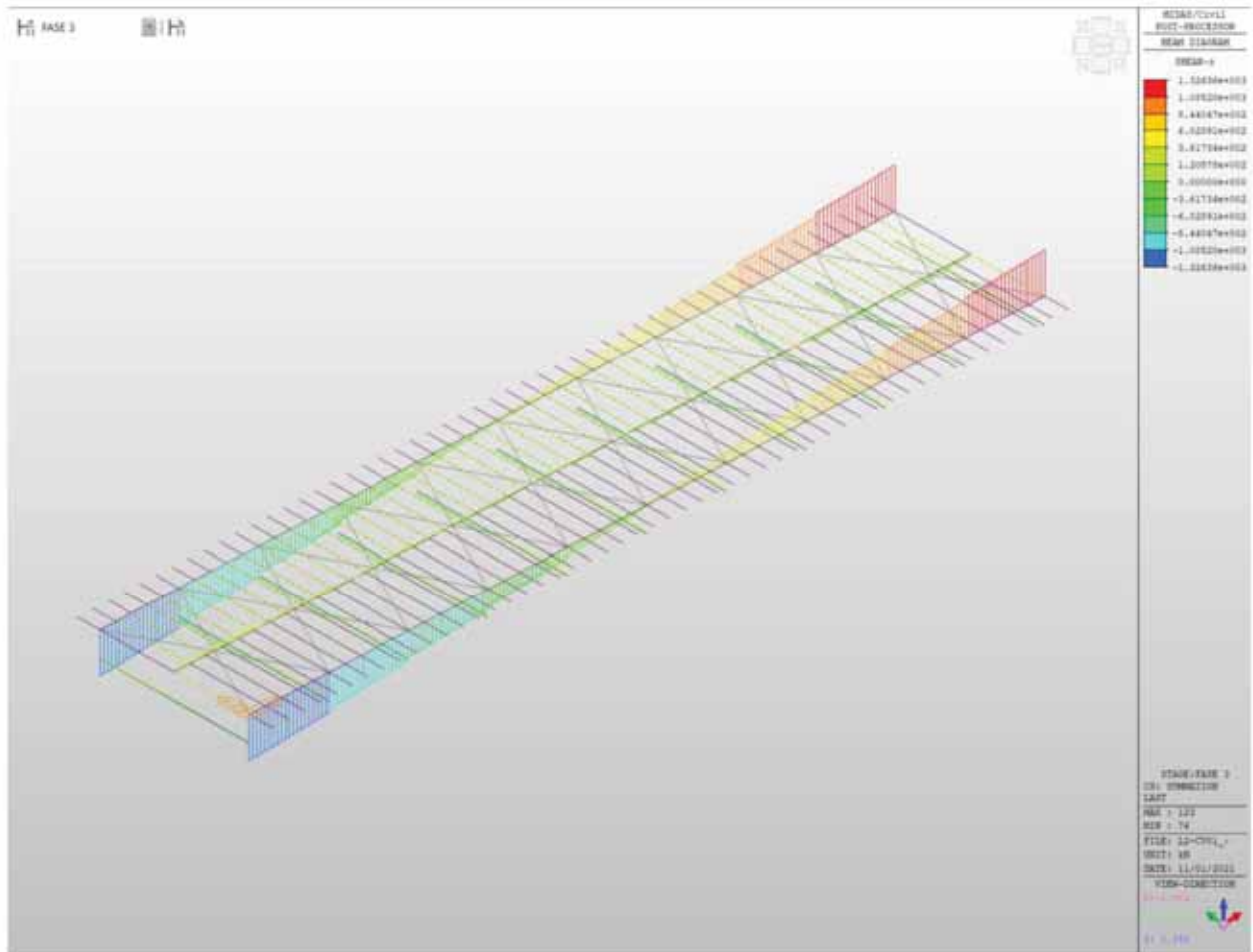


Figura 25 - Diagramma Tz - Fase 3

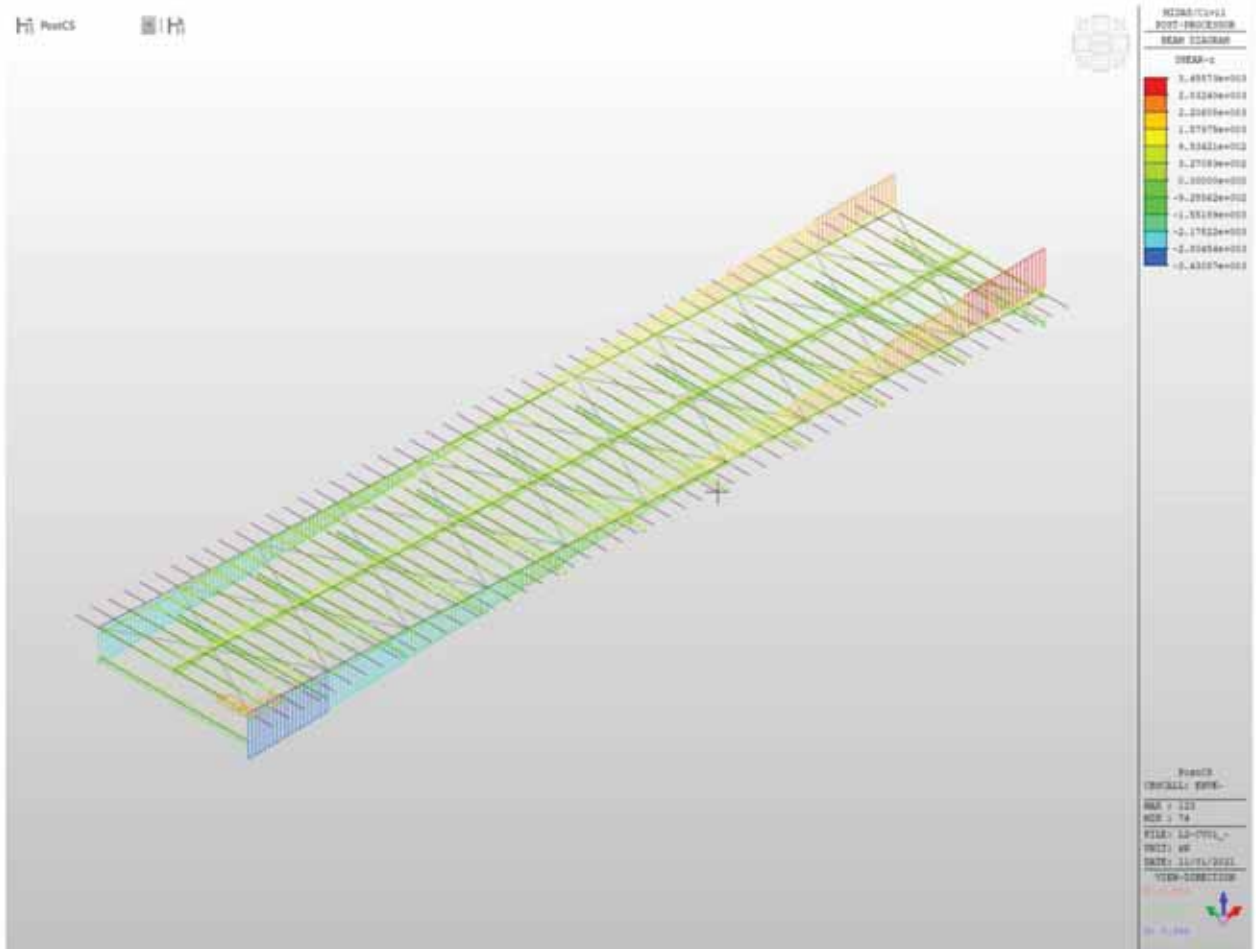


Figura 26 - Diagramma Tz - Involuppo SLU-SLV



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

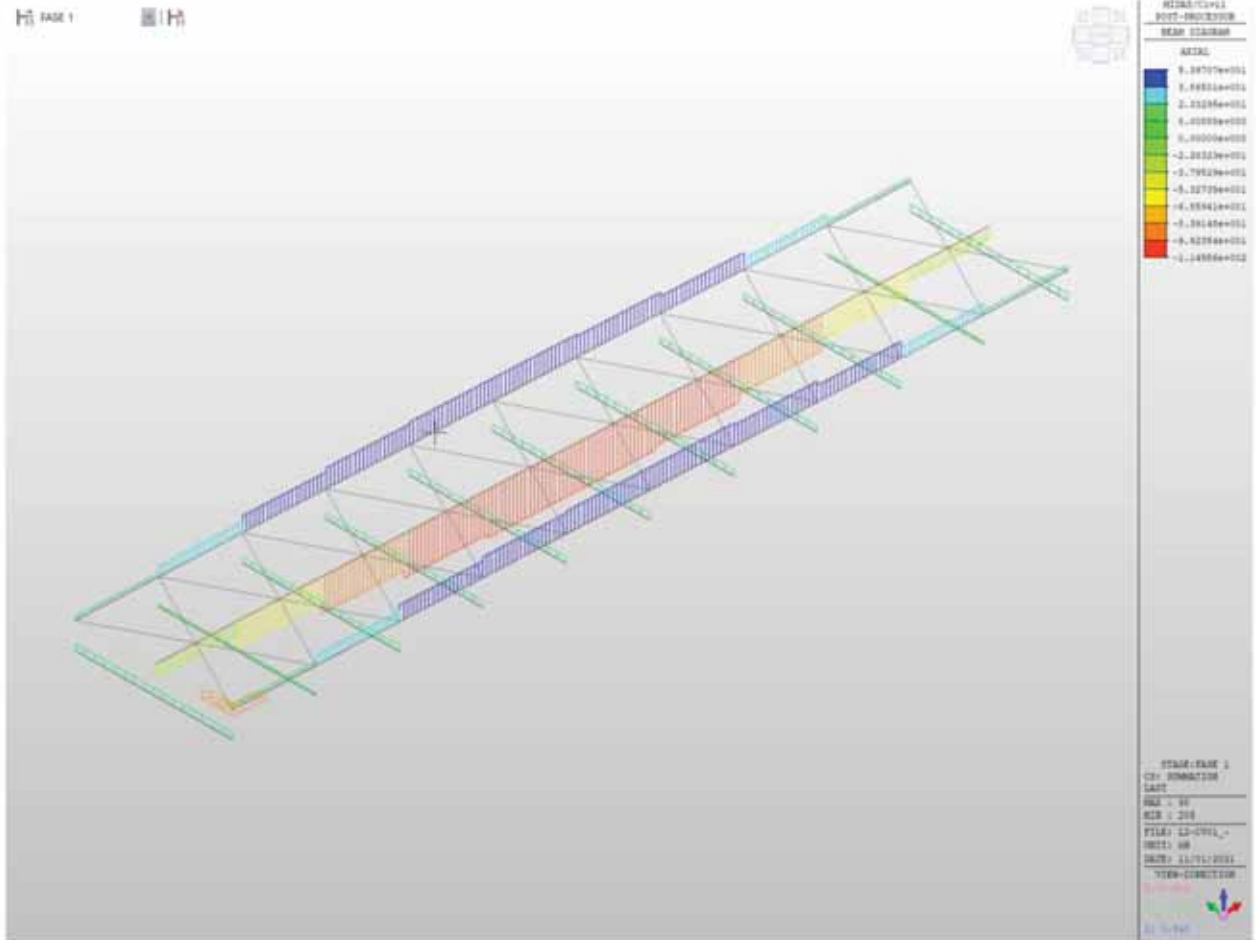


Figura 27 - Diagramma N - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

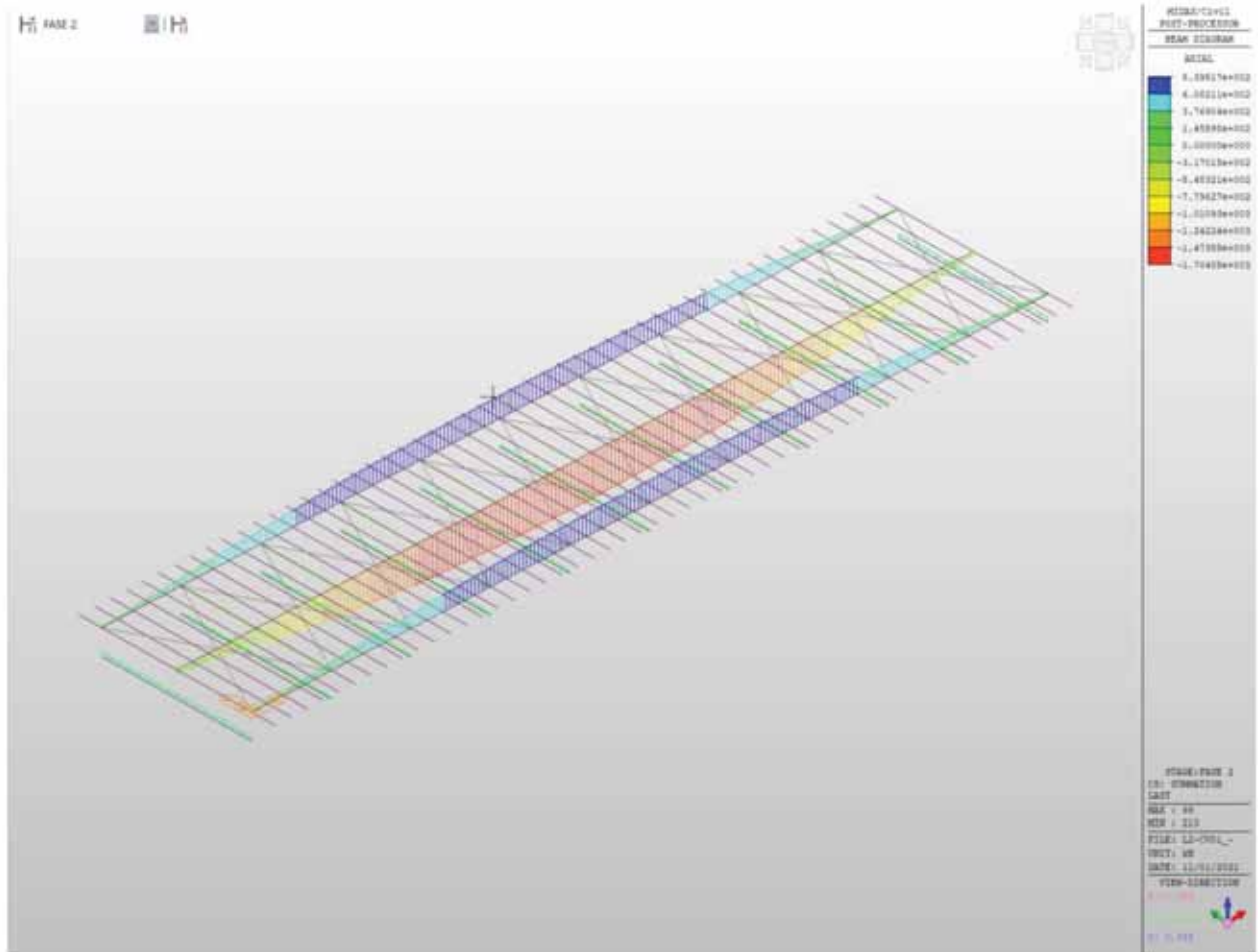


Figura 28 - Diagramma N - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

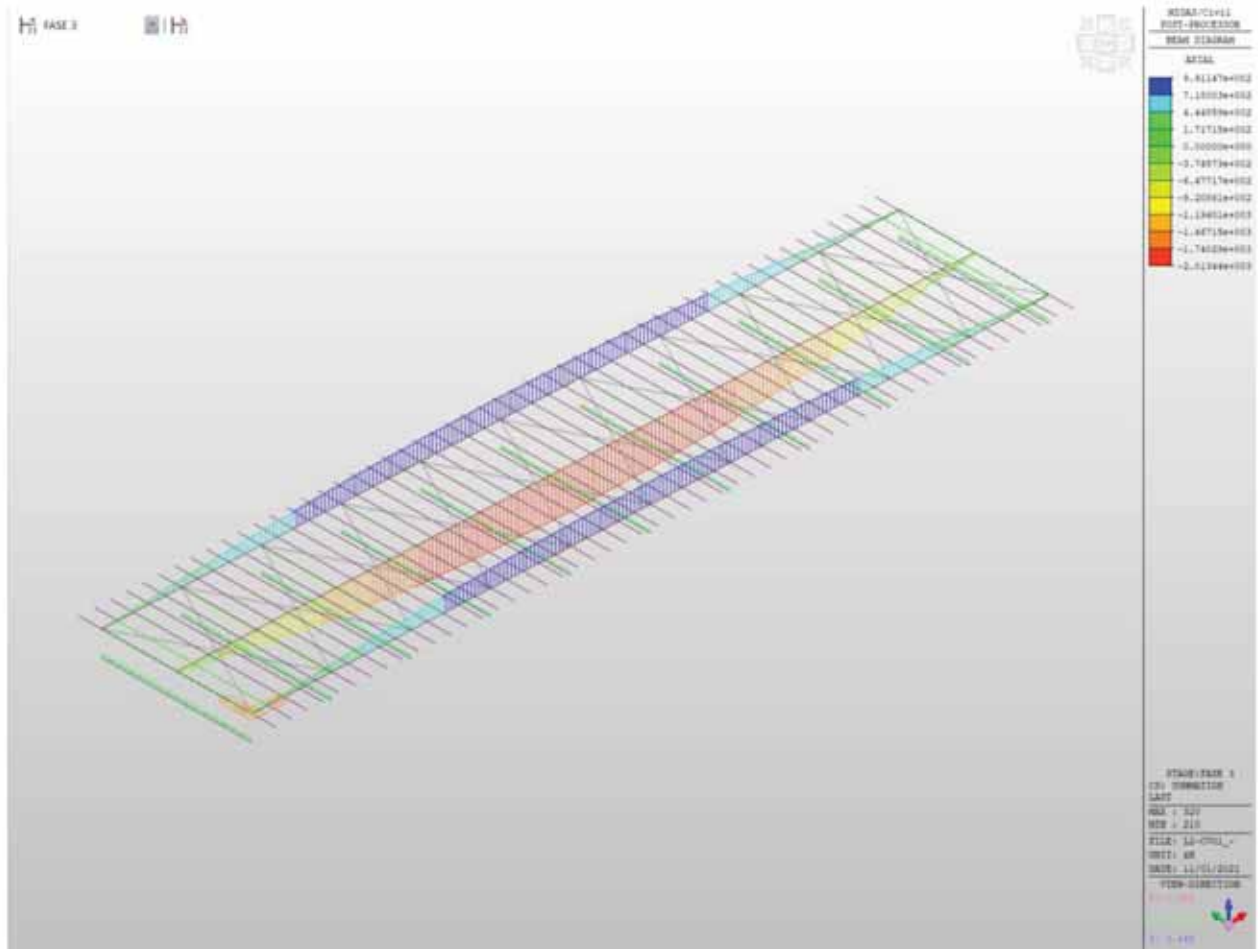


Figura 29 - Diagramma N - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

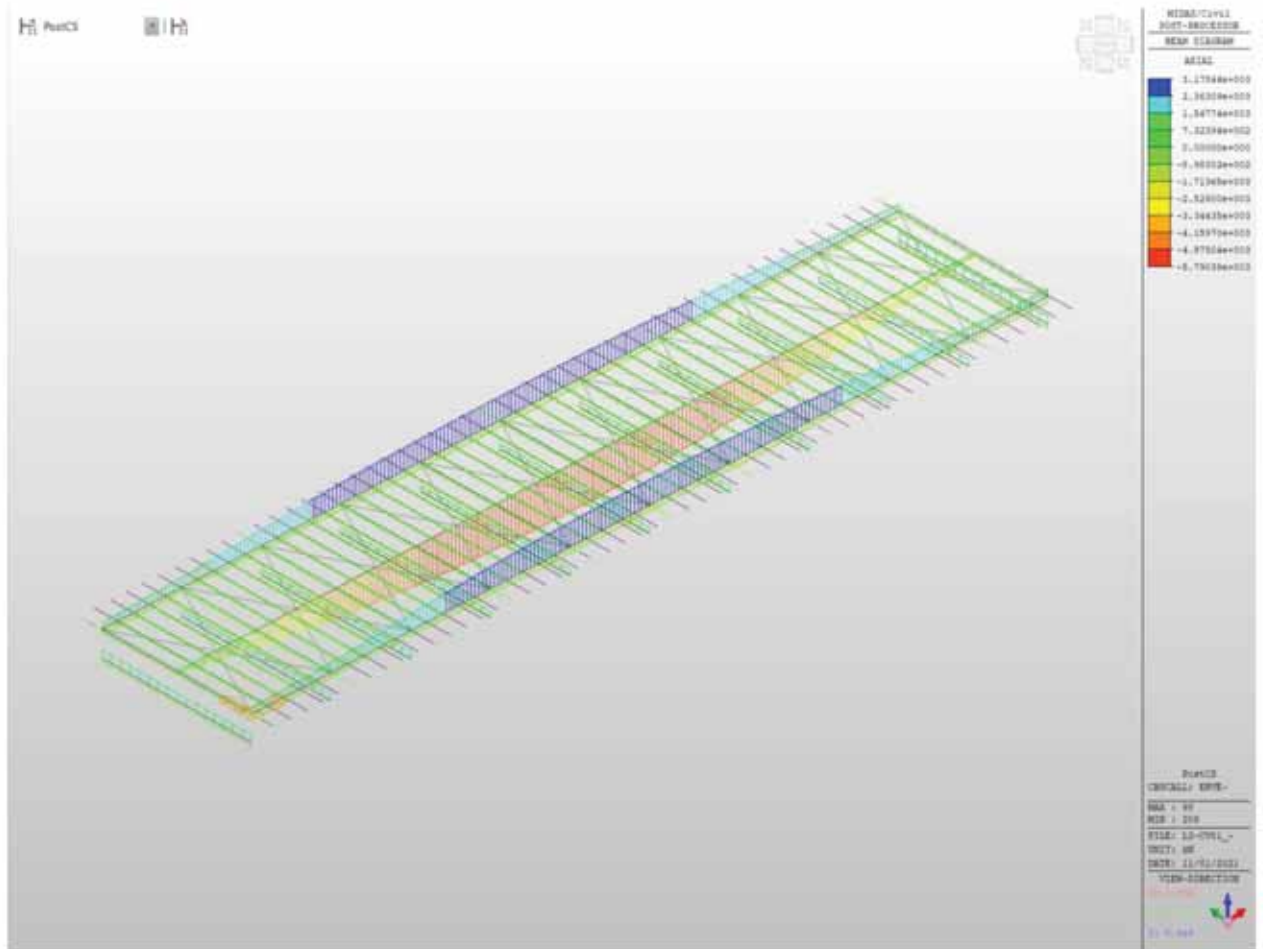


Figura 30 - Diagramma N - Involuppo SLU-SLV

**6.2.2 TRAVE PRINCIPALE CONCIO 1**

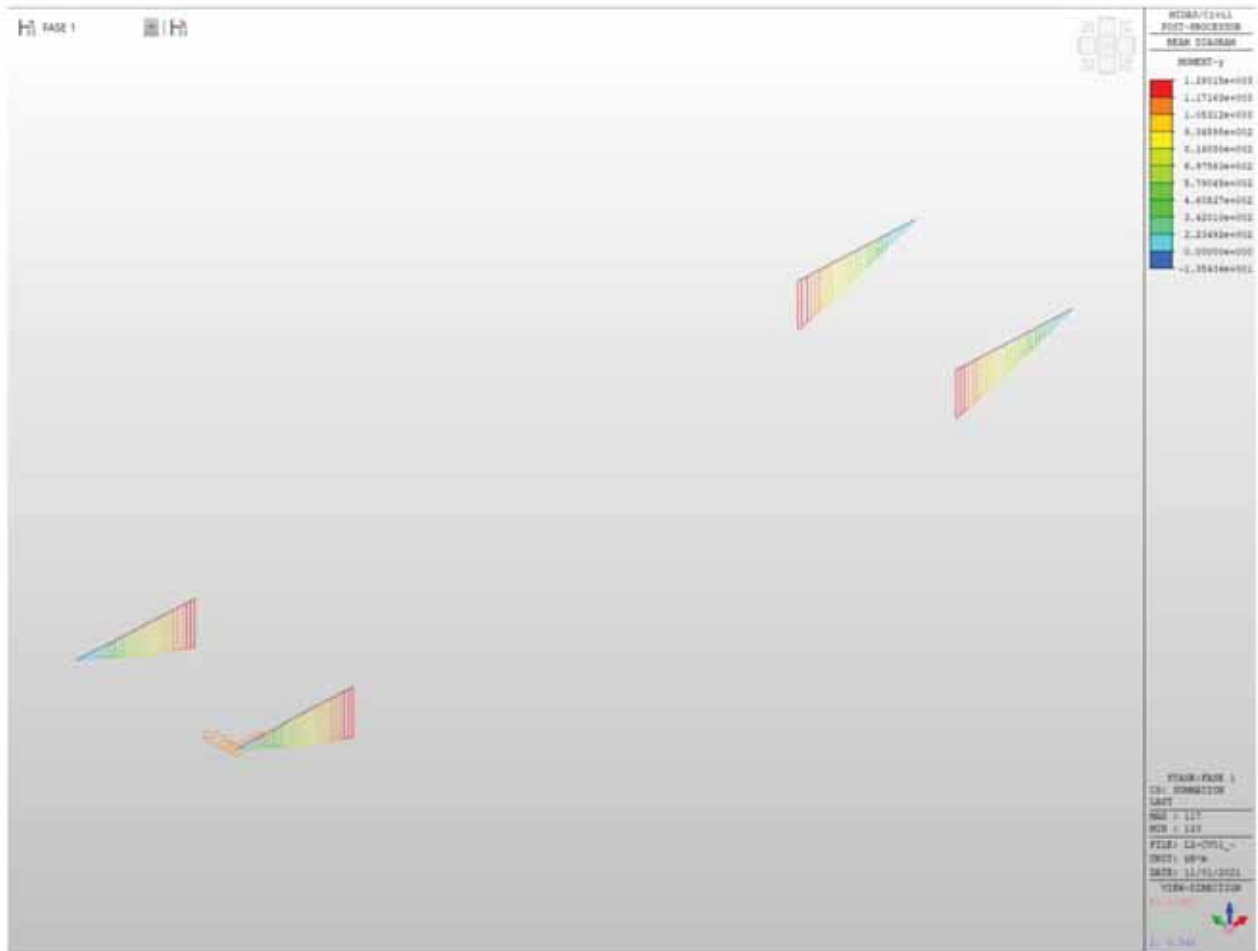


Figura 31 - Concio 1 - Diagramma My - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**



Figura 32 - Concio 1 - Diagramma My - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

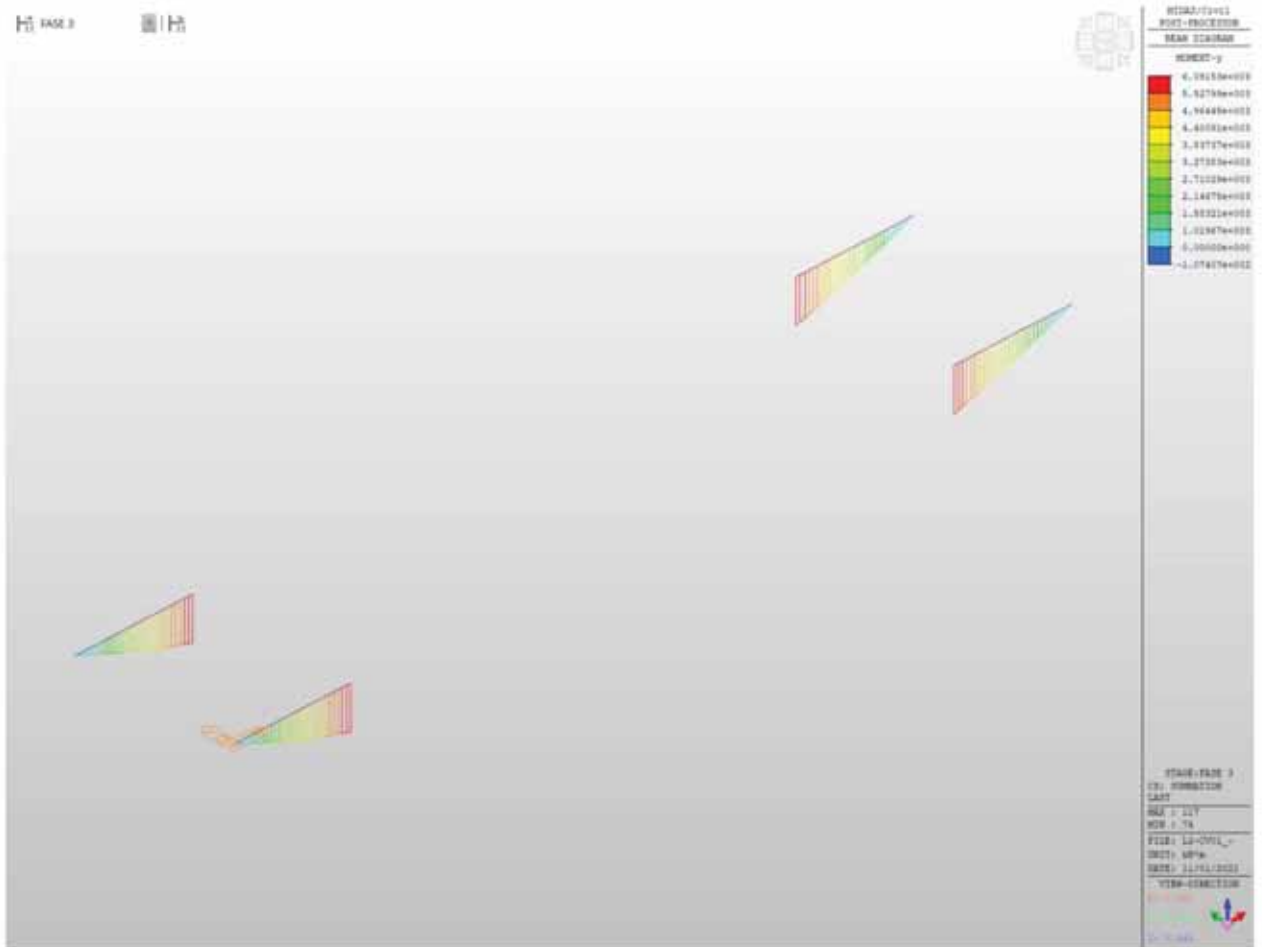


Figura 33 - Concio 1 - Diagramma My - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

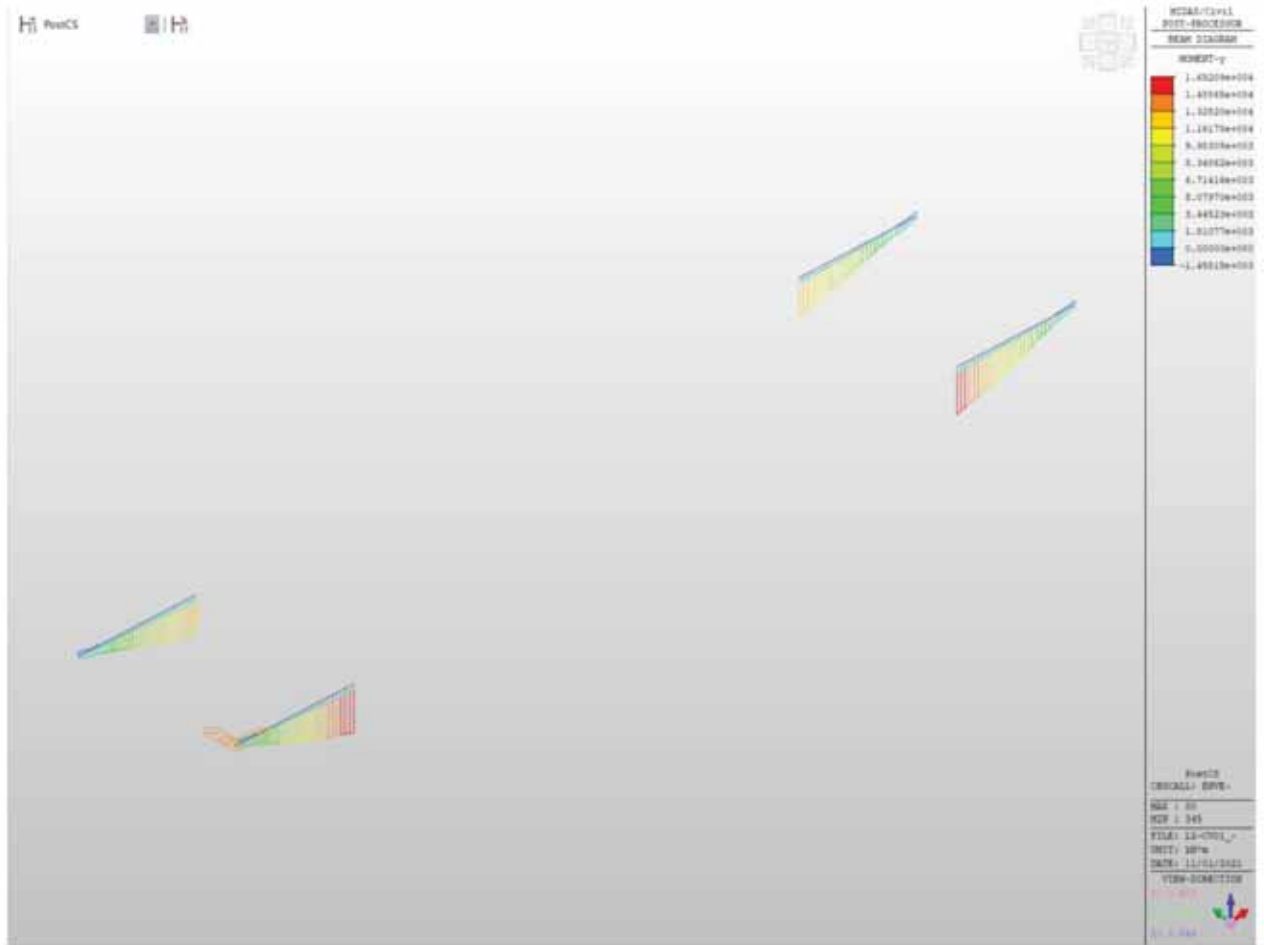


Figura 34 - Concio 1 - Diagramma My - Involuppo SLU-SLV



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

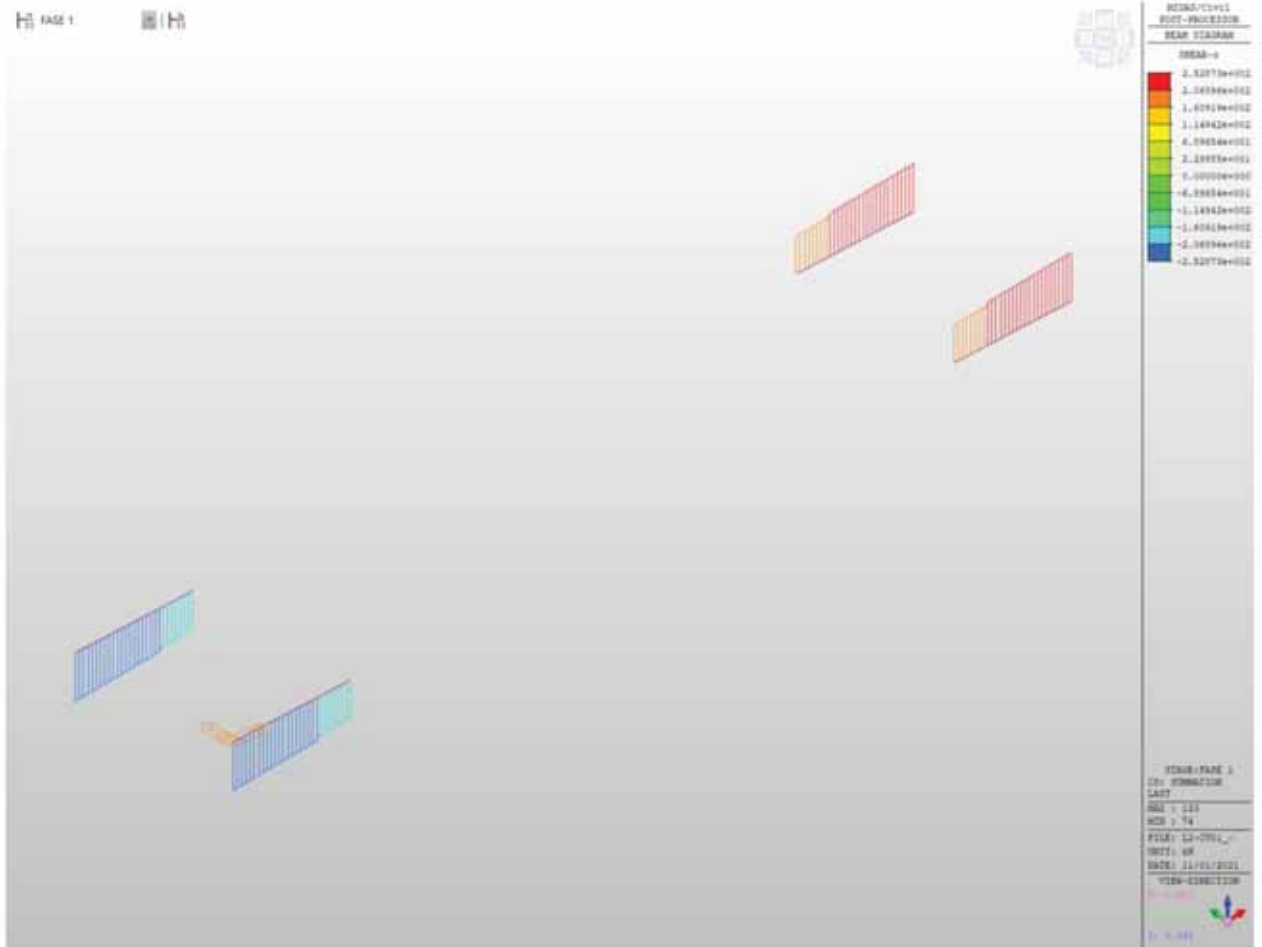


Figura 35 - Concio 1 - Diagramma Tz - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

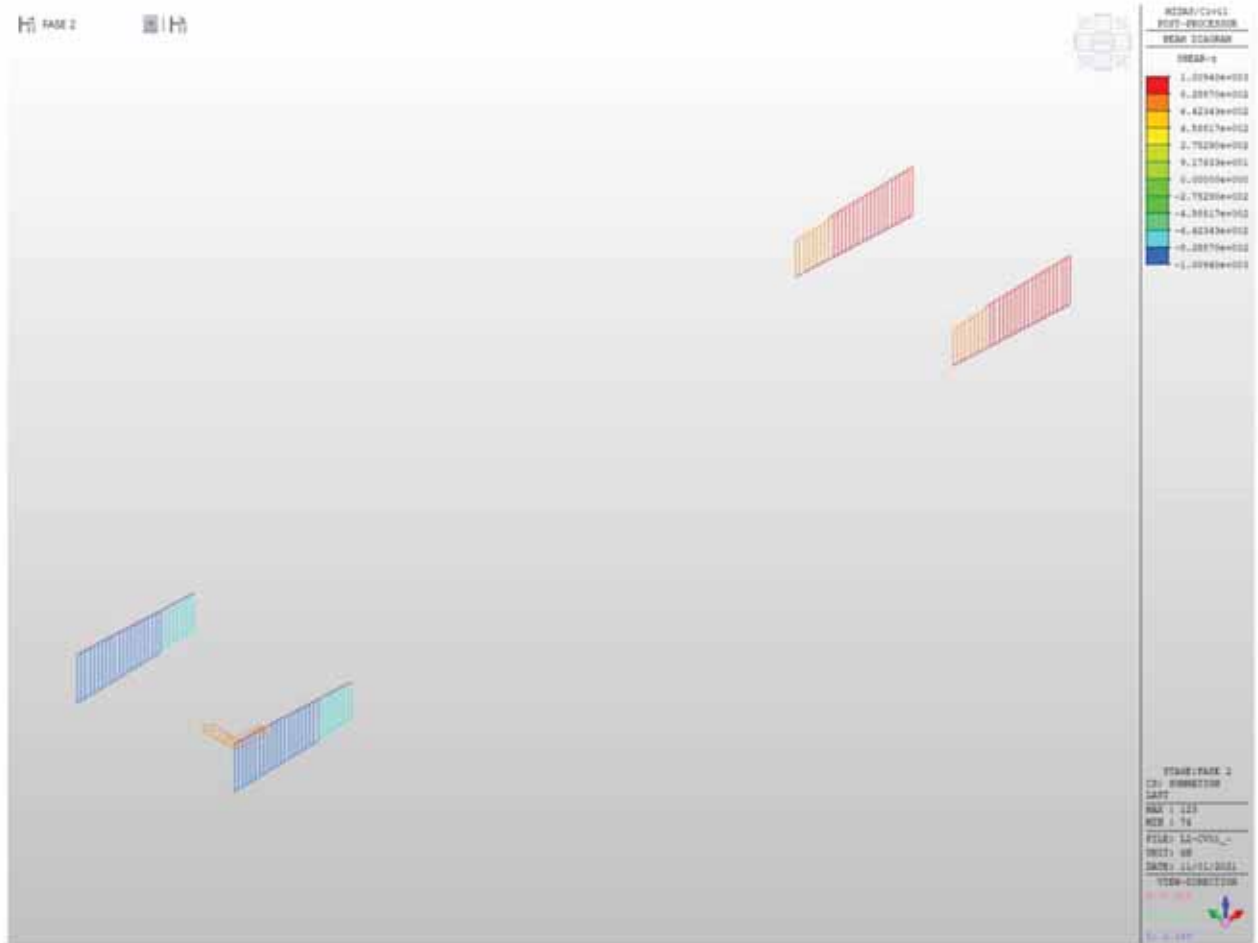


Figura 36 - Concio 1 - Diagramma Tz - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

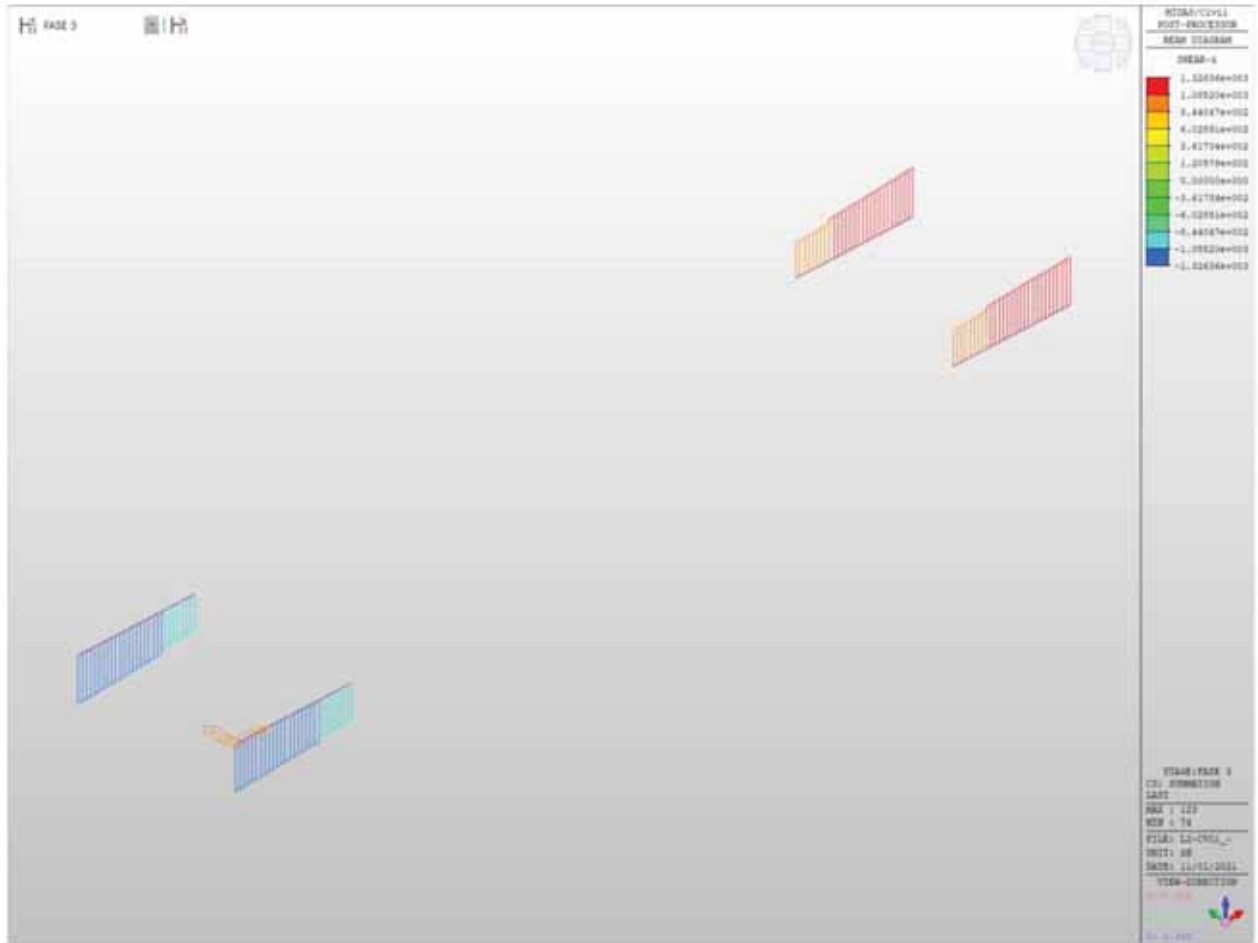


Figura 37 - Concio 1 - Diagramma Tz - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

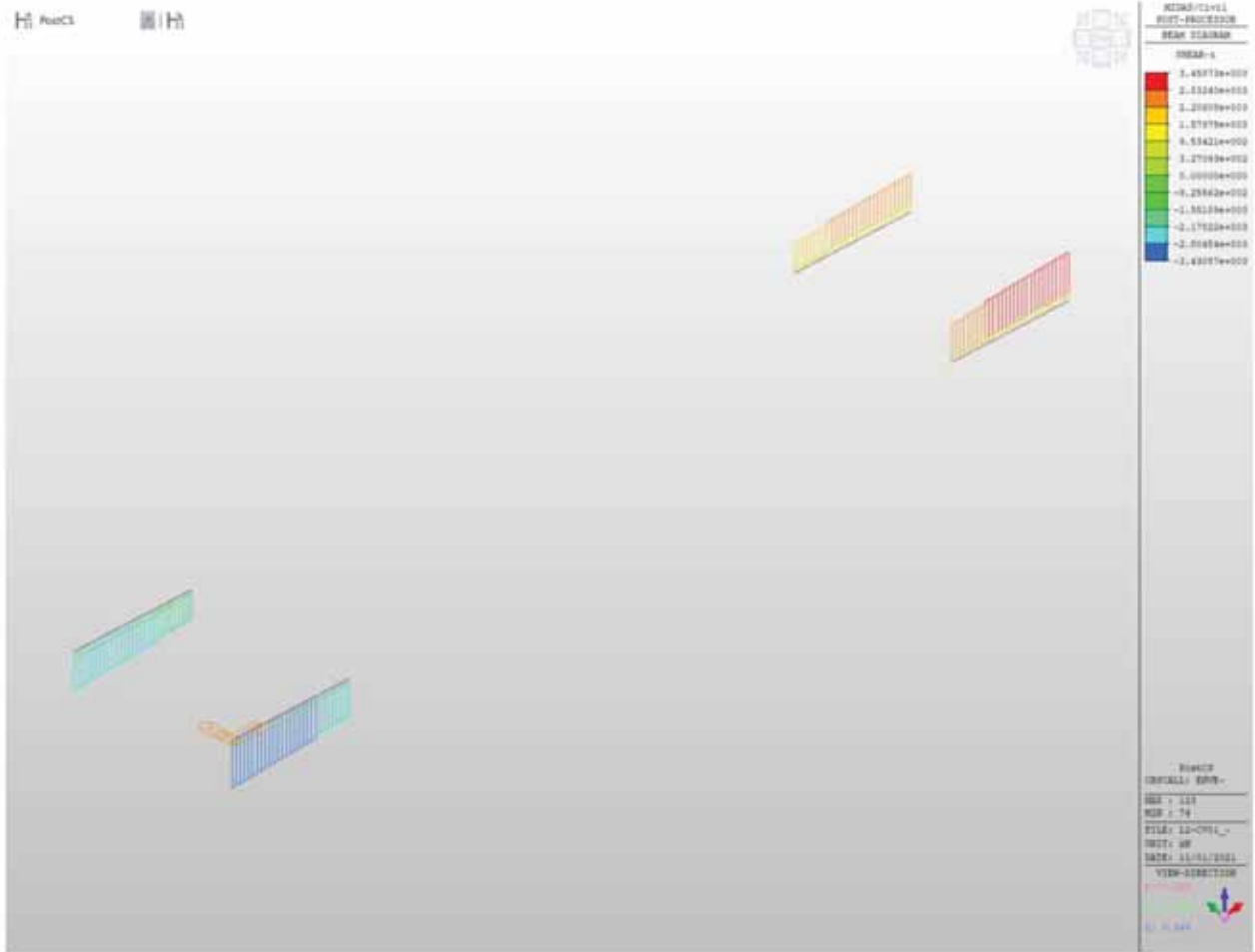


Figura 38 - Concio 1 - Diagramma Tz - Involuppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

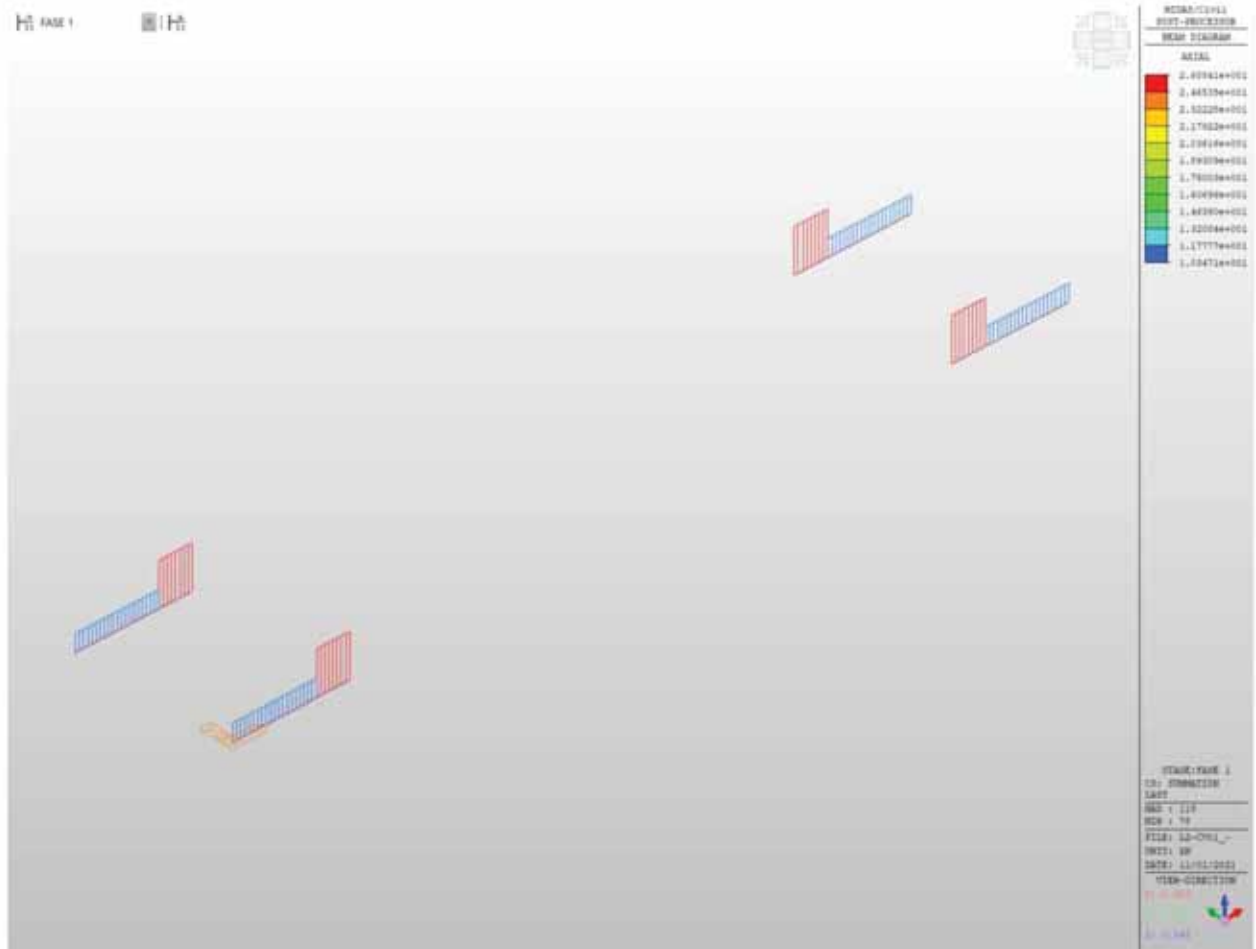


Figura 39 - Concio 1 - Diagramma N - Fase 1

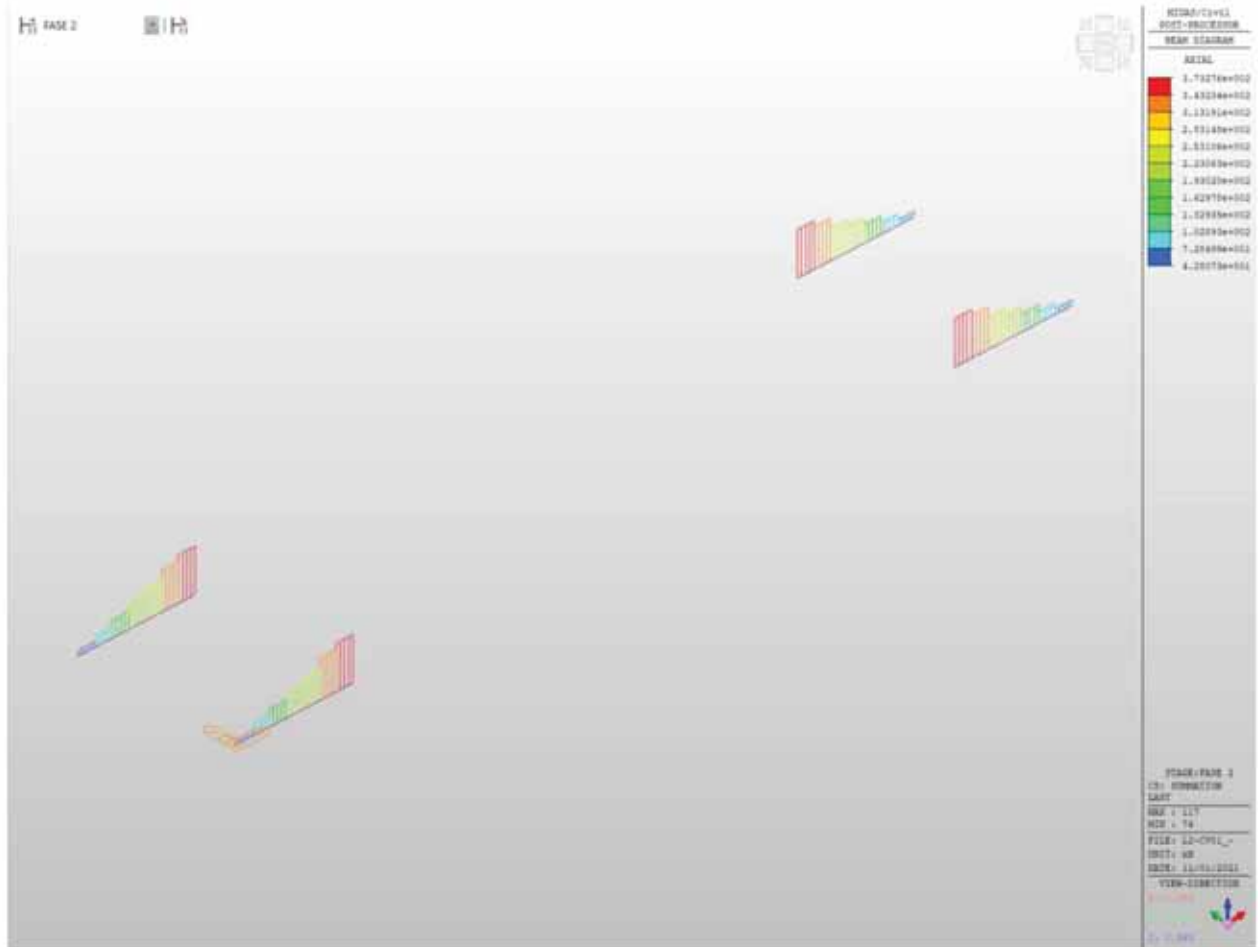
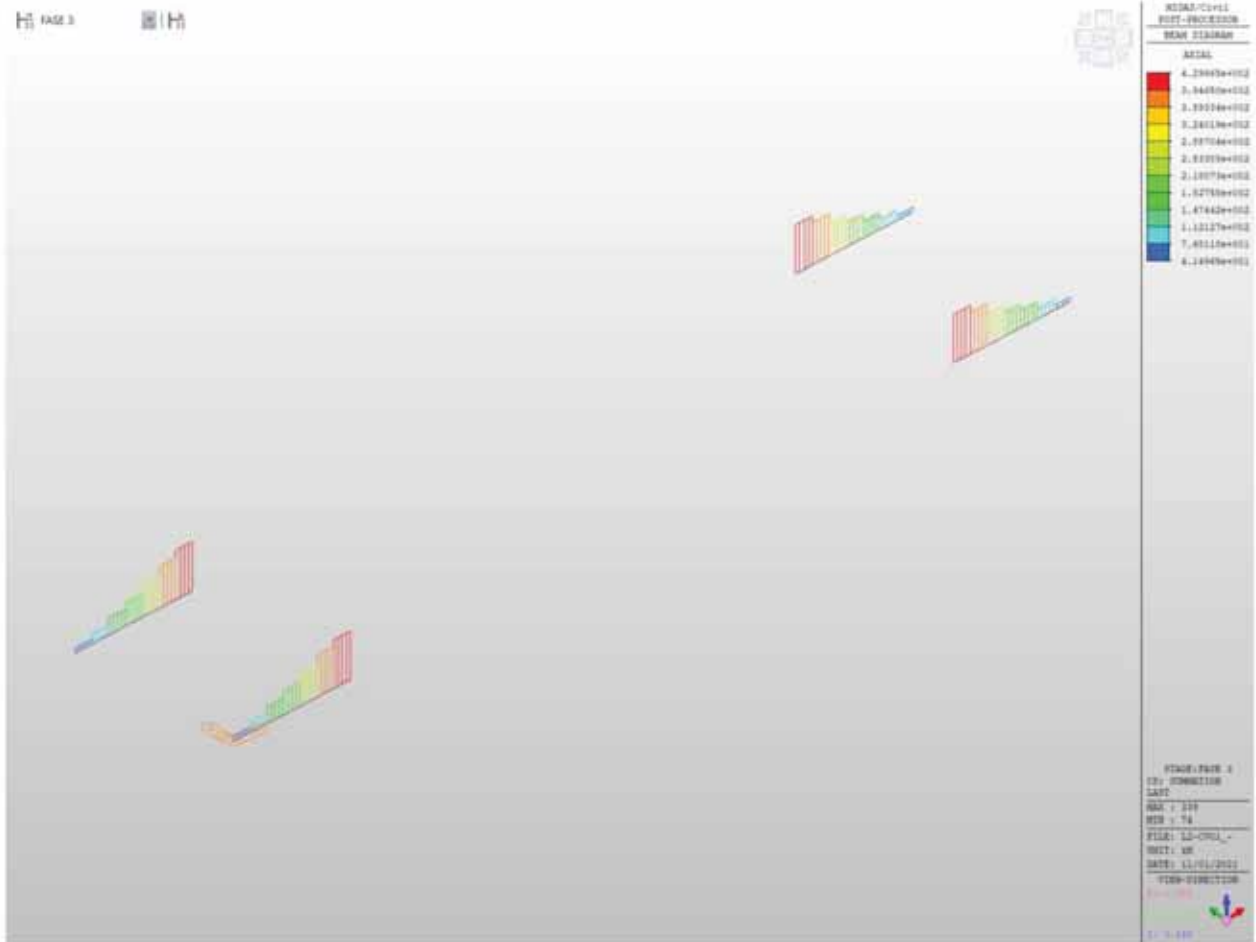


Figura 40 - Concio 1 - Diagramma N - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

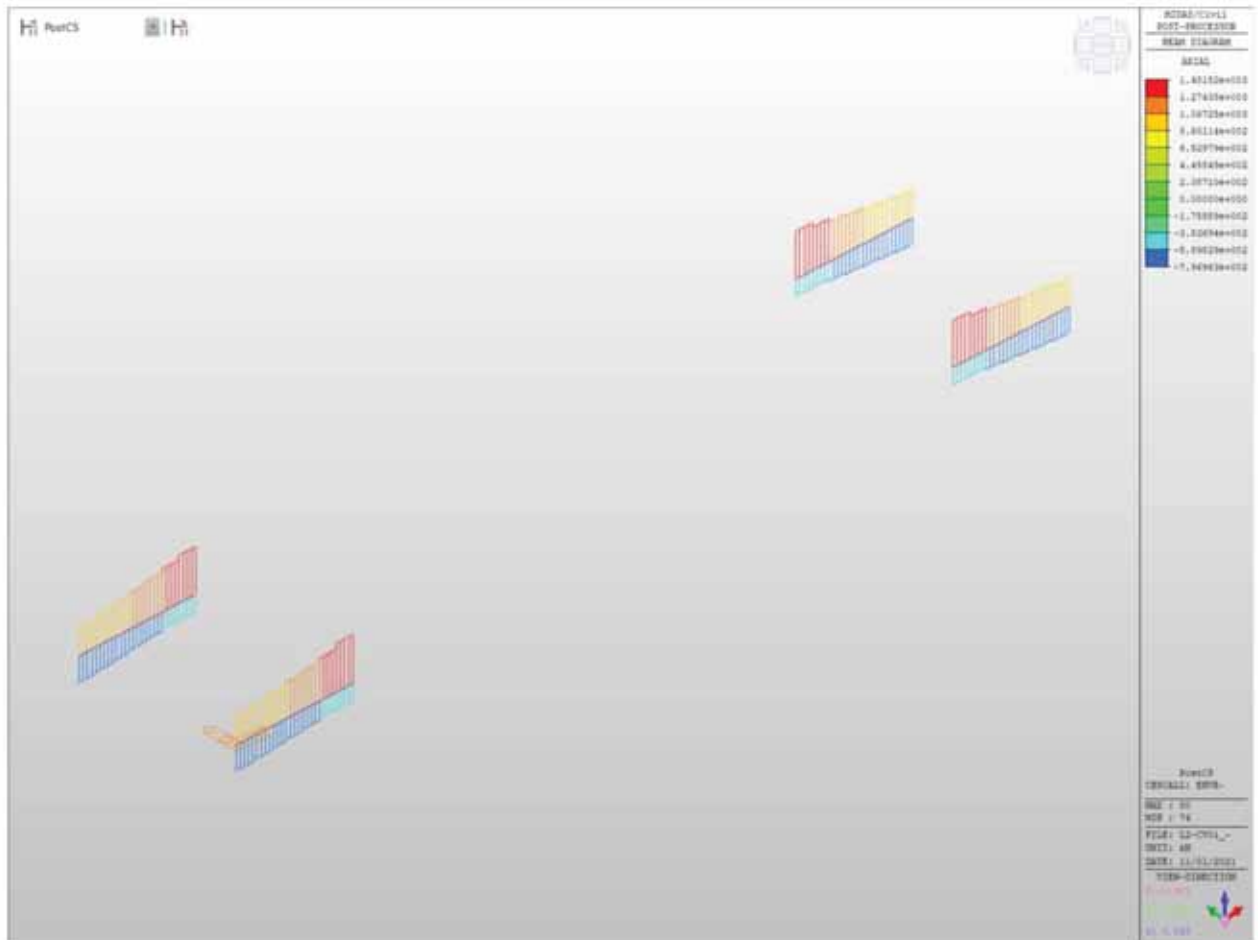
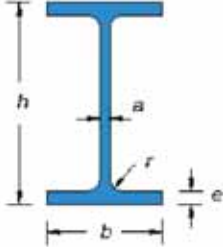


Figura 42 - Concio 1 - Diagramma N - Inviluppo SLU-SLV



VERIFICA SEZIONE IN ACCIAIO COMPOSTA

**CARATTERISTICHE PROFILO:**

Tipo: <input type="text" value="S355"/>					
B <sub>SUP</sub> =	700	mm		A <sub>r,sup</sub> =	14000.00 mm <sup>2</sup>
t <sub>r,sup</sub> =	20.0	mm			
B <sub>INF</sub> =	900	mm		A <sub>r,inf</sub> =	31500.00 mm <sup>2</sup>
t <sub>r,inf</sub> =	35.0	mm			
H =	2000	mm		A <sub>w</sub> =	38900.00 mm <sup>2</sup>
t <sub>w</sub> = a =	20.0	mm		n. traversi:	1
L =	4100	mm			
A <sub>TOT</sub> =	84400.00	mm <sup>2</sup>			
Y <sub>G,inf</sub> =	800.98	mm		Y <sub>G,sup</sub> =	1199.02 mm
J <sub>yy</sub> =	5.31E+10	mm <sup>4</sup>		J <sub>r</sub> =	19915833 mm <sup>4</sup>
J <sub>zz</sub> =	2699250000	mm <sup>4</sup>		J <sub>u</sub> =	1.069E+09 mm <sup>6</sup>
W <sub>el,yy,inf</sub> =	66237129	mm <sup>3</sup>		W <sub>el,yy,sup</sub> =	44248630 mm <sup>3</sup>
W <sub>pl,yy</sub> =	61094067	mm <sup>3</sup>			
W <sub>pl,xx</sub> =	$2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$				
ρ <sub>yy</sub> =	792.85	mm		ρ <sub>xx</sub> =	$\sqrt{\frac{J_{xx}}{A}}$
ρ <sub>zz</sub> =	178.83	mm			
Peso =	662.54	kg/m			
E =	210000	N/mm <sup>2</sup>	modulo elastico		
ν =	0.3			coeff. poisson	
G =	80769	N/mm <sup>2</sup>		G =	$\frac{E}{2(1 + \nu)}$ modulo elasticità trasversale
α =	0.000012	°C <sup>-1</sup>	coeff. espansione termica lineare		
f <sub>yk</sub> =	355	N/mm <sup>2</sup>	tensione snervamento caratteristica		
f <sub>sk</sub> =	510	N/mm <sup>2</sup>	tensione rottura caratteristica		
γ <sub>M0</sub> =	1.05		per sezioni classe 1,2,3 e 4		
γ <sub>M1</sub> =	1.05		per instabilità membrature		
γ <sub>M1</sub> =	1.10		per instabilità membrature ponti stradali e ferroviari		
γ <sub>M2</sub> =	1.25		per sezioni tese indebolite dai fori nei riguardi della frattura		

**SOLLECITAZIONI:**

N <sub>t,td</sub> =	0	N	caso per trave tesa
N <sub>c,td</sub> =	374000	N	caso per trave compressa
M <sub>td</sub> =	4638000000	Nmm	caso per trave inflessa in una direzione
V <sub>td</sub> =	1001000	N	taglio
Caso per trave a flessione deviata (Momenti flettenti agenti alle estremità  M <sub>B</sub>   <  M <sub>A</sub>  ):			
M <sub>B</sub> =	3344000000	Nmm	M <sub>A</sub> = 4638000000 Nmm
Valori del momento massimo e minimo dell'asta:			
M <sub>max,y,td</sub> =	0	Nmm	M <sub>max,y,td</sub> = 0 Nmm
M <sub>max,z,td</sub> =	0	Nmm	M <sub>max,z,td</sub> = 0 Nmm

LOTTO 1 - CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1):					
<b>ALA SUPERIORE</b>		<b>ALA INFERIORE</b>		<b>ANIMA</b>	
$C/t_{i,sup} =$	17.00	$C/t_{i,inf} =$	12.57	$C/t_w =$	97.25
$\varepsilon =$	0.81	$\varepsilon =$	0.81	$\varepsilon =$	0.81
Curva di instabilità "b" (Asse yy):					
Fattore di imperfezione $\alpha:$		0.34			
Curva di instabilità "c" (Asse zz):					
Fattore di imperfezione a:		0.49			

SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):					
$\frac{V_{sd}}{V_{c,Rd}} \leq 1$					
Area taglio resistente:		$A_v =$	39450	mm <sup>2</sup>	
$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}}$	$V_{c,Rd} =$	7700615	N	$\Rightarrow$	0.13 <b>OK!(&lt;1)</b>
Verifica in presenza di torsione uniforme:					
$\tau_{t,sd} =$	0	N/mm <sup>2</sup>			
$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,sd}}{f_{yk}} \frac{1}{\sqrt{3} \gamma_{M0}} \right] V_{c,Rd}$					
$V_{c,Rd,red} =$	7700615	N	$\Rightarrow$	0.13	<b>OK!(&lt;1)</b>
Verifica in termini tensionali nel punto più sollecitato:					
$\frac{\tau_{sd}}{f_{yk}} \frac{1}{\sqrt{3} \gamma_{M0}} \leq 1 \Rightarrow 0.00$ <b>OK!(&lt;1)</b>					

SOLLECITAZIONE DELL'ASTA A COMPRESSIONE SEMPLICE (NTC2008, 4.2.4.1.2):					
$\frac{N_{sd}}{N_{c,Rd}} \leq 1$	$N_{cr} = \frac{\pi^2 EJ}{(\eta l)^2}$	332808521	N	$N_{c,sd} < 0,04 N_{cr}$	<b>Instabilità trascurabile!</b>
0.04 Ncr =		13312341 N	<b>Instabilità trascurabile se <math>\lambda &lt; 0.2</math></b>		
Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):					
$\mu =$	1.00	$l_0 = \mu l =$	4100	mm	
$\lambda = l_0 / \rho =$	5				
Calcolo resistenza per sezioni in classe 1,2 e 3:					
$N_{c,Rd} = \frac{A f_{yk}}{\gamma_{M0}}$	$N_{c,Rd} =$	28535238	N	$\Rightarrow$	0.01 <b>OK!(&lt;1)</b>



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

**TRAVI INFLESSE (NTC2008, 4.2.4.1.3.2):**

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$\mu =$	0.70	$L_{cr} = L / \mu =$	2870	mm	
$W_{yy} =$	66237129	mm <sup>3</sup>	$W_{yy} =$	44248630	mm <sup>3</sup>
$\psi = 1.75 - 1.05 \frac{M_B}{M_A} + 0.3 \left( \frac{M_B}{M_A} \right)^2$			9.77		
$M_{cr} = \psi \frac{\pi}{L_{cr}} \sqrt{EJ_y GJ_T} \sqrt{1 + \left( \frac{\pi}{L_{cr}} \right)^2 \frac{EJ_{\Omega}}{GJ_T}}$			1.433E+12	Nmm	0.003
$M_{cr} / M_{cr}$	0.003	<b>Risulta &lt; 0.16 Non serve la verifica instabilità</b>			
$\bar{\lambda}_{LT} = \sqrt{\frac{W_{yy} f_{yk}}{M_{cr}}}$	0.10	<b>Risulta &lt; 0.4 Non serve la verifica instabilità</b>			

**Stabilità dei pannelli soggetti a taglio (NTC2008, 4.2.4.1.3.4.1):**

Se irrigiditori trasversali rigido o no,

$I_{it} =$	45000000	mm <sup>4</sup>	$0.75 h w t^3 =$	5710886	mm <sup>4</sup>	<b>Irrigiditore Trasversali rigidi</b>
------------	----------	-----------------	------------------	---------	-----------------	--

Coefficiente minimo di instabilità per taglio del pannello (In assenza di irrigiditori longitudinali),

$a/hw =$	2.05	$k_t =$	7.29
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Coefficiente minimo di instabilità per taglio del pannello (irrigiditori longitudinali più di due o  $a/hw > 3$ ),

$I_{il} =$	0	mm <sup>4</sup>	$k_{t1} =$	0.00
$a/hw =$	2.05	$k_t =$	7.29	

Coefficiente minimo di instabilità per taglio del pannello (irrigiditori longitudinali più di due o  $a/hw < 3$ ),

$I_{il} =$	0	mm <sup>4</sup>	$a/hw =$	2.05	$k_t =$	5.60
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$k_t =$	7.29	$hw/t =$	97.25	<b>Serve la verifica di instabilità</b>
$\eta =$	1.2			

$\sigma_c =$	19	MPa	$\tau_{cr} =$	138.53	$\lambda_w =$	1.22
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Montanti di appoggio rigidi,  $\chi_w =$  0.71  
Gli altri casi,  $\chi_w =$  0.68

$V_{hw,Rd} =$	5326791	N
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Momento resistente sole piattabande,

$Y_{G,inf} =$	624.42	mm	$Y_{G,sup} =$	1375.58	mm
$J_{yy} =$	37714089022	mm <sup>4</sup>	$W_{el,yy,sup} =$	27416925	mm <sup>3</sup>
$W_{el,yy,inf} =$	60398295	mm <sup>3</sup>			
$M_{t,Rd} =$	9044170614	Nmm	$V_{M,Rd} =$	15499	N

$bf =$	243	mm			
$V_{h,Rd} =$	5342290	N	0.187	<b>OKI(&lt;1)</b>	

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Stabilità dei pannelli soggetti a Compressione (NTC2008, 4.2.4.1.3.4.1):					
Stress,					
$\sigma_{sup} =$	104.82	MPa			
$\sigma_{inf} =$	-70.02	MPa			
$\sigma_{tr.} =$	60.23	MPa			
Irrigidati su entrambi i lati (Anima),					
$b =$	1945.0	mm	(Web bw)	$a =$	4100 mm
$\sigma_1 =$	104.8	(Max)		$\sigma_2 =$	-70.02 (Min)
$\psi =$	-0.7			$k\sigma =$	16.38
$\lambda_p =$	1.0			$\rho =$	0.96
$\sigma_{cr,p} =$	329.0	MPa		$\sigma_{cr,c} =$	4.516 MPa
$\zeta =$	1.0			$\lambda_c =$	8.866
$\alpha =$	0.34			$\Phi =$	116.298
$\chi_c =$	0.004				
$\rho_c =$	0.96				
$b_{eff} =$	1116.19	mm	$b_{e1} =$	446.48 mm	$b_{e2} =$ 669.71 mm
$b_t =$	778.96	mm			
Irrigidati a un lato (Piattebande), Piattebanda Superiore,					
$b =$	340	mm	(Flange Sup)		
$\sigma (lrr.) =$	104.8			$\sigma (Lib.) =$	104.82
$k\sigma =$	0.43				
$\lambda_p =$	1.1			$\rho =$	0.74
$b_{eff} =$	251.37	mm		$b_t =$	0 mm
Effective Section Properties,					
$A_{c,eff} (Piattebanda Sup) =$	10054.6	mm <sup>2</sup>			
$A_{c,eff} (Piattebanda Inf) =$	31500.0	mm <sup>2</sup>			
$A_{c,eff} (Anima) =$	22323.8	mm <sup>2</sup>			
$A_c (Anima tensione) =$	15579.1	mm <sup>2</sup>			
$A_{c,eff} (Total) =$	79457.6	mm <sup>2</sup>			
$Y_{g,int} =$	733.1	mm		$Y_{g,sup} =$	1266.9 mm
$I_{eff} =$	4.85E+10	mm <sup>4</sup>			
$W_{eff} (inf) =$	66191938	mm <sup>3</sup>		$W_{eff} (sup) =$	38299518 mm <sup>3</sup>
$e =$	67.92	mm			
$\eta =$	0.36				

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Fase 2

Soletta c.a. collaborante

$b_{eff}$	=	3.34	m
Altezza soletta:		0.26	m
Area soletta:		0.87	m <sup>2</sup>
$J_{yy}$	=	0.00489	m <sup>4</sup>
Peso	=	21.71	kN/m

Trave acciaio

Area trave acciaio:		0.0844	m <sup>2</sup>
H trave acciaio:		2.00	m
$Y_G$	=	0.801	m
$J_{yy}$	=	0.053055	m <sup>4</sup>
Peso	=	6.63	kN/m

Barre armatura soletta

1° strato barre (superiore)		2° strato barre (inferiore)	
n. Barre:	17	n. Barre:	17
Interasse:	0.20 m	Interasse:	0.20 m
Diametro:	0.02 m	Diametro:	0.02 m
Area:	0.005341 m <sup>2</sup>	Area:	0.005341 m <sup>2</sup>
$Y_1$	= 0.05 m	$Y_2$	= 0.21 m

n = 18.0      Coeff. omogeneizzazione con viscosità in atto

Materiali

cls	Barre armature	Acciaio
$R_{ck}$ = 40 MPa	$f_{yk}$ = 450 MPa	$f_{yk}$ = 355 MPa
$f_{ck}$ = 33.20 MPa	$\gamma_M$ = 1.15	$\gamma_{M0}$ = 1.05
$f_{ct}$ = 18.81 MPa	$f_{yk,s}$ = 391.30 MPa	$f_{yk,s}$ = 338.10 MPa
$f_{cm}$ = 41.2 MPa		
$f_{ctm}$ = 3.10 MPa		
$E_{cm}$ = 33643 MPa		

Caratteristiche geometriche

$A_{id}$ = 0.1433 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
$A_v$ = 39450 mm <sup>2</sup>	Area taglio sezione di acciaio
$Y_{G,inf,id}$ = 1.35 m	Asse neutro rispetto lembo inferiore:
$Y_{G,sup,id}$ = 0.91 m	Asse neutro rispetto lembo superiore
$J_{ax,id}$ = 0.114684 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{el,id}$ = 0.085116 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
$W_{sup,id}$ = 0.125666 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

Sollecitazioni

$M_{id}$ = 6092.00 kNm	(SLU)		
$M_{ed}$ = 28777.38 kNm	=>	0.21	ok!
$T_{id}$ = 1326.00 kN	(SLU)		
$T_{ed}$ = 7333.92 kN	=>	0.18	ok!

Tensioni

$\sigma_c$ = 2.69 MPa	=>	0.14	ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,sup}$ = 45.82 MPa	=>	0.12	ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf}$ = 37.32 MPa	=>	0.10	ok!(<1)	Tensione barre inferiori soletta
$\sigma_{s,sup}$ = 34.67 MPa	=>	0.10	ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{s,inf}$ = 71.57 MPa	=>	0.21	ok!(<1)	Tensione acciaio lembo inferiore

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Fase 3

Soletta c.a. collaborante

$b_{eff} =$	3.34	m
Altezza soletta:	0.26	m
Area soletta:	0.87	m <sup>2</sup>
$J_{xx} =$	0.00489	m <sup>4</sup>
Peso =	21.71	kN/m

Trave acciaio

Area trave acciaio:	0.0844	m <sup>2</sup>
H trave acciaio:	2.00	m
$Y_G =$	0.801	m
$J_{xx} =$	0.053055	m <sup>4</sup>
Peso =	6.63	kN/m

Barre armatura soletta

1° strato barre (superiore)		2° strato barre (inferiore)	
n. Barre:	17	n. Barre:	17
interasse:	0.20 m	interasse:	0.20 m
Diametro:	0.02 m	Diametro:	0.02 m
Area:	0.005341 m <sup>2</sup>	Area:	0.005341 m <sup>2</sup>
$Y_1 =$	0.05 m	$Y_2 =$	0.21 m

n = 6.2

Coeff. omogeneizzazione a tempo infinito

Materiali

c/c	Barre armature	Acciaio
$R_{ck} =$ 40 MPa	$f_{yk} =$ 450 MPa	$f_{yk} =$ 355 MPa
$f_{ck} =$ 33.20 MPa	$\gamma_M =$ 1.15	$\gamma_{M0} =$ 1.05
$f_{td} =$ 18.81 MPa	$f_{yk,t} =$ 391.30 MPa	$f_{yk,t} =$ 338.10 MPa
$f_{cm} =$ 41.2 MPa		
$f_{ctm} =$ 3.10 MPa		
$E_{cm} =$ 33643 MPa		

Caratteristiche geometriche

$A_c =$ 0.2351 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
$A_v =$ 39450 mm <sup>2</sup>	Area taglio sezione di acciaio
$Y_{G,inf,v} =$ 1.65 m	Asse neutro rispetto lembo inferiore:
$Y_{G,sup,v} =$ 0.61 m	Asse neutro rispetto lembo superiore
$J_{xx,v} =$ 0.14948 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{inf,v} =$ 0.090431 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
$W_{sup,v} =$ 0.246253 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

Sollecitazioni

$M_{ed} =$ 16520.00 kNm	(SLU)	
$M_{ed} =$ 30574.13 kNm	=>	0.54 ok!
$T_{ed} =$ 3459.00 kN	(SLU)	
$T_{ed} =$ 7333.92 kN	=>	0.47 ok!

Tensioni

$\sigma_c =$ 10.82 MPa	=>	0.58 ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,super} =$ 61.56 MPa	=>	0.16 ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf} =$ 43.88 MPa	=>	0.11 ok!(<1)	Tensione barre inferiori soletta
$\sigma_{a,super} =$ 38.35 MPa	=>	0.11 ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{a,inf} =$ 182.68 MPa	=>	0.54 ok!(<1)	Tensione acciaio lembo inferiore

### 6.2.3 TRAVE PRINCIPALE CONCIO 2

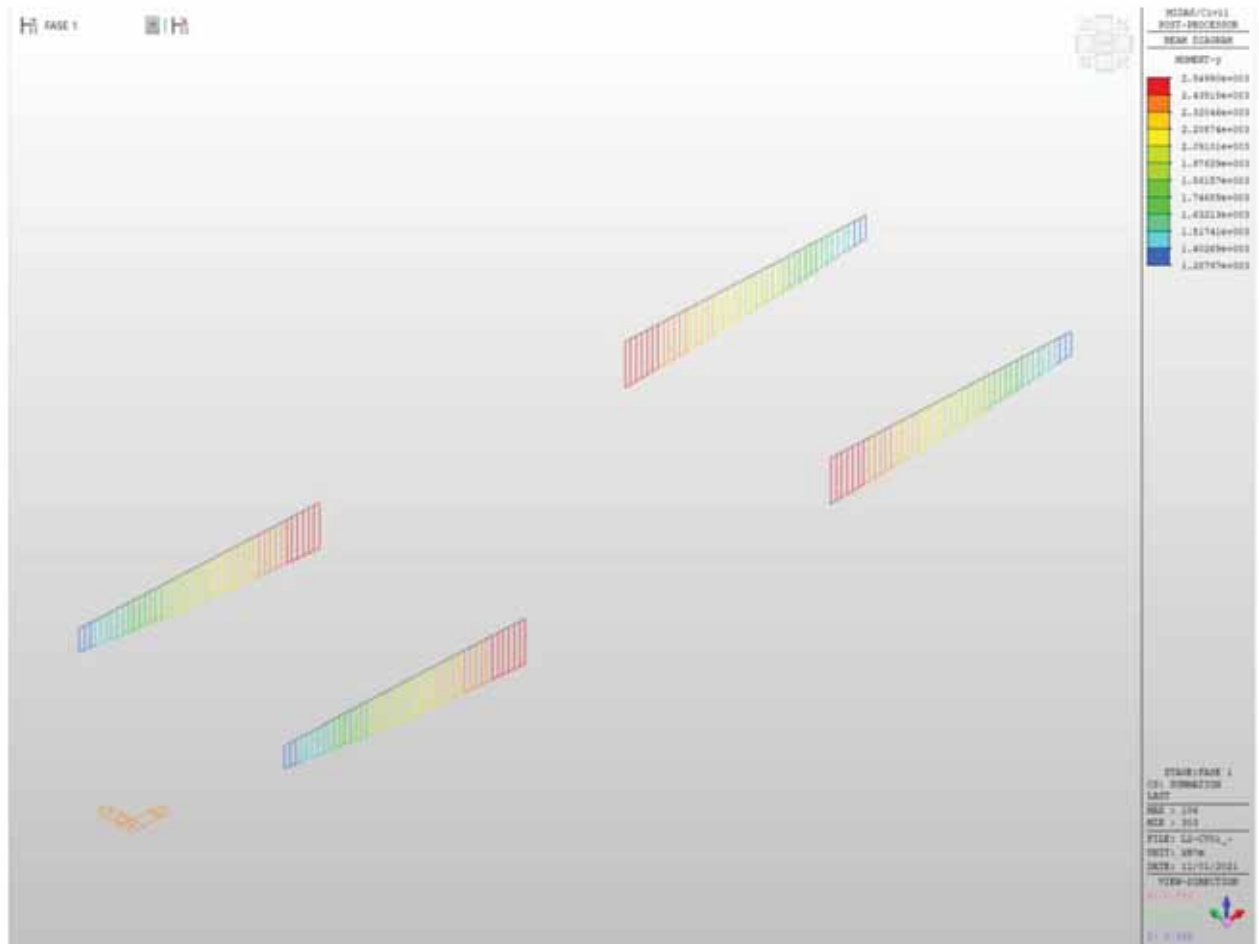


Figura 43 - Concio 2 - Diagramma My - Fase 1



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

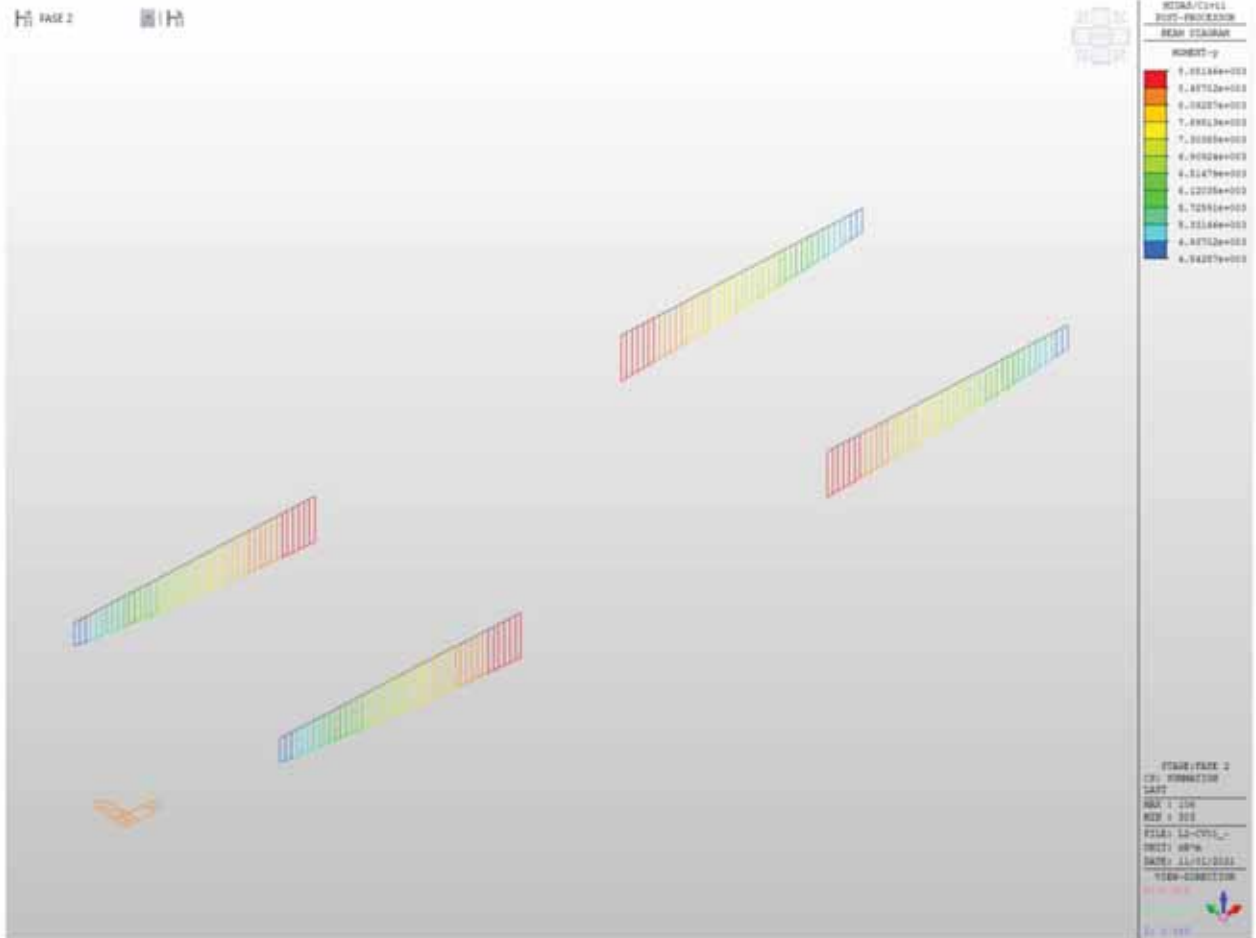


Figura 44 - Concio 2 - Diagramma My - Fase 2



Figura 45 - Concio 2 - Diagramma My - Fase 3

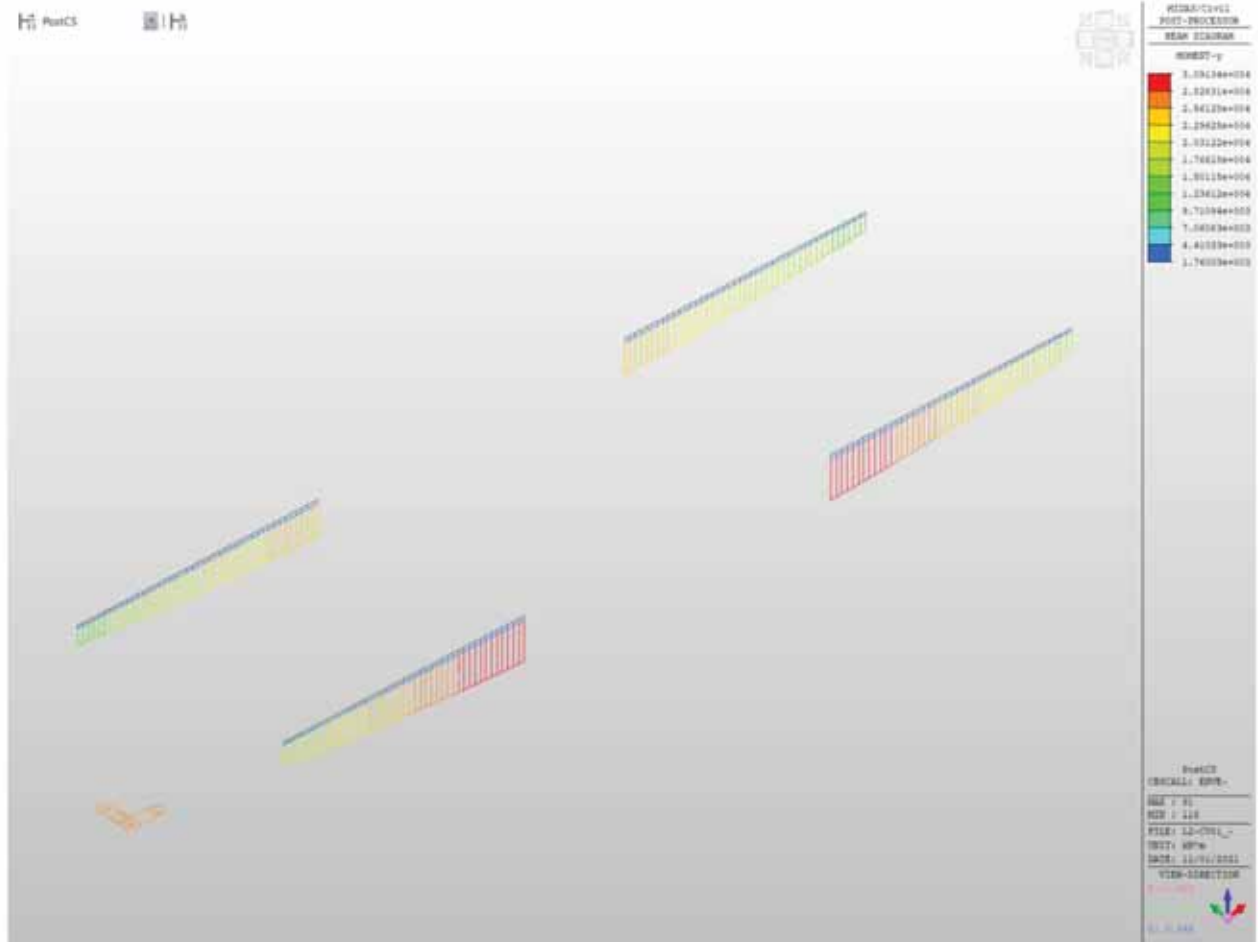


Figura 46 - Concio 2 - Diagramma My - Involuppo SLU-SLV

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

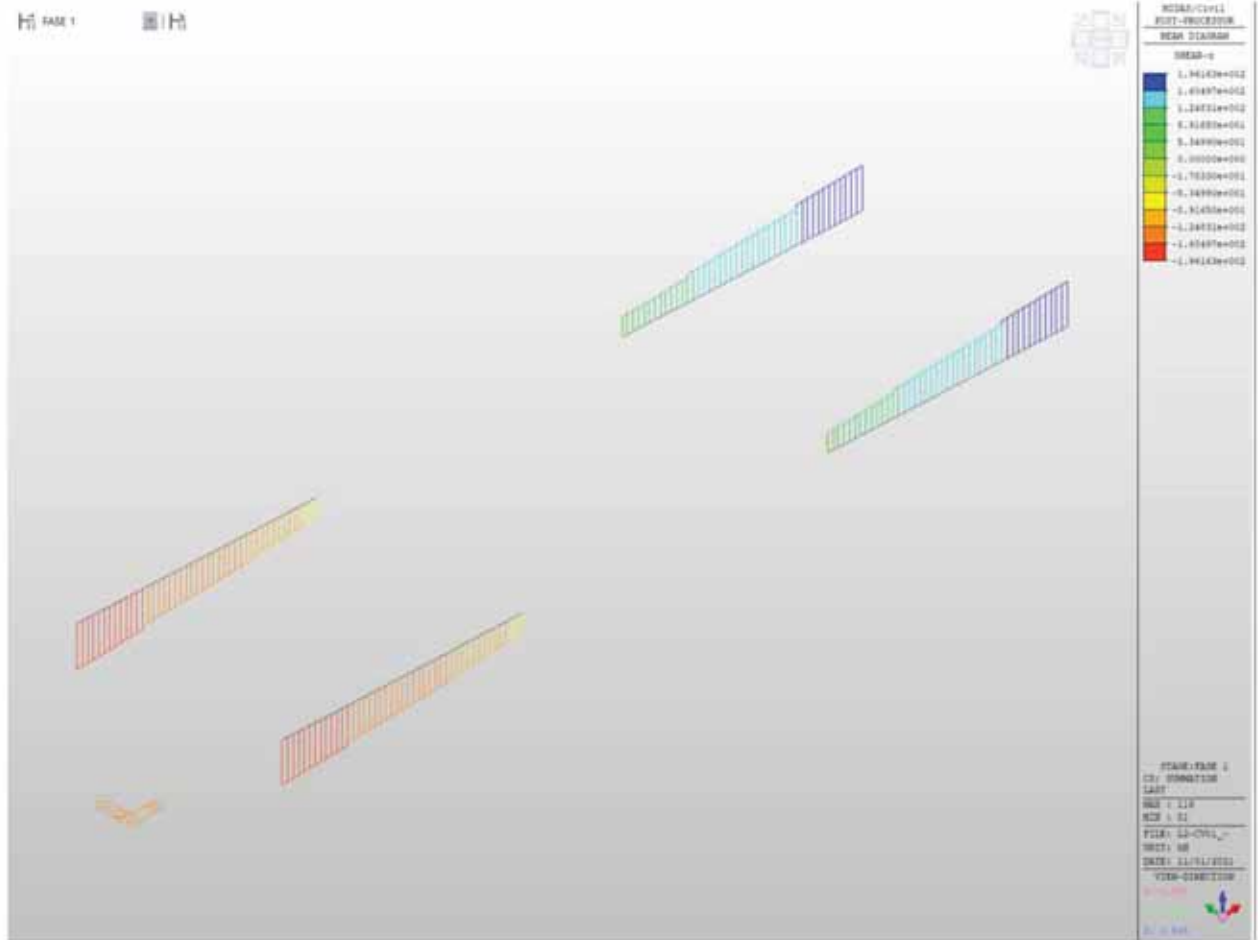


Figura 47 - Concio 2 - Diagramma Tz - Fase 1

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

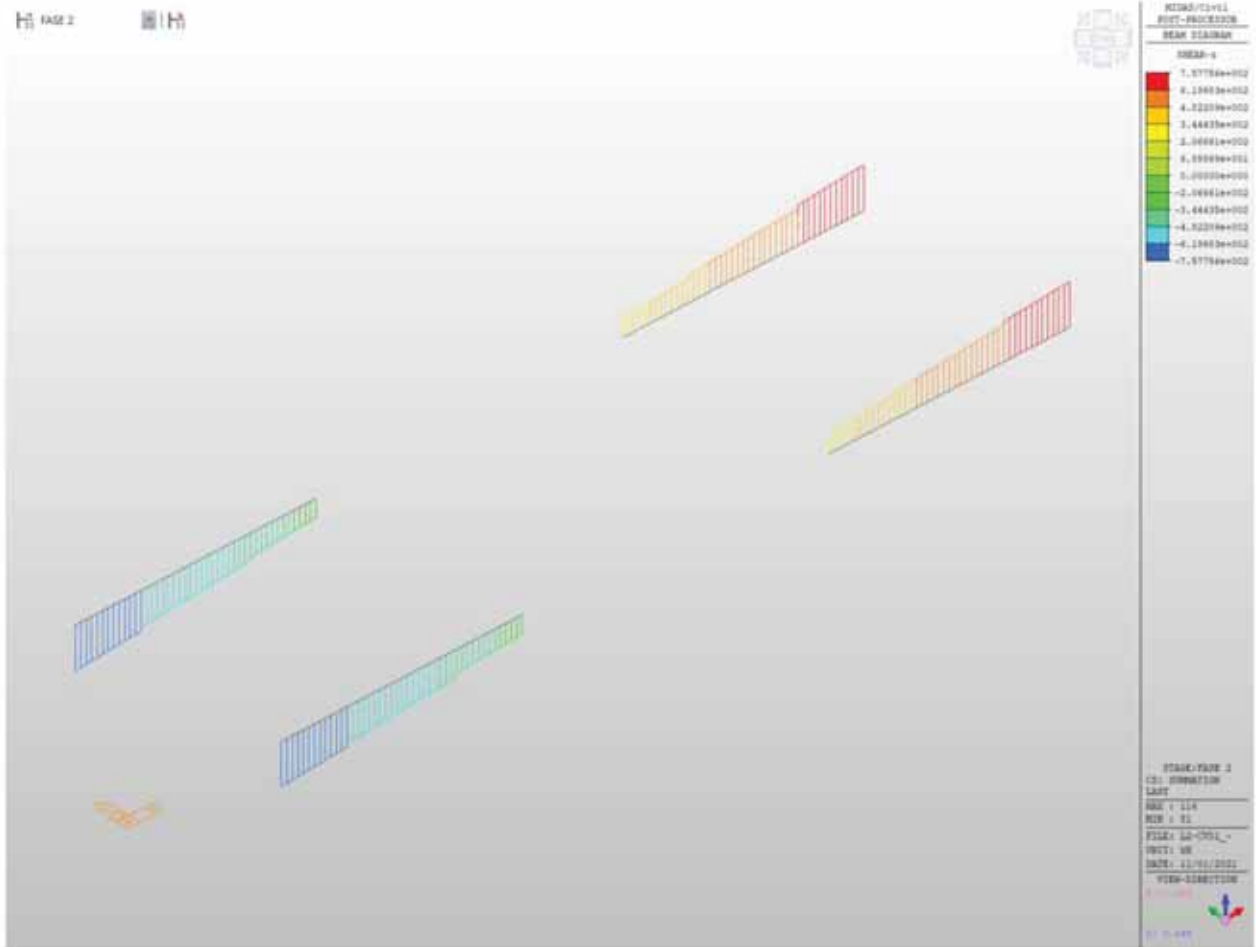


Figura 48 - Concio 2 - Diagramma Tz - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

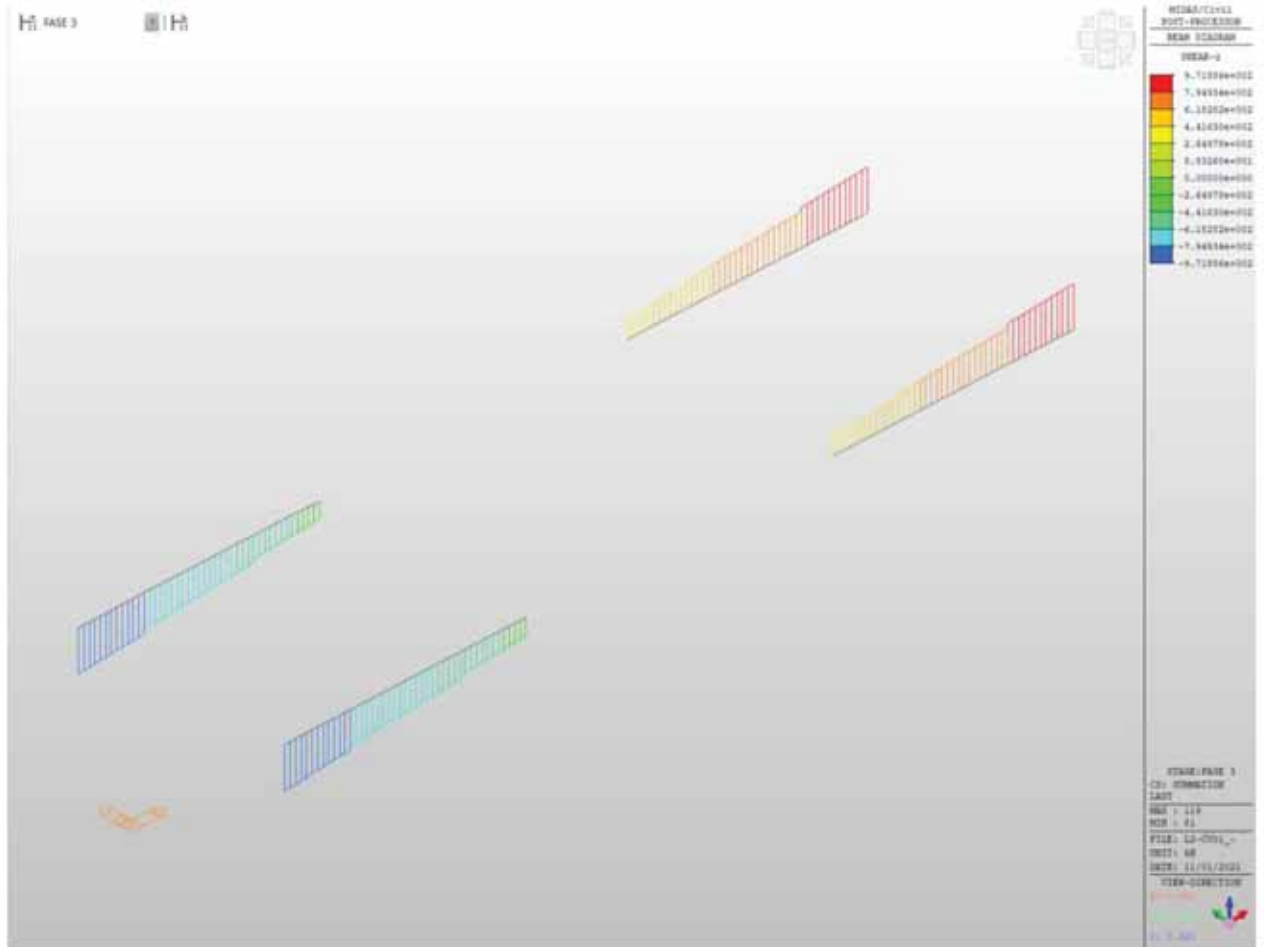


Figura 49 - Concio 2 - Diagramma Tz - Fase 3

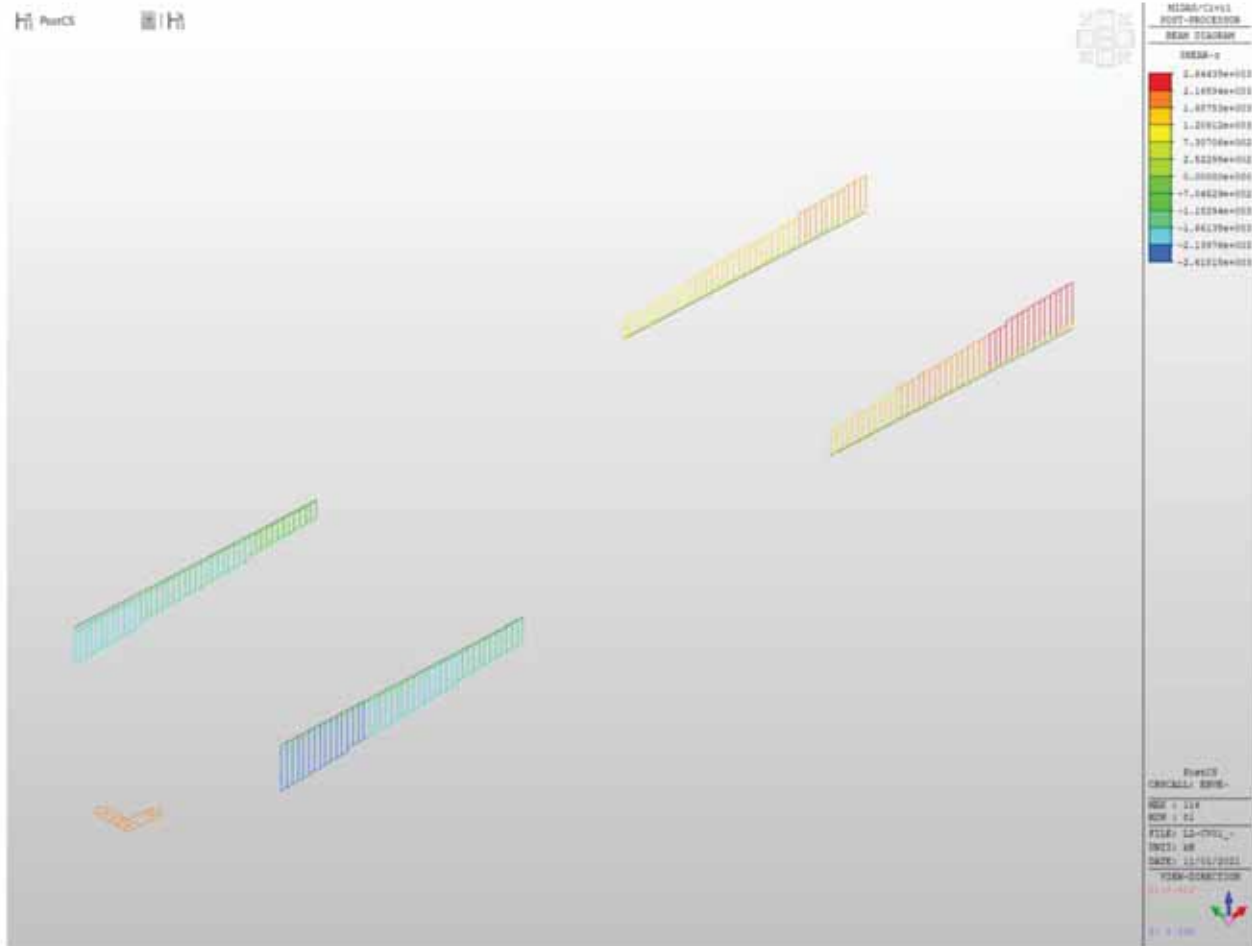


Figura 50 - Concio 2 - Diagramma Tz - Involuppo SLU-SLV

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

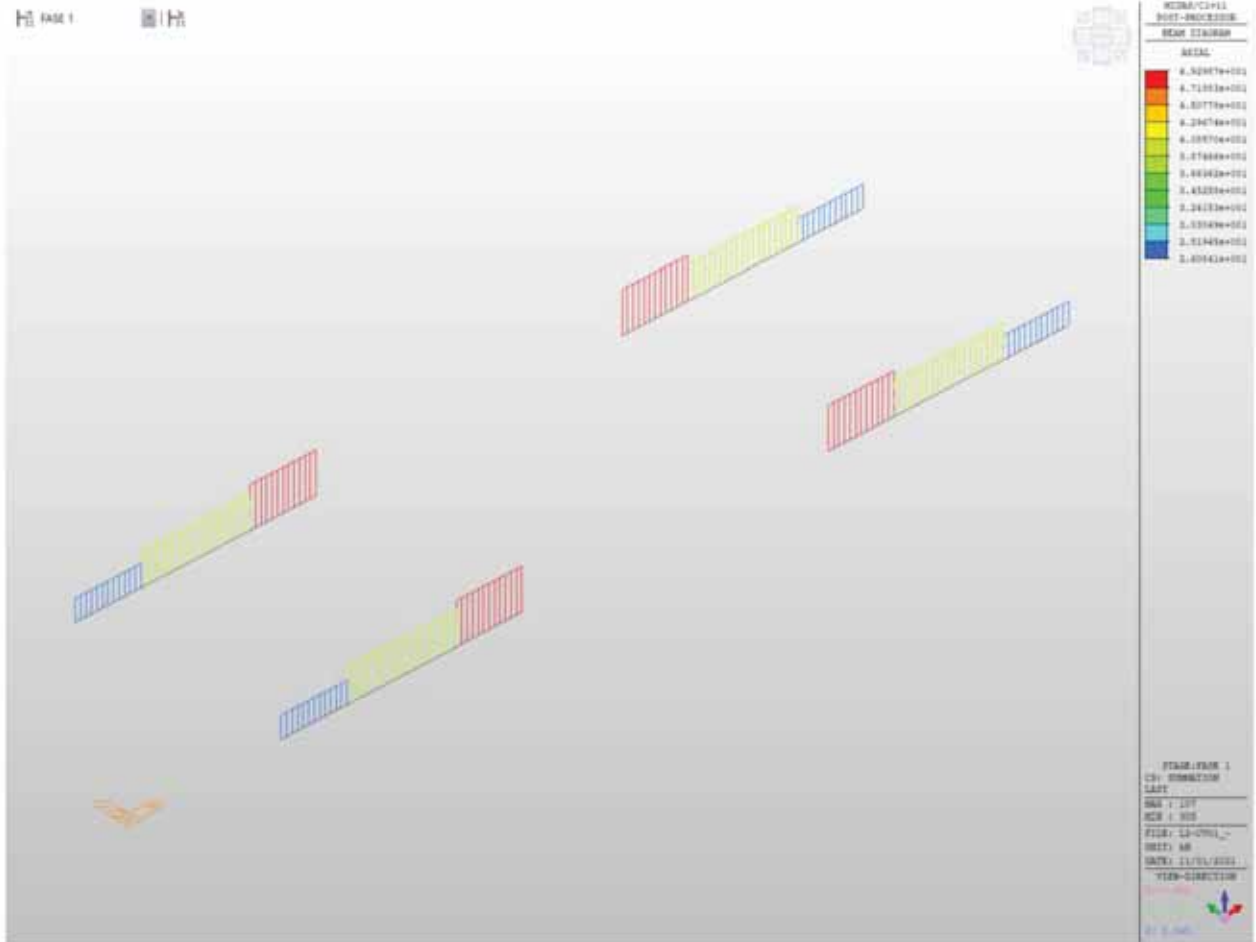


Figura 51 - Concio 2 - Diagramma N - Fase 1



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

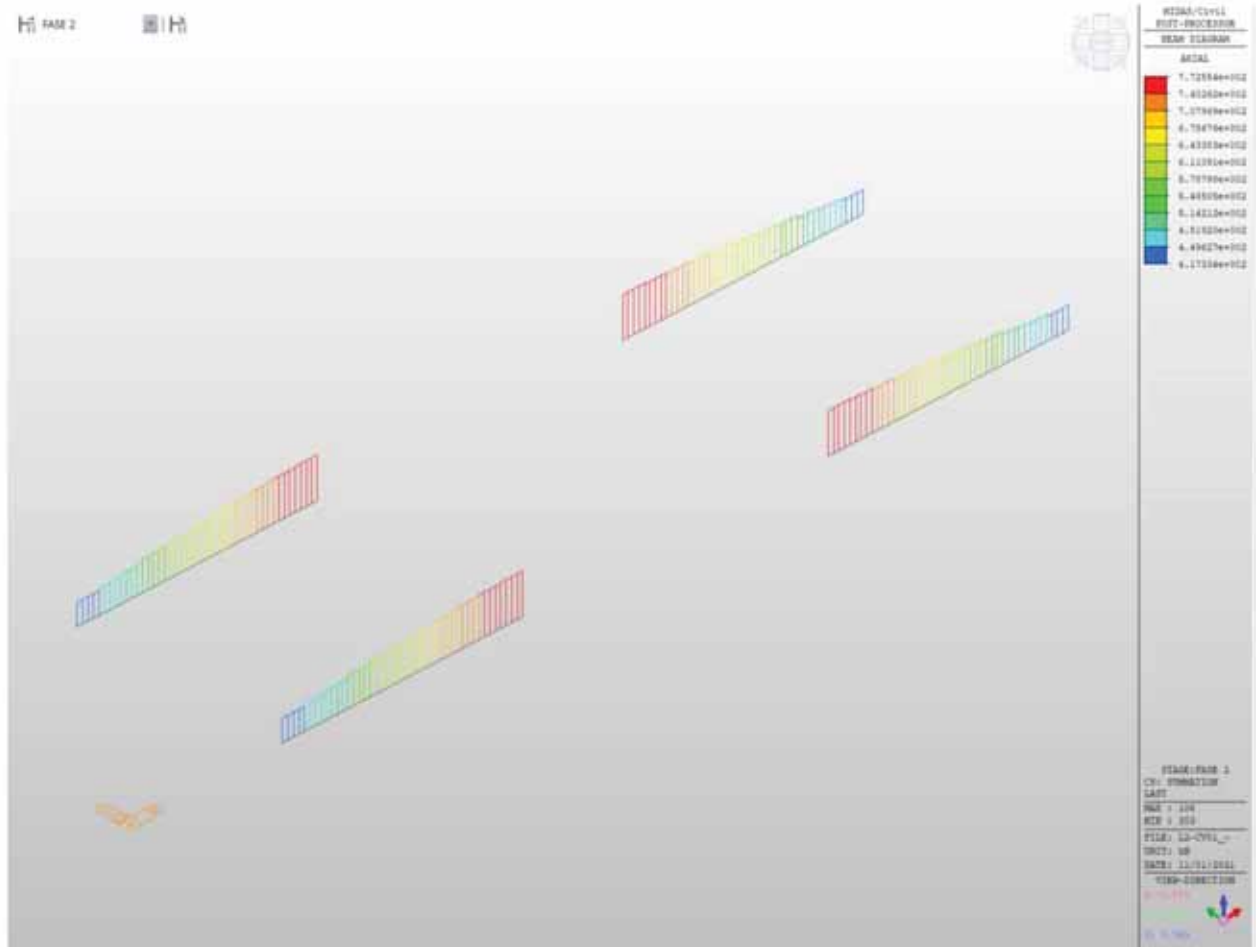


Figura 52 - Concio 2 - Diagramma N - Fase 2

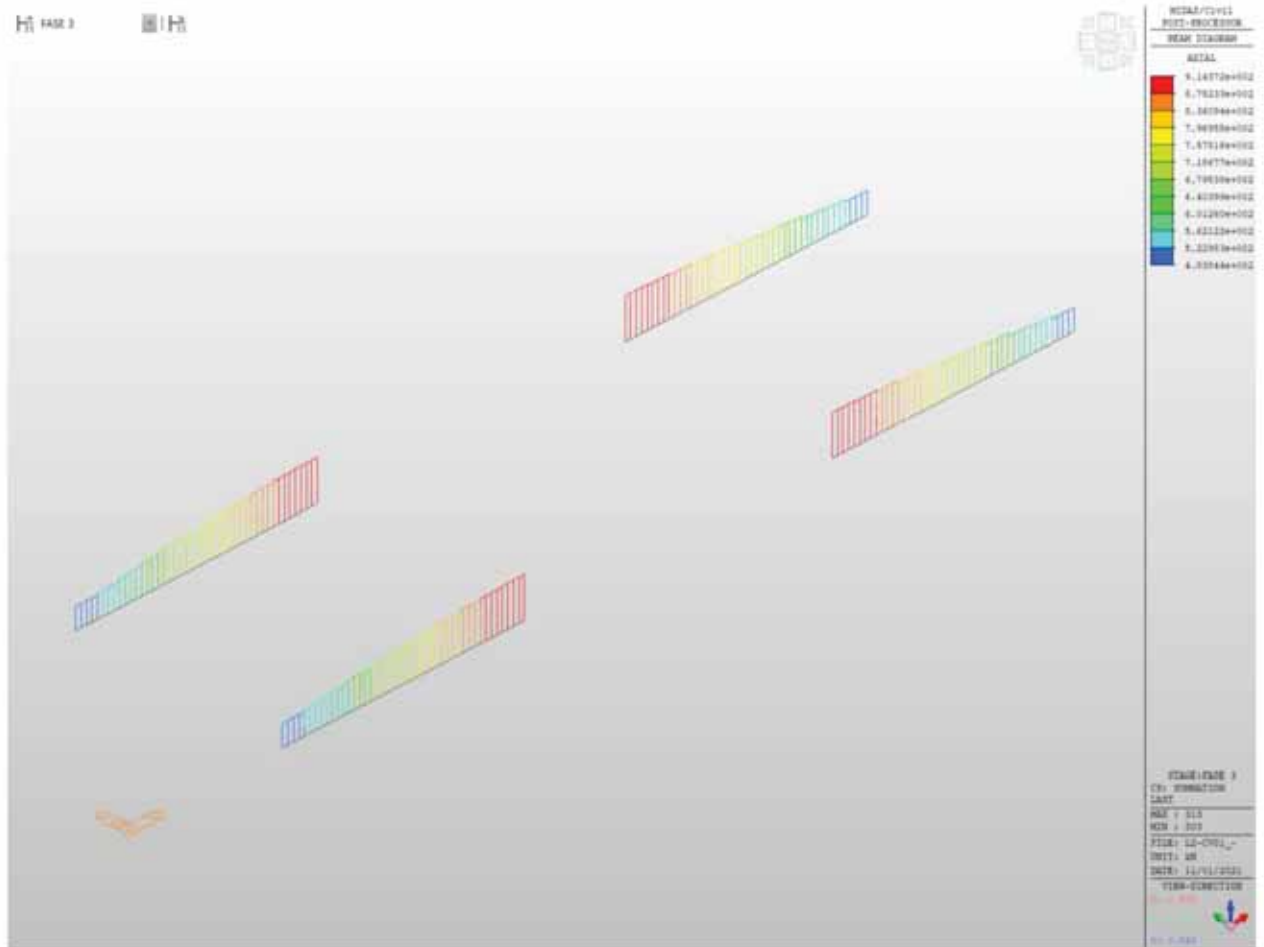


Figura 53 - Concio 2 - Diagramma N - Fase 3





**CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1):**

ALA SUPERIORE		ALA INFERIORE		ANIMA	
$C/t_{i,sup} =$	13.60	$C/t_{i,inf} =$	14.75	$C/t_w =$	96.75
$\varepsilon =$	0.81	$\varepsilon =$	0.81	$\varepsilon =$	0.81

Curva di instabilità "b" (Asse yy):  
Fattore di imperfezione  $\alpha$ : 0.34  
Curva di instabilità "c" (Asse zz):  
Fattore di imperfezione  $a$ : 0.49

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$\frac{V_{sd}}{V_{c,Rd}} \leq 1$

Area taglio resistente:  $A_v = 39350 \text{ mm}^2$

$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}}$   $V_{c,Rd} = 7681095 \text{ N} \Rightarrow 0.10 \text{ OK! (<1)}$

Verifica in presenza di torsione uniforme:

$\tau_{t,sd} = 0 \text{ N/mm}^2$

$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,sd}}{\frac{f_{yk}}{\sqrt{3} \gamma_{M0}}} \right] V_{c,Rd}$

$V_{c,Rd,red} = 7681095 \text{ N} \Rightarrow 0.10 \text{ OK! (<1)}$

Verifica in termini tensionali nel punto più sollecitato:

$\frac{\tau_{sd}}{\frac{f_{yk}}{\sqrt{3} \gamma_{M0}}} \leq 1 \Rightarrow 0.00 \text{ OK! (<1)}$

**SOLLECITAZIONE DELL'ASTA A COMPRESSIONE SEMPLICE (NTC2008, 4.2.4.1.2):**

$\frac{N_{sd}}{N_{c,Rd}} \leq 1$   $N_{cr} = \frac{\pi^2 EJ}{(\eta l)^2} = 798458922 \text{ N}$   $N_{c,sd} < 0,04 N_{cr}$   
*Instabilità trascurabile!*

$0,04 N_{cr} = 31938357 \text{ N}$  *Instabilità trascurabile se  $\lambda < 0.2$*

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$\mu = 1.00$   $l_0 = \mu l = 4100 \text{ mm}$

$\lambda = l_0 / \rho = 5$

Calcolo resistenza per sezioni in classe 1,2 e 3:

$N_{c,Rd} = \frac{A f_{yk}}{\gamma_{M0}}$   $N_{c,Rd} = 35229524 \text{ N} \Rightarrow 0.02 \text{ OK! (<1)}$



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Stabilità dei pannelli soggetti a taglio (NTC2008, 4.2.4.1.3.4.1):					
Se irrigiditori trasversali rigido o no,					
$I_{st} =$	45000000	mm <sup>4</sup>	$0.75hwt^3 =$	5710886	mm <sup>4</sup> <span style="color: red;">Irrigiditore Trasversali rigidi</span>
Coefficiente minimo di instabilità per taglio del pannello (In assenza di irrigiditori longitudinali),					
$a/hw =$	2.05		$k_t =$	7.29	
Coefficiente minimo di instabilità per taglio del pannello (irrigiditori longitudinali più di due o $a/hw > 3$ ),					
$I_{sl} =$	0	mm <sup>4</sup>	$k_{tl} =$	0.00	
$a/hw =$	2.05		$k_t =$	7.29	
Coefficiente minimo di instabilità per taglio del pannello (irrigiditori longitudinali più di due o $a/hw < 3$ ),					
$I_{sl} =$	0	mm <sup>4</sup>	$a/hw =$	2.05	$k_t =$ 5.60
$k_t =$	7.29				
$\eta =$	1.2		$hw/t =$	96.75	Serve la verifica di instabilità
$\sigma_c =$	19	MPa	$\tau_{cr} =$	138.53	$\lambda_w =$ 1.22
Montanti di appoggio rigidi,					
Gli altri casi,			$X_w =$	0.71	
			$X_w =$	0.68	
$V_{bw,Rd} =$	5326791	N			
Momento resistente sole piattabande,					
$Y_{G,inf} =$	545.67	mm	$Y_{G,sup} =$	1454.33	mm
$J_{yy} =$	49651391611	mm <sup>4</sup>			
$W_{el,yy,inf} =$	90991953	mm <sup>3</sup>	$W_{el,yy,sup} =$	34140340	mm <sup>3</sup>
$M_{t,red} =$	11139778375	Nmm			
$bf =$	303.75	mm	$V_{bf,Rd} =$	12502	N
$V_{b,Rd} =$	5339292	N	0.142	OK!(<1)	

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Stabilità dei pannelli soggetti a Compressione (NTC2008, 4.2.4.1.3.4.1):						
Stress,						
$\sigma_{sup} =$	170.27	MPa				
$\sigma_{inf} =$	-95.20	MPa				
$\sigma_{int.} =$	102.58	MPa				
Irrigidati su entrambi i lati (Anima),						
$b =$	1935.0	mm	(Web bw)	$a =$	4100 mm	
$\sigma_1 =$	170.3	(Max)		$\sigma_2 =$	-95.20 (Min)	
$\psi =$	-0.6			$k\sigma =$	14.38	
$\lambda_p =$	1.1			$\rho =$	0.90	
$\sigma_{cr,p} =$	292.0	MPa		$\sigma_{cr,c} =$	4.516 MPa	
$\zeta =$	1.0			$\lambda_c =$	8.866	
$\alpha =$	0.34			$\Phi =$	116.298	
$\chi_c =$	0.004					
$\rho_c =$	0.90					
$b_{eff} =$	1119.18	mm	$b_{e1} =$	447.67 mm	$b_{e2} =$	671.51 mm
$b_t =$	693.88	mm				
Irrigidati a un lato (Piattebande), Piattebanda Superiore,						
$b =$	340	mm	(Flange Sup)			
$\sigma$ (Irr.) =	170.3			$\sigma$ (Lib.) =	170.27	
$k\sigma =$	0.43					
$\lambda_p =$	1.1			$\rho =$	0.74	
$b_{eff} =$	251.37	mm		$b_t =$	0 mm	
Effective Section Properties,						
$A_{c,eff}$ (Piattebanda Sup) =	12568.3	mm <sup>2</sup>				
$A_{c,eff}$ (Piattebanda Inf) =	48000.0	mm <sup>2</sup>				
$A_{c,eff}$ (Anima) =	22383.5	mm <sup>2</sup>				
$A_c$ (Anima tensione) =	13877.7	mm <sup>2</sup>				
$A_{c,eff}$ (Total) =	96829.5	mm <sup>2</sup>				
$Y_{g,inf} =$	633.6	mm		$Y_{g,sup} =$	1366.4 mm	
$I_{eff} =$	5.98E+10	mm <sup>4</sup>				
$W_{eff}(inf) =$	94329960	mm <sup>3</sup>		$W_{eff}(sup) =$	43743341 mm <sup>3</sup>	
$e =$	83.57	mm				
$\eta =$	0.60					



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Fase 2

Soletta c.a. collaborante

$b_{eff}$ =	3.34	m
Altezza soletta:	0.26	m
Area soletta:	0.87	m <sup>2</sup>
$J_{yy}$ =	0.00489	m <sup>4</sup>
Peso =	21.71	kN/m

Trave acciaio

Area trave acciaio:	0.1042	m <sup>2</sup>
H trave acciaio:	2.00	m
$Y_G$ =	0.717	m
$J_{yy}$ =	0.066915	m <sup>4</sup>
Peso =	8.18	kN/m

Barre armatura soletta

1° strato barre (superiore)		2° strato barre (inferiore)	
n. Barre:	17	n. Barre:	17
Interasse:	0.20 m	Interasse:	0.20 m
Diametro:	0.02 m	Diametro:	0.02 m
Area:	0.005341 m <sup>2</sup>	Area:	0.005341 m <sup>2</sup>
$y_1$ =	0.05 m	$y_2$ =	0.21 m

n = 18.0

Coeff. omogeneizzazione con viscosità in atto

Materiali

cls	Barre armature	Acciaio
$R_{ck}$ = 40 MPa	$f_{yk}$ = 450 MPa	$f_{yk}$ = 355 MPa
$f_{ck}$ = 33.20 MPa	$\gamma_M$ = 1.15	$\gamma_{M0}$ = 1.05
$f_{ctd}$ = 18.81 MPa	$f_{yk,s}$ = 391.30 MPa	$f_{yk,s}$ = 338.10 MPa
$f_{cm}$ = 41.2 MPa		
$f_{ctm}$ = 3.10 MPa		
$E_{cm}$ = 33643 MPa		

Caratteristiche geometriche

$A_{oe}$ = 0.1631 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
$A_v$ = 39350 mm <sup>2</sup>	Area taglio sezione di acciaio
$Y_{G,inf,cl}$ = 1.23 m	Asse neutro rispetto lembo inferiore:
$Y_{G,sup,cl}$ = 1.03 m	Asse neutro rispetto lembo superiore
$J_{xx,cl}$ = 0.142386 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{pl,cl}$ = 0.115993 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
$W_{pl,cl}$ = 0.137909 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

Sollecitazioni

$M_{ed}$ = 11490.00 kNm	(SLU)	
$M_{ed}$ = 39216.61 kNm	=>	0.29 ok!
$T_{ed}$ = 972.00 kN	(SLU)	
$T_{ed}$ = 7315.33 kN	=>	0.13 ok!

Tensioni

$\sigma_c$ = 4.63 MPa	=>	0.25 ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,sup}$ = 79.28 MPa	=>	0.20 ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf}$ = 66.37 MPa	=>	0.17 ok!(<1)	Tensione barre inferiori soletta
$\sigma_{s,sup}$ = 62.33 MPa	=>	0.18 ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{s,inf}$ = 99.06 MPa	=>	0.29 ok!(<1)	Tensione acciaio lembo inferiore

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Fase 3

Soletta c.a. collaborante

$b_{eff}$ =	3.34	m
Altezza soletta:	0.26	m
Area soletta:	0.87	m <sup>2</sup>
$J_{xx}$ =	0.00489	m <sup>4</sup>
Peso =	21.71	kN/m

Trave acciaio

Area trave acciaio:	0.1042	m <sup>2</sup>
H trave acciaio:	2.00	m
$Y_G$ =	0.717	m
$J_{xx}$ =	0.066915	m <sup>4</sup>
Peso =	8.18	kN/m

Barre armatura soletta

1° strato barre (superiore)		2° strato barre (inferiore)	
n. Barre:	17	n. Barre:	17
Interasse:	0.20 m	Interasse:	0.20 m
Diametro:	0.02 m	Diametro:	0.02 m
Area:	0.005341 m <sup>2</sup>	Area:	0.005341 m <sup>2</sup>
$Y_1$ =	0.05 m	$Y_2$ =	0.21 m

n = 6.2      Coeff. omogeneizzazione a tempo infinito

Materiali

cls	Barre armature	Acciaio
$R_{ck}$ = 40 MPa	$f_{yk}$ = 450 MPa	$f_{yk}$ = 355 MPa
$f_{ck}$ = 33.20 MPa	$\gamma_M$ = 1.15	$\gamma_{M0}$ = 1.05
$f_{ot}$ = 18.81 MPa	$f_{yLk}$ = 391.30 MPa	$f_{yLk}$ = 338.10 MPa
$f_{cm}$ = 41.2 MPa		
$f_{ctm}$ = 3.10 MPa		
$E_{cm}$ = 33643 MPa		

Caratteristiche geometriche

$A_{eq}$ = 0.2549 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
$A_v$ = 39350 mm <sup>2</sup>	Area taglio sezione di acciaio
$Y_{G,inf,d}$ = 1.55 m	Asse neutro rispetto lembo inferiore:
$Y_{G,sup,d}$ = 0.71 m	Asse neutro rispetto lembo superiore
$J_{G,inf}$ = 0.190752 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{inf,d}$ = 0.122862 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
$W_{sup,d}$ = 0.269639 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

Sollecitazioni

$M_{ed}$ = 30913.00 kNm	(SLU)	
$M_{td}$ = 41539.10 kNm	=>	0.74 ok!
$T_{ed}$ = 2645.00 kN	(SLU)	
$T_{td}$ = 7315.33 kN	=>	0.36 ok!

Tensioni

$\sigma_c$ = 18.49 MPa	=>	0.98 ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,super}$ = 106.54 MPa	=>	0.27 ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf}$ = 80.61 MPa	=>	0.21 ok!(<1)	Tensione barre inferiori soletta
$\sigma_{a,super}$ = 72.51 MPa	=>	0.21 ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{a,inf}$ = 251.61 MPa	=>	0.74 ok!(<1)	Tensione acciaio lembo inferiore

### 6.2.4 TRAVE PRINCIPALE CONCIO 3

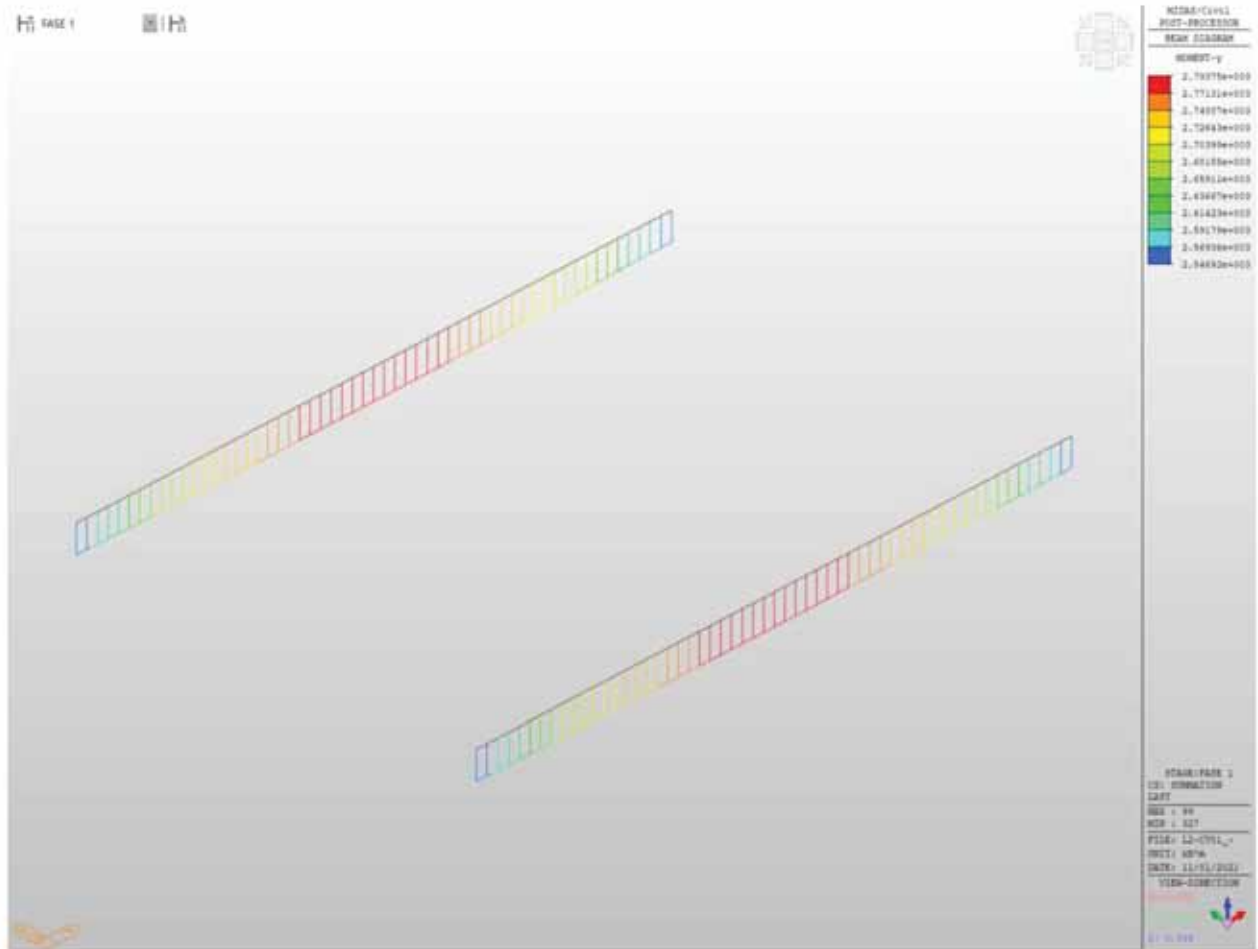
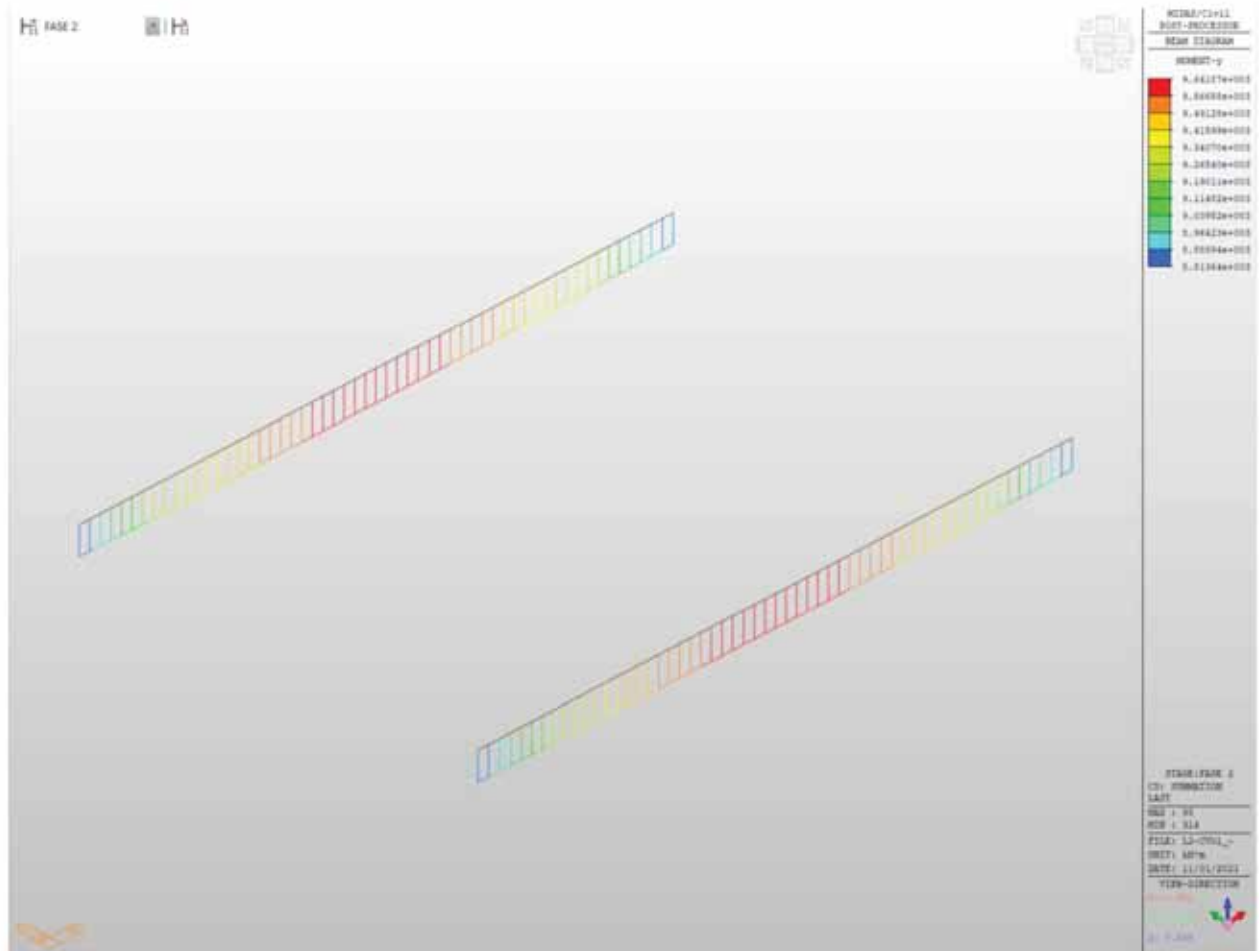


Figura 55 - Concio 3 - Diagramma My - Fase 1



**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

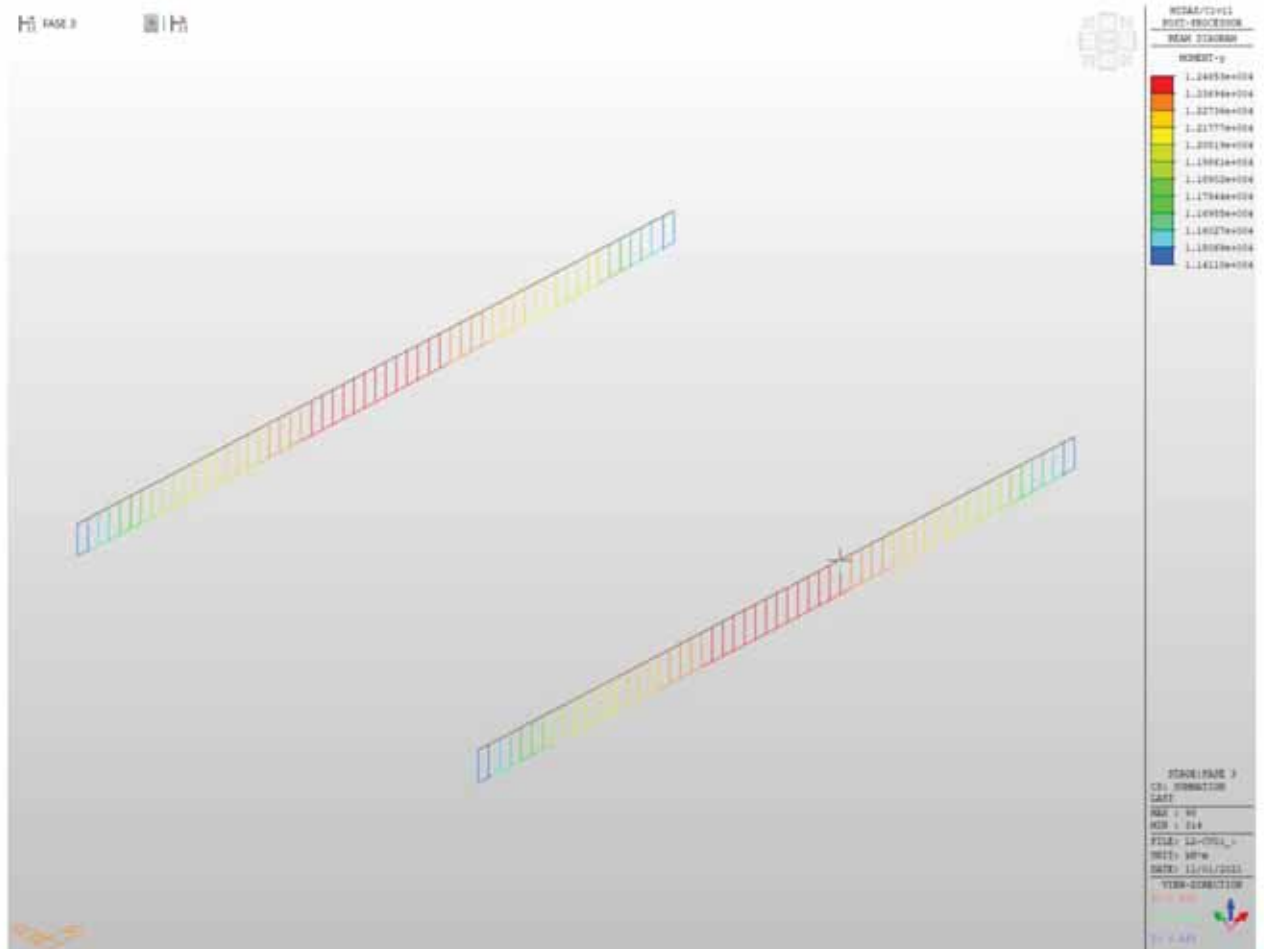


Figura 57 - Concio 3 - Diagramma My - Fase 3

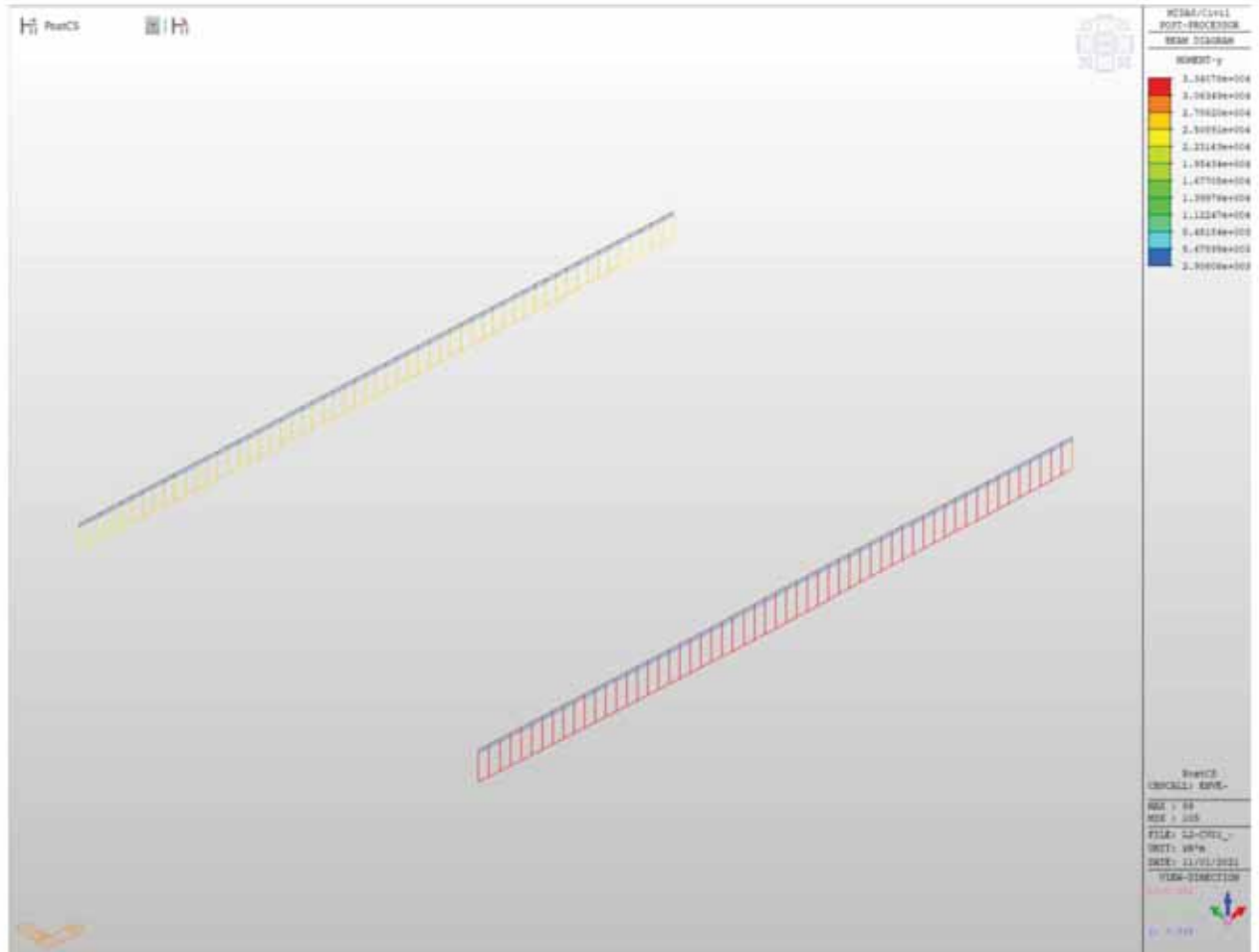


Figura 58 - Concio 3 - Diagramma My - Involuppo SLU-SLV

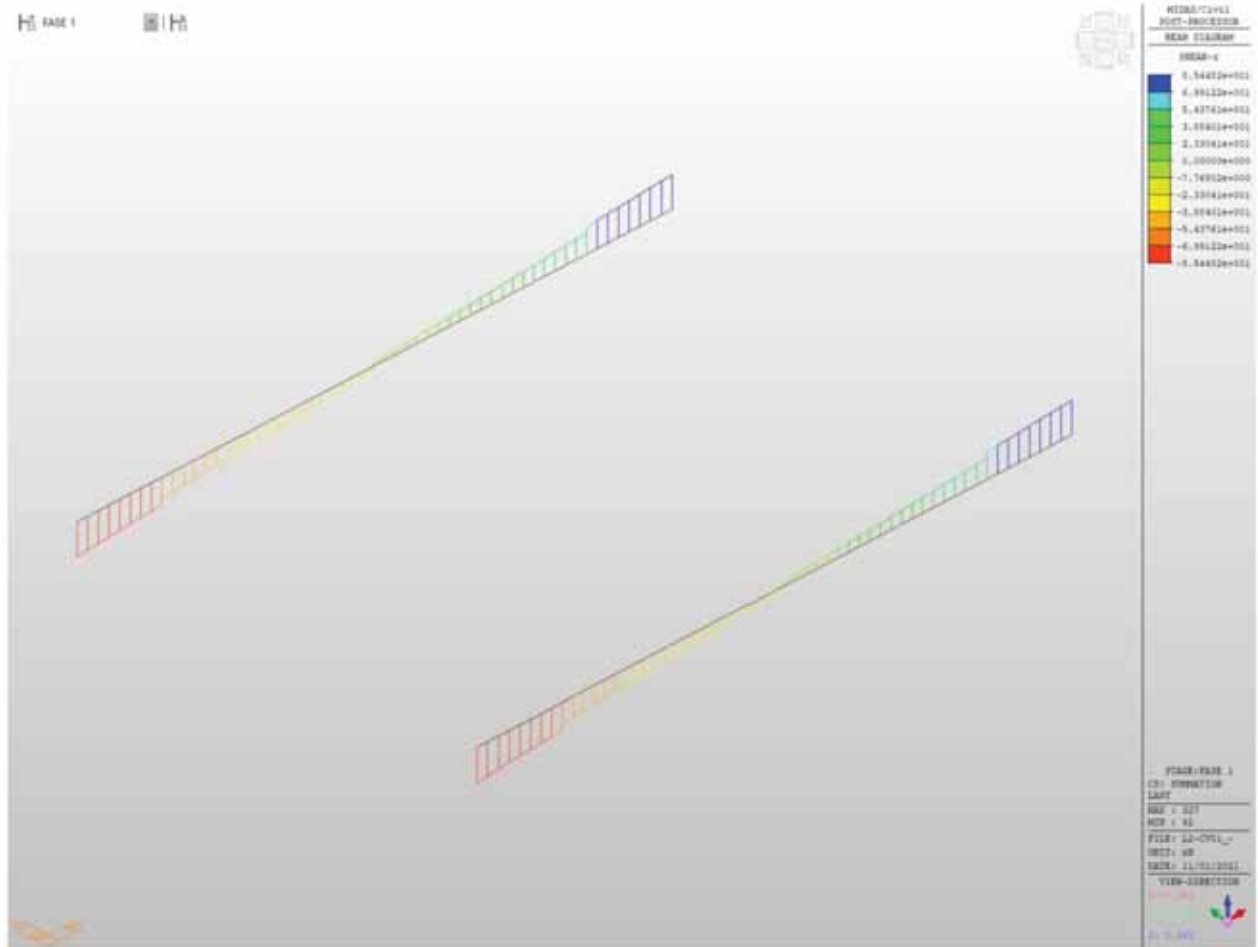


Figura 59 - Concio 3 - Diagramma Tz - Fase 1

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

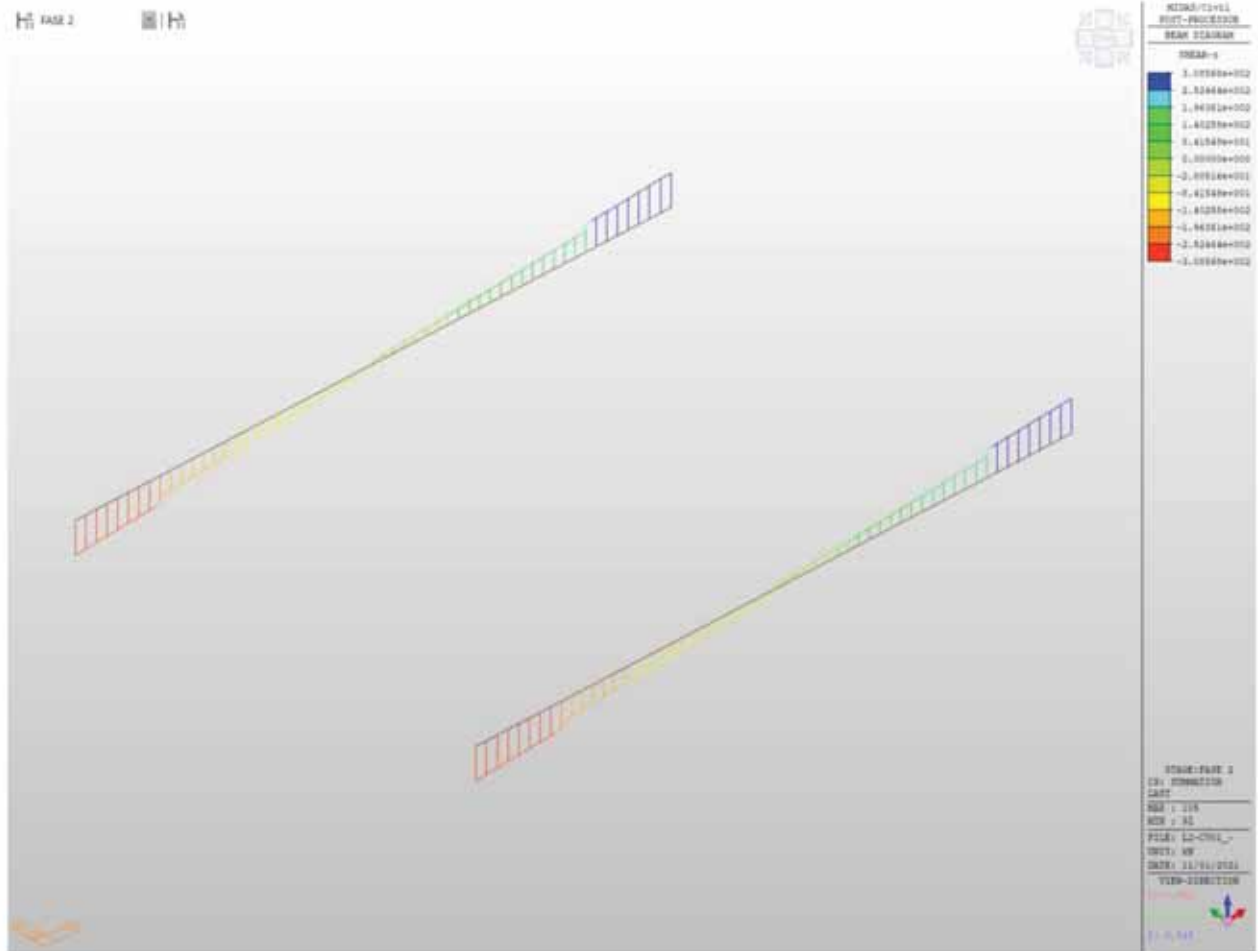


Figura 60 - Concio 3 - Diagramma Tz - Fase 2



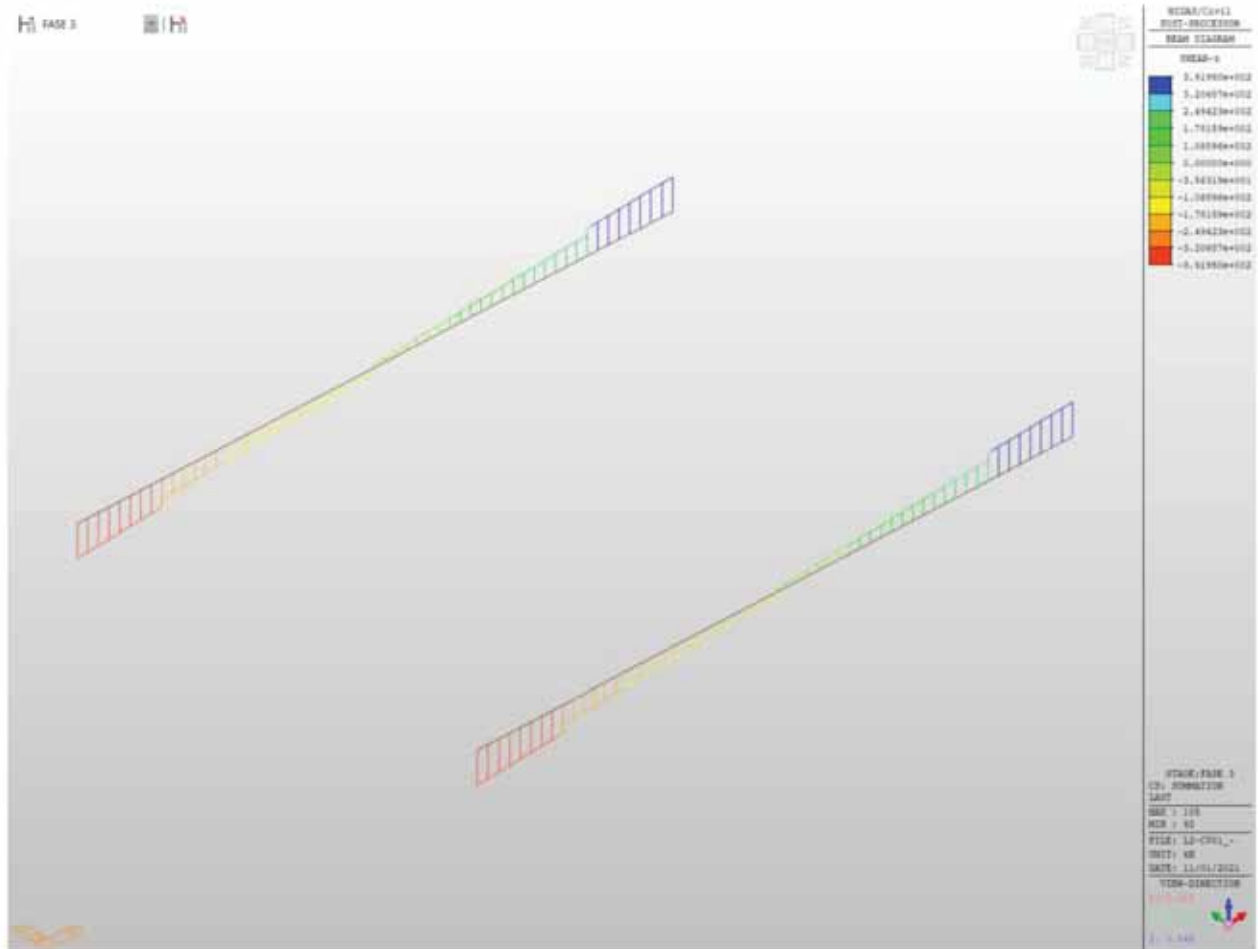


Figura 61 - Concio 3 - Diagramma Tz - Fase 3

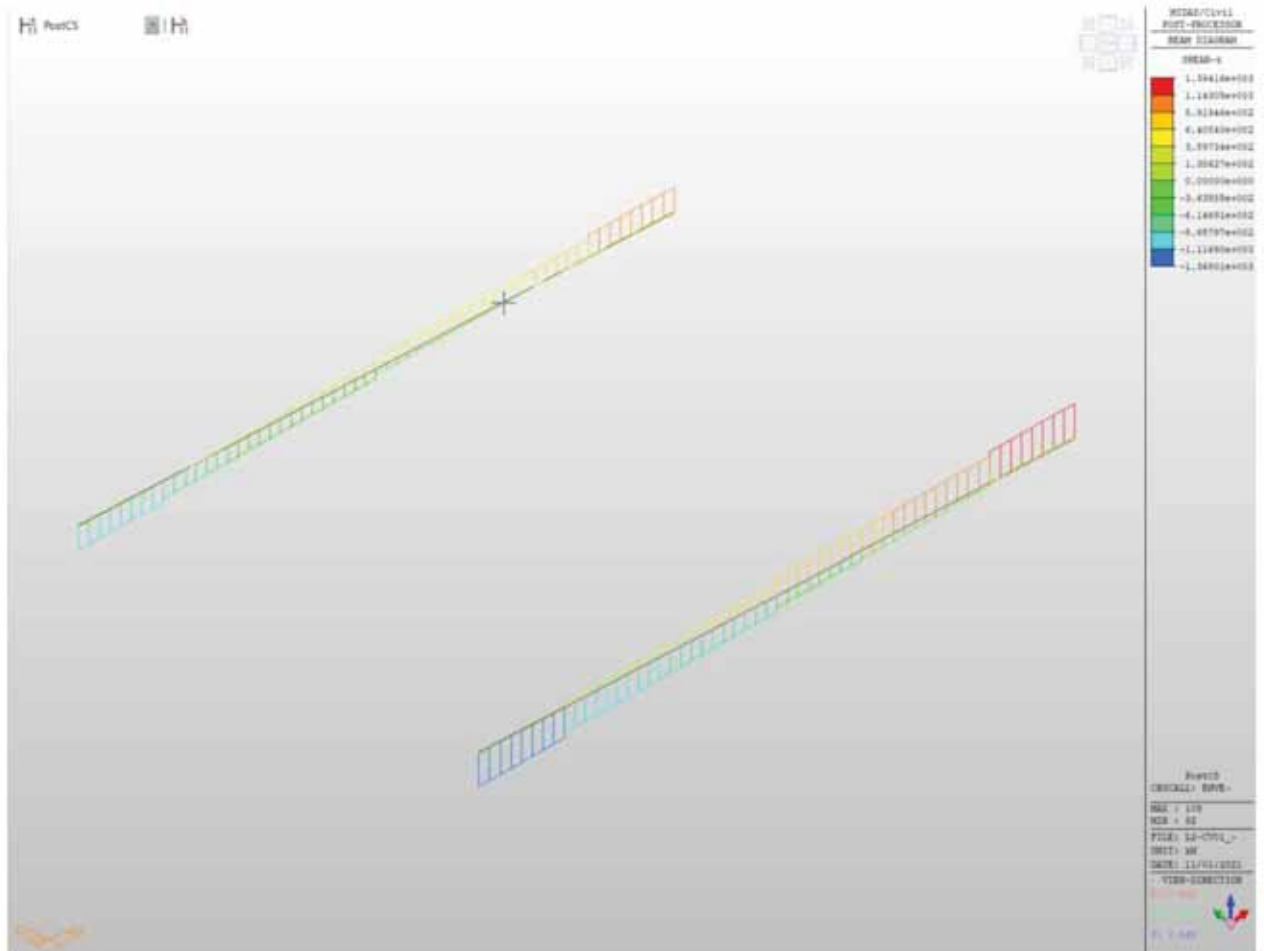


Figura 62 - Concio 3 - Diagramma Tz - Involuppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

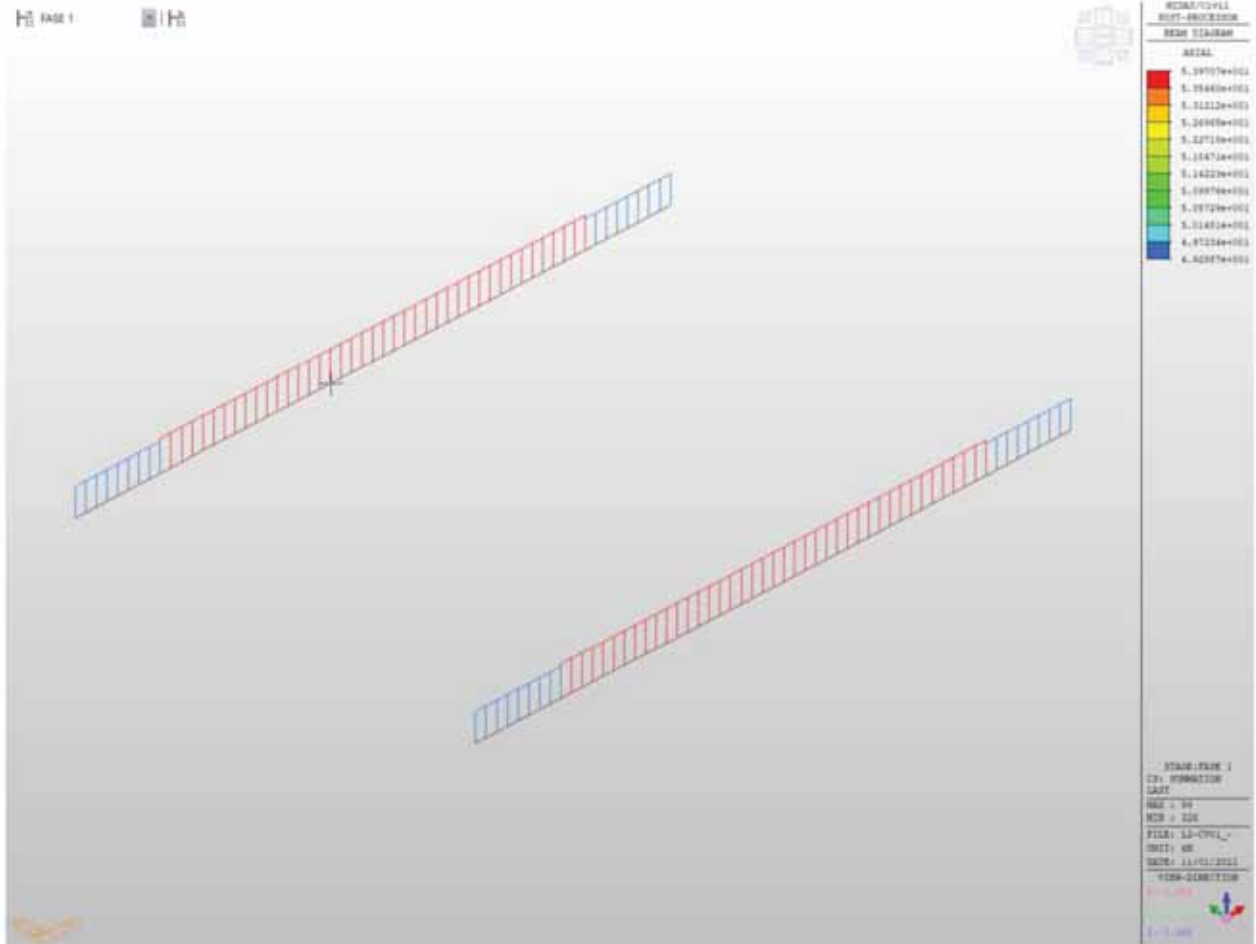


Figura 63 - Concio 3 - Diagramma N - Fase 1

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

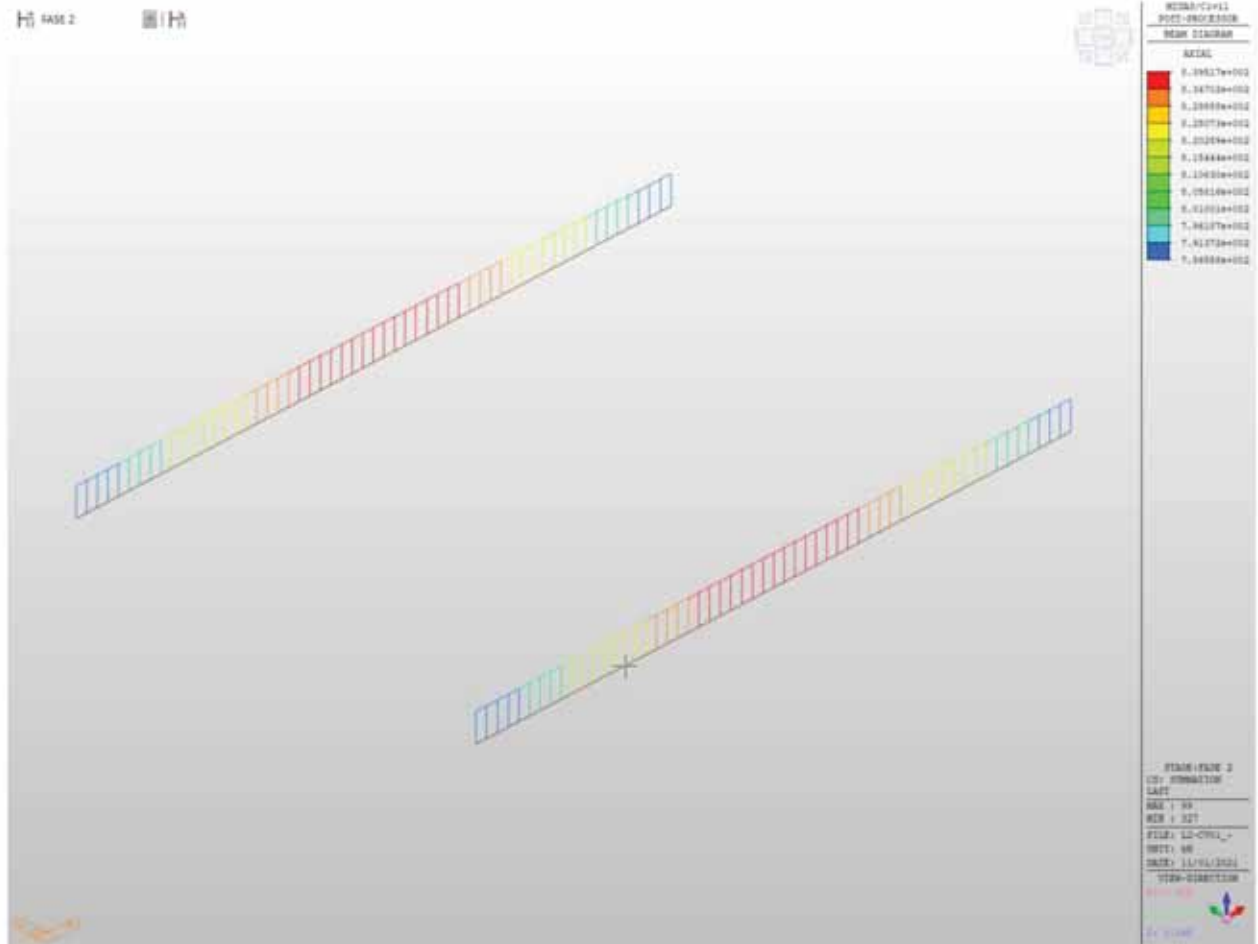


Figura 64 - Concio 3 - Diagramma N - Fase 2

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

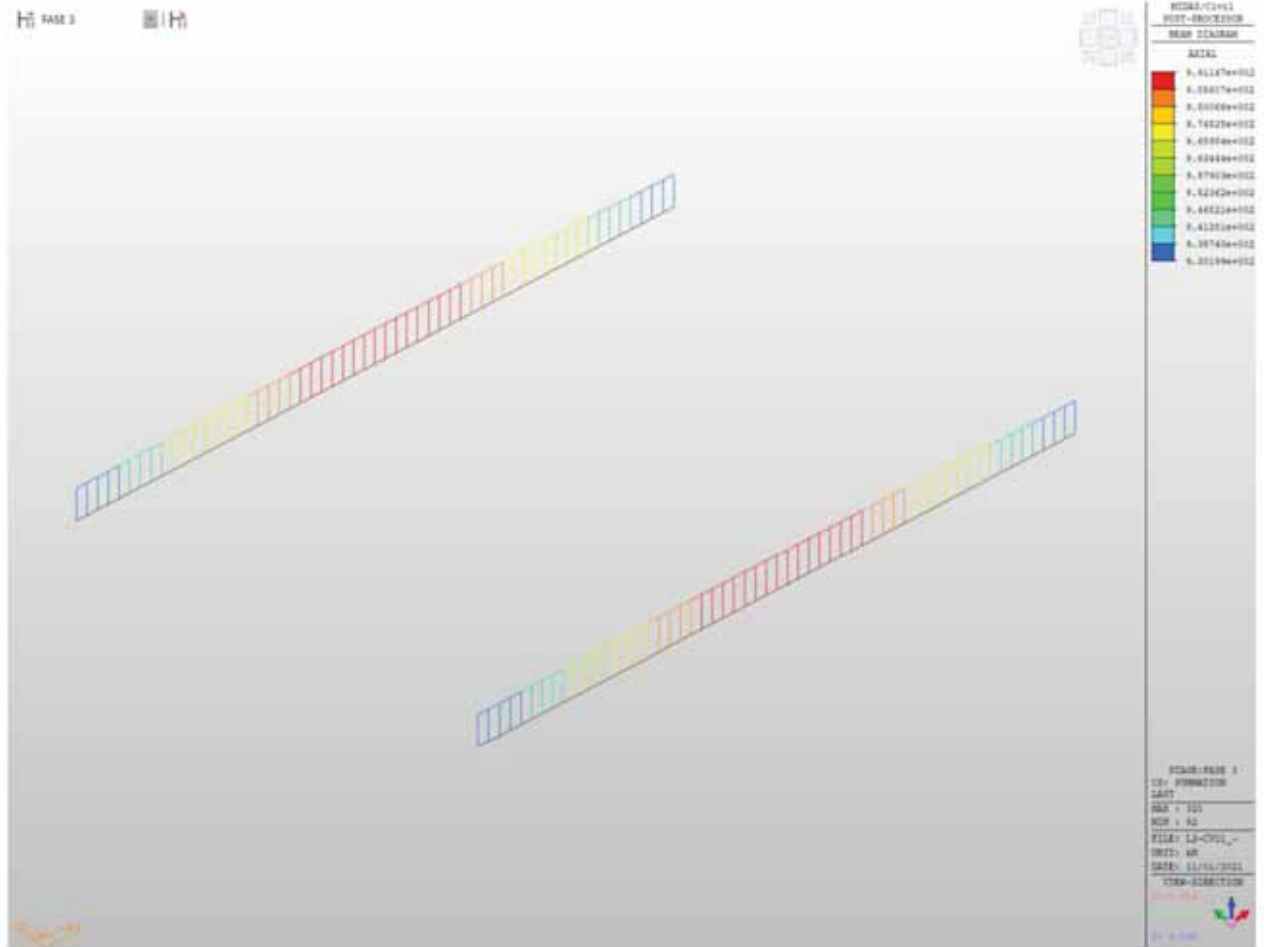


Figura 65 - Concio 3 - Diagramma N - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO**

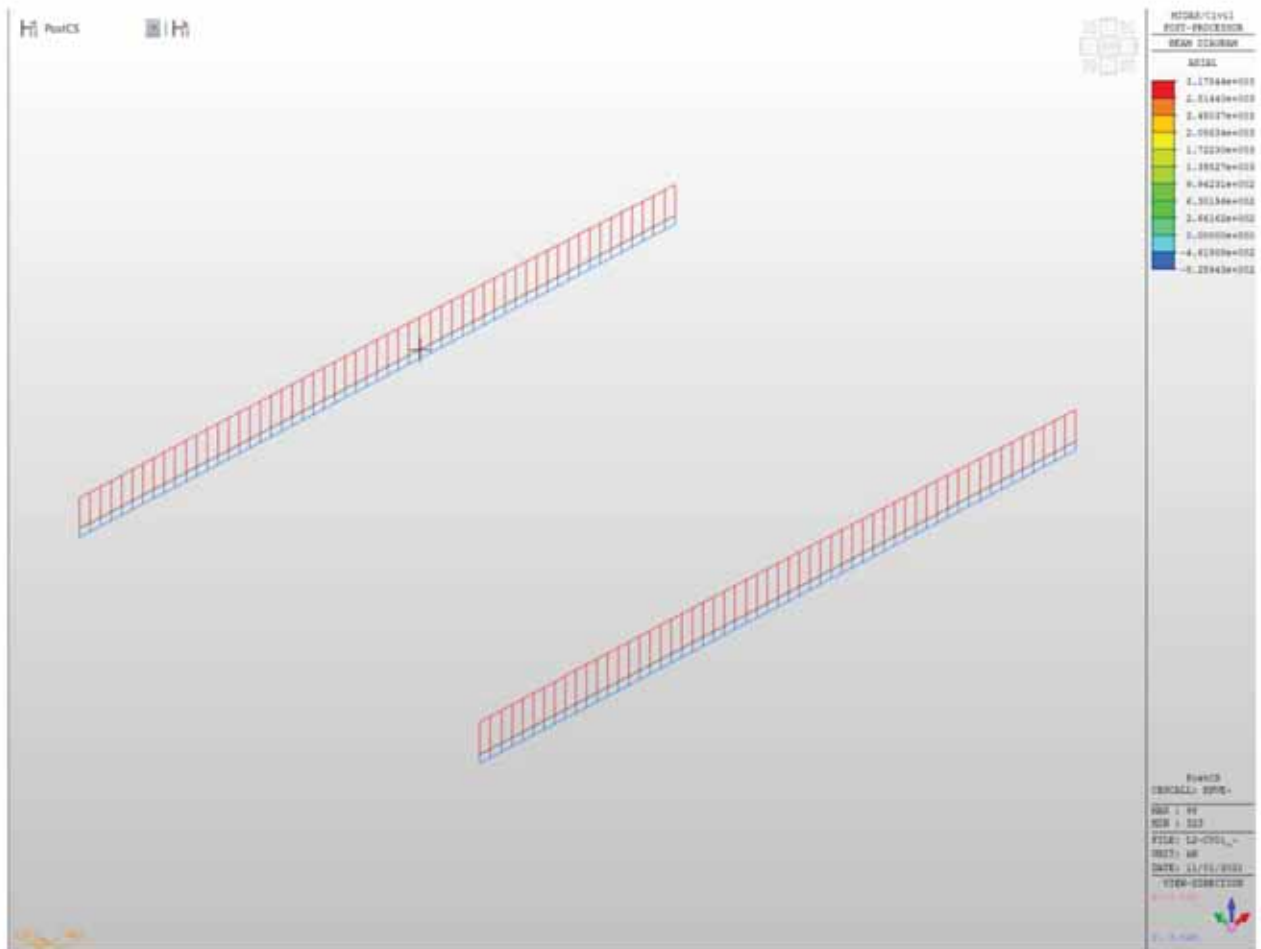
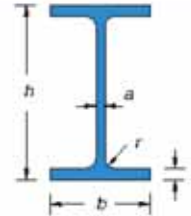


Figura 66 - Concio 3 - Diagramma N - Involuppo SLU-SLV

**VERIFICA SEZIONE IN ACCIAIO COMPOSTA**

**CARATTERISTICHE PROFILO:**

Tipo:	5355			
B <sub>sup</sub> =	700	mm		
t <sub>r, sup</sub> =	30.0	mm	A <sub>t, sup</sub> =	21000.00 mm <sup>2</sup>
B <sub>inf</sub> =	1200	mm		
t <sub>r, inf</sub> =	55.0	mm	A <sub>t, inf</sub> =	66000.00 mm <sup>2</sup>
H =	2000	mm		
t <sub>w</sub> = a =	20.0	mm	A <sub>w</sub> =	38300.00 mm <sup>2</sup>
L =	4100	mm	n. traversi:	1
A <sub>tot</sub> =	125300.00	mm <sup>2</sup>		
Y <sub>0, inf</sub> =	656.65	mm	Y <sub>0, sup</sub> =	1343.35 mm
J <sub>yy</sub> =	7.98E+10	mm <sup>4</sup>	J <sub>t</sub> =	77956667 mm <sup>4</sup>
J <sub>zz</sub> =	8778833333	mm <sup>4</sup>	J <sub>w</sub> =	1.069E+09 mm <sup>6</sup>
W <sub>el, yy, inf</sub> =	121452420	mm <sup>3</sup>	W <sub>el, yy, sup</sub> =	59368341 mm <sup>3</sup>
W <sub>el, yy</sub> =	90288083	mm <sup>3</sup>		
W <sub>p, Lxx</sub> =	$2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$			
ρ <sub>yy</sub> =	797.80	mm	ρ <sub>axx</sub> =	$\sqrt{\frac{J_{xx}}{A}}$
ρ <sub>zz</sub> =	264.69	mm		
Peso =	983.61	kg/m		
E =	210000	N/mm <sup>2</sup>	modulo elastico	
ν =	0.3		coeff. poisson	
G =	80769	N/mm <sup>2</sup>	$G = \frac{E}{2(1 + \nu)}$	modulo elasticità trasversale
α =	0.000012	°C <sup>-1</sup>	coeff. espansione termica lineare	
f <sub>yk</sub> =	355	N/mm <sup>2</sup>	tensione snervamento caratteristica	
f <sub>tk</sub> =	510	N/mm <sup>2</sup>	tensione rottura caratteristica	
γ <sub>M0</sub> =	1.05		per sezioni classe 1, 2, 3 e 4	
γ <sub>M1</sub> =	1.05		per instabilità membrature	
γ <sub>M1</sub> =	1.10		per instabilità membrature ponti stradali e ferroviari	
γ <sub>M2</sub> =	1.25		per sezioni tese indebolite dai fori nei riguardi della frattura	



**SOLLECITAZIONI:**

N <sub>c, inf</sub> =	0	N	caso per trave tesa
N <sub>c, sup</sub> =	840000	N	caso per trave compressa
M <sub>ed</sub> =	9642000000	Nmm	caso per trave inflessa in una direzione
V <sub>ed</sub> =	309000	N	taglio
Caso per trave a flessione deviata (Momenti flettenti agenti alle estremità  M <sub>s</sub>   <  M <sub>o</sub>  ):			
M <sub>o</sub> =	8756000000	Nmm	M <sub>s</sub> = 9642000000 Nmm
Valori del momento massimo e minimo dell'asta:			
M <sub>max, y, ed</sub> =	0	Nmm	M <sub>max, y, ed</sub> = 0 Nmm
M <sub>max, z, ed</sub> =	0	Nmm	M <sub>max, z, ed</sub> = 0 Nmm

**CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1):**

ALA SUPERIORE	ALA INFERIORE	ANIMA
C/t <sub>sup</sub> = 11.33	C/t <sub>inf</sub> = 10.73	C/t <sub>w</sub> = 95.75
c = 0.81	c = 0.81	c = 0.81
Curva di instabilità "b" (Asse yy):		
Fattore di imperfezione α: 0.34		
Curva di instabilità "c" (Asse zz):		
Fattore di imperfezione α: 0.49		

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{ed}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:  $A_v = 39150 \text{ mm}^2$

$$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}} \quad V_{c,Rd} = 7642056 \text{ N} \Rightarrow 0.04 \text{ OK! (<1)}$$

Verifica in presenza di torsione uniforme:

$$\tau_{Lsd} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{Lsd}}{f_{yk}} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 7642056 \text{ N} \Rightarrow 0.04 \text{ OK! (<1)}$$

Verifica in termini tensionali nel punto più sollecitato:

$$\frac{\tau_{ed}}{\frac{f_{yk}}{\sqrt{3} \gamma_{M0}}} \leq 1 \Rightarrow 0.00 \text{ OK! (<1)}$$

**SOLLECITAZIONE DELL'ASTA A COMPRESIONE SEMPLICE (NTC2008, 4.2.4.1.2):**

$$\frac{N_{ed}}{N_{c,Rd}} \leq 1 \quad N_{cr} = \frac{\pi^2 EJ}{(\eta l)^2} = 1082400865 \text{ N} \quad N_{c,Rd} < 0,04 N_{cr}$$

*Instabilità trascurabile!*

$0.04 N_{cr} = 43296035 \text{ N}$  *Instabilità trascurabile se  $\lambda < 0.2$*

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$$\mu = 1.00 \quad l_0 = \mu l = 4100 \text{ mm}$$

$$\lambda = l_0 / \rho = 5$$

Calcolo resistenza per sezioni in classe 1,2 e 3:

$$N_{c,Rd} = \frac{A f_{yk}}{\gamma_{M0}} \quad N_{c,Rd} = 42363333 \text{ N} \Rightarrow 0.02 \text{ OK! (<1)}$$

**STABILITA' DELLE MEMBRATURE (NTC2008, 4.2.4.1.3):**

Curva di instabilità "b" (Asse yy):  
Fattore di imperfezione  $\alpha = 0.49$

Verifica per sezioni classe 1,2 e 3:

$$\bar{\lambda} = \sqrt{\frac{A f_{yk}}{N_{cr}}} = 0.20 \quad \bar{\lambda} > 0.2 \text{ Serve verifica instabilità!}$$

$$\Phi = 0.5 \left[ 1 + \alpha (\bar{\lambda} - 0.2) + \bar{\lambda}^2 \right] = 0.52$$

$$\chi = \frac{1}{\Phi + \sqrt{\Phi^2 - \bar{\lambda}^2}} = 1.00$$

$$N_{b,Rd} = \frac{\chi A f_{yk}}{\gamma_{M1}} = 40381613 \text{ N}$$

$$\frac{N_{ed}}{N_{b,Rd}} \leq 1 \Rightarrow 0.02 \text{ OK! (<1)}$$





**Stabilità dei pannelli soggetti a taglio (NTC2008, 4.2.4.1.3.4.1):**

Se irrigiditori trasversali rigido o no,

$$I_{st} = 45000000 \text{ mm}^4 \quad 0.75hwt^3 = 5710886 \text{ mm}^4 \quad \text{Irrigiditore Trasversali rigidi}$$

Coefficiente minimo di instabilità per taglio del pannello (In assenza di irrigiditori longitudinali),

$$a/hw = 2.05 \quad k_t = 7.29$$

Coefficiente minimo di instabilità per taglio del pannello (irrigiditori longitudinali più di due o  $a/hw > 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad k_{tl} = 0.00$$

$$a/hw = 2.05 \quad k_t = 7.29$$

Coefficiente minimo di instabilità per taglio del pannello (irrigiditori longitudinali più di due o  $a/hw < 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad a/hw = 2.05 \quad k_t = 5.60$$

$$k_t = 7.29$$

$$\eta = 1.2 \quad hw/t = 95.75 \quad \text{Serve la verifica di instabilità}$$

$$\sigma_E = 19 \text{ MPa} \quad \tau_{cr} = 138.53 \quad \lambda_w = 1.22$$

Montanti di appoggio rigidi,

$$\chi_w = 0.71$$

Gli altri casi,

$$\chi_w = 0.68$$

$$V_{bw,Rd} = 5326791 \text{ N}$$

Momento resistente sole piattabande,

$$Y_{G,inf} = 500.00 \text{ mm} \quad Y_{G,sup} = 1500.00 \text{ mm}$$

$$J_{yy} = 61062850000 \text{ mm}^4$$

$$W_{el,yy,inf} = 122125700 \text{ mm}^3 \quad W_{el,yy,sup} = 40708567 \text{ mm}^3$$

$$M_{f,red} = 13370324310 \text{ Nmm}$$

$$bf = 364.5 \text{ mm} \quad V_{bf,Rd} = 29402 \text{ N}$$

$$V_{b,Rd} = 5356193 \text{ N} \quad 0.058 \quad \text{OK! (<1)}$$

Stabilità dei pannelli soggetti a Compressione (NTC2008, 4.2.4.1.3.4.1):					
Stress,					
$\sigma_{sup} =$	162.41	MPa			
$\sigma_{inf} =$	-79.39	MPa			
$\sigma_{irr.} =$	100.75	MPa			
Irrigidati su entrambi i lati (Anima),					
b =	1915.0	mm	(Web bw)	a =	4100 mm
$\sigma_1 =$	162.4	(Max)		$\sigma_2 =$	-79.39 (Min)
$\psi =$	-0.5			$k\sigma =$	13.22
$\lambda_p =$	1.1			$\rho =$	0.87
$\sigma_{cr,p} =$	274.0	MPa		$\sigma_{cr,c} =$	4.516 MPa
$\zeta =$	1.0			$\lambda_c =$	8.866
$\alpha =$	0.34			$\Phi =$	116.298
$\chi_c =$	0.004				
$\rho_c =$	0.87				
$b_{eff} =$	1123.65	mm	$b_{e1} =$	449.46	mm
				$b_{e2} =$	674.19 mm
$b_t =$	628.75	mm			
Irrigidati a un lato (Piattebande), Piattebanda Superiore,					
b =	340	mm	(Flange Sup)		
$\sigma (Irri.) =$	162.4			$\sigma (Lib.) =$	162.41
$k\sigma =$	0.43				
$\lambda_p =$	1.1			$\rho =$	0.74
$b_{eff} =$	251.37	mm		$b_t =$	0 mm
Effective Section Properties,					
$A_{c,eff} (Piattebanda Sup) =$	15082.0	mm <sup>2</sup>			
$A_{c,eff} (Piattebanda Inf) =$	66000.0	mm <sup>2</sup>			
$A_{c,eff} (Anima) =$	22473.1	mm <sup>2</sup>			
$A_c (Anima tensione) =$	12574.9	mm <sup>2</sup>			
$A_{c,eff} (Total) =$	116130.0	mm <sup>2</sup>			
$Y_{g,inf} =$	567.0	mm		$Y_{g,sup} =$	1433.0 mm
$I_{eff} =$	6.99E+10	mm <sup>4</sup>			
$W_{eff} (inf) =$	123356429	mm <sup>3</sup>		$W_{eff} (sup) =$	48814404 mm <sup>3</sup>
e =	89.61	mm			
$\eta =$	0.59				

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Fase 2

Soletta c.a.collaborante

$b_{eff}$ =	3.34	m
Altezza soletta:	0.26	m
Area soletta:	0.87	m <sup>2</sup>
$J_{yy}$ =	0.00489	m <sup>4</sup>
Peso =	21.71	kN/m

Trave acciaio

Area trave acciaio:	0.1253	m <sup>2</sup>
H trave acciaio:	2.00	m
$Y_c$ =	0.657	m
$J_{yy}$ =	0.079752	m <sup>4</sup>
Peso =	9.84	kN/m

Barre armatura soletta

1° strato barre (superiore)		2° strato barre (inferiore)	
n. Barre:	17	n. Barre:	17
Interasse:	0.20 m	Interasse:	0.20 m
Diametro:	0.02 m	Diametro:	0.02 m
Area:	0.005341 m <sup>2</sup>	Area:	0.005341 m <sup>2</sup>
$Y_1$ =	0.05 m	$Y_2$ =	0.21 m

n = 18.0

Coeff. omogeneizzazione con viscosità in atto

Materiali

cls	Barre armature	Acciaio
$R_{ck}$ = 40 MPa	$f_{yk}$ = 450 MPa	$f_{yk}$ = 355 MPa
$f_{ck}$ = 33.20 MPa	$\gamma_M$ = 1.15	$\gamma_{M0}$ = 1.05
$f_{cd}$ = 18.81 MPa	$f_{yk,s}$ = 391.30 MPa	$f_{yk,s}$ = 338.10 MPa
$f_{ctm}$ = 41.2 MPa		
$f_{ctm}$ = 3.10 MPa		
$E_{cm}$ = 33643 MPa		

Caratteristiche geometriche

$A_{oi}$ =	0.1842	m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
$A_v$ =	39150	mm <sup>2</sup>	Area taglio sezione di acciaio
$Y_{G,inf,oi}$ =	1.13	m	Asse neutro rispetto lembo inferiore:
$Y_{G,sup,oi}$ =	1.13	m	Asse neutro rispetto lembo superiore
$J_{xx,oi}$ =	0.167092	m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{inf,oi}$ =	0.148142	m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
$W_{sup,oi}$ =	0.147596	m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

Sollecitazioni

$M_{oi}$ =	12465.00	kNm	(SLU)	
$M_{oi}$ =	49901.58	kNm	=>	0.25 ok!
$T_{oi}$ =	391.00	kN	(SLU)	
$T_{oi}$ =	7278.15	kN	=>	0.05 ok!

Tensioni

$\sigma_c$ =	4.69	MPa	=>	0.25 ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,sup}$ =	80.72	MPa	=>	0.21 ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf}$ =	68.79	MPa	=>	0.18 ok!(<1)	Tensione barre inferiori soletta
$\sigma_{a,sup}$ =	65.06	MPa	=>	0.19 ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{a,inf}$ =	84.14	MPa	=>	0.25 ok!(<1)	Tensione acciaio lembo inferiore

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Fase 3

Soletta c.a. collaborante

$b_{eff}$ =	3.34	m
Altezza soletta:	0.26	m
Area soletta:	0.87	m <sup>2</sup>
$J_{xx}$ =	0.00489	m <sup>4</sup>
Peso =	21.71	kN/m

Trave acciaio

Area trave acciaio:	0.1253	m <sup>2</sup>
H trave acciaio:	2.00	m
$Y_{ei}$ =	0.657	m
$J_{xx}$ =	0.079752	m <sup>4</sup>
Peso =	9.84	kN/m

Barre armatura soletta

1° strato barre (superiore)		2° strato barre (inferiore)	
n. Barre:	17	n. Barre:	17
Interasse:	0.20 m	Interasse:	0.20 m
Diametro:	0.02 m	Diametro:	0.02 m
Area:	0.005341 m <sup>2</sup>	Area:	0.005341 m <sup>2</sup>
$Y_1$ =	0.05 m	$Y_2$ =	0.21 m

n = 6.2      Coeff. omogeneizzazione a tempo infinito

Materiali

cls	Barre armature	Acciaio
$R_{ck}$ = 40 MPa	$f_{yk}$ = 450 MPa	$f_{yk}$ = 355 MPa
$f_{ck}$ = 33.20 MPa	$\gamma_M$ = 1.15	$\gamma_{MO}$ = 1.05
$f_{cd}$ = 18.81 MPa	$f_{yck}$ = 391.30 MPa	$f_{yck}$ = 338.10 MPa
$f_{cm}$ = 41.2 MPa		
$f_{ctm}$ = 3.10 MPa		
$E_{cm}$ = 33643 MPa		

Caratteristiche geometriche

$A_{cl}$ = 0.2760 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
$A_s$ = 39150 mm <sup>2</sup>	Area taglio sezione di acciaio
$Y_{cl,inf}$ = 1.46 m	Asse neutro rispetto lembo inferiore:
$Y_{cl,sup}$ = 0.80 m	Asse neutro rispetto lembo superiore
$J_{xx,cl}$ = 0.229143 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{pl,cl}$ = 0.156815 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
$W_{pl,s}$ = 0.286871 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

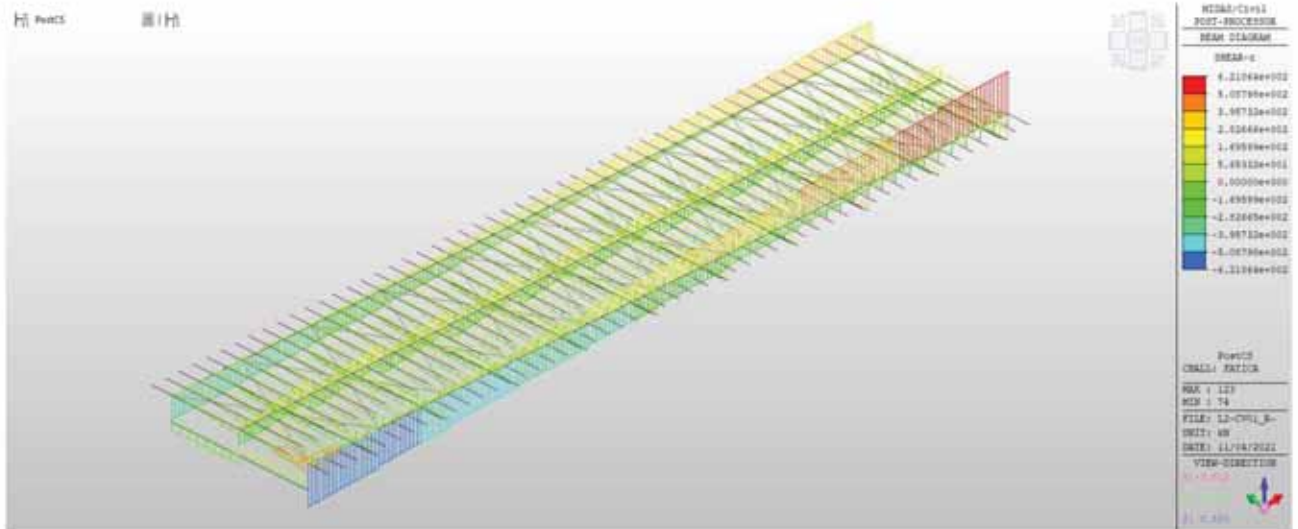
Sollecitazioni

$M_{cl}$ = 33408.00 kNm	(SLU)	
$M_{s,inf}$ = 53018.38 kNm	=>	0.63 ok!
$T_{cl}$ = 1394.00 kN	(SLU)	
$T_{s,inf}$ = 7278.15 kN	=>	0.19 ok!

Tensioni

$\sigma_c$ = 18.78 MPa	=>	0.998 ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,sup}$ = 109.17 MPa	=>	0.28 ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf}$ = 85.84 MPa	=>	0.22 ok!(<1)	Tensione barre inferiori soletta
$\sigma_{a,sup}$ = 78.55 MPa	=>	0.23 ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{a,inf}$ = 213.04 MPa	=>	0.63 ok!(<1)	Tensione acciaio lembo inferiore





Si procede con il calcolo delle variazioni di tensioni sulla trave in acciaio dovute al transito dei carichi suddetti.

#### Sollecitazioni

$M_{sd}$	6358,28	kNm	(SLU)		
$M_{rd}$	53018,38	kNm	=>	0,12	ok!
$T_{sd}$	621,86	kN	(SLU)		
$T_{rd}$	7278,15	kN	=>	0,09	ok!

#### Tensioni

$\sigma_{s,sup}$	20,78	MPa	=>	0,05	ok!(<1)
$\sigma_{s,inf}$	16,34	MPa	=>	0,04	ok!(<1)
$\sigma_{a,sup}$	14,95	MPa	=>	0,04	ok!(<1)
$\sigma_{a,inf}$	40,55	MPa	=>	0,12	ok!(<1)

#### VERIFICHE PER VITA ILLIMITATA

Le verifiche a fatica per vita illimitata possono essere condotte controllando che il massimo delta di tensione  $\Delta s_{max}$  indotto nel dettaglio stesso dallo spettro di carico risulti minore del limite di fatica del dettaglio stesso:

$$\Delta s_{max,d} = \gamma_{Mf} \Delta s_{max} \leq \Delta s_{\Delta}$$



oppure

$$\Delta t_{max,d} = \gamma_{Mf} \Delta t_{max} < \Delta t_D = \Delta t_L$$

$$\Delta s_D = 0,737 \Delta s_C$$

$\gamma_{Mf} = 1,35$  (strutture sensibili alla rottura per fatica - conseguenze significative)

**Travi in composizione saldata**

100	 	<p>5) Saldatura manuale a cordoni d'angolo o a piena penetrazione</p> <p>6) Saldatura a piena penetrazione manuale o automatica eseguita da un sol lato, in particolare per travi a cassone</p>	<p>5) e 6) Deve essere assicurato un corretto contatto tra anima e piattabanda. Il bordo dell'anima deve essere preparato in modo da garantire una penetrazione regolare alla radice, senza interruzioni</p>
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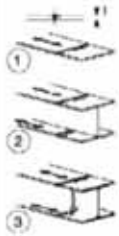



Delta massimo di tensione attacco anima-ala inf:

$$\Delta s_{max} = 40,55 \text{ MPa}$$

Limiti di fatica ad ampiezza costante:  $\Delta s_D = 0,737 \times 100 = 73,70 \text{ MPa}$

Verifica a fatica:  $\gamma_{Mf} \Delta s_{max} < \Delta s_D$   $1,35 \times 40,55 = 54,74 \text{ MPa} < 73,70 \text{ MPa} = \Delta s_D$

**Saldature di testa trasversali**

112	   	<p>Saldature senza piatto di sostegno</p> <ol style="list-style-type: none"> <li>Giunti trasversali in piatti e lamiere</li> <li>Giunti di anime e piattabande in travi composte eseguiti prima dell'assemblaggio</li> <li>Giunti trasversali completi di profili laminati, in assenza di lunette di scarico</li> <li>Giunti trasversali di lamiere e piatti con rastremazioni in larghezza e spessore con pendenza non maggiore di 1:4. Nelle zone di transizione gli intagli nelle saldature devono essere eliminati</li> </ol> <p>Per spessori <math>t &gt; 25 \text{ mm}</math>, si deve adottare una classe ridotta del coefficiente</p> $k_s = (25/t)^{1,2}$	<p>Saldature effettuate da entrambi i lati, molate in direzione degli sforzi e sottoposte a controlli non distruttivi</p> <p>Le saldature devono essere iniziate e terminate su tacchi d'estremità, da rimuovere una volta completata la saldatura</p> <p>I bordi esterni delle saldature devono essere molati in direzione degli sforzi</p> <p>3) Vale solo per profilati tagliati e risaldati</p>
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Delta massimo di tensione attacco anima-ala inf:

$$D s_{max} = 40,55 \text{ MPa}$$

Limiti di fatica ad ampiezza costante:  $\Delta s_D = 0,737 \times 112 = 82,54 \text{ MPa}$

Verifica a fatica:  $\gamma_{Mf} \Delta s_{max} < \Delta s_D$   $1,35 \times 40,55 = 54,74 \text{ MPa} < 82,54 \text{ MPa} = D s_D$



## 6.2.6 TRAVE DI SPINA

Si riporta di seguito la verifica della trave di spina. La trave presenta un valore di momento flettente di involucro positivo e negativo lungo tutto il suo sviluppo per effetto dell'involuppo delle azioni statiche e sismiche. In particolare, le azioni sismiche verticali, producono proprio un effetto di inversione di momento. Per tale motivo a favore di sicurezza si verifica la trave di spina solo come elemento in acciaio trascurando l'effetto della soletta collaborante.

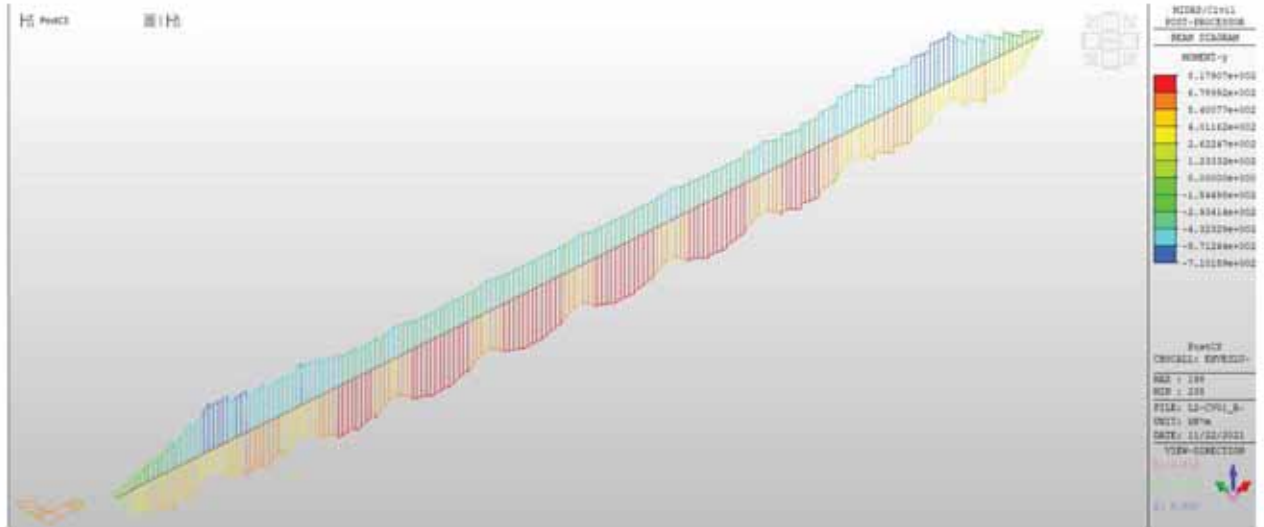


Figura 67- Trave di spina - Diagramma My - Involuppo SLU-SLV

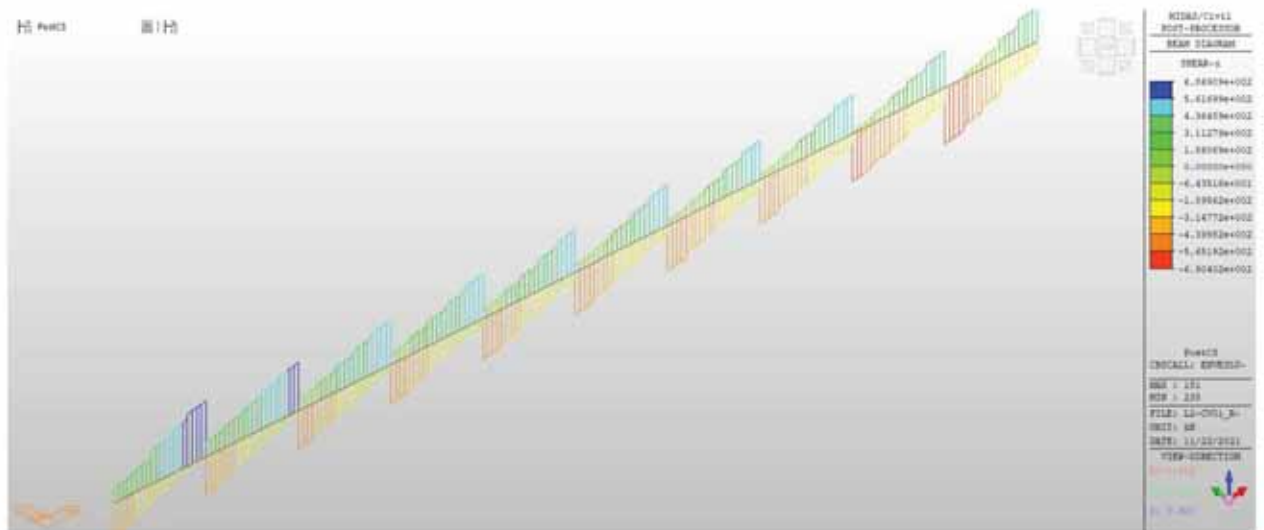


Figura 68 - Trave di spina - Diagramma Tz - Involuppo SLU-SLV

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

CARATTERISTICHE PROFILO:				
Tipo:	S355			
B <sub>sup</sub> =	300	mm		
t <sub>r, sup</sub> =	25.0	mm	A <sub>c, sup</sub> = 7500.00 mm <sup>2</sup>	
B <sub>inf</sub> =	300	mm	A <sub>c, inf</sub> = 7500.00 mm <sup>2</sup>	
t <sub>r, inf</sub> =	25.0	mm	A <sub>w</sub> = 7020.00 mm <sup>2</sup>	
H =	590	mm	n. traversi: 1	
t <sub>w</sub> = a =	13.0	mm		
L =	4100	mm		
A <sub>tot</sub> =	22020.00	mm <sup>2</sup>		
Y <sub>0, inf</sub> =	295.00	mm	Y <sub>0, sup</sub> = 295.00 mm	
J <sub>xx</sub> =	1368461000	mm <sup>4</sup>	J <sub>y</sub> = 3520460 mm <sup>4</sup>	
J <sub>yy</sub> =	112608019	mm <sup>4</sup>	J <sub>z</sub> = 5.070E+11 mm <sup>6</sup>	
W <sub>el, inf</sub> =	4638851	mm <sup>3</sup>	W <sub>el, sup</sub> = 4638851 mm <sup>3</sup>	
W <sub>pl</sub> =	5185200	mm <sup>3</sup>		
W <sub>pl, inf</sub> =	$2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$			
ρ <sub>xx</sub> =	249.29	mm	ρ <sub>yy</sub> = $\sqrt{\frac{J_{yy}}{A}}$	
ρ <sub>yy</sub> =	71.51	mm		
Peso =	172.86	kg/m		
E =	210000	N/mm <sup>2</sup>	modulo elastico	
ν =	0.3		coeff. poisson	
G =	80769	N/mm <sup>2</sup>	G = $\frac{E}{2(1+\nu)}$ modulo elasticità trasversale	
α =	0.000012	°C <sup>-1</sup>	coeff. espansione termica lineare	
f <sub>yk</sub> =	355	N/mm <sup>2</sup>	tensione snervamento caratteristica	
f <sub>tk</sub> =	510	N/mm <sup>2</sup>	tensione rottura caratteristica	
γ <sub>M0</sub> =	1.05		per sezioni classe 1,2,3 e 4	
γ <sub>M1</sub> =	1.05		per instabilità membrature	
γ <sub>M1</sub> =	1.10		per instabilità membrature ponti stradali e ferroviari	
γ <sub>M2</sub> =	1.25		per sezioni tese indebolite dai fori nei riguardi della frattura	
SOLLECITAZIONI:				
N <sub>1, sd</sub> =	0	N	caso per trave tesa	
N <sub>c, sd</sub> =	0	N	caso per trave compressa	
M <sub>sd</sub> =	817910000	Nmm	caso per trave inflessa in una direzione	
V <sub>sd</sub> =	690410	N	taglio	
Caso per trave a flessione deviata:				
M <sub>y, sd</sub> =	0	Nmm	M <sub>z, sd</sub> = 0 Nmm	
Valori del momento massimo e minimo dell'asta:				
M <sub>max, y, sd</sub> =	0	Nmm	M <sub>min, y, sd</sub> = 0 Nmm	
M <sub>max, z, sd</sub> =	0	Nmm	M <sub>min, z, sd</sub> = 0 Nmm	
CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1):				
<b>ALA SUPERIORE</b>		<b>ALA INFERIORE</b>		<b>ANIMA</b>
C/t <sub>sup</sub> =	5.74	C/t <sub>inf</sub> =	5.74	C/t <sub>w</sub> = 41.54
c =	0.81	c =	0.81	c = 0.81
Curva di instabilità "b":				
Fattore di imperfezione α:		0.34		
<input checked="" type="radio"/> Parte soggetta a flessione <input type="radio"/> Parte soggetta a compressione <input type="radio"/> Parte soggetta a flessione ed a compressione				<b>ANIMA</b> <b>Classe 1</b>

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):					
$\frac{V_{sd}}{V_{c,Rd}} \leq 1$					
Area taglio resistente:		$A_v =$	7345	mm <sup>2</sup>	
$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}}$	$V_{c,Rd} =$	1433739	N	$\Rightarrow$	0.48 <b>OK!(&lt;1)</b>

SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):					
$\frac{M_{sd}}{M_{c,Rd}} \leq 1$					
$V_{sd} < 0,5 V_{c,Rd}$	<i>Il taglio non influisce sulla verifica a flessione!</i>				
$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2$	0.00136	$f_{y,red} = (1 - \rho)f_{yk}$	355	N/mm <sup>2</sup>	
		$f_{yk} =$	355	N/mm <sup>2</sup>	
Calcolo resistenza plastica sezione lorda, classe 1 e 2:					
$M_{pl,Rd} = \frac{W_{pl} f_{yk}}{\gamma_{M0}}$	$M_{pl,Rd} =$	1753091429	Nmm	$\Rightarrow$	0.47 <b>OK!(&lt;1)</b>
Calcolo resistenza elastica sezione lorda, classe 3:					
$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}}$	$M_{el,Rd} =$	1568373382	Nmm	$\Rightarrow$	0.52 <b>OK!(&lt;1)</b>

### 6.2.7 TRAVERSO

Si riportano di seguito i diagrammi di sollecitazione e di verifica dei traversi. A favore di sicurezza si considerano le sollecitazioni estremali mentre per la sezione resistente si considera quella relativa ai traversi interni.

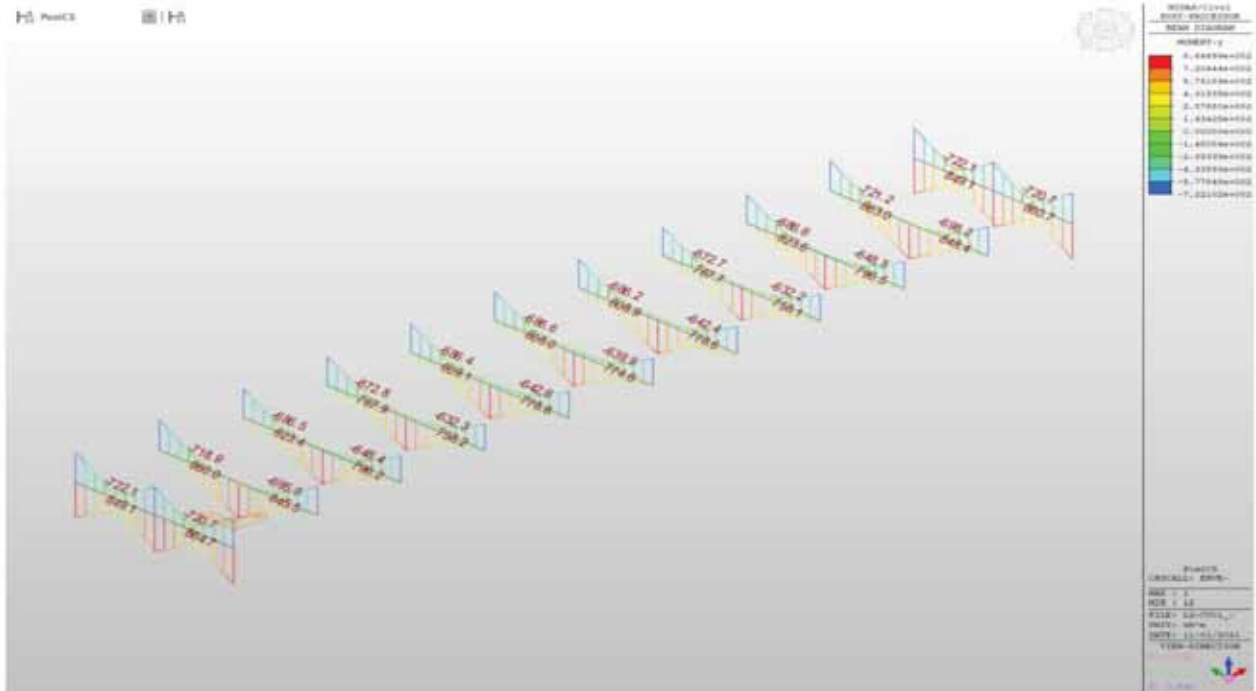


Figura 69 – Traverso - Momento SLU-SLV

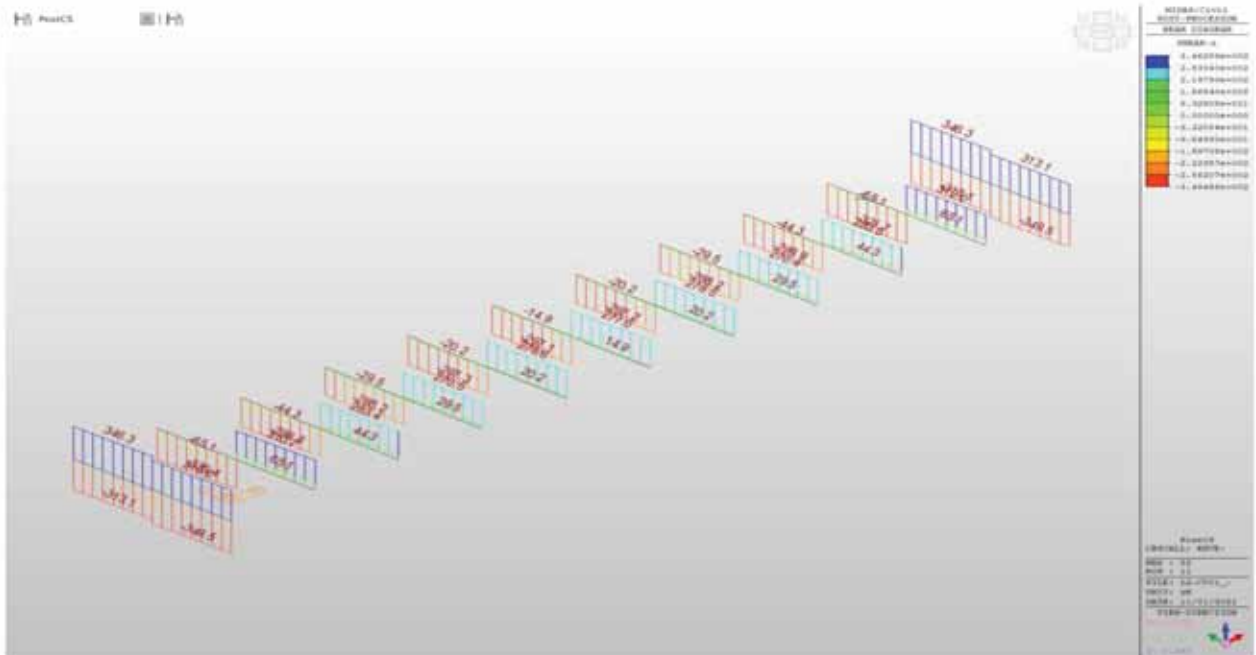


Figura 70 – Traverso - Taglio SLU-SLV

### 6.2.7.1 VERIFICA TRAVERSO DI CAMPATA

CARATTERISTICHE PROFILO:				
Tipo:	5355			
$B_{s,up}$	350	mm		
$t_{s,up}$	25,0	mm	$A_{s,up} = 8750,00$ mm <sup>2</sup>	
$B_{s,inf}$	350	mm	$A_{s,inf} = 8750,00$ mm <sup>2</sup>	
$t_{s,inf}$	25,0	mm	$A_w = 17000,00$ mm <sup>2</sup>	
H	900	mm	n. traversi: 1	
$t_w = a$	20,0	mm		
L	5000	mm		
$A_{TOT}$	34500,00	mm <sup>2</sup>		
$Y_{0,inf}$	450,00	mm	$Y_{0,up} = 450,00$ mm	
$J_{xx}$	4374062500	mm <sup>4</sup>	$J_y = 5912500$ mm <sup>4</sup>	
$J_{yy}$	179245833	mm <sup>4</sup>	$J_{\omega} = 2,731E+14$ mm <sup>6</sup>	
$W_{el,inf}$	9720139	mm <sup>3</sup>	$W_{el,up} = 9720139$ mm <sup>3</sup>	
$W_{pl}$	11268750	mm <sup>3</sup>		
$W_{pLxx} = 2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$				
$\rho_a$	356,07	mm	$\rho_{ax} = \sqrt{\frac{J_{xx}}{A}}$	
$\rho_w$	72,08	mm		
Peso	270,83	kg/m		
E	210000	N/mm <sup>2</sup>	modulo elastico	
$\nu$	0,3		coeff. poisson	
G	80769	N/mm <sup>2</sup>	$G = \frac{E}{2(1 + \nu)}$ modulo elasticità trasversale	
$\alpha$	0,000012	°C <sup>-1</sup>	coeff. espansione termica lineare	
$f_{tk}$	355	N/mm <sup>2</sup>	tensione snervamento caratteristica	
$f_{tk}$	510	N/mm <sup>2</sup>	tensione rottura caratteristica	
$\gamma_{M0}$	1,05		per sezioni classe 1,2,3 e 4	
$\gamma_{M1}$	1,05		per instabilità membrature	
$\gamma_{M1}$	1,10		per instabilità membrature ponti stradali e ferroviari	
$\gamma_{M2}$	1,25		per sezioni tese indebolite dai fori nei riguardi della frattura	
SOLLECITAZIONI:				
$N_{c,ld}$	0	N	caso per trave tesa	
$N_{c,ld}$	0	N	caso per trave compressa	
$M_{b,ld}$	860000000	Nmm	caso per trave inflessa in una direzione	
$V_{b,ld}$	312000	N	taglio	
Caso per trave a flessione deviata:				
$M_{x,ld}$	0	Nmm	$M_{y,ld} = 0$ Nmm	
Valori del momento massimo e minimo dell'asta:				
$M_{max,x,ld}$	0	Nmm	$M_{max,y,ld} = 0$ Nmm	
$M_{min,x,ld}$	0	Nmm	$M_{min,y,ld} = 0$ Nmm	
CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1):				
ALA SUPERIORE		ALA INFERIORE		ANIMA
$C/t_{s,up}$	6,60	$C/t_{s,inf}$	6,60	$C/t_w = 42,50$
$\epsilon$	0,81	$\epsilon$	0,81	$\epsilon = 0,81$
Curva di instabilità "b":				
Fattore di imperfezione $\alpha$ :		0,34		ANIMA
				Classe 1
<input checked="" type="radio"/> Parte soggetta a flessione <input type="radio"/> Parte soggetta a compressione <input type="radio"/> Parte soggetta a flessione ed a compressione				

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{sd}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:

$$A_v = 17500 \text{ mm}^2$$

$$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}}$$

$$V_{c,Rd} = 3415989 \text{ N} \Rightarrow 0,09 \text{ OK! (<1)}$$

Verifica in presenza di torsione uniforme:

$$\tau_{t,sd} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,sd}}{f_{yk}} \frac{\gamma_{M0}}{\sqrt{3}} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 3415989 \text{ N} \Rightarrow 0,09 \text{ OK! (<1)}$$

**SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):**

$$\frac{M_{sd}}{M_{c,Rd}} \leq 1$$

$$V_{sd} < 0,5 V_{c,Rd}$$

Il taglio non influisce sulla verifica a flessione!

$$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2$$

$$0,66803$$

$$f_{y,red} = (1 - \rho) f_{yk} = 118 \text{ N/mm}^2$$

$$f_{yk} = 355 \text{ N/mm}^2$$

Calcolo resistenza plastica sezione lorda, classe 1 e 2:

$$M_{pl,Rd} = \frac{W_{pl} f_{yk}}{\gamma_{M0}}$$

$$M_{pl,Rd} = 3809910714 \text{ Nmm} \Rightarrow 0,23 \text{ OK! (<1)}$$

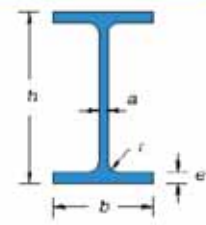
Calcolo resistenza elastica sezione lorda, classe 3:

$$M_{el,Rd} = \frac{W_{el, min} f_{yk}}{\gamma_{M0}}$$

$$M_{el,Rd} = 3286332672 \text{ Nmm} \Rightarrow 0,26 \text{ OK! (<1)}$$

### 6.2.7.2 VERIFICA TRAVERSO DI TESTATA

CARATTERISTICHE PROFILO:			
Tipi:	5355		
$B_{sup}$	350	mm	
$t_{sup}$	25,0	mm	$A_{sup} = 8750,00$ mm <sup>2</sup>
$B_{inf}$	350	mm	
$t_{inf}$	25,0	mm	$A_{inf} = 8750,00$ mm <sup>2</sup>
H	1300	mm	
$t_w = a$	20,0	mm	$A_w = 25000,00$ mm <sup>2</sup>
L	8000	mm	n. traversi: 1
$A_{TOT}$	42500,00	mm <sup>2</sup>	
$Y_{G,inf}$	650,00	mm	$Y_{G,sup} = 650,00$ mm
$J_{xx}$	10368229167	mm <sup>4</sup>	$J_y = 6979167$ mm <sup>4</sup>
$J_{yy}$	179512500	mm <sup>4</sup>	$J_x = 2,731E+14$ mm <sup>6</sup>
$W_{el,inf}$	15951122	mm <sup>3</sup>	$W_{el,sup} = 15951122$ mm <sup>3</sup>
$W_{pl}$	18968750	mm <sup>3</sup>	
$W_{pl,xx} = 2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{2r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$			
$\rho_{xx}$	493,92	mm	$\rho_{xx} = \sqrt{\frac{I_{xx}}{A}}$
$\rho_{yy}$	64,99	mm	
Peso	333,63	kg/m	
E	210000	N/mm <sup>2</sup>	modulo elastico
$\nu$	0,3		coeff. poisson
G	80769	N/mm <sup>2</sup>	$G = \frac{E}{2(1 + \nu)}$ modulo elasticità trasversale
$\alpha$	0,000012	°C <sup>-1</sup>	coeff. espansione termica lineare
$f_{yk}$	355	N/mm <sup>2</sup>	tensione snervamento caratteristica
$f_{tk}$	510	N/mm <sup>2</sup>	tensione rottura caratteristica
$\gamma_{M0}$	1,05		per sezioni classe 1,2,3 e 4
$\gamma_{M1}$	1,05		per instabilità membrature
$\gamma_{M2}$	1,10		per instabilità membrature ponti stradali e ferroviari
$\gamma_{M3}$	1,25		per sezioni tese indebolite dai fori nei riguardi della frattura



SOLLECITAZIONI:			
$N_{s,d}$	0	N	caso per trave tesa
$N_{c,d}$	0	N	caso per trave compressa
$M_{s,d}$	864700000	Nmm	caso per trave inflessa in una direzione
$V_{s,d}$	350000	N	taglio
Caso per trave a flessione deviata:			
$M_{y,d}$	0	Nmm	$M_{z,d} = 0$ Nmm
Valori del momento massimo e minimo dell'asta:			
$M_{max,y,d}$	0	Nmm	$M_{max,z,d} = 0$ Nmm
$M_{min,y,d}$	0	Nmm	$M_{min,z,d} = 0$ Nmm

CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1):				
<b>ALA SUPERIORE</b>		<b>ALA INFERIORE</b>		<b>ANIMA</b>
$C/t_{sup}$	6,60	$C/t_{inf}$	6,60	$C/t_w = 62,50$
$\epsilon$	0,81	$\epsilon$	0,81	$\epsilon = 0,81$
Curva di instabilità "b":				
Fattore di imperfezione $\alpha$ :		0,34		<b>ANIMA</b>
				<b>Classe 2</b>
<input checked="" type="radio"/> Parte soggetta a flessione <input type="radio"/> Parte soggetta a compressione <input type="radio"/> Parte soggetta a flessione ed a compressione				

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{sd}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:

$$A_v = 25500 \text{ mm}^2$$

$$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}} \quad V_{c,Rd} = 4977584 \text{ N} \quad \Rightarrow \quad 0,07 \quad \text{OK! (<1)}$$

Verifica in presenza di torsione uniforme:

$$\tau_{t,sd} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,sd}}{f_{yk}} \frac{1}{\sqrt{3} \gamma_{M0}} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 4977584 \text{ N} \quad \Rightarrow \quad 0,07 \quad \text{OK! (<1)}$$

**SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):**

$$\frac{M_{sd}}{M_{c,Rd}} \leq 1$$

$$V_{sd} < 0,5 V_{c,Rd}$$

*Il taglio non influisce sulla verifica a flessione!*

$$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2$$

$$0,73852$$

$$f_{y,red} = (1 - \rho) f_{yk} = 93 \text{ N/mm}^2$$

$$f_{yk} = 355 \text{ N/mm}^2$$

Calcolo resistenza plastica sezione lorda, classe 1 e 2:

$$M_{pl,Rd} = \frac{W_{pl} f_{yk}}{\gamma_{M0}} \quad M_{pl,Rd} = 6413244048 \text{ Nmm} \quad \Rightarrow \quad 0,13 \quad \text{OK! (<1)}$$

Calcolo resistenza elastica sezione lorda, classe 3:

$$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}} \quad M_{el,Rd} = 5392998321 \text{ Nmm} \quad \Rightarrow \quad 0,16 \quad \text{OK! (<1)}$$



## 6.2.8 VERIFICA PIOLI

Si riporta nel seguito la verifica del sistema di collegamento fra trave metallica e soletta gettata in opera. Il sistema di connessione è realizzato mediante pioli di tipo Nelson resistenti a taglio, saldati alla piattabanda superiore della trave principale ed annegati nella soletta in calcestruzzo. Si ritiene che l'intera forza di scorrimento sia assorbita solo dalla piolatura predisposta sulle travi principali di impalcato, trascurando, a favore di sicurezza il contributo della piolatura posta sulla trave di spina. Il dimensionamento dei connettori è stato eseguito in riferimento alle prescrizioni indicate dalla normativa. Scegliendo di inserire i pioli a passo costante lungo tutto lo sviluppo della trave, si è scelto di determinare la forza di scorrimento applicata tramite l'integrazione delle tensioni indotte sulla soletta collaborante considerando le tensioni indotte ai lembi della soletta stessa.

La collaborazione tra la trave metallica e la soletta è assicurata mediante pioli elettrosaldati all'ala della trave di acciaio. Per le verifiche si fa riferimento al punto 4.3.4.3.1.2 delle NTC.

$$P_{sds} = 0,8 f_u (\pi d^2 / 4) / \gamma_V \quad [4.3.9]$$

$$P_{sds} = 0,29 \alpha d^2 (f_{ck} E_{cm})^{0,5} / \gamma_V \quad [4.3.10]$$

In cui:

$\gamma_V$  è il fattore parziale definito al § 4.3.3;

$f_{tk}$  è la resistenza caratteristica a rottura dell'acciaio del piolo (comunque  $f_{tk} \leq 500$  MPa);

$f_{ck}$  è la resistenza cilindrica caratteristica del calcestruzzo della soletta;

$E_{cm}$  è il valore medio del modulo elastico secante del calcestruzzo della soletta definito al § 11.2.10.3;

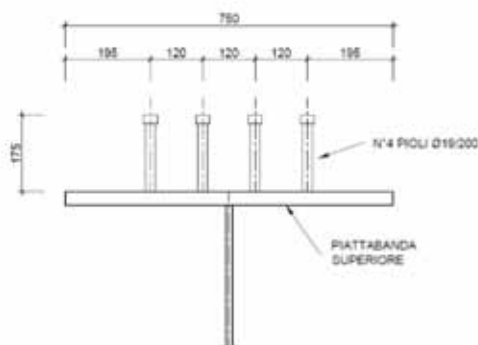
$d$  è il diametro del piolo, compreso tra 16 e 25 mm;

$h_{sc}$  è l'altezza del piolo dopo la saldatura;

$$\alpha = 0,2 (h_{sc} / d + 1) \quad \text{per } 3 \leq h_{sc} / d \leq 4, \quad [4.3.11 a]$$

$$\alpha = 1,0 \quad \text{per } h_{sc} / d > 4. \quad [4.3.11 b]$$

Per il caso in esame si ottiene:



$f_{tk} =$	450	Mpa
$f_{ck} =$	32	Mpa
$E_{cm} =$	33346	Mpa
$\alpha =$	1	
$d =$	19	mm
$\gamma_V =$	1,25	
$Prd,a =$	81,656	kN
$Prd,c =$	86,515	kN

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Si procedere ora con il calcolo della forza di scorrimento e quindi con il valore di carico massimo determinato per ciascun piolo.

<b>L =</b>	41	m
<b>Bsol =</b>	3340	mm
<b>H tot =</b>	2300	mm
<b>H sol =</b>	300	mm
<b>Y<sub>G,sup,ld</sub> =</b>	778,76	mm
<b>σ<sub>c1</sub> =</b>	17,28	Mpa
<b>σ<sub>c2</sub> =</b>	10,62	Mpa
<b>Fv<sub>(L/2)</sub> =</b>	681,9048	kN
<b>Δx =</b>	200	mm
<b>N<sub>Pioli</sub> =</b>	4	
<b>Fv<sub>-(PIOLO)</sub> =</b>	34,10	kN

Fv\_Piolo < min (Prd,a ; Prd,c) - Ok Verificato



6.2.9.1 VERIFICA GIUNTO TRAVERSO DI CAMPATA

$\gamma_{M0} =$	1,05	per sezioni classe 1,2,3 e 4		
$\gamma_{M1} =$	1,05	per instabilità membrature		
$\gamma_{M1} =$	1,10	per instabilità membrature ponti stradali e ferroviari		
$\gamma_{M2} =$	1,25	per sezioni tese indebolite dai fori nei riguardi della frattura		
<b>Resistenza a taglio bulloni 4.6, 5.6 e 8.8</b>				
27		10,9		
$F_{V,Rd} = \frac{0,6 \cdot f_{tb} \cdot A_{res}}{\gamma_{M2}}$		220,32	kN	
<b>Resistenza a taglio bulloni 6.8 e 10.9</b>				
$F_{V,Rd} = \frac{0,5 \cdot f_{tb} \cdot A_{res}}{\gamma_{M2}}$		183,6	kN	
<b>Resistenza a rifollamento</b>				
$F_{b,Rd} = \frac{k \cdot \alpha \cdot f_{tk} \cdot d \cdot t}{\gamma_{M2}}$		182,87	kN	di bordo
		182,87	kN	interni
dove:				
d è il diametro nominale del gambo del bullone.				
t è lo spessore della piastra collegata.				
$f_{tb}$ è la resistenza a rottura del materiale della piastra collegata,				
$\alpha = \min \{ e_1 / (3 \cdot d_0) ; f_{tb} / f_t ; 1 \}$ per bulloni di bordo nella direzione del carico applicato.				
$\alpha = \min \{ p_1 / (3 \cdot d_0) - 0,25 ; f_{tb} / f_t ; 1 \}$ per bulloni interni nella direzione del carico applicato.				
$k = \min \{ 2,8 \cdot e_2 / d_0 - 1,7 ; 2,5 \}$ per bulloni di bordo nella direzione perpendicolare al carico applicato.				
$k = \min \{ 1,4 \cdot p_2 / d_0 - 1,7 ; 2,5 \}$ per bulloni interni nella direzione perpendicolare al carico applicato.				
essendo $e_1$ , $e_2$ , $p_1$ e $p_2$ indicati in Fig. 4.2.3 e $d_0$ il diametro nominale del foro di alloggiamento del bullone.				
S355				
d =	27	mm		
$f_{tk} =$	510	mm		
$\alpha =$	0,50	mm		
$\alpha =$	0,50	mm		
k =	1,66	mm		
k =	1,66	mm		
$e_1 =$	36	mm		
$e_2 =$	36	mm		
$p_1 =$	66	mm		
$p_2 =$	72	mm		
$V_{S,d} =$	312,00	kN	taglio assorbito da anima	
$M_{S,d} =$	721,20	kNm		
Braccio =	0,900	m		
$F_{S,d} =$	801,33	kN	forza alle flange	

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

<b>Resistenza a taglio bulloni (flangia)</b>					
$t_{\text{piastra}} =$	20	mm			
$h_{\text{piastra}} =$	272	mm			
$n_{\text{piastre}} =$	1	piastre			
$n_f =$	1	file			
$n_{\text{pt}} =$	2	piani di taglio			
$n_b =$	3	bulloni			
$F_{V,Rd\_anima} = n \cdot n_b \cdot F_{V,Rd}$			1101,6	kN	<b>ok!</b>
<b>Resistenza a taglio delle piastre anima:</b>					
$n_f =$	2	file			
$n_{\text{pt}} =$	2	piani di taglio			
$n_b =$	9	bulloni			
$t_{\text{piastra}} =$	12	mm			
$h_{\text{piastra}} =$	759	mm			
$n_{\text{piastre}} =$	2	piastre			
$V_{V,Rd\_piastre} = n_{\text{piastre}} \cdot \frac{f_y \cdot A}{\gamma_{M_0} \cdot \sqrt{3}}$			7112	kN	<b>ok!</b>
<b>Resistenza a rifollamento flangia</b>					
$F_{V,Ed} =$	133,56	kN	Sollecitazione taglio bullone		
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>		
<b>Resistenza a rifollamento anima</b>					
$F_{V,Ed} =$	8,67	kN	Sollecitazione taglio bullone		
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>		

### 6.2.9.2 VERIFICA GIUNTO TRAVERSO DI TESTATA

$\gamma_{M0} =$	1,05	per sezioni classe 1,2,3 e 4		
$\gamma_{M1} =$	1,05	per instabilità membrane		
$\gamma_{M1} =$	1,10	per instabilità membrane ponti stradali e ferroviari		
$\gamma_{M2} =$	1,25	per sezioni tese indebolite dai fori nei riguardi della frattura		
<b>Resistenza a taglio bulloni 4.6, 5.6 e 8.8</b>				
27		10,9		
$F_{V,Rd} = \frac{0,6 \cdot f_{tb} \cdot A_{res}}{\gamma_{M2}}$		220,32	kN	
<b>Resistenza a taglio bulloni 6.8 e 10.9</b>				
$F_{V,Rd} = \frac{0,5 \cdot f_{tb} \cdot A_{res}}{\gamma_{M2}}$		183,6	kN	
<b>Resistenza a rifollamento</b>				
$F_{b,Rd} = \frac{k \cdot \alpha \cdot f_{tk} \cdot d \cdot t}{\gamma_{M2}}$		182,87	kN	di bordo
		182,87	kN	interni
dove:				
d è il diametro nominale del gambo del bullone,				
t è lo spessore della piastra collegata,				
$f_{td}$ è la resistenza a rottura del materiale della piastra collegata,				
$\alpha = \min \{e_1 / (3 \cdot d_0) ; f_{td} / f_t ; 1\}$ per bulloni di bordo nella direzione del carico applicato,				
$\alpha = \min \{p_1 / (3 \cdot d_0) - 0,25 ; f_{td} / f_t ; 1\}$ per bulloni interni nella direzione del carico applicato,				
$k = \min \{2,8 \cdot e_2 / d_0 - 1,7 ; 2,5\}$ per bulloni di bordo nella direzione perpendicolare al carico applicato,				
$k = \min \{1,4 \cdot p_2 / d_0 - 1,7 ; 2,5\}$ per bulloni interni nella direzione perpendicolare al carico applicato,				
essendo $e_1$ , $e_2$ , $p_1$ e $p_2$ indicati in Fig. 4.2.3 e $d_0$ il diametro nominale del foro di alloggiamento del bullone,				
S355				
d =	27	mm		
$f_{tk} =$	510	mm		
$\alpha =$	0,50	mm		
$\alpha =$	0,50	mm		
k =	1,66	mm		
k =	1,66	mm		
$e_1 =$	36	mm		
$e_2 =$	36	mm		
$p_1 =$	66	mm		
$p_2 =$	72	mm		
$V_{S,d} =$	349,50	kN	taglio assorbito da anima	
$M_{S,d} =$	722,10	kNm		
Braccio =	1,260	m		
$F_{S,d} =$	573,10	kN	forza alle flange	

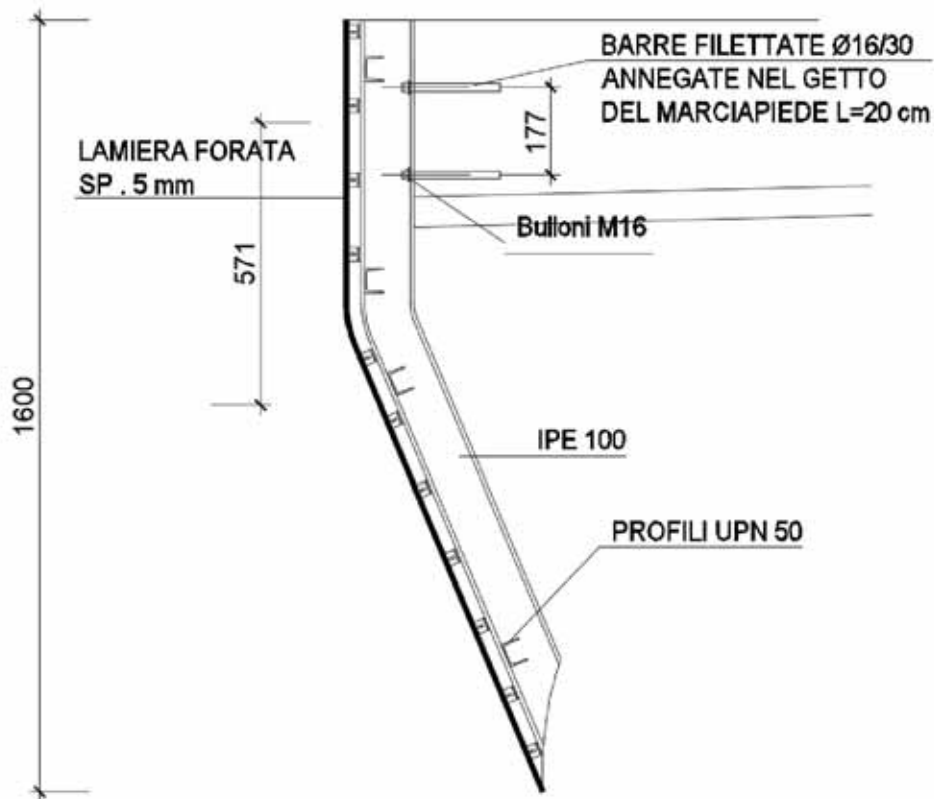
LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

<b>Resistenza a taglio bulloni (flangia)</b>				
$t_{\text{piastra}} =$	20	mm		
$h_{\text{piastra}} =$	272	mm		
$n_{\text{piastre}} =$	1	piastre		
$n_t =$	1	file		
$n_{\text{pt}} =$	2	piani di taglio		
$n_b =$	6	bulloni		
$F_{V,Rd\_anima} = n \cdot n_b \cdot F_{V,Rd}$			2203,2	kN <b>ok!</b>
<b>Resistenza a taglio delle piastre anima:</b>				
$n_t =$	2	file		
$n_{\text{pt}} =$	2	piani di taglio		
$n_b =$	13	bulloni		
$t_{\text{piastra}} =$	12	mm		
$h_{\text{piastra}} =$	1082	mm		
$n_{\text{piastre}} =$	2	piastre		
$V_{V,Rd\_piastre} = n_{\text{piastre}} \cdot \frac{f_y \cdot A}{\gamma_{M_0} \cdot \sqrt{3}}$			10138	kN <b>ok!</b>
<b>Resistenza a rifollamento flangia</b>				
$F_{V,Ed} =$	47,76	kN	Sollecitazione taglio bullone	
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>	
<b>Resistenza a rifollamento anima</b>				
$F_{V,Ed} =$	6,72	kN	Sollecitazione taglio bullone	
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>	

### 6.2.10 VERIFICA VELETTA METALLICA

Si riporta di seguito la verifica degli elementi di supporto previsti a sostegno della veletta metallica laterale al cordolo dell'impalcato.

La veletta è composta da elementi di sostegno in acciaio sia trasversali IPE100 che trasversali UPN 50 ed una lamiera forata in alluminio di spessore pari a 5mm.





LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

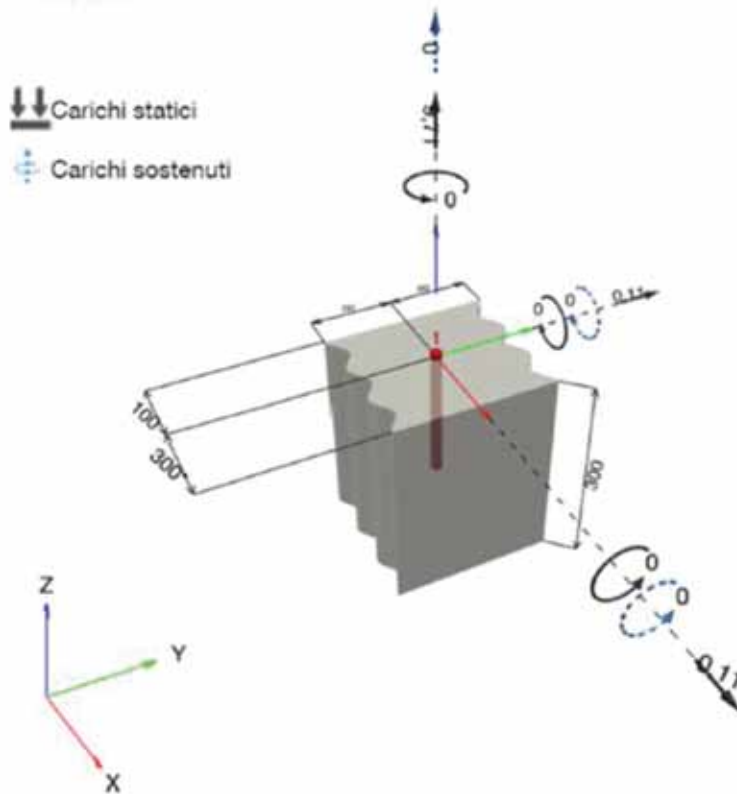
Peso Proprio Veletta		
Peso UPN 50 =	5,59	kg/ml
Peso Totale UPN 50 =	<b>22,36</b>	kg/ml
Peso IPE 100 =	8,10	kg/ml
Lunghezza IPE 100 =	1,40	m
Peso Totale IPE 100 =	<b>11,34</b>	kg/ml
Lunghezza Lamiera alluminio =	1,65	ml
Spessore Lamiera alluminio =	0,005	m
% Foratura Lamiera alluminio =	60%	
Peso Lamiera alluminio =	8,91	kg/mq
<b>Peso Totale =</b>	<b>42,61</b>	kg/mq
	<b>0,4261</b>	kN/ml
Forza Orizzontale Vento		
ph =	1,88	kN/mq
H =	1,6	m
Fv =	3,008	kN
Braccio Fv =	0,571	m
Ftrazione =	6,469	kN
Taglio =	0,071	kN
Ft_d =	9,704	kN
T_d =	0,107	kN

Si riporta di seguito la verifica di aderenza a trazione della barra filettata  $\phi 16$  prevista ammorsata per 20cm all'interno del cordolo laterale.

## 1 Dati da inserire

Tipo e dimensione dell'ancorante:	<b>HIT-RE 500 V4 + HAS-U 5.8 HDG M16</b>
Periodo di ritorno (durata in anni):	50
Codice articolo:	2223897 HAS-U 5.8 HDG M16x260 (inserire) / 2287552 HIT-RE 500 V4 (composto indurente)
Profondità di posa effettiva:	$h_{eff,act} = 200,0$ mm ( $h_{eff,inst} = -$ mm)
Materiale:	5.8
Certificazione No.:	ETA 20/0541
Emesso / Validato:	04/09/2021   -
Prova:	metodo di calcolo EN 1992-4, chimica
Fissaggio distanziato:	
Profilo:	
Materiale base:	non fessurato calcestruzzo, C25/30, $f_{c,cal} = 25,00$ N/mm <sup>2</sup> , h = 300,0 mm, Temp. Breve/Lungo: 0/0 °C, Coefficiente parziale di sicurezza materiale definito dall'utente $\gamma_c = 1,500$
Installazione:	<b>Foro eseguito con perforatore, Condizioni di installazione: asciutto</b>
Armatura:	nessuna armatura o interasse tra le armature $\geq 150$ mm (qualunque $\varnothing$ ) o $\geq 100$ mm ( $\varnothing \leq 10$ mm) senza armatura di bordo longitudinale

### Geometria [mm] & Carichi [kN, kNm]



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

1.1 Combinazione carichi

Caso	Descrizione	Forze [kN] / Momenti [kNm]	Sismico	Fuoco	Util. max. Tasseello [%]
1	Combinazione 1	N = 9,710; V <sub>x</sub> = 0,110; V <sub>y</sub> = 0,110; M <sub>x</sub> = 0,000; M <sub>y</sub> = 0,000; M <sub>z</sub> = 0,000; N <sub>tot</sub> = 0,000; M <sub>x,tot</sub> = 0,000; M <sub>y,tot</sub> = 0,000;	no	no	20

2 Condizione di carico/Carichi risultanti sull'ancorante

Carichi sull'ancorante [kN]

Trazione: (+ Trazione, - Compressione)

Ancorante	Trazione	Taglio	Taglio in dir. x	Taglio in dir. y
1	9,710	0,156	0,110	0,110

Compressione max. nel calcestruzzo: - [%]  
Max. sforzo di compressione nel calcestruzzo: - [N/mm<sup>2</sup>]  
risultante delle forze di trazione nel (x/y)=(0,0/0,0): 0,000 [kN]  
risultante delle forze di compressione (x/y)=(0,0/0,0): 0,000 [kN]

3 Carico di trazione (EN 1992-4, sezione 7.2.1)

	Carico [kN]	Resistenza [kN]	Utilizzo β <sub>N</sub> [%]	Stato
Rottura dell'acciaio*	9,710	52,333	19	OK
Rottura combinata conica del calcestruzzo e per sfilamento**	9,710	67,984	15	OK
Rottura conica del calcestruzzo**	9,710	55,311	18	OK
Fessurazione**	9,710	49,893	20	OK

\*ancorante più sollecitato \*\*gruppo di ancoranti (ancoranti sollecitati)

3.1 Rottura dell'acciaio

$$N_{Ed} \leq N_{Rd,s} = \frac{N_{Rp,s}}{\gamma_{M,s}} \quad \text{EN 1992-4, Tabella 7.1}$$

N <sub>Rp,s</sub> [kN]	γ <sub>M,s</sub>	N <sub>Rd,s</sub> [kN]	N <sub>Ed</sub> [kN]
78,500	1,500	52,333	9,710

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

3.2 Rottura combinata conica del calcestruzzo e per sfilamento

$$N_{Ed} \leq N_{Rd,c} = \frac{N_{Rk,c}}{\gamma_{M,c}} \quad \text{EN 1992-4, Tabella 7.1}$$

$$N_{Rk,c} = N_{Rk,c}^0 \cdot \frac{A_{c,N}^0}{A_{c,N}} \cdot \psi_{s,N} \cdot \psi_{s,Nb} \cdot \psi_{re,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{MN} \quad \text{EN 1992-4, Eq. (7.1)}$$

$$N_{Rk,c}^0 = \psi_{s,N} \cdot \tau_{Rk,c} \cdot z \cdot d \cdot h_{ef} \quad \text{EN 1992-4, Eq. (7.14)}$$

$$\psi_{s,N} = 1 \quad \text{EN 1992-4, Eq. (7.14a)}$$

$$s_{cr,Nb} = 7,3 \cdot d \cdot \sqrt{\psi_{s,N} \cdot \tau_{Rk,c}} \leq 3 \cdot h_{ef} \quad \text{EN 1992-4, Eq. (7.15)}$$

$$\psi_{s,Nb} = \psi_{s,Nb}^0 \cdot \left( \frac{s}{s_{cr,Nb}} \right)^{0,5} \cdot (\psi_{s,Nb}^0 - 1) \geq 1,00 \quad \text{EN 1992-4, Eq. (7.17)}$$

$$\psi_{s,Nb}^0 = \sqrt{n} \cdot (\sqrt{n} - 1) \cdot \left( \frac{\tau_{Rk,c}}{\tau_{Rk,c}^{LS}} \right)^{1,5} \geq 1,00 \quad \text{EN 1992-4, Eq. (7.18)}$$

$$\tau_{Rk,c} = \frac{k_3}{z \cdot d} \cdot \sqrt{h_{ef} \cdot f_{ct}} \quad \text{EN 1992-4, Eq. (7.10)}$$

$$\psi_{re,N} = 0,7 + 0,3 \cdot \frac{c}{c_{cr,Nb}} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.20)}$$

$$\psi_{ec1,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c1,N}}{s_{cr,Nb}} \right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.21)}$$

$$\psi_{ec2,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c2,N}}{s_{cr,Nb}} \right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.21)}$$

$A_{c,N} [mm^2]$	$A_{c,N}^0 [mm^2]$	$\tau_{Rk,c} [N/mm^2]$	$s_{cr,Nb} [mm]$	$c_{cr,Nb} [mm]$	$c_{cr,N} [mm]$	$f_{ct,cr} [N/mm^2]$
164.117	231.018	17,00	481,6	240,8	100,0	25,00
$\psi_{s,N}$	$\tau_{Rk,c} [N/mm^2]$	$k_3$	$\tau_{Rk,c} [N/mm^2]$	$\psi_{s,Nb}^0$	$\psi_{s,Nb}$	
1,023	17,38	11,000	15,47	1,000	1,000	
$e_{c1,N} [mm]$	$\psi_{ec1,N}$	$e_{c2,N} [mm]$	$\psi_{ec2,N}$	$\psi_{re,N}$	$\psi_{MN}$	
0,0	1,000	0,0	1,000	0,825	1,000	
$\psi_{s,Nb}^0$	$\alpha_{T,s}$	$\psi_{s,N}$				
0,880	0,000	1,000				
$N_{Rk,c}^0 [kN]$	$N_{Rd,c} [kN]$	$\gamma_{M,c}$	$N_{Rk,c} [kN]$	$N_{Ed} [kN]$		
174,750	101,970	1,500	67,084	0,710		

ID gruppo ancoranti  
1

3.3 Rottura conica del calcestruzzo

$$N_{Ed} \leq N_{Rd,c} = \frac{N_{Rk,c}}{\gamma_{M,c}} \quad \text{EN 1992-4, Tabella 7.1}$$

$$N_{Rk,c} = N_{Rk,c}^0 \cdot \frac{A_{c,N}^0}{A_{c,N}} \cdot \psi_{s,N} \cdot \psi_{s,Nb} \cdot \psi_{re,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{MN} \quad \text{EN 1992-4, Eq. (7.1)}$$

$$N_{Rk,c}^0 = k_1 \cdot \sqrt{z} \cdot h_{ef}^{1,5} \quad \text{EN 1992-4, Eq. (7.2)}$$

$$A_{c,N}^0 = s_{cr,N} \cdot s_{cr,N} \quad \text{EN 1992-4, Eq. (7.3)}$$

$$\psi_{s,N} = 0,7 + 0,3 \cdot \frac{c}{c_{cr,N}} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.4)}$$

$$\psi_{ec1,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c1,N}}{s_{cr,N}} \right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{ec2,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c2,N}}{s_{cr,N}} \right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{MN} = 1 \quad \text{EN 1992-4, Eq. (7.7)}$$

$A_{c,N} [mm^2]$	$A_{c,N}^0 [mm^2]$	$c_{cr,N} [mm]$	$s_{cr,N} [mm]$	$f_{ct,cr} [N/mm^2]$		
240.000	360.000	300,0	600,0	25,00		
$e_{c1,N} [mm]$	$\psi_{ec1,N}$	$e_{c2,N} [mm]$	$\psi_{ec2,N}$	$\psi_{s,N}$	$\psi_{MN}$	
0,0	1,000	0,0	1,000	0,800	1,000	
$z [mm]$	$\psi_{MN}$	$k_1$	$N_{Rk,c}^0 [kN]$	$\gamma_{M,c}$	$N_{Rd,c} [kN]$	$N_{Ed} [kN]$
0,0	1,000	11,000	155,563	1,500	55,311	0,710

ID gruppo ancoranti  
1

3.4 Fessurazione

$$N_{Ed} \leq N_{Rd,SP} = \frac{N_{Rk,SP}}{\gamma_{M20}} \quad \text{EN 1992-4, Tabella 7.1}$$

$$N_{Rd,SP} = N_{Rk,SP} \cdot \frac{A_{c,N}}{A_{c,N}^0} \cdot \psi_{s,N} \cdot \psi_{rc,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{h,SP} \quad \text{EN 1992-4, Eq. (7.23)}$$

$$N_{Rk,SP}^0 = \min(N_{Rk,SP}^t, N_{Rk,SP}^c) \quad \text{EN 1992-4, Eq. (7.3)}$$

$$A_{c,N}^0 = s_{cr,SP} \cdot s_{cr,SP} \quad \text{EN 1992-4, Eq. (7.3)}$$

$$\psi_{s,N} = 0,7 + 0,3 \cdot \frac{c}{c_{cr,SP}} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.4)}$$

$$\psi_{ec1,N} = \frac{1}{1 + \left(\frac{2 \cdot e_{c1,N}}{s_{cr,SP}}\right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{ec2,N} = \frac{1}{1 + \left(\frac{2 \cdot e_{c2,N}}{s_{cr,SP}}\right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{h,SP} = \left(\frac{h}{h_{ref}}\right)^{2/3} \leq \max\left\{1; \left(\frac{h_{ref} + 1,5 \cdot c_1}{h_{ref}}\right)^{2/3}\right\} \leq 2,00 \quad \text{EN 1992-4, Eq. (7.24)}$$

$A_{c,N}$ [mm <sup>2</sup> ]	$A_{c,N}^0$ [mm <sup>2</sup> ]	$c_{cr,SP}$ [mm]	$s_{cr,SP}$ [mm]	$\psi_{h,SP}$	$f_{c,eff}$ [N/mm <sup>2</sup> ]	
304.000	577.600	380,0	760,0	1,173	25,00	
$e_{c1,N}$ [mm]	$\psi_{ec1,N}$	$e_{c2,N}$ [mm]	$\psi_{ec2,N}$	$\psi_{s,N}$	$\psi_{rc,N}$	$k_1$
0,0	1,000	0,0	1,000	0,779	1,000	11,000
$N_{Rk,SP}^0$ [kN]	$\gamma_{M20}$	$N_{Rd,SP}$ [kN]	$N_{Ed}$ [kN]			
155,563	1,500	49,893	9,710			

ID gruppo ancoranti

1

#### 4 Carico di taglio (EN 1992-4, sezione 7.2.2)

	Carico [kN]	Resistenza [kN]	Utilizzo $\beta_v$ [%]	Stato
Rottura dell'acciaio (senza braccio di leva)*	0,156	37,728	1	OK
Rottura dell'acciaio (con braccio di leva)*	N/A	N/A	N/A	N/A
Rottura per pryout**	0,156	110,623	1	OK
Rottura del bordo del calcestruzzo in direzione x-**	0,110	33,825	1	OK

\*ancorante più sollecitato \*\*gruppo di ancoranti (ancoranti specifici)

#### 4.1 Rottura dell'acciaio (senza braccio di leva)

$$V_{Ed} \leq V_{Rd,s} = \frac{V_{Rd,s}}{\gamma_{M,s}} \quad \text{EN 1992-4, Tabella 7.2}$$

$$V_{Rd,s} = k_7 \cdot V_{Rk,s} \quad \text{EN 1992-4, Eq. (7.35)}$$

$V_{Rd,s}^0$ [kN]	$k_7$	$V_{Rk,s}$ [kN]	$\gamma_{M,s}$	$V_{Rd,s}$ [kN]	$V_{Ed}$ [kN]
47,160	1,000	47,160	1,250	37,728	0,156

#### 4.2 Rottura per pryout (cono del calcestruzzo)

$$V_{Ed} \leq V_{Rd,cp} = \frac{V_{Rd,cp}}{\gamma_{M,cp}} \quad \text{EN 1992-4, Tabella 7.2}$$

$$V_{Rd,cp} = k_8 \cdot \min(N_{Rk,c}; N_{Rk,p}) \quad \text{EN 1992-4, Eq. (7.39c)}$$

$$N_{Rk,c} = N_{Rk,c}^0 \cdot \frac{A_{c,N}}{A_{c,N}^0} \cdot \psi_{s,N} \cdot \psi_{re,N} \cdot \psi_{ec1,N} \cdot \psi_{ec2,N} \cdot \psi_{MN} \quad \text{EN 1992-4, Eq. (7.1)}$$

$$N_{Rk,c}^0 = k_1 \cdot \sqrt{f_{ct}} \cdot h_{ef}^{1.5} \quad \text{EN 1992-4, Eq. (7.2)}$$

$$A_{c,N}^0 = s_{cr,N} \cdot s_{cr,N} \quad \text{EN 1992-4, Eq. (7.3)}$$

$$\psi_{s,N} = 0,7 + 0,3 \cdot \frac{c}{c_{cr,N}} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.4)}$$

$$\psi_{ec1,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{v1}}{s_{cr,N}} \right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{ec2,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{v2}}{s_{cr,N}} \right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.6)}$$

$$\psi_{MN} = 1 \quad \text{EN 1992-4, Eq. (7.7)}$$

$A_{c,N}$ [mm <sup>2</sup> ]	$A_{c,N}^0$ [mm <sup>2</sup> ]	$c_{cr,N}$ [mm]	$s_{cr,N}$ [mm]	$k_8$	$f_{ct,eff}$ [N/mm <sup>2</sup> ]	
240.000	360.000	300,0	600,0	2,000	25,00	
$e_{v1}$ [mm]	$\psi_{ec1,N}$	$e_{v2}$ [mm]	$\psi_{ec2,N}$	$\psi_{s,N}$	$\psi_{re,N}$	$\psi_{MN}$
0,0	1,000	0,0	1,000	0,800	1,000	1,000
$k_1$	$N_{Rk,c}^0$ [kN]	$\gamma_{M,cp}$	$V_{Rd,cp}$ [kN]	$V_{Ed}$ [kN]		
11,000	155,563	1,500	110,623	0,156		

ID gruppo ancoranti

1

4.3 Rottura del bordo del calcestruzzo in direzione x-

$$V_{Ed} \leq V_{Rd,c} = \frac{V_{Rd,s}}{\gamma_{Mz}} \quad \text{EN 1992-4, Tabella 7.2}$$

$$V_{Rd,c} = k_T \cdot V_{Rd,c}^0 \cdot \frac{A_{c,v}}{A_{c,v}^0} \cdot \psi_{s,v} \cdot \psi_{n,v} \cdot \psi_{ec,v} \cdot \psi_{m,v} \quad \text{EN 1992-4, Eq. (7.40)}$$

$$V_{Rd,c}^0 = k_s \cdot d_{nom}^{\alpha} \cdot \rho_{ct} \cdot \sqrt{c_1} \cdot c_1^{0.5} \quad \text{EN 1992-4, Eq. (7.41)}$$

$$\alpha = 0,1 \cdot \left( \frac{l}{c_1} \right) \quad \text{EN 1992-4, Eq. (7.42)}$$

$$\beta = 0,1 \cdot \left( \frac{d_{nom}}{c_1} \right)^{0.2} \quad \text{EN 1992-4, Eq. (7.43)}$$

$$A_{c,v}^0 = 4,5 \cdot c_1^2 \quad \text{EN 1992-4, Eq. (7.44)}$$

$$\psi_{s,v} = 0,7 + 0,3 \cdot \frac{c_2}{1,5 \cdot c_1} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.45)}$$

$$\psi_{n,v} = \left( \frac{1,5 \cdot c_1}{h} \right)^{0.5} \geq 1,00 \quad \text{EN 1992-4, Eq. (7.46)}$$

$$\psi_{ec,v} = \frac{1}{1 + \left( \frac{2 \cdot e_v}{3 \cdot c_1} \right)} \leq 1,00 \quad \text{EN 1992-4, Eq. (7.47)}$$

$$\psi_{m,v} = \sqrt{\frac{1}{(\cos \alpha_v)^2 + (0,5 \cdot \sin \alpha_v)^2}} \geq 1,00 \quad \text{EN 1992-4, Eq. (7.48)}$$

$l$ [mm]	$d_{nom}$ [mm]	$k_s$	$\alpha$	$\beta$	$f_{ctd}$ [N/mm <sup>2</sup> ]
192,0	16,00	2,400	0,139	0,069	26,00
$c_1$ [mm]	$A_{c,v}$ [mm <sup>2</sup> ]	$A_{c,v}^0$ [mm <sup>2</sup> ]			
100,0	45.000	45.000			
$\psi_{s,v}$	$\psi_{n,v}$	$\psi_{ec,v}$	$e_{c,v}$ [mm]	$\psi_{m,v}$	$\psi_{m,v}$
1,000	1,000	2,000	0,0	1,000	1,000
$V_{Rd,c}^0$ [kN]	$k_T$	$\gamma_{Mz}$	$V_{Rd,c}$ [kN]	$V_{Ed}$ [kN]	
25,369	1,0	1,500	33,825	0,110	

5 Carichi combinati di trazione e di taglio (EN 1992-4, sezione 7.2.3)

Rottura dell'acciaio

$\beta_N$	$\beta_V$	$\alpha$	Utilizzo $\beta_{N,V}$ [%]	Stato
0,186	0,004	2,000	4	OK

$$\beta_N^2 + \beta_V^2 \leq 1,0$$

Rottura del calcestruzzo

$\beta_N$	$\beta_V$	$\alpha$	Utilizzo $\beta_{N,V}$ [%]	Stato
0,195	0,003	1,500	9	OK

$$\beta_N^2 + \beta_V^2 \leq 1,0$$

6 Spostamenti (ancorante più sollecitato)

Carichi a breve termine:

$$N_{sk} = 7,193 \text{ [kN]} \quad \delta_N = 0,0429 \text{ [mm]}$$

$$V_{sk} = 0,115 \text{ [kN]} \quad \delta_V = 0,0046 \text{ [mm]}$$

$$\delta_{wV} = 0,0432 \text{ [mm]}$$

Carichi a lungo termine:

$$N_{sk} = 7,193 \text{ [kN]} \quad \delta_N = 0,0930 \text{ [mm]}$$

$$V_{sk} = 0,115 \text{ [kN]} \quad \delta_V = 0,0069 \text{ [mm]}$$

$$\delta_{wV} = 0,0933 \text{ [mm]}$$

## 7 VERIFICA SOLETTA

Si riportano di seguito le verifiche effettuate per il dimensionamento delle solette di impalcato.

### 7.1 VERIFICA SOLETTA FASE TRANSITORIA

In questa fase di calcolo viene verificata la lastra di predalles con relativi tralicci al fine di sostenere i carichi della soletta, del peso degli operatori e di eventuali barriere di protezione laterali provvisorie per la sicurezza degli operatori.

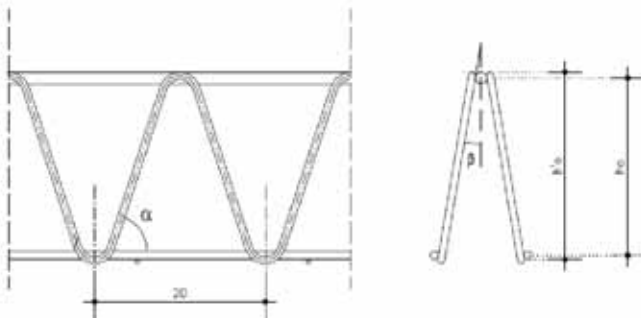
Si riportano di seguito i valori dei carichi utilizzati ed il calcolo delle sollecitazioni in campata ed in appoggio al fine di dimensionare il traliccio da prevedere nel predalles.

#### 7.1.1 ANALISI DEI CARICHI

Si considera il peso proprio della lastra di predalles, il peso del getto della soletta, il peso di un parapetto di protezione e di un mezzo operativo per le varie lavorazioni.

		Sp [m]	$\gamma$ [kN/m <sup>3</sup> ]		
Gk1	Peso Proprio	0.05	25	1.25	kN/m <sup>2</sup>
Gk2	Peso Soletta	0.21	25	5.25	kN/m <sup>2</sup>
Gk2	Peso Parapetto Sic.			0.4	kN/m
Qk	Peso Mezzo Operativo			1	kN/m <sup>2</sup>

Le verifiche vengono eseguite facendo riferimento allo Stato Limite Ultimo della sezione. Si verifica che le tensioni sugli elementi metallici che costituiscono il traliccio siano inferiori alla tensione di calcolo e che la stabilità degli elementi compressi risulti soddisfatta. Di seguito si riporta una rappresentazione schematica del traliccio utilizzato.



Altezza totale del traliccio:	$h'_0 = 165 \text{ mm}$
Diametro correnti superiori:	$f_{cs} = 16 \text{ mm}$
Diametro correnti inferiori:	$f_{ci} = 14 \text{ mm}$
Diametro staffa:	$f_{ci} = 10 \text{ mm}$

#### 7.1.2 VERIFICA A MOMENTO POSITIVO - CAMPATA

Analizzando gli elaborati grafici allegati al presente progetto esecutivo, si evince che la distanza tra le anime delle travi di appoggio è pari a 3.50m e considerando 5 cm di appoggio su entrambe le strutture,



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

si considera una luce di calcolo pari a 3.65m. Si riporta di seguito il calcolo delle sollecitazioni di progetto.

SEZIONE IN CAMPATA				
	LC	3.65		
	MOMENTO FLETTENTE	TAGLIO	Med	Ted
<b>Gk1</b>	$1.25 \times 1.20 \times 3.65^2 / 8$	$1.25 \times 1.20 \times 3.65/2$	2.50	2.74
<b>Gk2</b>	$5.25 \times 1.20 \times 3.65^2 / 8$	$5.25 \times 1.20 \times 3.65/2$	10.49	11.50
<b>Qk</b>	$1 \times 1.20 \times 3.65^2 / 8$	$1 \times 1.20 \times 3.65/2$	2.00	2.19
	<b>Med</b>		<b>24.41</b>	<b>21.35</b>
	<b>Ted</b>			
	<b>Ned = (Med / h<sub>0</sub>) / 3</b>		<b>54.25</b>	

Altezza totale traliccio	ht=	16.50	cm	
Larghezza totale traliccio	B=	9.00	cm	
Interasse tralicci	i=	40.00	cm	
Numero di tralicci /m	n=	3		
Passo traliccio	p=	20.00	cm	
Quota corrente inferiore	s=	2.00	cm	
Corrente superiore	1	∅s=	1.60	cm
Ferro integr sup (I)	0	∅s'=	0.00	cm
Ferro integr sup (II)	0	∅s"=	0.00	cm
Corrente inferiore	2	∅i=	1.40	cm
Diagonale		∅d=	1.00	cm
Altezza utile traliccio	d=	13.0	cm	
Interasse ferri inferiori	pi=	7.6	cm	
Interasse trasv. diagonale	bd=	5.2	cm	
Inclinazione longitudinale	α=	1.065	rad	61.0 °
Inclinazione trasversale	β=	0.192	rad	11.0 °
Area corrente superiore	As=	2.01	cm <sup>2</sup>	
Area ferro integr sup (I)	As'=	0.00	cm <sup>2</sup>	
Area ferro integr sup (II)	As"=	0.00	cm <sup>2</sup>	
Area corrente inferiore	Ai=	3.08	cm <sup>2</sup>	
Area diagonale	Ad=	0.79	cm <sup>2</sup>	
Inerzia corrente superiore	Js=	0.3217	cm <sup>4</sup>	
Inerzia ferro integr sup (I)	Js'=	0.0000	cm <sup>4</sup>	
Inerzia ferro integr sup (II)	Js"=	0.0000	cm <sup>4</sup>	
Inerzia corrente inferiore	Ji=	0.3771	cm <sup>4</sup>	
Inerzia diagonale	Jd=	0.0491	cm <sup>4</sup>	

### CARATTERISTICHE DI RESISTENZA TRALICCIO

	Area cm <sup>2</sup>	Quota cm	J cm <sup>4</sup>	Jtrasp cm <sup>4</sup>	J+Jtrasp cm <sup>4</sup>
Corr sup + ferro (I)	2.01	15.7	0.322	124.3	124.7
Ferro (II)	0.00	14.9	0.000	0.0	0.0
Corrente inf	3.08	2.7	0.377	81.2	81.6

Quota baricentro	YG=	7.84	cm
Area totale	Atot=	5.1	cm <sup>2</sup>
Inerzia totale	Jtot=	206.3	cm <sup>4</sup>
Modulo di resistenza c. sup.	Ws=	26.23	cm <sup>3</sup>
Modulo di resistenza ferro (II)	Ws''=	29.2	cm <sup>3</sup>
Modulo di resistenza c. inf.	Wi=	40.2	cm <sup>3</sup>

### CARATTERISTICHE DI RESISTENZA ALL'INSTABILITÀ TRALICCIO

#### Corrente superiore

Lunghezza asta	L=	20.0	cm
Coefficiente riduttivo	$\beta$ =	0.7	
Lunghezza libera inflessione	L0=	14.0	cm
Carico critico Euleriano	Ncr=	340183	N
Snellezza adimensionale	$\lambda$ =	0.52	
Fattore di imperfezione	$\alpha$ =	0.49	
Coefficiente	$\Phi$ =	0.71	
Fattore di riduzione ( $\leq 1$ )	$\chi$ =	0.83	
Resistenza asta compressione	NbRd=	-65.6	kN

### Corrente inferiore

Lunghezza asta	L=	20.0	cm
Coefficiente riduttivo	$\beta$ =	0.7	
Lunghezza libera inflessione	L0=	14.0	cm
Carico critico Euleriano	Ncr=	199409	N
Snellezza adimensionale	$\lambda$ =	0.59	
Fattore di imperfezione	$\alpha$ =	0.49	
Coefficiente	$\Phi$ =	0.77	
Fattore di riduzione ( $\leq 1$ )	$\chi$ =	0.79	
Resistenza asta compressione	NbRd=	-47.7	kN

### Diagonale

Lunghezza asta	L=	16.6	
Coefficiente riduttivo	$\beta$ =	0.7	
Lunghezza libera inflessione	L0=	11.6	cm
Carico critico Euleriano	Ncr=	75294	N
Snellezza adimensionale	$\lambda$ =	0.69	
Fattore di imperfezione	$\alpha$ =	0.49	
Coefficiente	$\Phi$ =	0.85	
Fattore di riduzione ( $\leq 1$ )	$\chi$ =	0.73	
Resistenza asta compressione	NbRd=	-22.6	kN

### VERIFICA TRALICCIO

Momento sollecitante	Msd=	24.41	kNm	campata
Taglio sollecitante	Vsd=	21.35	kN	appoggio
Momento soll. /traliccio	msd=	8.14	kNm	campata
Taglio sollec. /traliccio	vsd=	7.12	kN	appoggio

### Corrente superiore compresso

#### Verifica di resistenza

Tensione limite di calcolo	f <sub>yd</sub> =	-391	N/mm <sup>2</sup>
Tensione corrente superiore	$\sigma_s$ =	-310	N/mm <sup>2</sup>
Coefficiente di sicurezza	c.s.=	1.26	VERIFICATO

#### Verifica di instabilità

Resistenza asta compressa	NbRd=	-65.6	kN
Compressione	Nsd=	-62.4	kN
Coefficiente di sicurezza	c.s.=	1.05	VERIFICATO

Si esegue ora la verifica della struttura allo SLU.

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Verifica C.A. S.L.U. - File: TRALICCIO M+

File Materiali Opzioni Visualizza Progetto Sez. Rett. Sismica Normativa: NTC 2008

Titolo : \_\_\_\_\_

N° figure elementari  Zoom N° strati barre  Zoom

N°	b [cm]	h [cm]
1	0.1	14
2	120	5

N°	As [cm²]	d [cm]
1	6.03	0.8
2	9.24	16.5

Tipo Sezione  
 Rettang. re  Trapezi  
 a T  Circolare  
 Rettangoli  Coord.

Sollecitazioni  
 S.L.U.  Metodo n

N<sub>Ed</sub>   kN  
M<sub>xEd</sub>   kNm  
M<sub>yEd</sub>

P.to applicazione N  
 Centro  Baricentro cls  
 Coord. [cm] xN  yN

Tipo rottura  
**Lato calcestruzzo - Acciaio elastico**

Metodo di calcolo  
 S.L.U. +  S.L.U. -  
 Metodo n

Tipo flessione  
 Retta  Deviata

N° rett.

Calcola MRd

L<sub>0</sub>  cm

Precompresso

Materiali  
 B450C  C30/37

$\epsilon_{su}$   ‰  $\epsilon_{c2}$   ‰  
 $I_{yd}$   N/mm<sup>4</sup>  $\epsilon_{cu}$   ‰  
 $E_s$   N/mm<sup>2</sup>  $I_{cd}$

$E_s/E_c$    $I_{cc}/I_{cd}$

$\epsilon_{syd}$   ‰  $\sigma_{c,adm}$    
 $\sigma_{s,adm}$   N/mm<sup>2</sup>  $\tau_{co}$    
 $\tau_{c1}$

M<sub>xRd</sub>  kNm  
 $\sigma_c$   N/mm<sup>2</sup>  
 $\sigma_s$   N/mm<sup>2</sup>  
 $\epsilon_c$   ‰  
 $\epsilon_s$   ‰  
d  cm  
x  x/d   
 $\delta$

Med < Mrd Verifica soddisfatta

### 7.1.3 VERIFICA A MOMENTO NEGATIVO APPOGGIO

Analizzando gli elaborati grafici allegati al presente progetto esecutivo, si evince che la distanza dello sbalzo rispetto all'asse dell'anima della trave di appoggio è pari a 1.25m. Si riporta di seguito il calcolo delle sollecitazioni e la verifica strutturale allo SLU.

SEZIONE IN APPOGGIO		
	LC	1.25
	MOMENTO FLETTENTE	
Gk1	$1.25 \times 1.20 \times 1.25^2 / 2$	1.17
Gk2	$5.25 \times 1.20 \times 1.25^2 / 2$	4.92
Gk2	$0.4 \times 1.20 \times 1.25$	0.60
Qk	$1 \times 1.20 \times 1.25^2 / 2$	0.94
	<b>Med</b>	<b>11.27</b>

Si esegue ora la verifica della struttura allo SLU.

LOTTO 1 - CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

Verifica C.A. S.L.U. - File: TRALICCIO M-

File Materiali Opzioni Visualizza Progetto Sez. Rett. Sismica Normativa: NTC 2008 ?

Titolo : \_\_\_\_\_

N° figure elementari  Zoom N° strati barre  Zoom

N°	b [cm]	h [cm]	N°	As [cm²]	d [cm]
1	0.1	14	1	6.03	0.8
2	120	5	2	9.239999	16.5

Tipo Sezione  
 Rettan.re  Trapezi  
 a T  Circolare  
 Rettangoli  Coord.

Sollecitazioni  
 S.L.U.  Metodo n

P.to applicazione N  
 Centro  Baricentro cls  
 Coord.[cm] xN  yN

Metodo di calcolo  
 S.L.U.+  S.L.U.-  
 Metodo n

Tipo flessione  
 Retta  Deviata

N° rett.

Calcola MRd Dominio M-N  
 L<sub>0</sub>  cm Col. modello

Precompresso

Materiali

B450C		C30/37	
$\epsilon_{cu}$	67.5 ‰	$\epsilon_{c2}$	2 ‰
$f_{yd}$	391.3 N/mm <sup>2</sup>	$\epsilon_{cu}$	3.5 ‰
$E_s$	200,000 N/mm <sup>2</sup>	$f_{cd}$	17
$E_s/E_c$	15	$f_{cc}/f_{cd}$	0.8
$\epsilon_{syd}$	1.957 ‰	$\sigma_{c,adm}$	11.5
$\sigma_{s,adm}$	255 N/mm <sup>2</sup>	$\tau_{co}$	0.6933
		$\tau_{c1}$	2.029

M<sub>xPd</sub> -42.71 kN m

$\sigma_c$  -17 N/mm<sup>2</sup>  
 $\sigma_s$  391.3 N/mm<sup>2</sup>  
 $\epsilon_c$  3.5 ‰  
 $\epsilon_s$  26.51 ‰  
 d 18.2 cm  
 $\lambda$  2.122  $\lambda/d$  0.1166  
 $\delta$  0.7

Med < Mrd Verifica soddisfatta

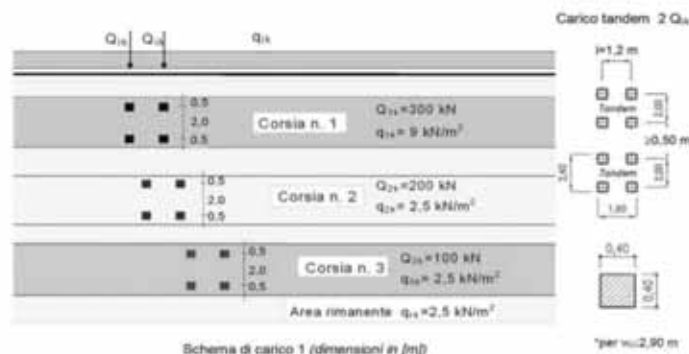
## 7.2 VERIFICA SOLETTA FASE DEFINITIVA

In questa fase di calcolo viene effettuato il calcolo della soletta considerando i seguenti carichi permanenti compatibilmente a quanto utilizzato per il dimensionamento dell'impalcato metallico.

PESO SOLETTA		
Hsoletta =	<b>0.26</b>	m
$\gamma$ =	25	kN/mc
peso =	6.5	kN/mq
CORDOLI		
Hcordoli =	<b>0.15</b>	m
$\gamma$ =	25	kN/mc
peso =	3.75	kN/mq
PERMANENTE PORTATO		
Hsoletta =	<b>0.11</b>	cm
$\gamma$ =	24	kN/mc
peso =	<b>2.64</b>	kN/mq
BARRIERE+VELETTE E TUBAZIONI		
Barriere =	1.5	kN/ml
Tubazioni =	0.1	kN/ml
Veletta =	0.4	kN/ml
Totale	<b>2.000</b>	kN/ml

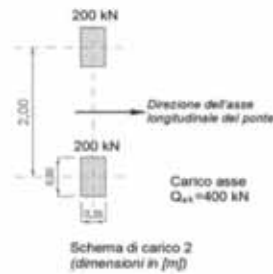
I carichi accidentali considerati sono quelli prescritti dalla normativa ed in particolare lo schema di carico 1, costituito da carichi concentrati su due assi in tandem, applicati su impronte di pneumatico di forma quadrata e lato pari a 0,40 m ( $Q_{ik}$ ), oltre che da un carico distribuito a metro quadrato ( $q_{ik}$ ) applicato alla corsia di carico.

I valori adottati per i carichi variano in base alla corsia di carico, secondo lo schema di normativa riportato nel seguito.



Oltre a questo, è stato adottato anche lo schema di carico 2, costituito da un singolo asse applicato su specifiche impronte di pneumatico di forma rettangolare, con larghezza 0,60 m ed altezza 0,35 m. Nel seguito l'immagine estratta dalla normativa.

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO



In aggiunta a quanto sopra, occorre tenere in conto anche l'urto di veicoli in svio ( $q_8$ ). La normativa, a tal proposito, indica che le forze causate da collisioni accidentali sugli elementi di sicurezza, possono essere considerate attraverso una forza orizzontale equivalente di collisione pari a 100 kN. Tale forza è da intendersi applicata trasversalmente ed orizzontalmente 100 mm sotto la sommità dell'elemento su cui insiste, oppure, 1,0 m sopra il piano di marcia, a seconda di quale sia il minore per una striscia di 0.5m. Ai fini dell'applicazione di tale forza alla soletta di impalcato, è stata considerata una diffusione a terra ottenendo una larghezza di carico pari a  $1.00 + 0.50 + 1.00$  ottenendo un effetto torcente sul bordo della soletta pari a  $M_t = 100 \text{ kN}/2.5\text{m} = 40 \text{ kNm/m}$ . I carichi concentrati previsti nei casi di carico, sono stati applicati tenendo in considerazione dell'effettiva geometria della mesh di calcolo utilizzata e considerando (come previsto dalla norma) una diffusione di carico a  $45^\circ$  fino al baricentro della soletta in cls.

Si riportano di seguito alcune immagini relative al modello e ai carichi applicati.

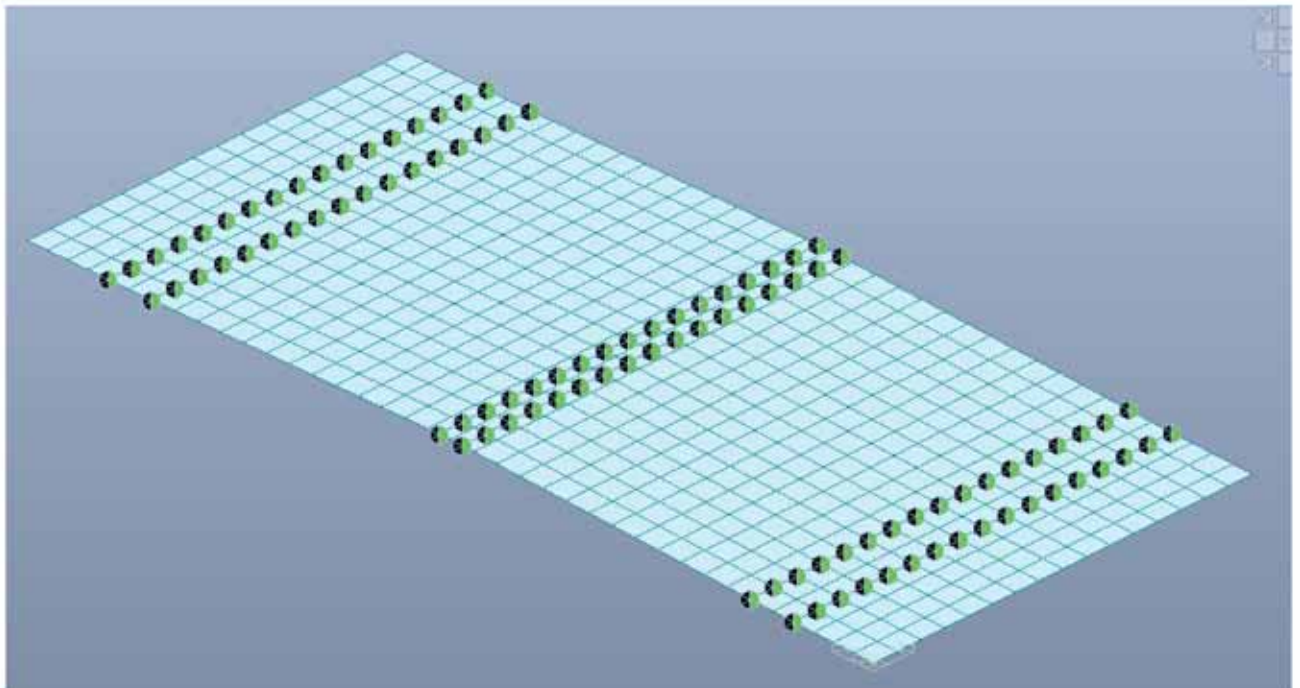


Figura 73 - Modello di calcolo

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

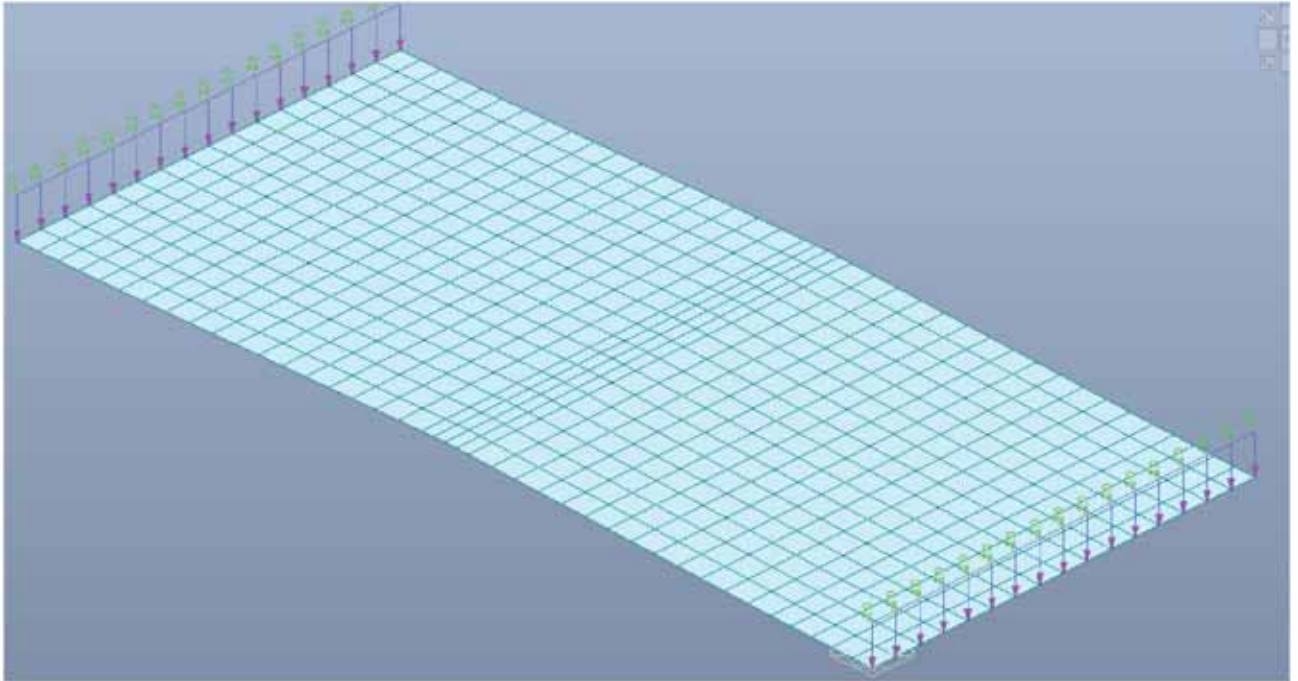


Figura 74 – Peso Barriera + Veletta + Tubazioni

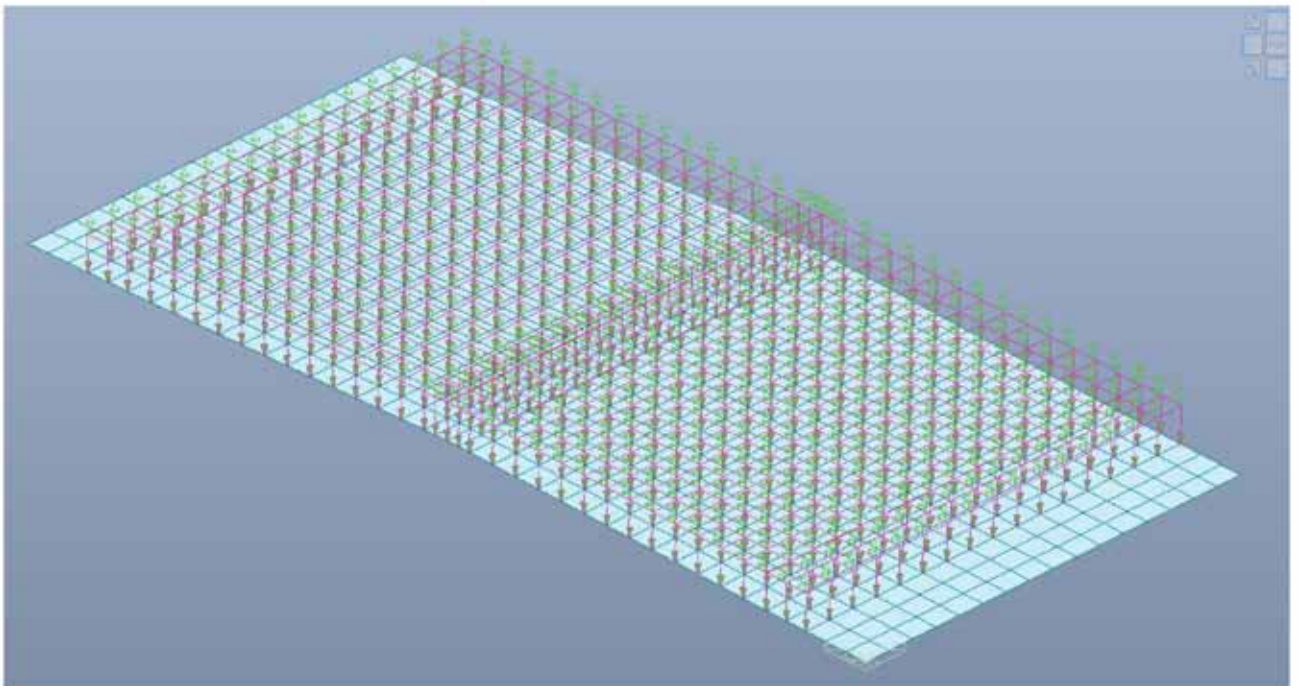


Figura 75 - Peso permanente portato



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

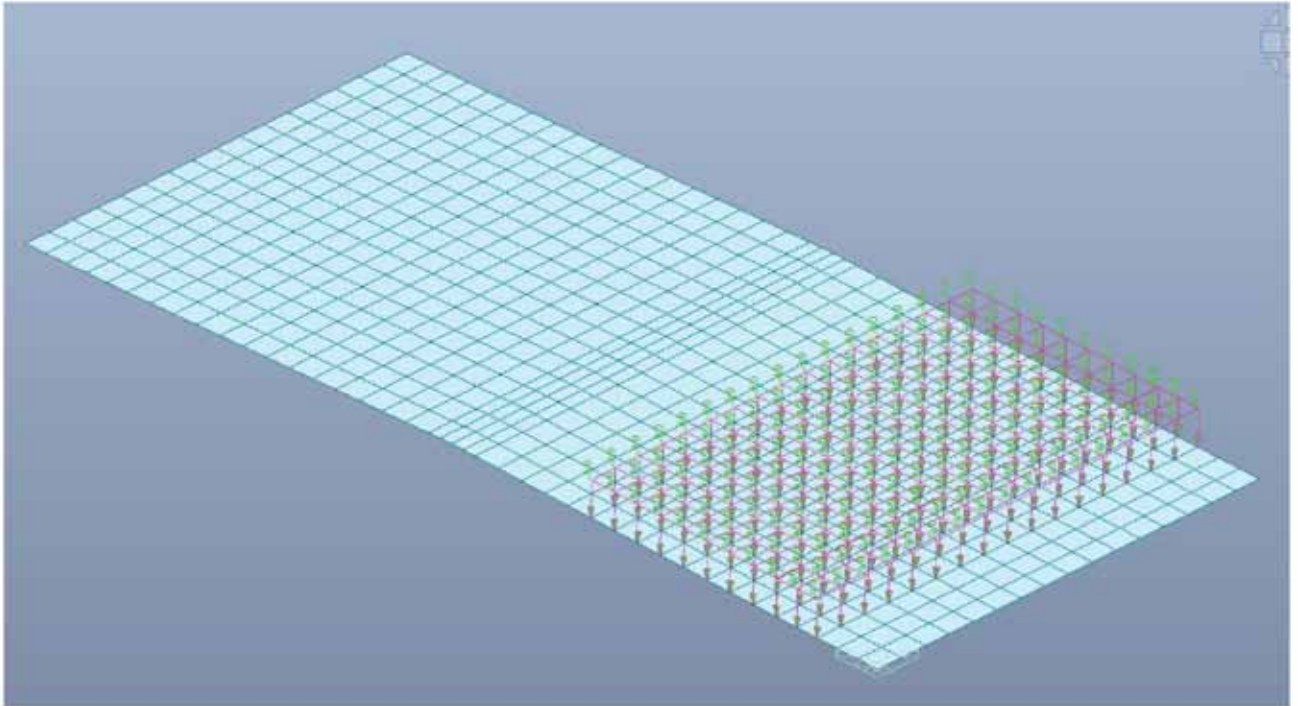


Figura 76 - Carico Distribuito prima colonna di carico

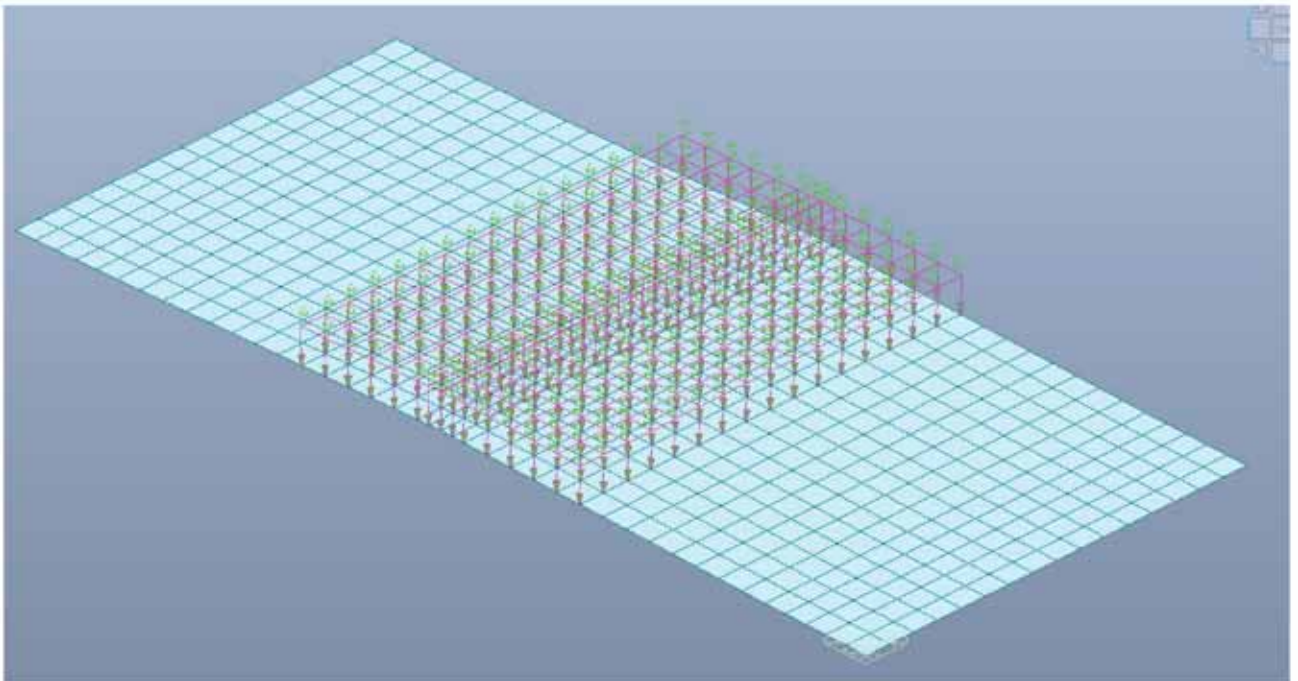


Figura 77 - Carico Distribuito seconda colonna di carico

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

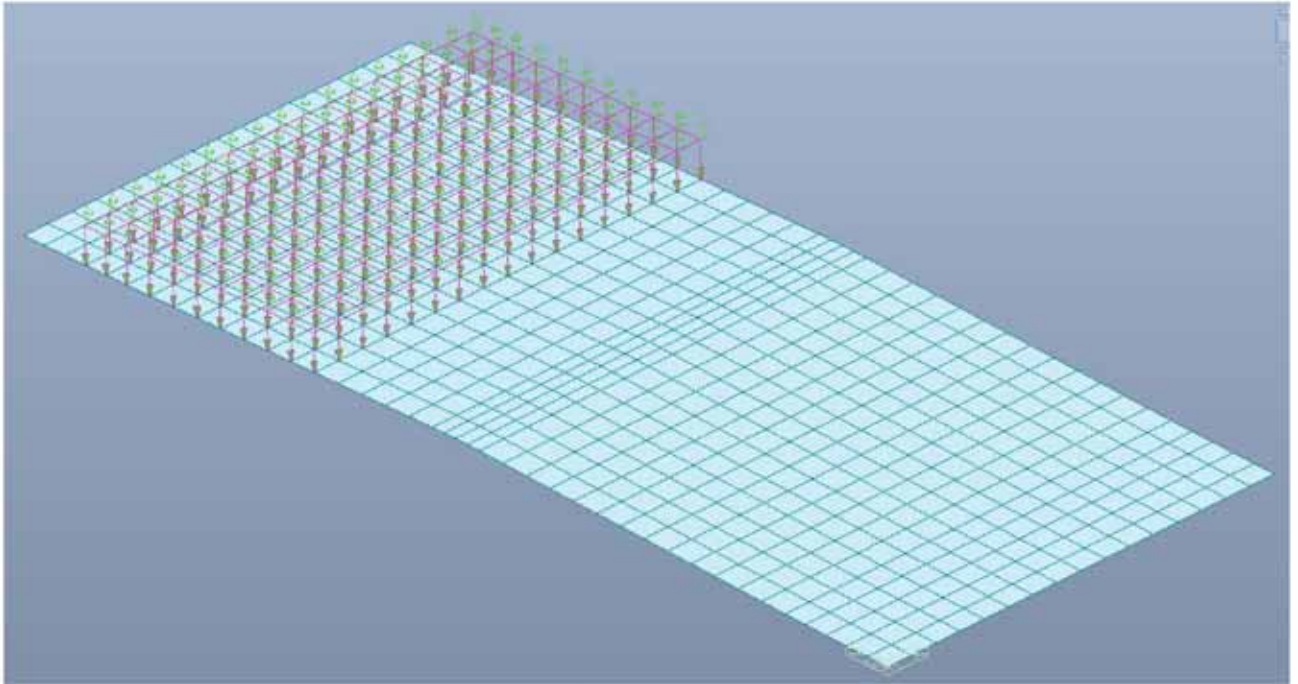


Figura 78 - Carico Distribuito terza colonna di carico

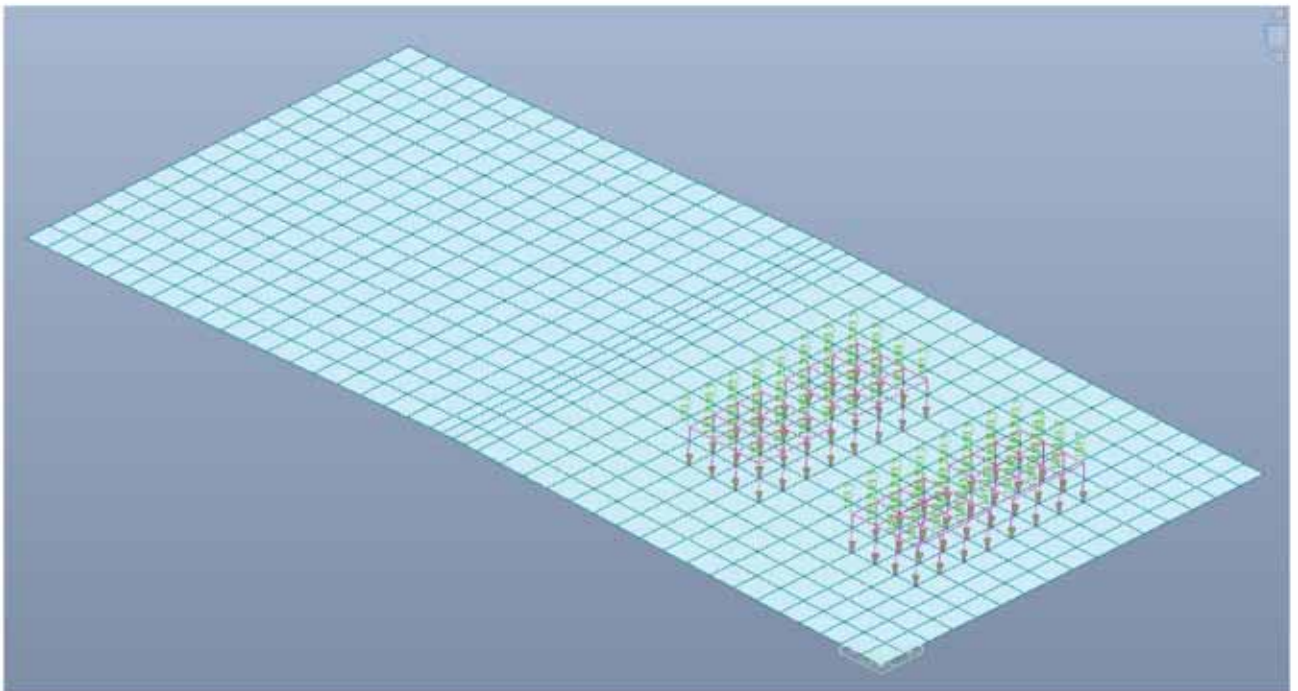


Figura 79 - Caso di carico Q1a

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

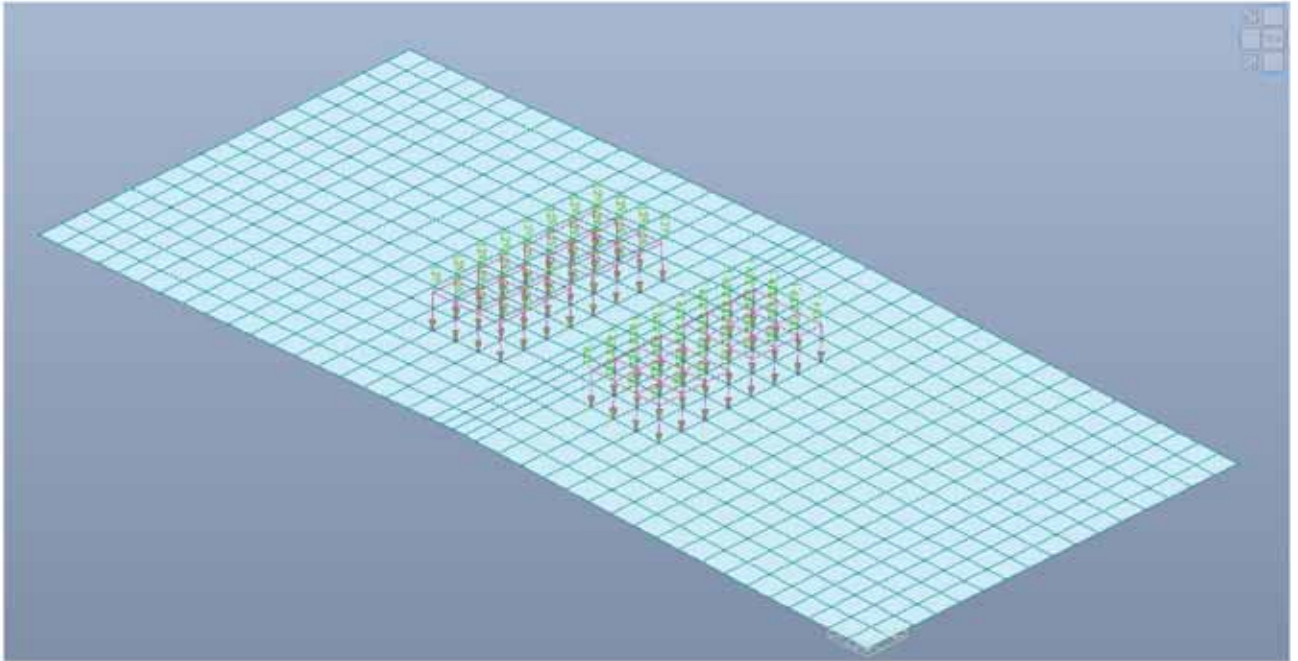


Figura 80 - Caso di carico Q1b

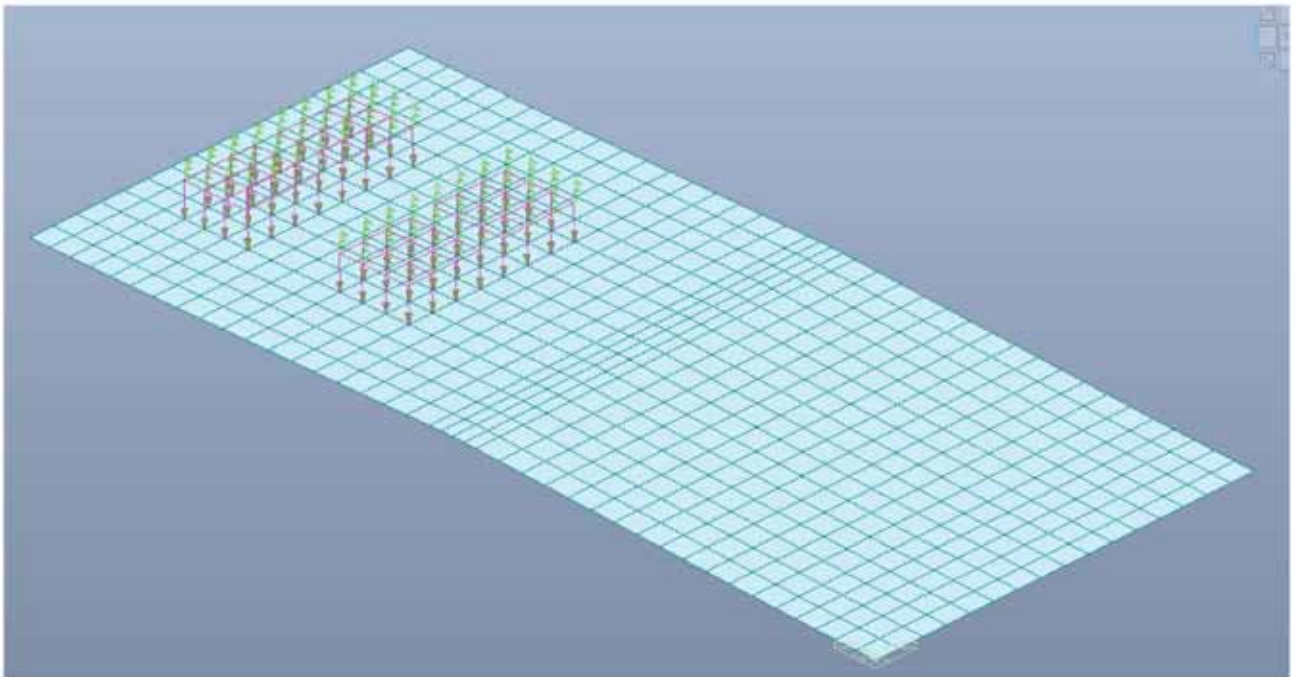


Figura 81 - Caso di carico Q1c

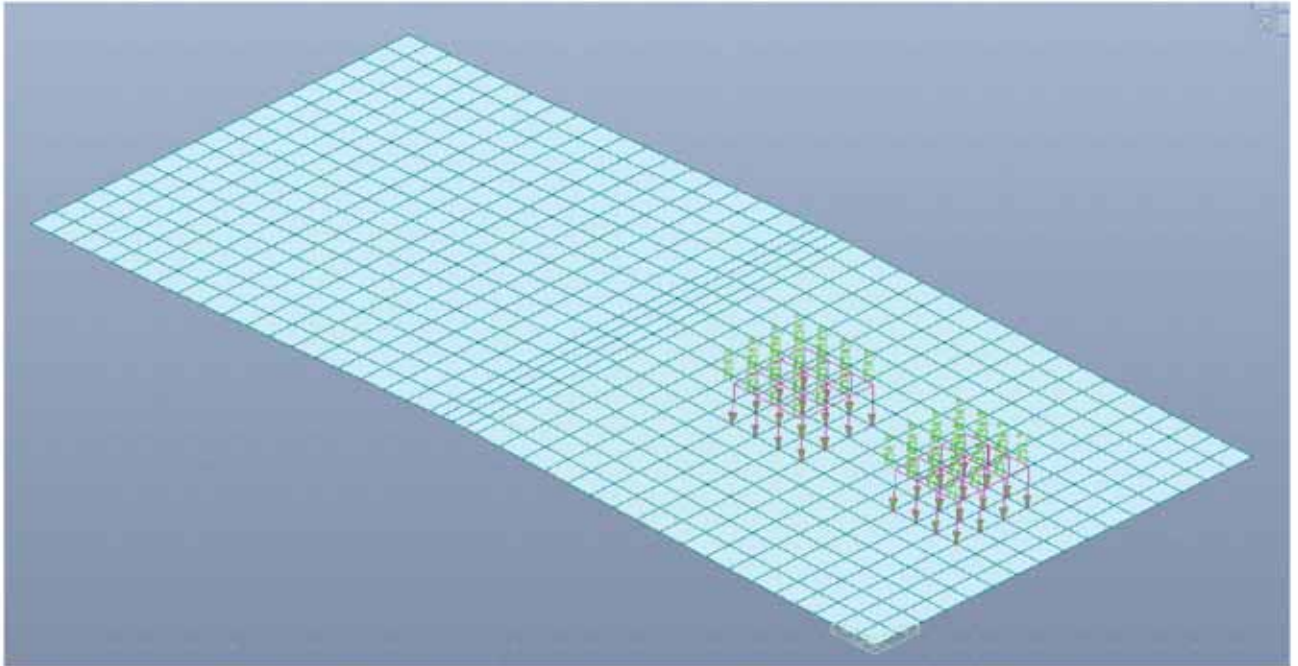


Figura 82 – Schema Carico 2

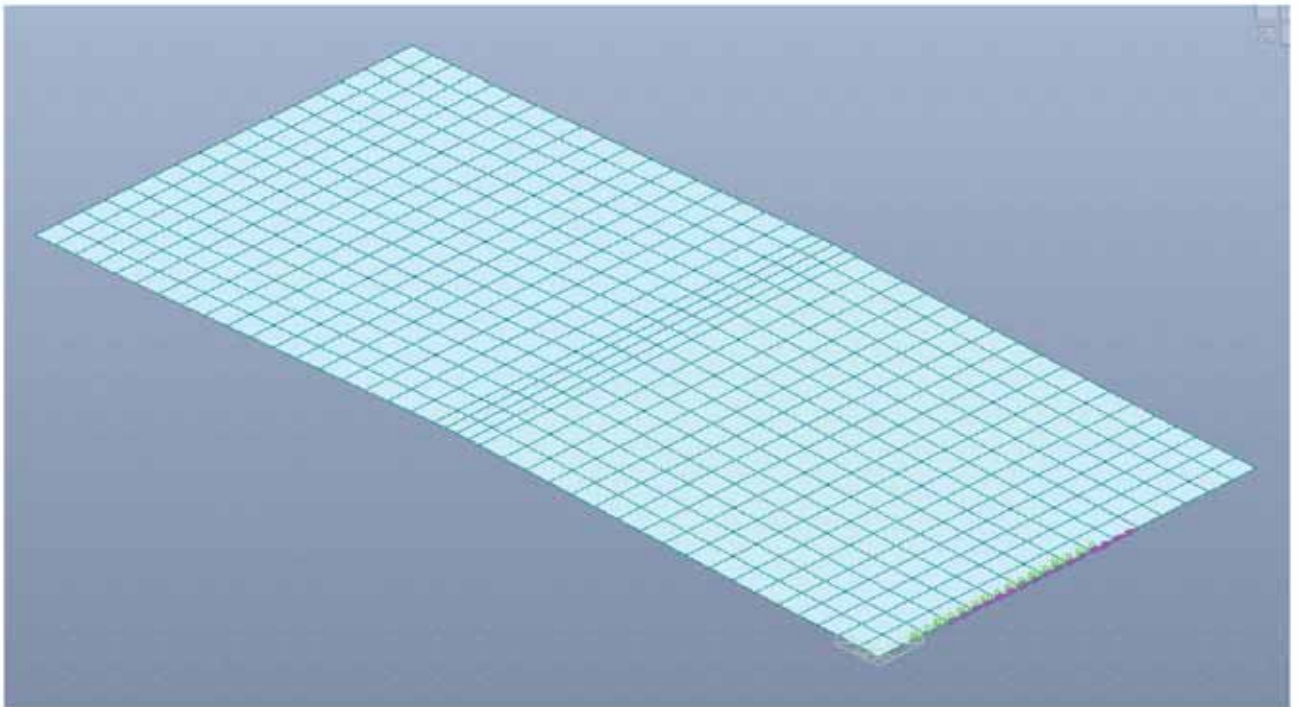


Figura 83 - Momento torcente che simula l'effetto dell'urto

7.2.1 VERIFICA STRUTTURALE SOLETTA

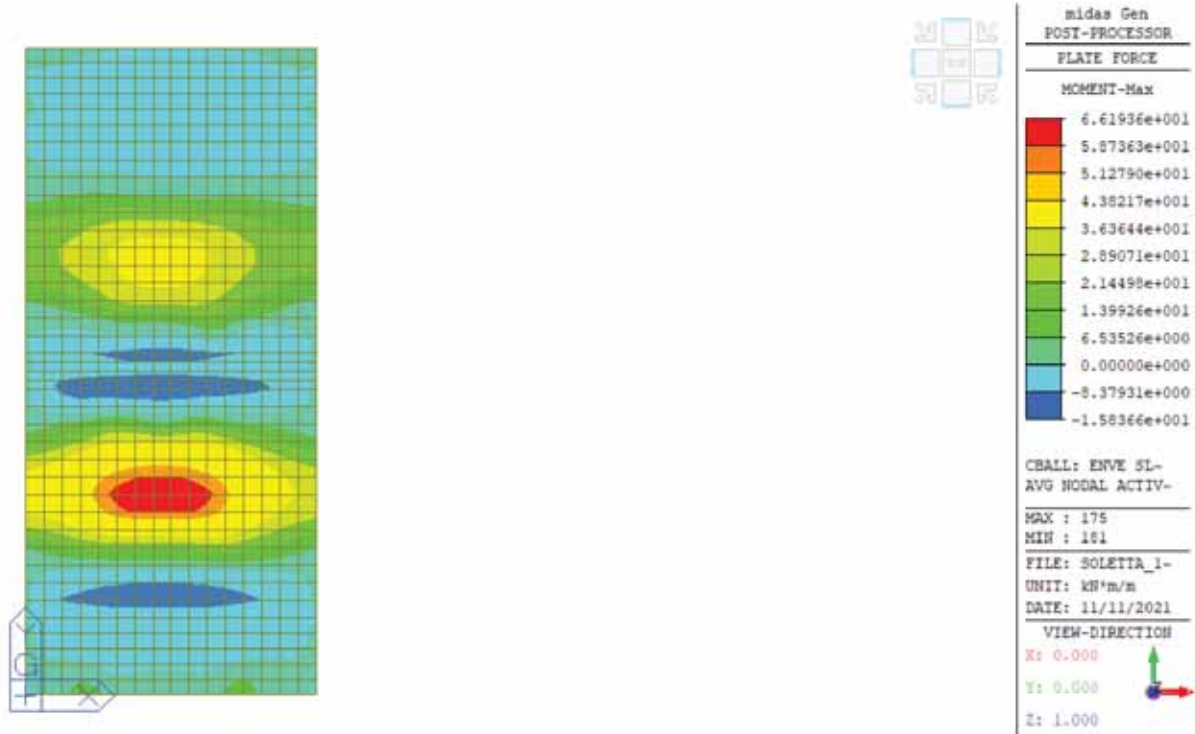


Figura 84 - Max Momento SLU

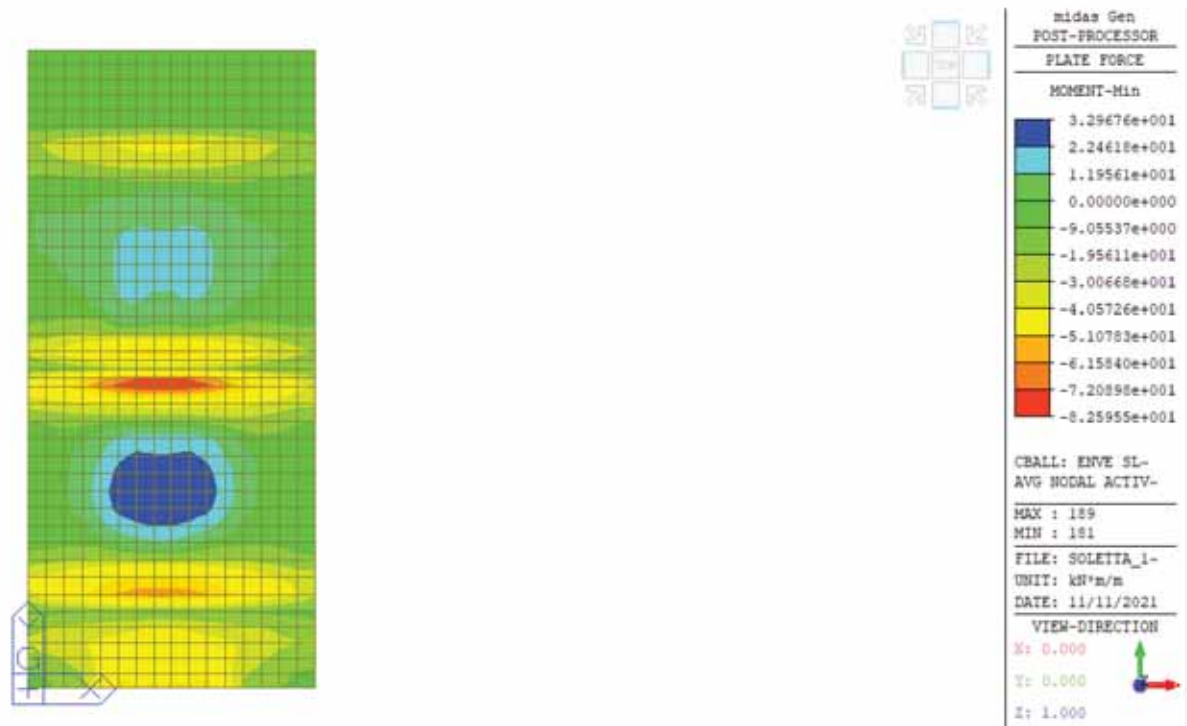


Figura 85 - Min Momento SLU

LOTTO 1 - CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

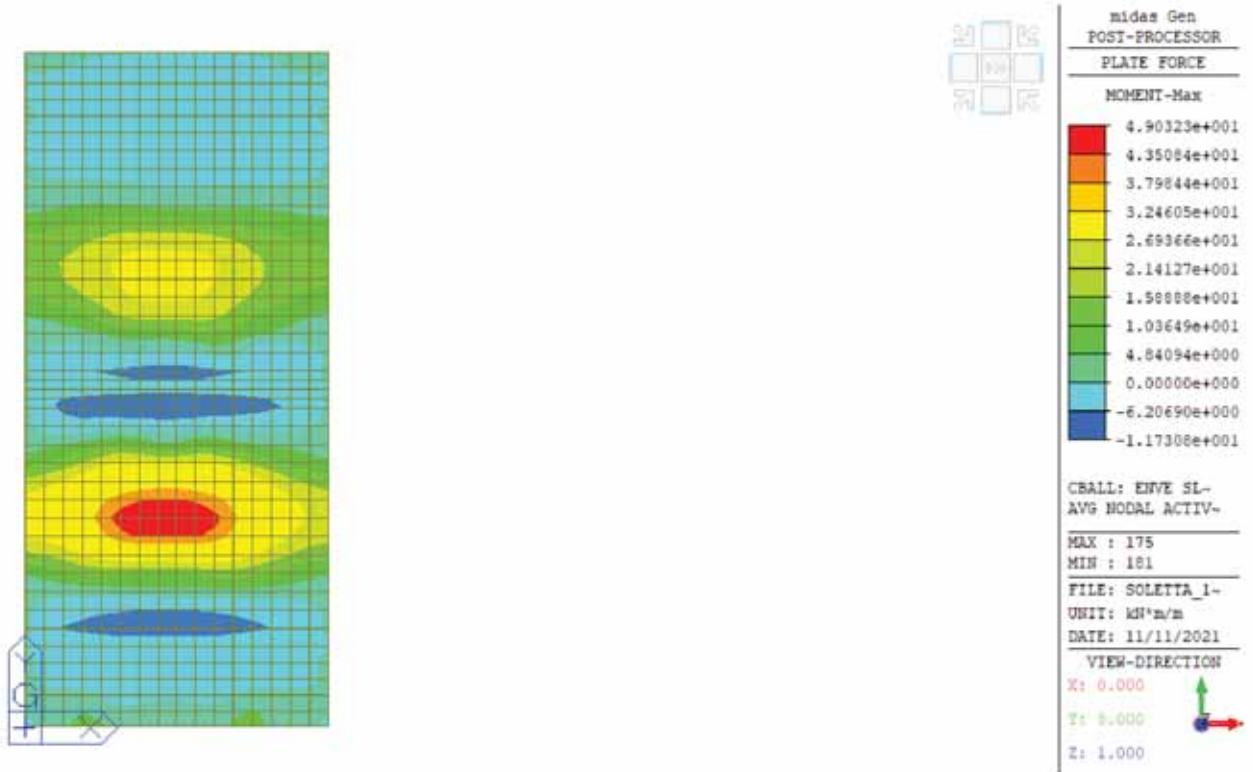


Figura 86 - Max Momento SLE

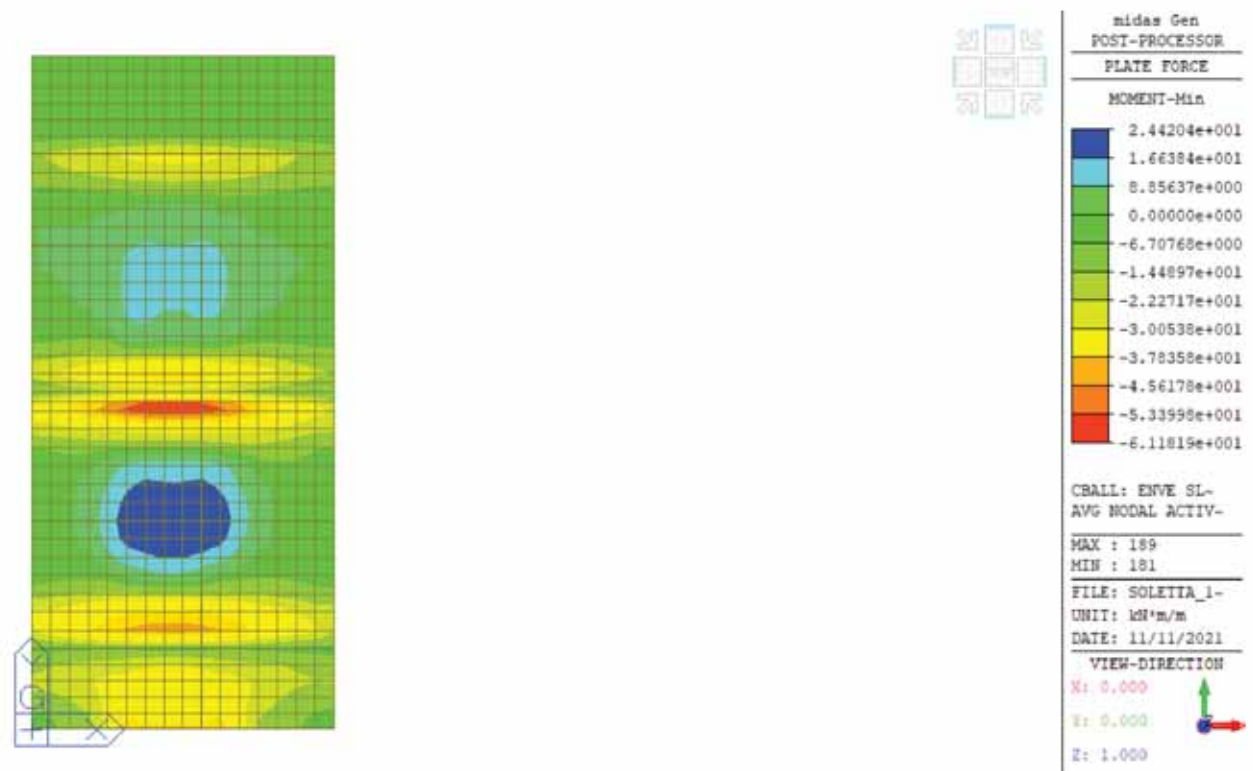


Figura 87 - Min Momento SLE

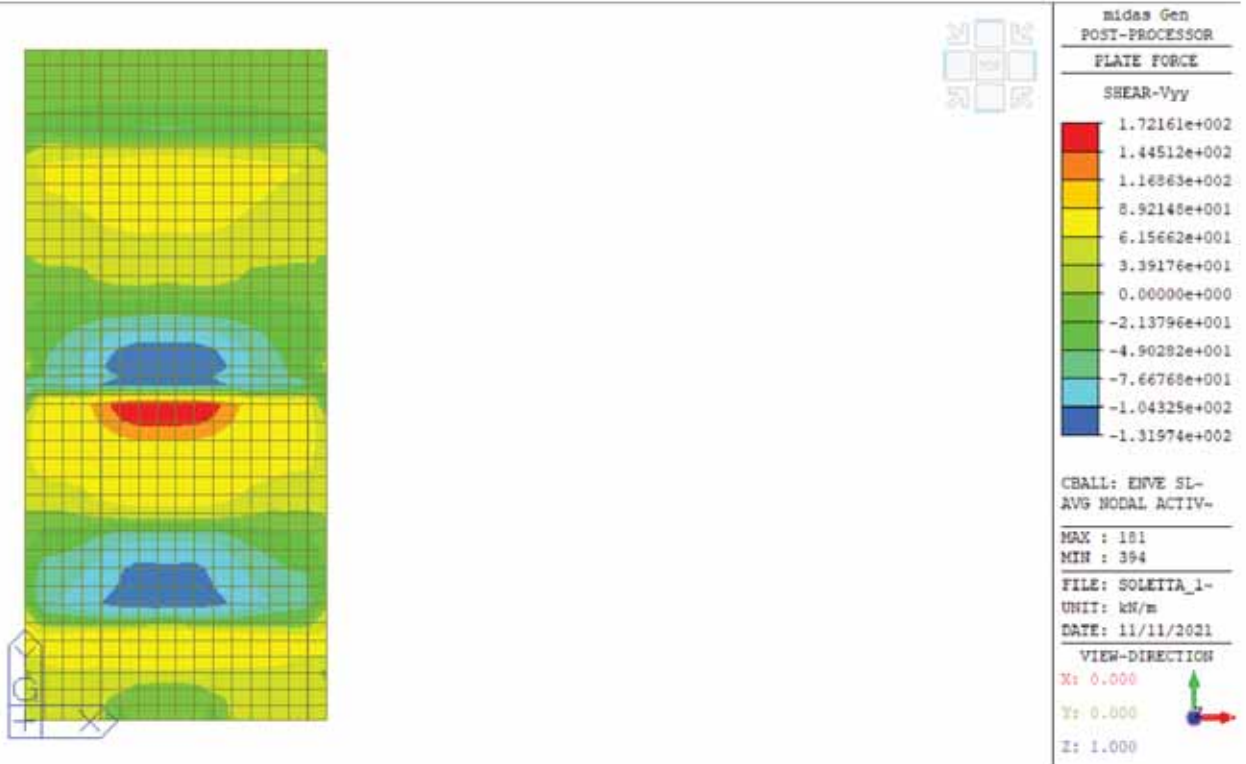
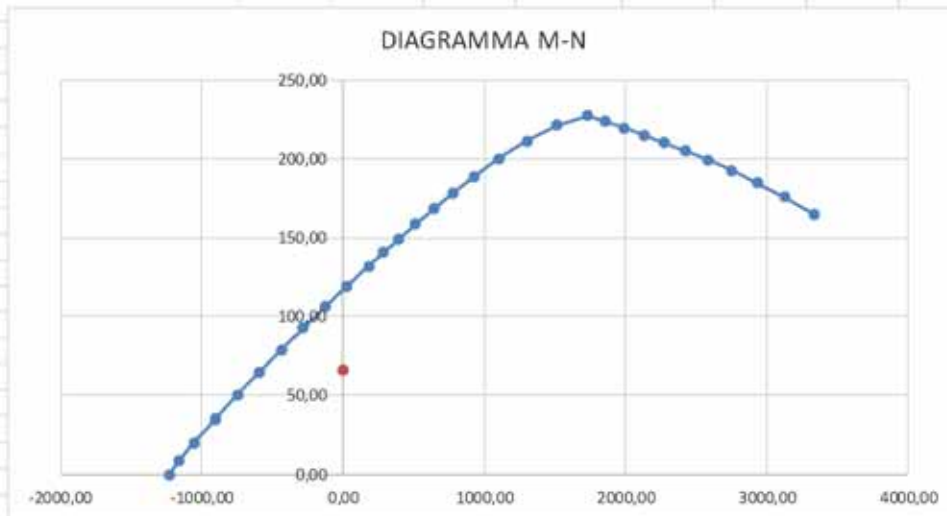


Figura 88 - Taglio SLU

**VERIFICA A FLESSIONE MOMENTO POSITIVO**

RIEPILOGO SOLLECITAZIONI		RIEPILOGO ARMATURE			RIEPILOGO GEOMETRIA			RIEPILOGO MATERIALI			
	M	N	n°	φ (mm)	y	B =	100	cm	fck	33,2	N/mm <sup>2</sup>
	kNm	kN		(mm)	(cm)	H =	26	cm	Rck	40	N/mm <sup>2</sup>
SLU/SLV	66,20	0,0							fcd =	18,81	
RARA	49,02	0,0	5	20	5				c	5	cm
FREQUENTE	49,02	0,0	1	0	0				fmax	16	mm
QUASI PERMANENTE	49,02	0,0	1	0	21				i'	0	cm
			5	20	21				tyk	450	N/mm <sup>2</sup>



VERIFICHE TENSIONALI		σc	σs	σcmax	σsmax
		N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>
RARA		6,14	172,16	19,92	360,00
	QUASI PERMANENTE	6,14		14,94	

VERIFICHE A FESSURAZIONE		Wk	Mf	M	Wklim	
		mm	kNm	kNm		
QUASI PERMANENTE		0,1030	42,42	35,63	0,2	Controllo Wk
	FREQUENTE	0,1030	42,42	35,63	0,3	Controllo Wk

**VERIFICA TENSIONALE COMBINAZIONE QUASI PERMANENTE :**

M = 49,02 kNm

N = 0 kN

H/6 = 4,33 cm

B = 100 cm

H = 26 cm

Verifica:

n = 15



LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

$$x = 7,32 \text{ cm}$$

$$s-c = 6,14 \text{ N/mm}^2$$

$$s-a = -29,17 \text{ N/mm}^2 \text{ (} y = 5 \text{ cm: } 5 \text{ f } 20, A_a = 15,71 \text{ cm}^2, \text{ passo } 20,0 \text{ cm)}$$

$$s-a' = 172,16 \text{ N/mm}^2 \text{ (} y = 21 \text{ cm: } 5 \text{ f } 20, A_a' = 15,71 \text{ cm}^2, \text{ passo } 20,0 \text{ cm)}$$

VERIFICA TENSIONALE COMBINAZIONE RARA :

$$M = 49,02 \text{ kNm}$$

$$N = 0 \text{ kN}$$

$$H/6 = 4,33 \text{ cm}$$

$$B = 100 \text{ cm}$$

$$H = 26 \text{ cm}$$

Verifica:

$$n = 15$$

$$x = 7,32 \text{ cm}$$

$$s-c = 6,14 \text{ N/mm}^2$$

$$s-a = -29,17 \text{ N/mm}^2 \text{ (} y = 5 \text{ cm: } 5 \text{ f } 20, A_a = 15,71 \text{ cm}^2, \text{ passo } 20,0 \text{ cm)}$$

$$s-a' = 172,16 \text{ N/mm}^2 \text{ (} y = 21 \text{ cm: } 5 \text{ f } 20, A_a' = 15,71 \text{ cm}^2, \text{ passo } 20,0 \text{ cm)}$$

VERIFICA A FESSURAZIONE COMBINAZIONE QUASI PERMANENTE :

$$M = 49,02 \text{ kNm}$$

$$N = 0 \text{ kNm}$$

$$R_{ck} = 40 \text{ N/mm}^2$$

$$f_{ctm} = 3,16 \text{ N/mm}^2 \text{ (} 0,27 \times R_{ck}^{2/3} \text{)}$$

$$B = 100 \text{ cm (larghezza sezione)}$$

$$H = 26 \text{ cm (altezza sezione)}$$

$$y = 5 \text{ cm (posizione di armatura)}$$

$$A_a = 15,71 \text{ cm}^2 \text{ (} n_i \text{ } 5 \text{ f } 20 \text{ passo } 20 \text{ cm)}$$

$$y' = 21 \text{ cm (posizione di armatura)}$$

$$A_a' = 15,71 \text{ cm}^2 \text{ (} n_i \text{ } 5 \text{ f } 20 \text{ passo } 20 \text{ cm)}$$

STADIO NON FESSURATO

$$n = 15 \text{ (coeff. omogeneizzazione)}$$

$$J_{id(I)} = 174'615 \text{ cm}^4$$

$$A_{id} = 3'040 \text{ cm}^2$$

$$x(I) = 13,000 \text{ cm (asse neutro stato I, dal lembo compresso)}$$

$$M = 35,63 \text{ kNm (Momento di formazione fessura } f_{cfk} = 0,7 \times 1,2 \times f_{ctm} \text{)}$$

$$M_f = 42,42 \text{ kNm (Momento di fessurazione } f_{ctm} \text{)}$$

STADIO FESSURATO (II)

$$n = 15 \text{ (coeff. omogeneizzazione)}$$

$$x(II) = 7,3 \text{ cm (asse neutro stato II)}$$

$$J_{id(II)} = 58'437 \text{ cm}^4$$

$$s = 172,16 \text{ N/mm}^2 \text{ (M,N)}$$

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

$$e s = 0,000820$$

Geometria tirante ideale

$$b = 100 \text{ cm}$$

$$h' = 17,8 \text{ cm } (c + f/2 + i' + 7,5xf)$$

$$A_s = 15,71 \text{ cm}^2$$

$$A_c = 1780,00 \text{ cm}^2$$

$$A_s/A_c = 0,0088$$

Distanza media fessure:

$$c = 5 \text{ cm (copriferro, da estradosso cls ad estradosso barra)}$$

$$i = 10,0 \text{ cm (interasse ferri in orizzontali)}$$

$$i' = 0 \text{ cm (interasse ferri verticale, nel caso di due strati di barre)}$$

$$f \text{ max} = 16 \text{ mm diametro massimo barre}$$

$$r = 0,0088 (A_s/A_c \text{ tirante ideale})$$

$$k_1 = 0,4 \text{ (barre a.m.)}$$

$$k_2 = 0,079 (0,25x[1-h'/(2x(H-x1))])$$

$$s_m = 17,72 \text{ cm } (2x(c+i/10)+k_1xk_2xf/r)$$

Deformazione media:

$$b_1 = 1 \text{ (barre a.m.)}$$

$$b_2 = 0,5 \text{ (carichi di lunga durata o numerosi cicli di carico)}$$

$$s_{sr} = 148,97 \text{ N/mm}^2 \text{ (stato II, Mf)}$$

$$s_{ss} = 172,16 \text{ N/mm}^2 \text{ (stato II, Magente)}$$

$$e_{sm} = 0,000513 (s_s/E_sx(1-b_1xb_2x(s_{sr}/s_s)^2))$$

Ampiezza teorica di fessura:

$$w_k = 0,154 \text{ mm } (1,7x e_{sm}x s_m)$$

VERIFICA A FESSURAZIONE COMBINAZIONE FREQUENTE :

$$M = 49,02 \text{ kNm}$$

$$N = 0 \text{ kNm}$$

$$R_{ck} = 40 \text{ N/mm}^2$$

$$f_{ctm} = 3,16 \text{ N/mm}^2 (0,27xR_{ck}^{2/3})$$

$$B = 100 \text{ cm (larghezza sezione)}$$

$$H = 26 \text{ cm (altezza sezione)}$$

$$y = 5 \text{ cm (posizione di armatura)}$$

$$A_a = 15,71 \text{ cm}^2 (n_i \text{ 5 f 20 passo 20 cm})$$

$$y' = 21 \text{ cm (posizione di armatura)}$$

$$A_a' = 15,71 \text{ cm}^2 (n_i \text{ 5 f 20 passo 20 cm})$$

STADIO NON FESSURATO

$$n = 15 \text{ (coeff. omogeneizzazione)}$$

$$J_{id(I)} = 174'615 \text{ cm}^4$$

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

$$Aid = 3'040 \text{ cm}^2$$

$$x(I) = 13,000 \text{ cm (asse neutro stato I, dal lembo compresso)}$$

$$M = 35,63 \text{ kNm (Momento di formazione fessura } f_{ctk} = 0,7 \times 1,2 \times f_{ctm})$$

$$M_f = 42,42 \text{ kNm (Momento di fessurazione } f_{ctm})$$

STADIO FESSURATO (II)

$$n = 15 \text{ (coeff. omogeneizzazione)}$$

$$x(II) = 7,3 \text{ cm (asse neutro stato II)}$$

$$Jid(II) = 58'437 \text{ cm}^2$$

$$s_s = 172,16 \text{ N/mm}^2 \text{ (M,N)}$$

$$e_s = 0,000820$$

Geometria tirante ideale

$$b = 100 \text{ cm}$$

$$h' = 17,8 \text{ cm (} c + f/2 + i' + 7,5 \times f)$$

$$A_s = 15,71 \text{ cm}^2$$

$$A_c = 1780,00 \text{ cm}^2$$

$$A_s/A_c = 0,0088$$

Distanza media fessure:

$$c = 5 \text{ cm (copriferro, da estradosso cls ad estradosso barra)}$$

$$i = 10,0 \text{ cm (interasse ferri in orizzontali)}$$

$$i' = 0 \text{ cm (interasse ferri verticale, nel caso di due strati di barre)}$$

$$f_{\max} = 16 \text{ mm diametro massimo barre}$$

$$r = 0,0088 \text{ (} A_s/A_c \text{ tirante ideale)}$$

$$k_1 = 0,4 \text{ (barre a.m.)}$$

$$k_2 = 0,079 \text{ (} 0,25 \times [1 - h' / (2 \times (H - x_1))] \text{)}$$

$$s_m = 17,72 \text{ cm (} 2 \times (c + i/10) + k_1 \times k_2 \times f/r)$$

Deformazione media:

$$b_1 = 1 \text{ (barre a.m.)}$$

$$b_2 = 0,5 \text{ (carichi di lunga durata o numerosi cicli di carico)}$$

$$s_{sr} = 148,97 \text{ N/mm}^2 \text{ (stato II, } M_f)$$

$$s_{ss} = 172,16 \text{ N/mm}^2 \text{ (stato II, Magente)}$$

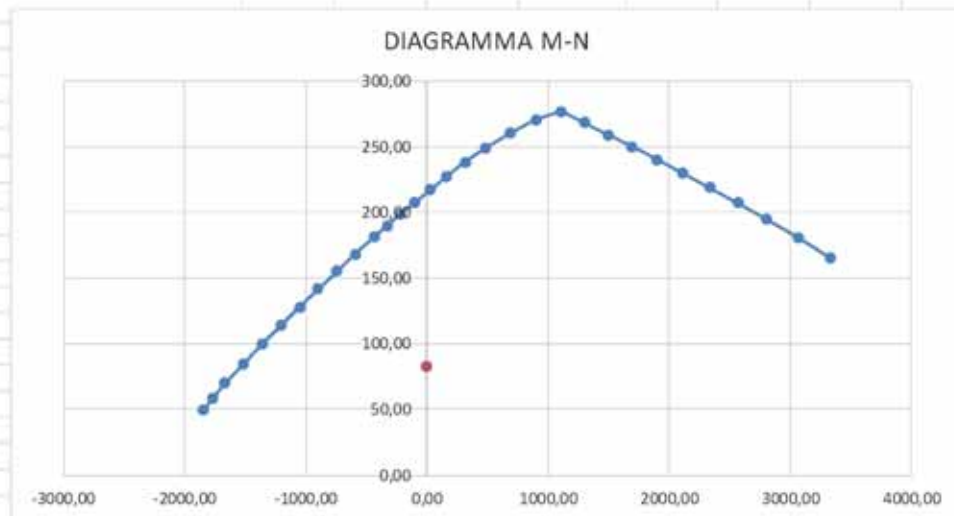
$$e_{sm} = 0,000513 \text{ (} s_s/E_s \times (1 - b_1 \times b_2 \times (s_{sr}/s_s)^2 \text{))}$$

Ampiezza teorica di fessura:

$$w_k = 0,154 \text{ mm (} 1,7 \times e_{sm} \times s_m)$$

**VERIFICA A FLESSIONE MOMENTO NEGATIVO**

RIEPILOGO SOLLECITAZIONI			RIEPILOGO ARMATURE			RIEPILOGO GEOMETRIA			RIEPILOGO MATERIALI		
	M	N	n°	φ (mm)	y	B =	100	cm			
	kNm	kN		(mm)	(cm)	H =	26	cm	fck	33,2	N/mm <sup>2</sup>
SLU/SLV	82,59	0,0							Rck	40	N/mm <sup>2</sup>
RARA	61,18	0,0	5	20	5				fcd =	18,81	
FREQUENTE	61,18	0,0	1	0	0				c	4	cm
QUASI PERMANENTE	61,18	0,0	1	0	21				fmax	16	mm
			10	20	21				i	10	cm
									i'	0	cm
									fyk	450	N/mm <sup>2</sup>



VERIFICHE TENSIONALI	RARA	QUASI PERMANENTE	σc	σs	σcmax	σsmax
			N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>
			6,02	111,22	19,92	360,00
			6,02		14,94	

VERIFICHE A FESSURAZIONE	QUASI PERMANENTE	FREQUENTE	Wk	Mf	M	Wkim
			mm	kNm	kNm	
			0,0535	47,58	39,97	0,2
			0,0535	47,58	39,97	0,3

**VERIFICA TENSIONALE COMBINAZIONE QUASI PERMANENTE :**

M = 61,18 kNm  
 N = 0 kN  
 H/6 = 4,33 cm  
 B = 100 cm  
 H = 26 cm  
 Verifica:  
 n = 15  
 x = 9,41 cm  
 s-c = 6,02 N/mm<sup>2</sup>

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

$s-a = -42,28 \text{ N/mm}^2$  ( $y = 5 \text{ cm}$ : 5 f 20,  $A_a = 15,71 \text{ cm}^2$ , passo 20,0 cm)

$s-a' = 111,22 \text{ N/mm}^2$  ( $y = 21 \text{ cm}$ : 10 f 20,  $A_{a'} = 31,42 \text{ cm}^2$ , passo 10,0 cm)

VERIFICA TENSIONALE COMBINAZIONE RARA :

$M = 61,18 \text{ kNm}$

$N = 0 \text{ kN}$

$H/6 = 4,33 \text{ cm}$

$B = 100 \text{ cm}$

$H = 26 \text{ cm}$

Verifica:

$n = 15$

$x = 9,41 \text{ cm}$

$s-c = 6,02 \text{ N/mm}^2$

$s-a = -42,28 \text{ N/mm}^2$  ( $y = 5 \text{ cm}$ : 5 f 20,  $A_a = 15,71 \text{ cm}^2$ , passo 20,0 cm)

$s-a' = 111,22 \text{ N/mm}^2$  ( $y = 21 \text{ cm}$ : 10 f 20,  $A_{a'} = 31,42 \text{ cm}^2$ , passo 10,0 cm)

VERIFICA A FESSURAZIONE COMBINAZIONE QUASI PERMANENTE :

$M = 61,18 \text{ kNm}$

$N = 0 \text{ kNm}$

$R_{ck} = 40 \text{ N/mm}^2$

$f_{ctm} = 3,16 \text{ N/mm}^2$  ( $0,27 \times R_{ck}^{2/3}$ )

$B = 100 \text{ cm}$  (larghezza sezione)

$H = 26 \text{ cm}$  (altezza sezione)

$y = 5 \text{ cm}$  (posizione di armatura)

$A_a = 15,71 \text{ cm}^2$  ( $n_i$  5 f 20 passo 20 cm)

$y' = 21 \text{ cm}$  (posizione di armatura)

$A_{a'} = 31,42 \text{ cm}^2$  ( $n_i$  10 f 20 passo 10 cm)

STADIO NON FESSURATO

$n = 15$  (coeff. omogeneizzazione)

$J_{id(I)} = 187'740 \text{ cm}^4$

$A_{id} = 3'260 \text{ cm}^2$

$x(I) = 13,540 \text{ cm}$  (asse neutro stato I, dal lembo compresso)

$M = 39,97 \text{ kNm}$  (Momento di formazione fessura  $f_{cfk} = 0,7 \times 1,2 \times f_{ctm}$ )

$M_f = 47,58 \text{ kNm}$  (Momento di fessurazione  $f_{ctm}$ )

STADIO FESSURATO (II)

$n = 15$  (coeff. omogeneizzazione)

$x(II) = 9,4 \text{ cm}$  (asse neutro stato II)

$J_{id(II)} = 95'658 \text{ cm}^4$

$s = 111,22 \text{ N/mm}^2$  (M,N)

$e = 0,000530$

Geometria tirante ideale

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

$$b = 100 \text{ cm}$$

$$h' = 16,8 \text{ cm } (c + f/2 + i' + 7,5xf)$$

$$A_s = 31,42 \text{ cm}^2$$

$$A_c = 1680,00 \text{ cm}^2$$

$$A_s/A_c = 0,0187$$

Distanza media fessure:

$$c = 4 \text{ cm (copriferro, da estradosso cls ad estradosso barra)}$$

$$i = 10,0 \text{ cm (interasse ferri in orizzontali)}$$

$$i' = 0 \text{ cm (interasse ferri verticale, nel caso di due strati di barre)}$$

$$f \text{ max} = 16 \text{ mm diametro massimo barre}$$

$$r = 0,0187 (A_s/A_c \text{ tirante ideale})$$

$$k_1 = 0,4 \text{ (barre a.m.)}$$

$$k_2 = 0,081 (0,25x[1-h'/(2x(H-x1))])$$

$$s_m = 12,79 \text{ cm } (2x(c+i/10)+k_1xk_2xf/r)$$

Deformazione media:

$$b_1 = 1 \text{ (barre a.m.)}$$

$$b_2 = 0,5 \text{ (carichi di lunga durata o numerosi cicli di carico)}$$

$$s_{sr} = 86,50 \text{ N/mm}^2 \text{ (stato II, Mf)}$$

$$s_{ss} = 111,22 \text{ N/mm}^2 \text{ (stato II, Magente)}$$

$$e_{sm} = 0,000369 (s_{ss}/E_sx(1-b_1xb_2x(s_{sr}/s_{ss})^2))$$

Ampiezza teorica di fessura:

$$w_k = 0,080 \text{ mm } (1,7x e_{sm}x s_m)$$

VERIFICA A FESSURAZIONE COMBINAZIONE FREQUENTE :

$$M = 61,18 \text{ kNm}$$

$$N = 0 \text{ kNm}$$

$$R_{ck} = 40 \text{ N/mm}^2$$

$$f_{ctm} = 3,16 \text{ N/mm}^2 (0,27xR_{ck}^2/3)$$

$$B = 100 \text{ cm (larghezza sezione)}$$

$$H = 26 \text{ cm (altezza sezione)}$$

$$y = 5 \text{ cm (posizione di armatura)}$$

$$A_a = 15,71 \text{ cm}^2 (n_j \text{ 5 f 20 passo 20 cm})$$

$$y' = 21 \text{ cm (posizione di armatura)}$$

$$A_a' = 31,42 \text{ cm}^2 (n_j \text{ 10 f 20 passo 10 cm})$$

STADIO NON FESSURATO

$$n = 15 \text{ (coeff. omogeneizzazione)}$$

$$J_{id}(I) = 187'740 \text{ cm}^4$$

$$A_{id} = 3'260 \text{ cm}^2$$

$$x(I) = 13,540 \text{ cm (asse neutro stato I, dal lembo compresso)}$$

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

$M = 39,97 \text{ kNm}$  (Momento di formazione fessura  $f_{cfk} = 0,7 \times 1,2 \times f_{ctm}$ )

$M_f = 47,58 \text{ kNm}$  (Momento di fessurazione  $f_{ctm}$ )

STADIO FESSURATO (II)

$n = 15$  (coeff. omogeneizzazione)

$x(II) = 9,4 \text{ cm}$  (asse neutro stato II)

$J_{id(II)} = 95'658 \text{ cm}^2$

$s_s = 111,22 \text{ N/mm}^2$  (M,N)

$e_s = 0,000530$

Geometria tirante ideale

$b = 100 \text{ cm}$

$h' = 16,8 \text{ cm}$  ( $c + f/2 + i' + 7,5 \times f$ )

$A_s = 31,42 \text{ cm}^2$

$A_c = 1680,00 \text{ cm}^2$

$A_s/A_c = 0,0187$

Distanza media fessure:

$c = 4 \text{ cm}$  (copriferro, da estradosso cls ad estradosso barra)

$i = 10,0 \text{ cm}$  (interasse ferri in orizzontali)

$i' = 0 \text{ cm}$  (interasse ferri verticale, nel caso di due strati di barre)

$f_{\max} = 16 \text{ mm}$  diametro massimo barre

$r = 0,0187$  ( $A_s/A_c$  tirante ideale)

$k_1 = 0,4$  (barre a.m.)

$k_2 = 0,081$  ( $0,25 \times [1 - h' / (2 \times (H - x_1))]$ )

$s_m = 12,79 \text{ cm}$  ( $2 \times (c + i/10) + k_1 \times k_2 \times f/r$ )

Deformazione media:

$b_1 = 1$  (barre a.m.)

$b_2 = 0,5$  (carichi di lunga durata o numerosi cicli di carico)

$s_{sr} = 86,50 \text{ N/mm}^2$  (stato II,  $M_f$ )

$s_{ss} = 111,22 \text{ N/mm}^2$  (stato II, Magente)

$e_{sm} = 0,000369$  ( $s_s/E_s \times (1 - b_1 \times b_2 \times (s_{sr}/s_s)^2)$ )

Ampiezza teorica di fessura:

$w_k = 0,080 \text{ mm}$  ( $1,7 \times e_{sm} \times s_m$ )





## 8 VERIFICA ELEMENTI DI APPOGGIO

Si riporta di seguito l'andamento degli spostamenti orizzontali necessari per il dimensionamento dei varchi, nonché gli scarichi necessari per il dimensionamento dei boggioni e dei ritegni sismici.

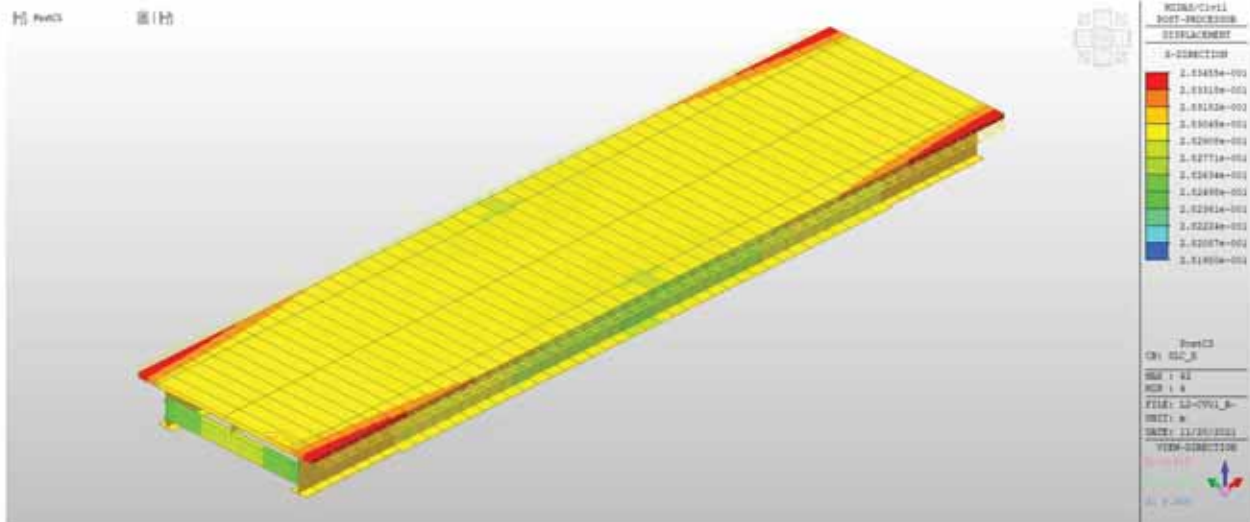


Figura 89 - Spostamenti Stato Limite Collasso X (SISMAX+0.30SISMAY)

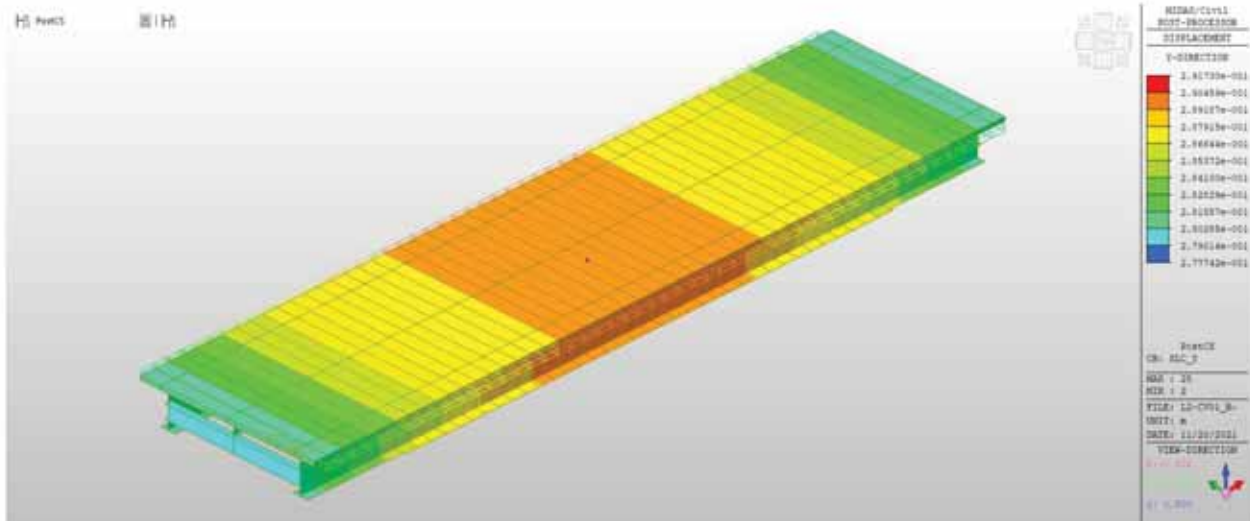


Figura 90 - Spostamenti Stato Limite Collasso Y (SISMAY+0.30SISMAX)

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

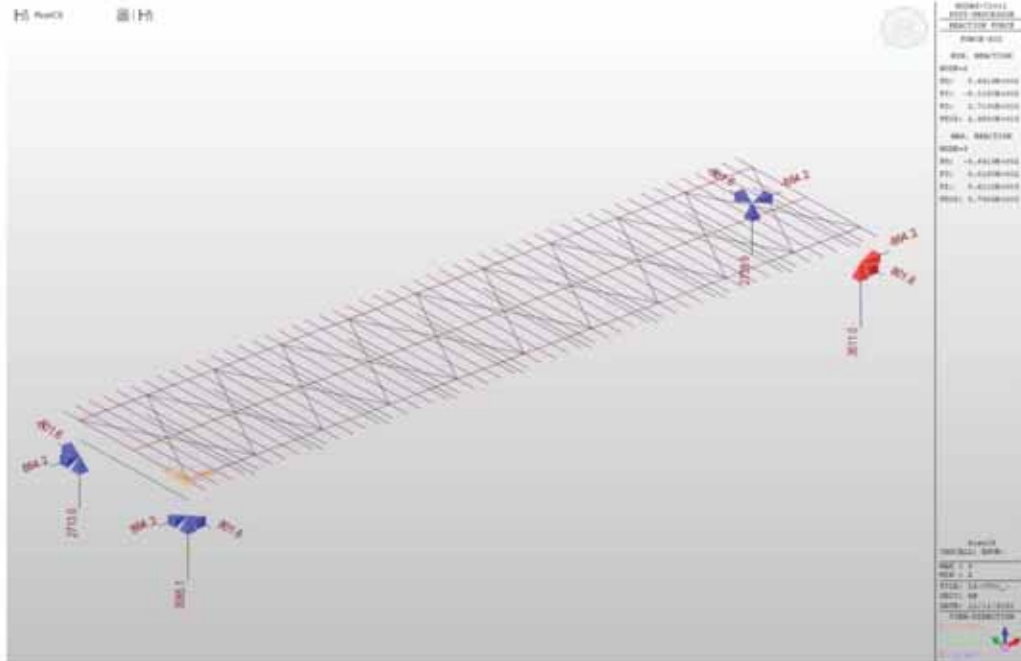


Figura 91 – Reazioni agli appoggi

Node	Load	FX (kN)	FY (kN)	FZ (kN)
1	FASE 3	40.820	1.180	1315.085
2	FASE 3	40.820	-1.180	1315.085
1	VENTO PONTE SCARICO	0.143	46.191	29.861
2	VENTO PONTE SCARICO	-0.143	46.203	-29.861
1	VENTO PONTE CARICO	-0.075	103.942	46.118
2	VENTO PONTE CARICO	0.076	103.969	-46.118
1	TERMICA UNIFORME +	31.960	10.191	0.000
2	TERMICA UNIFORME +	31.960	-10.191	0.000
1	TERMICA UNIFORME -	-29.898	-9.533	0.000
2	TERMICA UNIFORME -	-29.898	9.533	0.000
1	TERMICA GRADIENTE +	-10.699	-0.589	0.000
2	TERMICA GRADIENTE +	-10.699	0.589	0.000
1	TERMICA GRADIENTE -	12.839	0.706	0.000
2	TERMICA GRADIENTE -	12.839	-0.706	0.000
1	FRENATURA	-114.513	0.001	-9.603
2	FRENATURA	-114.513	-0.001	-9.603
1	EX(RS)	807.283	0.023	75.568
2	EX(RS)	807.283	0.023	75.568
1	EY(RS)	0.346	795.323	382.831
2	EY(RS)	0.346	795.323	382.831
1	EZ(RS)	19.230	0.229	743.037
2	EZ(RS)	19.230	0.229	743.037
1	DX(RS)	152.987	0.004	14.330
2	DX(RS)	152.987	0.004	14.330
1	CX(RS)	1189.687	0.034	111.350
2	CX(RS)	1189.687	0.034	111.350
1	LM01(all)	17.648	0.842	1001.604
2	LM01(all)	3.812	-0.793	143.115
1	LM02(all)	4.766	1.164	273.948
2	LM02(all)	4.766	-1.164	273.948
1	LM03(all)	1.159	0.256	43.861
2	LM03(all)	5.336	-0.271	306.962

## 8.1 VERIFICA BAGGIOLI

DATI DI INPUT		
Forza orizzontale	$V_{Sd}$ [kN] =	864.200
Forza verticale (+ se di trazione)	$N_{Sd} / 4$ [kN] =	-902.750
Altezza di applicazione forza orizzontale	a [m] =	0.27
Base baggiolo	h [m] =	1.05
Copriferro	c [m] =	0.05
Altezza utile	d [m] =	0.875
Profondità di incastro assunta	h' [m] =	0.00
Luce di calcolo mensola	L [m] =	0.27
Profondità baggiolo	b [m] =	1.00
Angolo inclinazione traliccio [°]	$\alpha$ [°] =	18.92
Angolo inclinazione traliccio [rad]	$\alpha$ [rad] =	0.33
Angolo inclinazione traliccio [°]	$\psi$ [°] =	71.08
Angolo inclinazione traliccio [rad]	$\psi$ [rad] =	1.24
VERIFICA CORRENTE TESO		
Trazione sulle barre d'armatura corrente teso	$Z_{Sd}$ [kN] =	-606.45
Classe acciaio		B450C
Resistenza a snervamento armatura	$f_{yd}$ [MPa] =	391.3
Area armatura necessaria	$A_{nec}$ [cm <sup>2</sup> ] =	15.5
Diametro armatura 1	$\varnothing 1$ [mm] =	16
Diametro armatura 2	$\varnothing 2$ [mm] =	0
Numero ferri $\varnothing 1$	$n_{\varnothing 1}$ [-] =	9
Numero ferri $\varnothing 2$	$n_{\varnothing 2}$ [-] =	0
Area armatura 1 (singola barra)	$A_{s1}$ [cm <sup>2</sup> ] =	2.01
Area armatura 2 (singola barra)	$A_{s2}$ [cm <sup>2</sup> ] =	0.00
Area armatura totale	$A_{sTOT}$ [cm <sup>2</sup> ] =	18.1
Resistenza corrente teso	$Z_{Rd}$ [kN] =	708.09
Coefficiente di sicurezza	$Z_{Rd} / Z_{Sd} = c.s.$ =	1.17
VERIFICA PUNTO CLS COMPRESSO		
Resistenza cubica caratteristica	$R_{ck}$ [MPa] =	40
Resistenza cilindrica caratteristica	$f_{ck}$ [MPa] =	32
Coefficiente parziale di sicurezza	$\gamma_c$ [-] =	1.50
Resistenza di calcolo a compressione	$f_{cd}$ [MPa] =	18.13
Presenza di staffe		Si
Coefficiente presenza staffe (1 senza staffe; 1.5 con staffe)	c [-] =	1.50
Pendenza del puntone di calcestruzzo	$\cotg \psi = \tan \alpha = \lambda$ =	0.34
Altezza puntone	$0.4 * c * d * \sin \psi = hp$ [m] =	0.497
Compressione nel puntone	$C_{Sd}$ [kN] =	1869.90
Resistenza del puntone di calcestruzzo	$0.4 * b * d * f_{cd} * c / (1 + \lambda^2) = C_{Rd}$ [kN] =	8519
Coefficiente di sicurezza	$C_{Rd} / C_{Sd} = c.s.$ =	4.56

## 8.2 ISOLATORI – EFFETTO VARIABILITA' RIGIDEZZA

Si riportano di seguito le caratteristiche degli isolatori individuati.

V	Fzd	Ke	$\xi_e$	Kv	Dg	te	h	H	Z
kN	kN	kN/mm	%	kN/mm	mm	mm	mm	mm	mm
15080	33560	3,83	21	3737	1000	180	316	396	1050
<b>V</b>	Carico verticale massimo agente in presenza del sisma allo SLC								
<b>Fzd</b>	Carico verticale massimo in assenza di SISMA e spostamento 10mm								
<b>Ke</b>	Rigidezza orizzontale equivalente								
<b><math>\xi_e</math></b>	Coefficiente di smorzamento viscoso equivalente								
<b>Kv</b>	Rigidezza verticale								
<b>Dg</b>	Diametro elastomero								
<b>te</b>	Spessore totale gomma								
<b>h</b>	Altezza escluse piastre di ancoraggio								
<b>H</b>	Altezza totale incluse piastre di ancoraggio								
<b>Z</b>	Lato piastre di ancoraggio								

Il valore di  $F_z$  max = 3611 kN inferiore ai limiti previsti per l'isolatore introdotto ( $F_{zd} = 33560$  kN). Nel caso specifico, sono stati scelti isolatori elastomerici a nucleo di piombo e nell'ambito della rigidezza è stata effettuata una simulazione (che ha fornito sostanzialmente i medesimi risultati in termini di spostamento e reazioni agli appoggi) modificando la rigidezza dell'isolatore di +/-20% così come previsto dalle NTC2008 al punto 11.9.7.

## 8.3 VERIFICA GIUNTI

Per la determinazione dei varchi e quindi l'individuazione dei corretti giunti da utilizzare, sono stati considerati gli spostamenti sia dovuti alle azioni statiche che sismiche. Nel dettaglio si nota che lo spostamento massimo, dovuto all'azione sismica in condizione SLC è pari a  $d_{max} = 28.34$  cm. Si introduce quindi un giunto che permetta tale quantità di spostamento.

## 8.4 VERIFICA RITEGNI SISMICI.

Il presente cavalcavia presenta come appoggi 4 isolatori di appoggio che permettono uno spostamento massimo orizzontale pari a 300mm. Si è scelto di introdurre comunque dei ritegni sismici per bloccare lo spostamento trasversale dell'impalcato in conseguenza dei valori di spostamento generato dal sisma in condizione di salvaguardia della vita. In altre parole, si è scelto di limitare al massimo lo spostamento trasversale a valore riferito al sisma SLV e pertanto anche il ritegno stesso viene dimensionato considerando la forza orizzontale residua che non viene assorbita dall'isolatore stesso.

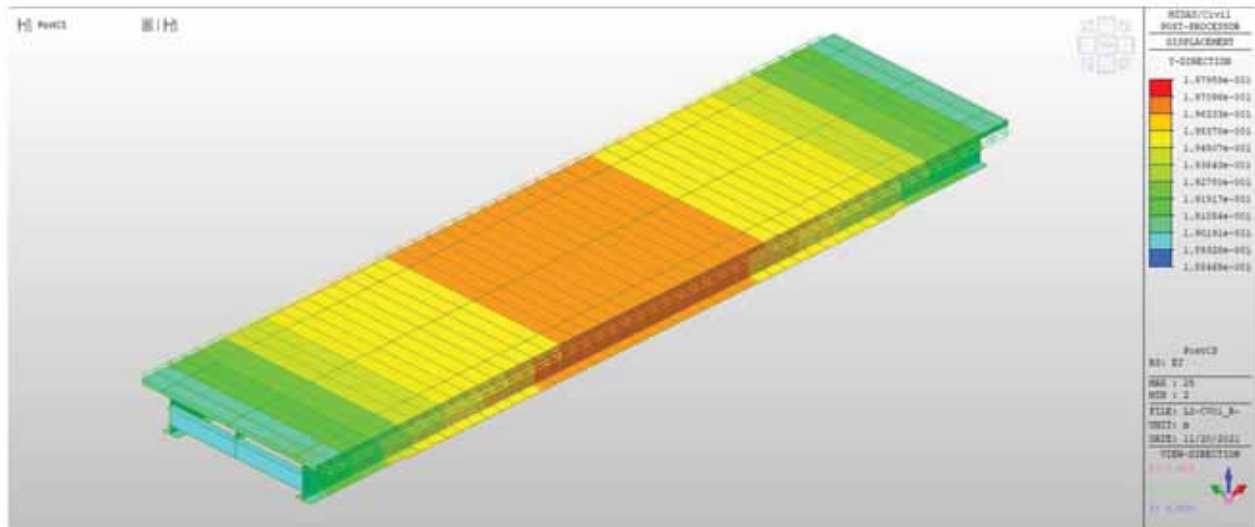
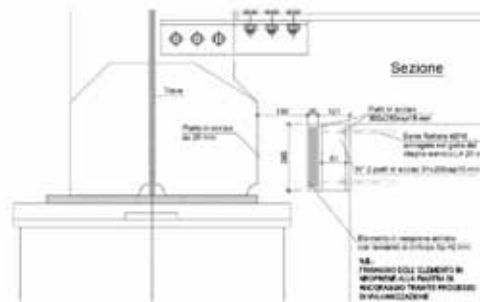


Figura 92 - Spostamento Y SLY (SISMA Y + 0.3 SISMA X)



Spostamento residuo =  $29.17 - 19.00 = 10.17\text{cm}$

$K_{\text{isolatore}} = 3.83 \text{ kN/mm}$

$F_{\text{residua}} = 389.51 \text{ kN}$

Con tale forza vengono dimensionate le armature dei ritegni sismici

LOTTO 1 – CAVALCAVIA AL KM. 12+325 - RELAZIONE DI CALCOLO IMPALCATO

DATI DI INPUT		
Forza orizzontale	$V_{sd}$ [kN] =	779.020
Forza verticale (+ se di trazione)	$N_{sd} / 4$ [kN] =	0
Altezza di applicazione forza orizzontale	$a$ [m] =	0.75
Base baggiolo	$h$ [m] =	1.00
Copriferro	$c$ [m] =	0.05
Altezza utile	$d$ [m] =	0.825
Profondità di incastro assunta	$h'$ [m] =	0.00
Luce di calcolo mensola	$L$ [m] =	0.75
Profondità baggiolo	$b$ [m] =	1.00
Angolo inclinazione traliccio [°]	$\alpha$ [°] =	45.29
Angolo inclinazione traliccio [rad]	$\alpha$ [rad] =	0.79
Angolo inclinazione traliccio [°]	$\psi$ [°] =	44.71
Angolo inclinazione traliccio [rad]	$\psi$ [rad] =	0.78
VERIFICA CORRENTE TESO		
Trazione sulle barre d'armatura corrente teso	$Z_{sd}$ [kN] =	786.89
Classe acciaio		B450C
Resistenza a snervamento armatura	$f_{yd}$ [MPa] =	391.3
Area armatura necessaria	$A_{nec}$ [cm <sup>2</sup> ] =	20.1
Diametro armatura 1	$\varnothing 1$ [mm] =	16
Diametro armatura 2	$\varnothing 2$ [mm] =	0
Numero ferri $\varnothing 1$	$n_{\varnothing 1}$ [-] =	10
Numero ferri $\varnothing 2$	$n_{\varnothing 2}$ [-] =	0
Area armatura 1 (singola barra)	$A_{s1}$ [cm <sup>2</sup> ] =	2.01
Area armatura 2 (singola barra)	$A_{s2}$ [cm <sup>2</sup> ] =	0.00
Area armatura totale	$A_{sTOT}$ [cm <sup>2</sup> ] =	20.1
Resistenza corrente teso	$Z_{Rd}$ [kN] =	786.76
Coefficiente di sicurezza	$Z_{Rd} / Z_{sd} = c.s.$ =	1.001
VERIFICA PUNTO CLS COMPRESSO		
Resistenza cubica caratteristica	$R_{ck}$ [MPa] =	40
Resistenza cilindrica caratteristica	$f_{ck}$ [MPa] =	32
Coefficiente parziale di sicurezza	$\gamma_c$ [-] =	1.50
Resistenza di calcolo a compressione	$f_{cd}$ [MPa] =	18.13
Presenza di staffe		Si
Coefficiente presenza staffe (1 senza staffe; 1.5 con staffe)	$c$ [-] =	1.50
Pendenza del puntone di calcestruzzo	$\cotg \psi = \tan \alpha = \lambda$ =	1.01
Altezza puntone	$0.4 * c * d * \sin \psi = h_p$ [m] =	0.348
Compressione nel puntone	$C_{sd}$ [kN] =	1107.28
Resistenza del puntone di calcestruzzo	$0.4 * b * d * f_{cd} * c / (1 + \lambda^2) = C_{Rd}$ [kN] =	4443
Coefficiente di sicurezza	$C_{Rd} / C_{sd} = c.s.$ =	4.01

### 8.5 VERIFICA DEFORMAZIONI

Si riportano di seguito le deformazioni dell'impalcato dovuto al peso proprio, soletta e permanente, nonché la freccia in condizioni di esercizio.

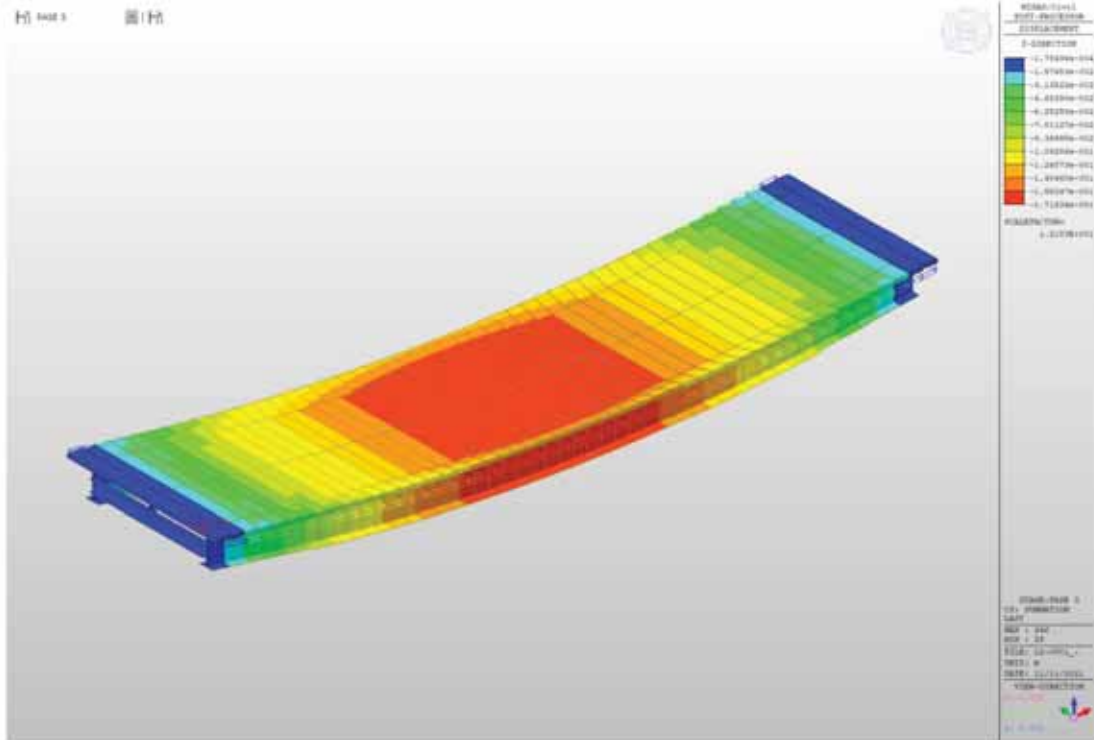


Figura 93 – Deformate - Fase 3

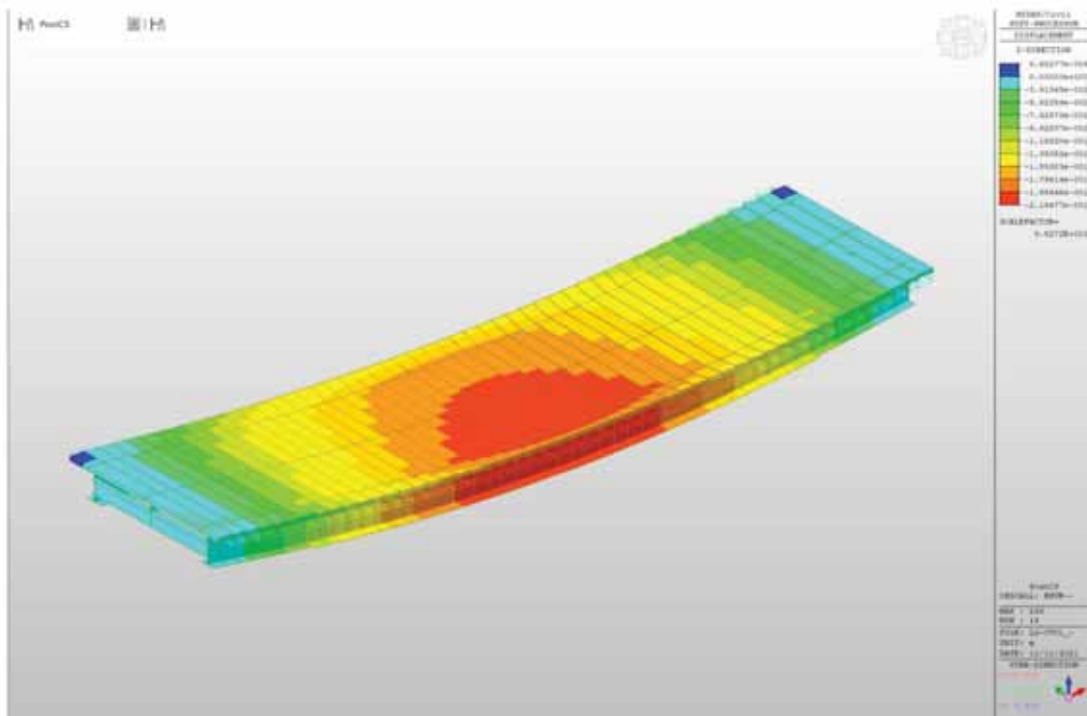


Figura 94 – Deformate - Inviluppo SLE

Per effetto del peso proprio dell'impalcato, della soletta e dei permanenti portati, si ottiene uno spostamento in mezzzeria pari a circa 17.16 cm (monta da applicare in fase di costruzione).

Considerando quindi tale valore come monta iniziale dai imporre all'impalcato, in condizione di esercizio si ottiene il seguente valore di freccia.

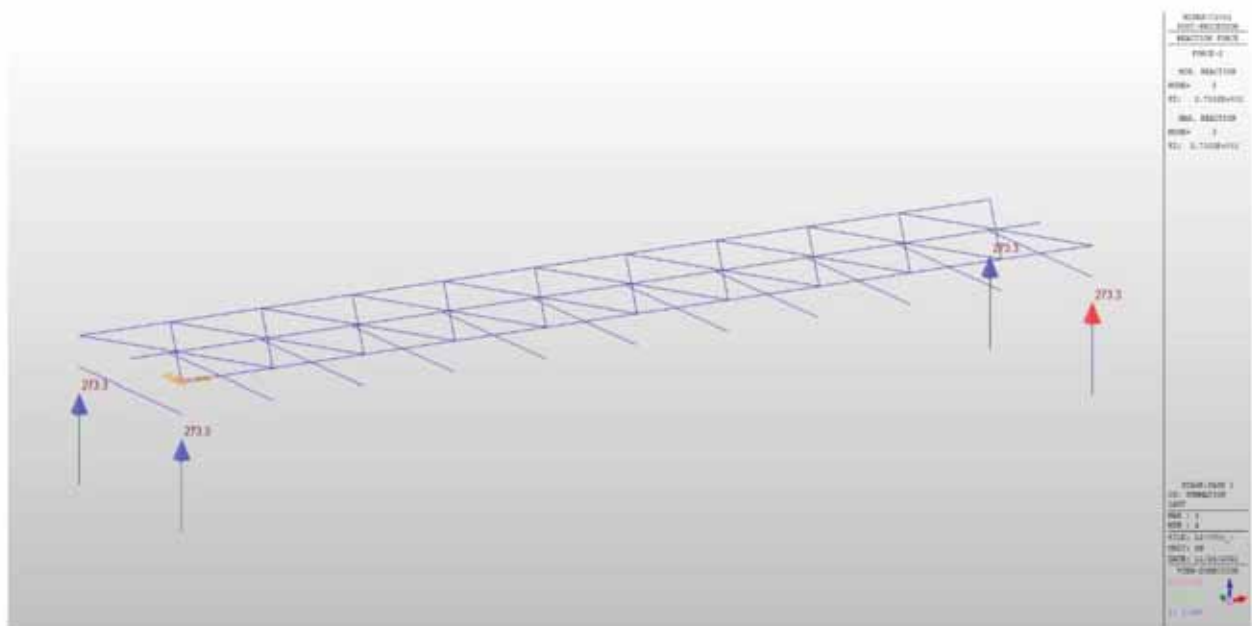
$$f = 21.95\text{cm} - 17.16\text{cm} = 4.79 \text{ cm}$$



## 9 VALIDAZIONE CODICE DI CALCOLO

Per effettuare la validazione del codice di calcolo, è stato determinato il peso della struttura ed è stato confrontato con gli scarichi derivanti dal modello dovuto ai soli pesi propri.

In base al punto 10.2 delle norme per le costruzioni NTC2008, qualora per la determinazione delle sollecitazioni di progetto si ricorra all'utilizzo di codici di calcolo, dovrà essere verificata l'attendibilità dei risultati tramite una validazione del modello di carico. Si riporta pertanto di seguito la validazione del modello andando a confrontare le reazioni vincolari per effetto del peso proprio della struttura ottenuti dal modello di calcolo e manualmente calcolando i singoli Pesì strutturali.



Calcolando i pesi in modo manuale e considerando l'incremento del peso proprio dovuto alla presenza delle piastre varie si ottiene:


	LUNGHEZZA CONCIO		PIATTABANDA SUP		PIATTABANDA INF		ANIMA			AREA	VOLME	PESO	
	mm	NUMERO	B mm	Sp mm	B mm	Sp mm	Htrave mm	B mm	Sp mm				
C1	6250	1	700	20	900	35	2000	1945	20	0.0844	0.5275	4140.875	
C2	9000	1	700	25	1200	40	2000	1935	20	0.1042	0.9378	7361.73	
C3	12000	1	700	30	1200	55	2000	1915	20	0.1253	1.5036	11803.26	34808.47
C4	9000	1	700	25	1200	40	2000	1935	20	0.1042	0.9378	7361.73	
C5	6250	1	700	20	900	35	2000	1945	20	0.0844	0.5275	4140.875	69616.94
TRAVE DI SPINA	42500	1	300	25	300	25	590	1935	13	0.1042	0.9378	7361.73	7361.73
TRAVERSI	7400	11	350	25	350	25	900	850	20	0.3795	2.8083	22045.16	22045.16
													Peso tot 99023.83
													Reazione 24755.96
													<b>247.5596</b> kN
													12% <b>277.2667</b> kN

La reazione vincolare ottenuta risulta essere compatibile con quella calcolata manualmente.

## 10 ALLEGATO 1 - TABULATO MIDAS

# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

```
-----  
; MIDAS/Civil Text (MCT) File.  
; Date : 2021/11/22  
-----
```

\*VERSION  
8.7.5

\*UNIT ; Unit System  
; FORCE, LENGTH, HEAT, TEMPER  
KN , M, BTU, C

\*PROJINFO ; Project Information  
USER=.  
ADDRESS=.

\*STRUCTYPE ; Structure Type  
; iSTYP, iMASS, iSMAS, bMASSOFFSET, bSELFWEIGHT, GRAV, TEMPER, bALIGNBEAM, bALIGNSLAB,  
bROTRIGID  
0, 1, 1, NO, YES, 9.806, 18, NO, NO, NO

\*REBAR-MATL-CODE ; Rebar Material Code  
; CONC\_CODE, CONC\_MDB, SRC\_CODE, SRC\_MDB  
NTC08(RC), B450C, ASTM(RC), Grade 60


\*NODE ; Nodes  
; iNO, X, Y, Z  
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23, 12.3, 4, -1.27  
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26, 24.6, 4, -1.27  
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28, 32.8, 4, -1.27  
29, 36.9, 4, -1.27  
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32, 4.1, 8, -1.27  
33, 8.2, 8, -1.27  
34, 12.3, 8, -1.27  
35, 16.4, 8, -1.27  
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PROJECT TITLE :

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	Author		File Name	L2-CV01_REVISIONE_2

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PROJECT TITLE :

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	Author		File Name	L2-CV01_REVISIONE_2

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PROJECT TITLE :

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	Author		File Name	L2-CV01_REVISIONE_2


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PROJECT TITLE :

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	Author		File Name	L2-CV01_REVISIONE_2

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 252, 4.92, 9.25, 0  
 253, 5.74, 9.25, 0  
 254, 6.56, 9.25, 0  
 255, 7.38, 9.25, 0  
 256, 8.2, 9.25, 0  
 257, 9.02, 9.25, 0  
 258, 9.84, 9.25, 0  
 259, 10.66, 9.25, 0  
 260, 11.48, 9.25, 0  
 261, 12.3, 9.25, 0  
 262, 13.12, 9.25, 0  
 263, 13.94, 9.25, 0  
 264, 14.76, 9.25, 0  
 265, 15.58, 9.25, 0  
 266, 16.4, 9.25, 0  
 267, 17.22, 9.25, 0  
 268, 18.04, 9.25, 0  
 269, 18.86, 9.25, 0  
 270, 19.68, 9.25, 0  
 271, 20.5, 9.25, 0  
 272, 21.32, 9.25, 0  
 273, 22.14, 9.25, 0  
 274, 22.96, 9.25, 0  
 275, 23.78, 9.25, 0  
 276, 24.6, 9.25, 0  
 277, 25.42, 9.25, 0  
 278, 26.24, 9.25, 0  
 279, 27.06, 9.25, 0  
 280, 27.88, 9.25, 0  
 281, 28.7, 9.25, 0  
 282, 29.52, 9.25, 0  
 283, 30.34, 9.25, 0  
 284, 31.16, 9.25, 0  
 285, 31.98, 9.25, 0  
 286, 32.8, 9.25, 0  
 287, 33.62, 9.25, 0

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288, 34.44, 9.25, 0  
 289, 35.26, 9.25, 0  
 290, 36.08, 9.25, 0  
 291, 36.9, 9.25, 0  
 292, 37.72, 9.25, 0  
 293, 38.54, 9.25, 0  
 294, 39.36, 9.25, 0  
 295, 40.18, 9.25, 0  
 296, 41, 9.25, 0

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*ELEMENT      ; Elements
; iEL, TYPE, iMAT, iPRO, iN1, iN2, ANGLE, iSUB, EXVAL, iOPT(EXVAL2) ; Frame Element
; iEL, TYPE, iMAT, iPRO, iN1, iN2, ANGLE, iSUB, EXVAL, EXVAL2, bLMT ; Comp/Tens Truss
; iEL, TYPE, iMAT, iPRO, iN1, iN2, iN3, iN4, iSUB, iWID , LCAXIS      ; Planar Element
; iEL, TYPE, iMAT, iPRO, iN1, iN2, iN3, iN4, iN5, iN6, iN7, iN8      ; Solid Element

  1, BEAM , 1, 4, 20, 9, 0, 0
  2, BEAM , 1, 4, 21, 10, 0, 0
  3, BEAM , 1, 4, 22, 11, 0, 0
  4, BEAM , 1, 4, 23, 12, 0, 0
  5, BEAM , 1, 4, 24, 13, 0, 0
  6, BEAM , 1, 4, 25, 14, 0, 0
  7, BEAM , 1, 4, 26, 15, 0, 0
  8, BEAM , 1, 4, 27, 16, 0, 0
  9, BEAM , 1, 4, 28, 17, 0, 0
 10, BEAM , 1, 4, 29, 18, 0, 0
 11, BEAM , 1, 4, 30, 19, 0, 0
 12, BEAM , 1, 4, 31, 20, 0, 0
 13, BEAM , 1, 4, 32, 21, 0, 0
 14, BEAM , 1, 4, 33, 22, 0, 0
 15, BEAM , 1, 4, 34, 23, 0, 0
 16, BEAM , 1, 4, 35, 24, 0, 0
 17, BEAM , 1, 4, 36, 25, 0, 0
 18, BEAM , 1, 4, 37, 26, 0, 0
 19, BEAM , 1, 4, 38, 27, 0, 0
 20, BEAM , 1, 4, 39, 28, 0, 0
 21, BEAM , 1, 4, 40, 29, 0, 0
 22, BEAM , 1, 4, 41, 30, 0, 0
 23, BEAM , 3, 6, 42, 93, 0, 0
 24, BEAM , 3, 6, 43, 94, 0, 0
 25, BEAM , 3, 6, 44, 95, 0, 0
 26, BEAM , 3, 6, 45, 96, 0, 0
 27, BEAM , 3, 6, 46, 97, 0, 0
 28, BEAM , 3, 6, 47, 98, 0, 0
 29, BEAM , 3, 6, 48, 99, 0, 0
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 31, BEAM , 3, 6, 50, 101, 0, 0
 32, BEAM , 3, 6, 51, 102, 0, 0
 33, BEAM , 3, 6, 52, 103, 0, 0
 34, BEAM , 3, 6, 53, 104, 0, 0
 35, BEAM , 3, 6, 54, 105, 0, 0
 36, BEAM , 3, 6, 55, 106, 0, 0
 37, BEAM , 3, 6, 56, 107, 0, 0
 38, BEAM , 3, 6, 57, 108, 0, 0
 39, BEAM , 3, 6, 58, 109, 0, 0
 40, BEAM , 3, 6, 59, 110, 0, 0
 41, BEAM , 3, 6, 60, 111, 0, 0
 42, BEAM , 3, 6, 61, 112, 0, 0
 43, BEAM , 3, 6, 62, 113, 0, 0
 44, BEAM , 3, 6, 63, 114, 0, 0
 45, BEAM , 3, 6, 64, 115, 0, 0
 46, BEAM , 3, 6, 65, 116, 0, 0
 47, BEAM , 3, 6, 66, 117, 0, 0
    
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48, BEAM , 3, 6, 67, 118, 0, 0
49, BEAM , 3, 6, 68, 119, 0, 0
50, BEAM , 3, 6, 69, 120, 0, 0
51, BEAM , 3, 6, 70, 121, 0, 0
52, BEAM , 3, 6, 71, 122, 0, 0
53, BEAM , 3, 6, 72, 123, 0, 0
54, BEAM , 3, 6, 73, 124, 0, 0
55, BEAM , 3, 6, 74, 125, 0, 0
56, BEAM , 3, 6, 75, 126, 0, 0
57, BEAM , 3, 6, 76, 127, 0, 0
58, BEAM , 3, 6, 77, 128, 0, 0
59, BEAM , 3, 6, 78, 129, 0, 0
60, BEAM , 3, 6, 79, 130, 0, 0
61, BEAM , 3, 6, 80, 131, 0, 0
62, BEAM , 3, 6, 81, 132, 0, 0
63, BEAM , 3, 6, 82, 133, 0, 0
64, BEAM , 3, 6, 83, 134, 0, 0
65, BEAM , 3, 6, 84, 135, 0, 0
66, BEAM , 3, 6, 85, 136, 0, 0
67, BEAM , 3, 6, 86, 137, 0, 0
68, BEAM , 3, 6, 87, 138, 0, 0
69, BEAM , 3, 6, 88, 139, 0, 0
70, BEAM , 3, 6, 89, 140, 0, 0
71, BEAM , 3, 6, 90, 141, 0, 0
72, BEAM , 3, 6, 91, 142, 0, 0
73, BEAM , 3, 6, 92, 143, 0, 0
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75, BEAM , 1, 7, 94, 95, 0, 0
76, BEAM , 1, 7, 95, 96, 0, 0
77, BEAM , 1, 7, 96, 97, 0, 0
78, BEAM , 1, 7, 97, 98, 0, 0
79, BEAM , 1, 7, 98, 99, 0, 0
80, BEAM , 1, 7, 99, 100, 0, 0
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82, BEAM , 1, 8, 101, 102, 0, 0
83, BEAM , 1, 8, 102, 103, 0, 0
84, BEAM , 1, 8, 103, 104, 0, 0
85, BEAM , 1, 8, 104, 105, 0, 0
86, BEAM , 1, 8, 105, 106, 0, 0
87, BEAM , 1, 8, 106, 107, 0, 0
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89, BEAM , 1, 8, 108, 109, 0, 0
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93, BEAM , 1, 9, 112, 113, 0, 0
94, BEAM , 1, 9, 113, 114, 0, 0
95, BEAM , 1, 9, 114, 115, 0, 0
96, BEAM , 1, 9, 115, 116, 0, 0
97, BEAM , 1, 9, 116, 117, 0, 0
98, BEAM , 1, 9, 117, 118, 0, 0
99, BEAM , 1, 9, 118, 119, 0, 0
100, BEAM , 1, 9, 119, 120, 0, 0
101, BEAM , 1, 9, 120, 121, 0, 0
102, BEAM , 1, 9, 121, 122, 0, 0
103, BEAM , 1, 9, 122, 123, 0, 0
104, BEAM , 1, 9, 123, 124, 0, 0
105, BEAM , 1, 9, 124, 125, 0, 0
106, BEAM , 1, 8, 125, 126, 0, 0
107, BEAM , 1, 8, 126, 127, 0, 0
108, BEAM , 1, 8, 127, 128, 0, 0
109, BEAM , 1, 8, 128, 129, 0, 0

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110, BEAM , 1, 8, 129, 130, 0, 0
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112, BEAM , 1, 8, 131, 132, 0, 0
113, BEAM , 1, 8, 132, 133, 0, 0
114, BEAM , 1, 8, 133, 134, 0, 0
115, BEAM , 1, 8, 134, 135, 0, 0
116, BEAM , 1, 8, 135, 136, 0, 0
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118, BEAM , 1, 7, 137, 138, 0, 0
119, BEAM , 1, 7, 138, 139, 0, 0
120, BEAM , 1, 7, 139, 140, 0, 0
121, BEAM , 1, 7, 140, 141, 0, 0
122, BEAM , 1, 7, 141, 142, 0, 0
123, BEAM , 1, 7, 142, 143, 0, 0
124, BEAM , 3, 6, 93, 144, 0, 0
125, BEAM , 3, 6, 94, 145, 0, 0
126, BEAM , 3, 6, 95, 146, 0, 0
127, BEAM , 3, 6, 96, 147, 0, 0
128, BEAM , 3, 6, 97, 148, 0, 0
129, BEAM , 3, 6, 98, 149, 0, 0
130, BEAM , 3, 6, 99, 150, 0, 0
131, BEAM , 3, 6, 100, 151, 0, 0
132, BEAM , 3, 6, 101, 152, 0, 0
133, BEAM , 3, 6, 102, 153, 0, 0
134, BEAM , 3, 6, 103, 154, 0, 0
135, BEAM , 3, 6, 104, 155, 0, 0
136, BEAM , 3, 6, 105, 156, 0, 0
137, BEAM , 3, 6, 106, 157, 0, 0
138, BEAM , 3, 6, 107, 158, 0, 0
139, BEAM , 3, 6, 108, 159, 0, 0
140, BEAM , 3, 6, 109, 160, 0, 0
141, BEAM , 3, 6, 110, 161, 0, 0
142, BEAM , 3, 6, 111, 162, 0, 0
143, BEAM , 3, 6, 112, 163, 0, 0
144, BEAM , 3, 6, 113, 164, 0, 0
145, BEAM , 3, 6, 114, 165, 0, 0
146, BEAM , 3, 6, 115, 166, 0, 0
147, BEAM , 3, 6, 116, 167, 0, 0
148, BEAM , 3, 6, 117, 168, 0, 0
149, BEAM , 3, 6, 118, 169, 0, 0
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151, BEAM , 3, 6, 120, 171, 0, 0
152, BEAM , 3, 6, 121, 172, 0, 0
153, BEAM , 3, 6, 122, 173, 0, 0
154, BEAM , 3, 6, 123, 174, 0, 0
155, BEAM , 3, 6, 124, 175, 0, 0
156, BEAM , 3, 6, 125, 176, 0, 0
157, BEAM , 3, 6, 126, 177, 0, 0
158, BEAM , 3, 6, 127, 178, 0, 0
159, BEAM , 3, 6, 128, 179, 0, 0
160, BEAM , 3, 6, 129, 180, 0, 0
161, BEAM , 3, 6, 130, 181, 0, 0
162, BEAM , 3, 6, 131, 182, 0, 0
163, BEAM , 3, 6, 132, 183, 0, 0
164, BEAM , 3, 6, 133, 184, 0, 0
165, BEAM , 3, 6, 134, 185, 0, 0
166, BEAM , 3, 6, 135, 186, 0, 0
167, BEAM , 3, 6, 136, 187, 0, 0
168, BEAM , 3, 6, 137, 188, 0, 0
169, BEAM , 3, 6, 138, 189, 0, 0
170, BEAM , 3, 6, 139, 190, 0, 0
171, BEAM , 3, 6, 140, 191, 0, 0

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172, BEAM , 3, 6, 141, 192, 0, 0
173, BEAM , 3, 6, 142, 193, 0, 0
174, BEAM , 3, 6, 143, 194, 0, 0
175, BEAM , 1, 3, 144, 145, 0, 0
176, BEAM , 1, 3, 145, 146, 0, 0
177, BEAM , 1, 3, 146, 147, 0, 0
178, TRUSS , 1, 5, 195, 98, 0, 0
179, TRUSS , 1, 5, 200, 93, 0, 0
180, BEAM , 1, 3, 147, 148, 0, 0
181, BEAM , 1, 3, 148, 149, 0, 0
182, BEAM , 1, 3, 149, 150, 0, 0
183, BEAM , 1, 3, 150, 151, 0, 0
184, BEAM , 1, 3, 151, 152, 0, 0
185, TRUSS , 1, 5, 205, 98, 0, 0
186, TRUSS , 1, 5, 200, 103, 0, 0
187, BEAM , 1, 3, 152, 153, 0, 0
188, BEAM , 1, 3, 153, 154, 0, 0
189, BEAM , 1, 3, 154, 155, 0, 0
190, BEAM , 1, 3, 155, 156, 0, 0
191, BEAM , 1, 3, 156, 157, 0, 0
192, TRUSS , 1, 5, 210, 103, 0, 0
193, TRUSS , 1, 5, 205, 108, 0, 0
194, BEAM , 1, 3, 157, 158, 0, 0
195, BEAM , 1, 3, 158, 159, 0, 0
196, BEAM , 1, 3, 159, 160, 0, 0
197, BEAM , 1, 3, 160, 161, 0, 0
198, BEAM , 1, 3, 161, 162, 0, 0
199, TRUSS , 1, 5, 215, 108, 0, 0
200, TRUSS , 1, 5, 210, 113, 0, 0
201, BEAM , 1, 3, 162, 163, 0, 0
202, BEAM , 1, 3, 163, 164, 0, 0
203, BEAM , 1, 3, 164, 165, 0, 0
204, BEAM , 1, 3, 165, 166, 0, 0
205, BEAM , 1, 3, 166, 167, 0, 0
206, TRUSS , 1, 5, 215, 118, 0, 0
207, TRUSS , 1, 5, 220, 113, 0, 0
208, BEAM , 1, 3, 167, 168, 0, 0
209, BEAM , 1, 3, 168, 169, 0, 0
210, BEAM , 1, 3, 169, 170, 0, 0
211, BEAM , 1, 3, 170, 171, 0, 0
212, BEAM , 1, 3, 171, 172, 0, 0
213, TRUSS , 1, 5, 220, 123, 0, 0
214, TRUSS , 1, 5, 225, 118, 0, 0
215, BEAM , 1, 3, 172, 173, 0, 0
216, BEAM , 1, 3, 173, 174, 0, 0
217, BEAM , 1, 3, 174, 175, 0, 0
218, BEAM , 1, 3, 175, 176, 0, 0
219, BEAM , 1, 3, 176, 177, 0, 0
220, TRUSS , 1, 5, 230, 123, 0, 0
221, TRUSS , 1, 5, 225, 128, 0, 0
222, BEAM , 1, 3, 177, 178, 0, 0
223, BEAM , 1, 3, 178, 179, 0, 0
224, BEAM , 1, 3, 179, 180, 0, 0
225, BEAM , 1, 3, 180, 181, 0, 0
226, TRUSS , 1, 5, 230, 133, 0, 0
227, TRUSS , 1, 5, 235, 128, 0, 0
228, BEAM , 1, 3, 181, 182, 0, 0
229, BEAM , 1, 3, 182, 183, 0, 0
230, BEAM , 1, 3, 183, 184, 0, 0
231, BEAM , 1, 3, 184, 185, 0, 0
232, BEAM , 1, 3, 185, 186, 0, 0
233, BEAM , 1, 3, 186, 187, 0, 0


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234,	TRUSS	,	1,	5,	235,	138,	0,	0
235,	TRUSS	,	1,	5,	240,	133,	0,	0
236,	BEAM	,	1,	3,	187,	188,	0,	0
237,	BEAM	,	1,	3,	188,	189,	0,	0
238,	BEAM	,	1,	3,	189,	190,	0,	0
239,	BEAM	,	1,	3,	190,	191,	0,	0
240,	TRUSS	,	1,	5,	240,	143,	0,	0
241,	BEAM	,	1,	3,	191,	192,	0,	0
242,	TRUSS	,	1,	5,	245,	138,	0,	0
243,	BEAM	,	1,	3,	192,	193,	0,	0
244,	BEAM	,	1,	3,	193,	194,	0,	0
245,	BEAM	,	3,	6,	144,	195,	0,	0
246,	BEAM	,	3,	6,	145,	196,	0,	0
247,	BEAM	,	3,	6,	146,	197,	0,	0
248,	BEAM	,	3,	6,	147,	198,	0,	0
249,	BEAM	,	3,	6,	148,	199,	0,	0
250,	BEAM	,	3,	6,	149,	200,	0,	0
251,	BEAM	,	3,	6,	150,	201,	0,	0
252,	BEAM	,	3,	6,	151,	202,	0,	0
253,	BEAM	,	3,	6,	152,	203,	0,	0
254,	BEAM	,	3,	6,	153,	204,	0,	0
255,	BEAM	,	3,	6,	154,	205,	0,	0
256,	BEAM	,	3,	6,	155,	206,	0,	0
257,	BEAM	,	3,	6,	156,	207,	0,	0
258,	BEAM	,	3,	6,	157,	208,	0,	0
259,	BEAM	,	3,	6,	158,	209,	0,	0
260,	BEAM	,	3,	6,	159,	210,	0,	0
261,	BEAM	,	3,	6,	160,	211,	0,	0
262,	BEAM	,	3,	6,	161,	212,	0,	0
263,	BEAM	,	3,	6,	162,	213,	0,	0
264,	BEAM	,	3,	6,	163,	214,	0,	0
265,	BEAM	,	3,	6,	164,	215,	0,	0
266,	BEAM	,	3,	6,	165,	216,	0,	0
267,	BEAM	,	3,	6,	166,	217,	0,	0
268,	BEAM	,	3,	6,	167,	218,	0,	0
269,	BEAM	,	3,	6,	168,	219,	0,	0
270,	BEAM	,	3,	6,	169,	220,	0,	0
271,	BEAM	,	3,	6,	170,	221,	0,	0
272,	BEAM	,	3,	6,	171,	222,	0,	0
273,	BEAM	,	3,	6,	172,	223,	0,	0
274,	BEAM	,	3,	6,	173,	224,	0,	0
275,	BEAM	,	3,	6,	174,	225,	0,	0
276,	BEAM	,	3,	6,	175,	226,	0,	0
277,	BEAM	,	3,	6,	176,	227,	0,	0
278,	BEAM	,	3,	6,	177,	228,	0,	0
279,	BEAM	,	3,	6,	178,	229,	0,	0
280,	BEAM	,	3,	6,	179,	230,	0,	0
281,	BEAM	,	3,	6,	180,	231,	0,	0
282,	BEAM	,	3,	6,	181,	232,	0,	0
283,	BEAM	,	3,	6,	182,	233,	0,	0
284,	BEAM	,	3,	6,	183,	234,	0,	0
285,	BEAM	,	3,	6,	184,	235,	0,	0
286,	BEAM	,	3,	6,	185,	236,	0,	0
287,	BEAM	,	3,	6,	186,	237,	0,	0
288,	BEAM	,	3,	6,	187,	238,	0,	0
289,	BEAM	,	3,	6,	188,	239,	0,	0
290,	BEAM	,	3,	6,	189,	240,	0,	0
291,	BEAM	,	3,	6,	190,	241,	0,	0
292,	BEAM	,	3,	6,	191,	242,	0,	0
293,	BEAM	,	3,	6,	192,	243,	0,	0
294,	BEAM	,	3,	6,	193,	244,	0,	0
295,	BEAM	,	3,	6,	194,	245,	0,	0

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296, BEAM , 1, 7, 195, 196, 0, 0
297, BEAM , 1, 7, 196, 197, 0, 0
298, BEAM , 1, 7, 197, 198, 0, 0
299, BEAM , 1, 7, 198, 199, 0, 0
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306, BEAM , 1, 8, 205, 206, 0, 0
307, BEAM , 1, 8, 206, 207, 0, 0
308, BEAM , 1, 8, 207, 208, 0, 0
309, BEAM , 1, 8, 208, 209, 0, 0
310, BEAM , 1, 8, 209, 210, 0, 0
311, BEAM , 1, 8, 210, 211, 0, 0
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313, BEAM , 1, 8, 212, 213, 0, 0
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315, BEAM , 1, 9, 214, 215, 0, 0
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320, BEAM , 1, 9, 219, 220, 0, 0
321, BEAM , 1, 9, 220, 221, 0, 0
322, BEAM , 1, 9, 221, 222, 0, 0
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329, BEAM , 1, 8, 228, 229, 0, 0
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341, BEAM , 1, 7, 240, 241, 0, 0
342, BEAM , 1, 7, 241, 242, 0, 0
343, BEAM , 1, 7, 242, 243, 0, 0
344, BEAM , 1, 7, 243, 244, 0, 0
345, BEAM , 1, 7, 244, 245, 0, 0
346, BEAM , 3, 6, 195, 246, 0, 0
347, BEAM , 3, 6, 196, 247, 0, 0
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350, BEAM , 3, 6, 199, 250, 0, 0
351, BEAM , 3, 6, 200, 251, 0, 0
352, BEAM , 3, 6, 201, 252, 0, 0
353, BEAM , 3, 6, 202, 253, 0, 0
354, BEAM , 3, 6, 203, 254, 0, 0
355, BEAM , 3, 6, 204, 255, 0, 0
356, BEAM , 3, 6, 205, 256, 0, 0
357, BEAM , 3, 6, 206, 257, 0, 0

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358, BEAM , 3, 6, 207, 258, 0, 0
359, BEAM , 3, 6, 208, 259, 0, 0
360, BEAM , 3, 6, 209, 260, 0, 0
361, BEAM , 3, 6, 210, 261, 0, 0
362, BEAM , 3, 6, 211, 262, 0, 0
363, BEAM , 3, 6, 212, 263, 0, 0
364, BEAM , 3, 6, 213, 264, 0, 0
365, BEAM , 3, 6, 214, 265, 0, 0
366, BEAM , 3, 6, 215, 266, 0, 0
367, BEAM , 3, 6, 216, 267, 0, 0
368, BEAM , 3, 6, 217, 268, 0, 0
369, BEAM , 3, 6, 218, 269, 0, 0
370, BEAM , 3, 6, 219, 270, 0, 0
371, BEAM , 3, 6, 220, 271, 0, 0
372, BEAM , 3, 6, 221, 272, 0, 0
373, BEAM , 3, 6, 222, 273, 0, 0
374, BEAM , 3, 6, 223, 274, 0, 0
375, BEAM , 3, 6, 224, 275, 0, 0
376, BEAM , 3, 6, 225, 276, 0, 0
377, BEAM , 3, 6, 226, 277, 0, 0
378, BEAM , 3, 6, 227, 278, 0, 0
379, BEAM , 3, 6, 228, 279, 0, 0
380, BEAM , 3, 6, 229, 280, 0, 0
381, BEAM , 3, 6, 230, 281, 0, 0
382, BEAM , 3, 6, 231, 282, 0, 0
383, BEAM , 3, 6, 232, 283, 0, 0
384, BEAM , 3, 6, 233, 284, 0, 0
385, BEAM , 3, 6, 234, 285, 0, 0
386, BEAM , 3, 6, 235, 286, 0, 0
387, BEAM , 3, 6, 236, 287, 0, 0
388, BEAM , 3, 6, 237, 288, 0, 0
389, BEAM , 3, 6, 238, 289, 0, 0
390, BEAM , 3, 6, 239, 290, 0, 0
391, BEAM , 3, 6, 240, 291, 0, 0
392, BEAM , 3, 6, 241, 292, 0, 0
393, BEAM , 3, 6, 242, 293, 0, 0
394, BEAM , 3, 6, 243, 294, 0, 0
395, BEAM , 3, 6, 244, 295, 0, 0
396, BEAM , 3, 6, 245, 296, 0, 0

```

```

*GROUP ; Group
; NAME, NODE LIST, ELEM LIST, PLANE TYPE
FASE 1 , 1to4 9to41 93to245, 1to22 74to123 175to244 296to345, 0
FASE 2 , 42to296, 23to73 124to174 245to295 346to396, 0

```

```

*BNDR-GROUP ; Boundary Group
; NAME, AUTOTYPE
FASE 1, 0

```

```

*LOAD-GROUP ; Load Group
; NAME
FASE 1
FASE 2
FASE 3

```

```

*MATERIAL ; Material
; iMAT, TYPE, MNAME, SPHEAT, HEATCO, PLAST, TUNIT, bMASS, DAMPRATIO, [DATA1]
; STEEL, CONC, USER
; iMAT, TYPE, MNAME, SPHEAT, HEATCO, PLAST, TUNIT, bMASS, DAMPRATIO, [DATA2], [DATA2]
; SRC
; [DATA1] : 1, DB, NAME, CODE, USEELAST, ELAST
; [DATA1] : 2, ELAST, POISN, THERMAL, DEN, MASS

```

# MIDAS

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```
; [DATA1] : 3, Ex, Ey, Ez, Tx, Ty, Tz, Sxy, Sxz, Syz, Pxy, Pxz, Pyz, DEN, MASS
; Orthotropic
; [DATA2] : 1, DB, NAME, CODE, USEELAST, ELAST or 2, ELAST, POISN, THERMAL, DEN, MASS
1, STEEL, S355, 0, 0, C, NO, 0.02, 1, EN05-PS(S), S3
55, NO, 2.1e+008
2, CONC, C32/40, 0, 0, C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e
-005, 0, 0
3, CONC, C32/40_NOP, 0, 0, C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e
-005, 0, 0
```

\*MATL-COLOR

```
; iMAT, W_R, W_G, W_B, HF_R, HF_G, HF_B, HE_R, HE_G, HE_B, bBLEND, FACT
1, 115, 0, 0, 255, 87, 87, 115, 0, 0, NO, 0.5
2, 0, 66, 45, 0, 192, 128, 0, 66, 45, NO, 0.5
3, 38, 57, 0, 128, 192, 0, 38, 57, 0, NO, 0.5
```


\*TDM-TYPE ; Time Dependent Material

```
; NAME=NAME, CODE, STR, HU, VOL, AGE, CFACTA, CFACTB, TYPE, [ACI1 or ACI2] ; CODE=
ACI
; NAME=NAME, CODE, STR, HU, MSIZE, CTYPE, AGE ; CODE=
CEB1990,KS,CEB1978,KSCE 2010,KCI-USD12
; NAME=NAME, CODE, N1, PHI1, N2, PHI2 ; CODE=
MEM
; NAME=NAME, CODE, STR, HU, USS, UCS, VOL, RR, MOD ; CODE=
PCA
; NAME=NAME, CODE, STR, HU, VOL, UCS, VSR1, LAF, US, VSR, PST, bRCE, RR, MOD ; CODE=
COMBINED
; NAME=NAME, CODE, STR, HTYPE, HU, MSIZE, CTYPE, AGE, CM ; CODE=
JAPAN
; NAME=NAME, CODE, ELAST, HU, VOL, CC, WC, AGE ; CODE=
JSCE
; NAME=NAME, CODE, STR, HTYPE, HU, MSIZE, AGE ; CODE=
CHINA
; NAME=NAME, CODE, STR, HU, MSIZE, BSC, AGE ; CODE=
JTG
; NAME=NAME, CODE, STR, HU, VSR, AGE, bEXPOSE ; CODE=
AASHTO
; NAME=NAME, CODE, STR, HU, MSIZE, AGE ; CODE=
INDIA(IRC:18-2000)
; NAME=NAME, CODE, STR, HU, MSIZE, CTYPE, AGE ; CODE=
INDIA(IRC:112-2011)
; NAME=NAME, CODE, STR, HU, MSIZE, CTYPE, AGE, TCode, bSILICA ; CODE=
European
; NAME=NAME, CODE, STR, EE(Not Use), FS, HT, DSE, DSC, AGE ; CODE=
NZ Bridge(SP/M/022)
; NAME=NAME, CODE, STR, HU, AGE, M, CMETH, CTYPE, CREEP, CONCT, W, MAXS, A, PZ ; CODE=
Russian
; NAME=NAME, CODE, STR, HU, MSIZE, BSC, AGE, FLYASH ; CODE=
China(JTG D62-2016)
; NAME=NAME, CODE, STR, EE, HT, DSC, AGE ; CODE=
Australia
; NAME=NAME, CODE, bSSF, SSFNAME ; CODE=
USER(line1)
; CREEPFUNC1, AGE1, CREEPFUNC2, AGE2, ... ; USER(
from line 2)
; [ACI1] : CURE, SLUMP, FAP, AIR, CC
; [ACI2] : UCC, USS
NAME=C+S, European, 35000, 70, 0.25, Class N, 3, 1, NO
```

```
*TDM-ELAST ; Time Dependent Material(Comp. Strength)
; NAME=NAME, TYPE, CODE, STRENGTH, A, B ; TYPE=CODE(Korean St
andard, ACI)
```

# MIDAS

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```
; NAME=NAME, TYPE, CODE, STRENGTH, iCTYPE ; TYPE=CODE (CEB-FIP,
Ohzagi)
; NAME=NAME, TYPE, CODE, STRENGTH ; TYPE=CODE (Chinese S
tandard)
; NAME=NAME, TYPE, CODE, STRENGTH, bUSE, [DATA] ; TYPE=CODE (Japan (Hyd
ration))
; NAME=NAME, TYPE, CODE, STRENGTH, iTYPE ; TYPE=CODE (Japan (Ela
stic))
; NAME=NAME, TYPE, CODE, STRENGTH ; TYPE=CODE (INDIA (IRC
:18-2000))
; NAME=NAME, TYPE, CODE, STRENGTH, iCTYPE ; TYPE=CODE (INDIA (IRC
:112-2011))
; NAME=NAME, TYPE, CODE, STRENGTH, iCTYPE ; TYPE=CODE (European)

; NAME=NAME, TYPE, CODE, STRENGTH, ; TYPE=CODE (CEB-FIP (1
978))
; NAME=NAME, TYPE, CODE, STRENGTH, TYPE, CMETH, CTYPE, MAXS, PZ ; TYPE=CODE (Russian))

; NAME=NAME, TYPE, CODE, STRENGTH, ; CODE=Australia
; NAME=NAME, TYPE, SCALE ; TYPE=USER (line 1)
; DAY1, COMPL, TENS1, ELAST1, DAY2, COMP2, ... ; USER (from line
2)
; [DATA] : A, B, D, TSF or iCTYPE, TSF
NAME=MATURAZIONE, CODE, EUROPEAN, 43000, 2
```

```
*TDM-LINK ; Time Dependent Material Link
; iMAT, TDM-TYPE1 (CREEP/SHRINKAGE), TDM-TYPE2 (ELASTICITY)
2, C+S, MATURAZIONE
```

```
*SECTION ; Section
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, [DATA1], [DATA2] ;
1st line - DB/USER
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, BLT, D1, ..., D8, iCEL ;
1st line - VALUE
; AREA, ASy, ASz, Ixx, Iyy, Izz ;
2nd line
; CyP, CyM, CzP, CzM, QyB, QzB, PERI_OUT, PERI_IN, Cy, Cz ;
3rd line
; Y1, Y2, Y3, Y4, Z1, Z2, Z3, Z4, Zyy, Zzz ;
4th line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, ELAST, DEN, POIS, POIC, SF, THERMAL ;
1st line - SRC
; D1, D2, [SRC] ;
2nd line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 1, DB, NAME1, NAME2, D1, D2 ;
1st line - COMBINED
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 2, D11, D12, D13, D14, D15, D21, D22,
D23, D24
; iSEC, TYPE, SNAME, [OFFSET2], bSD, bWE, SHAPE, iyVAR, izVAR, STYPE ;
1st line - TAPERED
; DB, NAME1, NAME2 ;
2nd line (STYPE=DB)
; [DIM1], [DIM2] ;
2nd line (STYPE=USER)
; D11, D12, D13, D14, D15, D16, D17, D18 ;
2nd line (STYPE=VALUE)
; AREA1, ASy1, ASz1, Ixx1, Iyy1, Izz1 ;
3rd line (STYPE=VALUE)
; CyP1, CyM1, CzP1, CzM1, QyB1, QzB1, PERI_OUT1, PERI_IN1, Cy1, Cz1 ;
4th line (STYPE=VALUE)
; Y11, Y12, Y13, Y14, Z11, Z12, Z13, Z14, Zyy1, Zyy2 ;
```



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5th line(STYPE=VALUE)
;   D21, D22, D23, D24, D25, D26, D27, D28                               ;
6th line(STYPE=VALUE)
;   AREA2, ASy2, ASz2, Ixx2, Iyy2, Izz2                                 ;
7th line(STYPE=VALUE)
;   CyP2, CyM2, CzP2, CzM2, QyB2, QzB2, PERI_OUT2, PERI_IN2, Cy2, Cz2   ;
8th line(STYPE=VALUE)
;   Y21, Y22, Y23, Y24, Z21, Z22, Z23, Z24, Zyy2, Zzz2                 ;
9th line(STYPE=VALUE)
;   OPT1, OPT2, [JOINT]                                                ; 2nd l
ine(STYPE=PSC)
;   ELAST, DEN, POIS, POIC, THERMAL                                     ; 2nd l
ine(STYPE=PSC-CMPW)
;   bSHEARCHK, [SCHK-I], [SCHK-J], [WT-I], [WT-J], WI, WJ, bSYM, bSIDEHOLE ; 3rd l
ine(STYPE=PSC)
;   bSHEARCHK, bSYM, bHUNCH, [CMPWEB-I], [CMPWEB-J]                   ; 3rd l
ine(STYPE=PSC-CMPW)
;   bUSERDEFMESH SIZE, MESH SIZE, bUSERINPSTIFF, [STIFF-I], [STIFF-J]   ; 4th l
ine(STYPE=PSC)
;   [SIZE-A]-i                                                         ; 5th l
ine(STYPE=PSC)
;   [SIZE-B]-i                                                         ; 6th l
ine(STYPE=PSC)
;   [SIZE-C]-i                                                         ; 7th l
ine(STYPE=PSC)
;   [SIZE-D]-i                                                         ; 8th l
ine(STYPE=PSC)
;   [SIZE-A]-j                                                         ; 9th l
ine(STYPE=PSC)
;   [SIZE-B]-j                                                         ; 10th
line(STYPE=PSC)
;   [SIZE-C]-j                                                         ; 11th
line(STYPE=PSC)
;   [SIZE-D]-j                                                         ; 12th
line(STYPE=PSC)
;   GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMULTI, EsEc-L, EsEc-S     ; 2nd l
ine(STYPE=CMP-B/I)
;   SW_i, Hw_i, tw_i, B_i, Bf1_i, tf1_i, B2_i, Bf2_i, tf2_i             ; 3rd l
ine(STYPE=CMP-B/I)
;   SW_j, Hw_j, tw_j, B_j, Bf1_j, tf1_j, B2_j, Bf2_j, tf2_j           ; 4th l
ine(STYPE=CMP-B/I)
;   N1, N2, Hr, Hr2, tr1, tr2                                          ; 5th l
ine(STYPE=CMP-B)
;   GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb, bSYM, SW_i, SW_j    ; 2nd l
ine(STYPE=CMP-CI/CT)
;   OPT1, OPT2, [JOINT]                                                ; 3rd l
ine(STYPE=CMP-CI/CT)
;   [SIZE-A]-i                                                         ; 4th l
ine(STYPE=CMP-CI/CT)
;   [SIZE-B]-i                                                         ; 5th l
ine(STYPE=CMP-CI/CT)
;   [SIZE-C]-i                                                         ; 6th l
ine(STYPE=CMP-CI/CT)
;   [SIZE-D]-i                                                         ; 7th l
ine(STYPE=CMP-CI/CT)
;   [SIZE-A]-j                                                         ; 8th l
ine(STYPE=CMP-CI/CT)
;   [SIZE-B]-j                                                         ; 9th l
ine(STYPE=CMP-CI/CT)
;   [SIZE-C]-j                                                         ; 10th
line(STYPE=CMP-CI/CT)
;   [SIZE-D]-j                                                         ; 11th

```

# MIDAS

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```


line(STYPE=CMP-CI/CT)
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, STYPE1, STYPE2 ; 1st l
ine - CONSTRUCT ; Befor
; SHAPE, ...(same with other type data from shape) ; After
e (STYPE1)
; SHAPE, ...(same with other type data from shape) ; 1st l
(STYPE2)
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st l
ine - COMPOSITE-SB ; 2nd l
; Hw, tw, B, Bf1, tf1, B2, Bf2, tf2 ; 3rd l
ine N1, N2, Hr, Hr2, tr1, tr2 ; 4th l
; SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 1st l
ine - COMPOSITE-SI ; 2nd l
; Hw, tw, B, tf1, B2, tf2 ; 3rd l
; SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 1st l
ine - COMPOSITE-CI/CT ; 2nd l
; OPT1, OPT2, [JOINT] ; 3rd l
; [SIZE-A] ; 4th l
; [SIZE-B] ; 5th l
; [SIZE-C] ; 6th l
; [SIZE-D] ; 7th l
; SW, GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb ; 1st l
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 2nd l
ine - PSC ; 3rd l
; OPT1, OPT2, [JOINT] ; 4th l
; bSHEARCHK, [SCHK], [WT], WIDTH, bSYM, bSIDEHOLE ; 5th l
; bUSERDEFMESH SIZE, MESH SIZE, bUSERINPSTIFF, [STIFF] ; 6th l
; bWE, [WARPING POINT]-i, [WARPING POINT]-j ; 7th l
; [SIZE-A] ; 8th l
; [SIZE-B] ; 9th l
; [SIZE-C] ;
; [SIZE-D] ;
; [DATA1] : 1, DB, NAME or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10
; [DATA2] : CCSHAPE or iCEL or iN1, iN2
; [SRC] : 1, DB, NAME1, NAME2 or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10, iN1, iN2

; [DIM1], [DIM2] : D1, D2, D3, D4, D5, D6, D7, D8
; [OFFSET] : OFFSET, iCENT, iREF, iHORZ, HUSER, iVERT, VUSER
; [OFFSET2]: OFFSET, iCENT, iREF, iHORZ, HUSERI, HUSERJ, iVERT, VUSERI, VUSERJ
; [JOINT] : 8(1CELL, 2CELL), 13(3CELL), 9(PSCM), 8(PSCH), 9(PSCT), 2(PSCB), 0(nCELL), 2(nCEL2)

```

# MIDAS

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```
; [SIZE-A] : 6(1CELL, 2CELL), 10(3CELL), 10(PSCM), 6(PSCH), 8(PSCT), 10(PSCB), 5(nCELL), 11(nCEL2)
; [SIZE-B] : 6(1CELL, 2CELL), 12(3CELL), 6(PSCM), 6(PSCH), 8(PSCT), 6(PSCB), 8(nCELL), 18(nCEL2)
; [SIZE-C] : 10(1CELL, 2CELL), 13(3CELL), 9(PSCM), 10(PSCH), 7(PSCT), 8(PSCB), 0(nCELL), 11(nCEL2)
; [SIZE-D] : 8(1CELL, 2CELL), 13(3CELL), 6(PSCM), 7(PSCH), 8(PSCT), 5(PSCB), 0(nCELL), 18(nCEL2)
; [STIFF] : AREA, ASy, ASz, Ixx, Iyy, Izz
; [SCHK] : bAUTO_Z1, Z1, bAUTO_Z3, Z3
; [WT] : bAUTO_TOR, TOR, bAUTO_SHR1, SHR1, bAUTO_SHR2, SHR2, bAUTO_SHR3, SHR3
; [CMPWEB] : EFD, LRF, A, B, H, T
; [WARPING POINT] : nWarpingCheck, X1,X2,X3,X4,X5,X6, Y1,Y2,Y3,Y4,Y5,Y6
1, COMPOSITE , TRAVE PRINCIPALE CAMPATA, CT, 0, 0, 0, 0, 0, 0, 0, YES, NO, I
1.93, 0.022, 0.6, 0.03, 1, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.35, 1, 3.35, 3.35, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
2, COMPOSITE , TRAVE PRINCIPALE APPOGGIO, CT, 0, 0, 0, 0, 0, 0, 0, YES, NO, I
1.93, 0.022, 0.6, 0.03, 0.8, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.35, 1, 3.35, 3.35, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
3, COMPOSITE , TRAVE SPINA , CT, 0, 0, 0, 0, 0, 0, 0, YES, NO, I
0.54, 0.013, 0.3, 0.025, 0.3, 0.025
0, 0, 0, 0, 0, 0
0
0
0
3.8, 1, 3.8, 3.8, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
4, DBUSER , TRAVERSO , CC, 0, 0, 0, 0, 0, 0, 0, YES, NO, H , 2, 0.8, 0.3
5, 0.02, 0.028, 0.35, 0.028, 0, 0, 0, 0
5, DBUSER , CONTROVENTI , CC, 0, 0, 0, 0, 0, 0, 0, YES, NO, L , 1, UNI, L70
x7
6, DBUSER , SOLETTA , CC, 0, 0, 0, 0, 0, 0, 0, YES, NO, SB , 2, 0.25, 0.
82, 0, 0, 0, 0, 0, 0, 0, 0
7, COMPOSITE , CONCIO C1 , CT, 0, 0, 0, 0, 0, 0, 0, YES, NO, I
1.945, 0.02, 0.7, 0.02, 0.9, 0.035
0, 0, 0, 0, 0, 0
0
0
0
3.34, 1, 3.34, 3.34, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
8, COMPOSITE , CONCIO C2 , CT, 0, 0, 0, 0, 0, 0, 0, YES, NO, I
1.935, 0.02, 0.7, 0.025, 1.2, 0.04
0, 0, 0, 0, 0, 0
0
0
0
3.34, 1, 3.34, 3.34, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
9, COMPOSITE , CONCIO C3 , CT, 0, 0, 0, 0, 0, 0, 0, YES, NO, I
1.915, 0.02, 0.7, 0.03, 1.2, 0.055
0, 0, 0, 0, 0, 0
0
0
0
3.34, 1, 3.34, 3.34, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
```

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\*SECT-COLOR

```
; iSEC, W_R, W_G, W_B, HF_R, HF_G, HF_B, HE_R, HE_G, HE_B, bBLEND, FACT
1, 0, 33, 66, 0, 128, 255, 0, 33, 66, NO, 0.5
2, 31, 0, 81, 148, 87, 255, 31, 0, 81, NO, 0.5
3, 108, 54, 0, 255, 128, 0, 108, 54, 0, NO, 0.5
4, 0, 66, 45, 0, 192, 128, 0, 66, 45, NO, 0.5
5, 65, 65, 65, 255, 255, 255, 65, 65, 65, NO, 0.5
6, 100, 0, 100, 192, 0, 192, 100, 0, 100, NO, 0.5
7, 100, 0, 100, 255, 0, 0, 100, 0, 100, NO, 0.5
8, 70, 70, 70, 0, 255, 255, 70, 70, 70, NO, 0.5
9, 85, 32, 0, 0, 255, 0, 85, 32, 0, NO, 0.5
```

\*COMP-GEN-SECT-PSC-DESIGN ; Composite Section for PSC Design


```
; SECT, bCompPSC, (Z1, Z2, Z3, t1, t2, t3, TotT)-I, (Z1, Z2, Z3, t1, t2, t3, TotT)-J
1, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
2, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
3, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
7, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
8, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
9, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
```

\*DGN-SECT

```
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, [DATA1], [DATA2] ;
1st line - DB/USER
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, BLT, D1, ..., D8, iCEL ;
1st line - VALUE
; AREA, ASy, ASz, Ixx, Iyy, Izz ;
2nd line
; CyP, CyM, CzP, CzM, QyB, QzB, PERI_OUT, PERI_IN, Cy, Cz ;
3rd line
; Y1, Y2, Y3, Y4, Z1, Z2, Z3, Z4, Zyy, Zzz ;
4th line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, ELAST, DEN, POIS, POIC, SF, THERMAL ;
1st line - SRC
; D1, D2, [SRC] ;
2nd line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 1, DB, NAME1, NAME2, D1, D2 ;
1st line - COMBINED
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 2, D11, D12, D13, D14, D15, D21, D22, D23, D24 ;
; iSEC, TYPE, SNAME, [OFFSET2], bSD, bWE, SHAPE, iyVAR, izVAR, STYPE ;
1st line - TAPERED
; DB, NAME1, NAME2 ;
2nd line(STYPE=DB)
; [DIM1], [DIM2] ;
2nd line(STYPE=USER)
; D11, D12, D13, D14, D15, D16, D17, D18 ;
2nd line(STYPE=VALUE)
; AREA1, ASy1, ASz1, Ixx1, Iyy1, Izz1 ;
3rd line(STYPE=VALUE)
; CyP1, CyM1, CzP1, CzM1, QyB1, QzB1, PERI_OUT1, PERI_IN1, Cy1, Cz1 ;
4th line(STYPE=VALUE)
; Y11, Y12, Y13, Y14, Z11, Z12, Z13, Z14, Zyy1, Zyy2 ;
5th line(STYPE=VALUE)
; D21, D22, D23, D24, D25, D26, D27, D28 ;
6th line(STYPE=VALUE)
; AREA2, ASy2, ASz2, Ixx2, Iyy2, Izz2 ;
7th line(STYPE=VALUE)
; CyP2, CyM2, CzP2, CzM2, QyB2, QzB2, PERI_OUT2, PERI_IN2, Cy2, Cz2 ;
8th line(STYPE=VALUE)
; Y21, Y22, Y23, Y24, Z21, Z22, Z23, Z24, Zyy2, Zzz2 ;
9th line(STYPE=VALUE)
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;      OPT1, OPT2, [JOINT] ; 2nd l
ine (STYPE=PSC)
;      ELAST, DEN, POIS, POIC, THERMAL ; 2nd l
ine (STYPE=PSC-CMPW)
;      bSHEARCHK, [SCHK-I], [SCHK-J], [WT-I], [WT-J], WI, WJ, bSYM, bSIDEHOLE ; 3rd l
ine (STYPE=PSC)
;      bSHEARCHK, bSYM, bHUNCH, [CMPWEB-I], [CMPWEB-J] ; 3rd l
ine (STYPE=PSC-CMPW)
;      bUSERDEFMESH SIZE, MESH SIZE, bUSERINPSTIFF, [STIFF-I], [STIFF-J] ; 4th l
ine (STYPE=PSC)
;      [SIZE-A]-i ; 5th l
ine (STYPE=PSC)
;      [SIZE-B]-i ; 6th l
ine (STYPE=PSC)
;      [SIZE-C]-i ; 7th l
ine (STYPE=PSC)
;      [SIZE-D]-i ; 8th l
ine (STYPE=PSC)
;      [SIZE-A]-j ; 9th l
ine (STYPE=PSC)
;      [SIZE-B]-j ; 10th
line (STYPE=PSC)
;      [SIZE-C]-j ; 11th
line (STYPE=PSC)
;      [SIZE-D]-j ; 12th
line (STYPE=PSC)
;      GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMULTI, EsEc-L, EsEc-S ; 2nd l
ine (STYPE=CMP-B/I)
;      SW i, Hw i, tw_i, B_i, Bf1_i, tf1_i, B2_i, Bf2_i, tf2_i ; 3rd l
ine (STYPE=CMP-B/I)
;      SW_j, Hw_j, tw_j, B_j, Bf1_j, tf1_j, B2_j, Bf2_j, tf2_j ; 4th l
ine (STYPE=CMP-B/I)
;      N1, N2, Hr, Hr2, tr1, tr2 ; 5th l
ine (STYPE=CMP-B)
;      GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb, bSYM, SW_i, SW_j ; 2nd l
ine (STYPE=CMP-CI/CT)
;      OPT1, OPT2, [JOINT] ; 3rd l
ine (STYPE=CMP-CI/CT)
;      [SIZE-A]-i ; 4th l
ine (STYPE=CMP-CI/CT)
;      [SIZE-B]-i ; 5th l
ine (STYPE=CMP-CI/CT)
;      [SIZE-C]-i ; 6th l
ine (STYPE=CMP-CI/CT)
;      [SIZE-D]-i ; 7th l
ine (STYPE=CMP-CI/CT)
;      [SIZE-A]-j ; 8th l
ine (STYPE=CMP-CI/CT)
;      [SIZE-B]-j ; 9th l
ine (STYPE=CMP-CI/CT)
;      [SIZE-C]-j ; 10th
line (STYPE=CMP-CI/CT)
;      [SIZE-D]-j ; 11th
line (STYPE=CMP-CI/CT)
;      iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, STYPE1, STYPE2 ; 1st l
ine - CONSTRUCT
;      SHAPE, ... (same with other type data from shape) ; Befor
e (STYPE1)
;      SHAPE, ... (same with other type data from shape) ; After
(STYPE2)
;      iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st l
ine - COMPOSITE-SB

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;      Hw, tw, B, Bf1, tf1, B2, Bf2, tf2                ; 2nd l
ine
;      N1, N2, Hr, Hr2, tr1, tr2                        ; 3rd l
ine
;      SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 4th l
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE        ; 1st l
ine - COMPOSITE-SI
;      Hw, tw, B, tf1, B2, tf2                          ; 2nd l
ine
;      SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 3rd l
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE        ; 1st l
ine - COMPOSITE-CI/CT
;      OPT1, OPT2, [JOINT]                              ; 2nd l
ine
;      [SIZE-A]                                         ; 3rd l
ine
;      [SIZE-B]                                         ; 4th l
ine
;      [SIZE-C]                                         ; 5th l
ine
;      [SIZE-D]                                         ; 6th l
ine
;      SW, GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb ; 7th l
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE        ; 1st l
ine - PSC
;      OPT1, OPT2, [JOINT]                              ; 2nd l
ine
;      bsHEARCHK, [SCHK], [WT], WIDTH, bSYM, bSIDEHOLE ; 3rd l
ine
;      bUSERDEFMESH SIZE, MESH SIZE, bUSERINPSTIFF, [STIFF] ; 4th l
ine
;      bWE, [WARPING POINT]-i, [WARPING POINT]-j       ; 5th l
ine
;      [SIZE-A]                                         ; 6th l
ine
;      [SIZE-B]                                         ; 7th l
ine
;      [SIZE-C]                                         ; 8th l
ine
;      [SIZE-D]                                         ; 9th l
ine
; [DATA1] : 1, DB, NAME or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10
; [DATA2] : CCSHAPE or iCEL or iN1, iN2
; [SRC]   : 1, DB, NAME1, NAME2 or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10, iN1, iN2

; [DIM1], [DIM2] : D1, D2, D3, D4, D5, D6, D7, D8
; [OFFSET] : OFFSET, iCENT, iREF, iHORZ, HUSER, iVERT, VUSER
; [OFFSET2] : OFFSET, iCENT, iREF, iHORZ, HUSERI, HUSERJ, iVERT, VUSERI, VUSERJ
; [JOINT] : 8(1CELL, 2CELL), 13(3CELL), 9(PSCM), 8(PSCH), 9(PSCT), 2(PSCB), 0(nCELL), 2(nCEL2)
; [SIZE-A] : 6(1CELL, 2CELL), 10(3CELL), 10(PSCM), 6(PSCH), 8(PSCT), 10(PSCB), 5(nCELL), 11(nCEL2)
; [SIZE-B] : 6(1CELL, 2CELL), 12(3CELL), 6(PSCM), 6(PSCH), 8(PSCT), 6(PSCB), 8(nCELL), 18(nCEL2)
; [SIZE-C] : 10(1CELL, 2CELL), 13(3CELL), 9(PSCM), 10(PSCH), 7(PSCT), 8(PSCB), 0(nCELL), 11(nCEL2)
; [SIZE-D] : 8(1CELL, 2CELL), 13(3CELL), 6(PSCM), 7(PSCH), 8(PSCT), 5(PSCB), 0(nCELL), 18(nCEL2)
; [STIFF] : AREA, ASy, ASz, Ixx, Iyy, Izz

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; [SCHK]      : bAUTO_Z1, Z1, bAUTO_Z3, Z3
; [WT]       : bAUTO_TOR, TOR, bAUTO_SHR1, SHR1, bAUTO_SHR2, SHR2, bAUTO_SHR3, SHR3
; [CMPWEB]   : EFD, LRF, A, B, H, T
; [WARPING POINT] : nWarpingCheck, X1,X2,X3,X4,X5,X6, Y1,Y2,Y3,Y4,Y5,Y6
1, COMPOSITE , TRAVE PRINCIPALE CAMPATA, CT, 0, 0, 0, 0, 0, 0, 0, YES, NO, I
  1.93, 0.022, 0.6, 0.03, 1, 0.04
  0, 0, 0, 0, 0, 0
  0
  0
  0
  3.35, 1, 3.35, 3.35, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
2, COMPOSITE , TRAVE PRINCIPALE APPOGGIO, CT, 0, 0, 0, 0, 0, 0, YES, NO, I
  1.93, 0.022, 0.6, 0.03, 0.8, 0.04
  0, 0, 0, 0, 0, 0
  0
  0
  0
  3.35, 1, 3.35, 3.35, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
3, COMPOSITE , TRAVE SPINA , CT, 0, 0, 0, 0, 0, 0, YES, NO, I
  0.54, 0.013, 0.3, 0.025, 0.3, 0.025
  0, 0, 0, 0, 0, 0
  0
  0
  0
  3.8, 1, 3.8, 3.8, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
4, DBUSER , TRAVERSO , CC, 0, 0, 0, 0, 0, 0, YES, NO, H , 2, 0.8, 0.3
5, 0.02, 0.028, 0.35, 0.028, 0, 0, 0, 0
5, DBUSER , CONTROVENTI , CC, 0, 0, 0, 0, 0, 0, YES, NO, L , 1, UNI, L70
x7
6, DBUSER , SOLETTA , CC, 0, 0, 0, 0, 0, 0, YES, NO, SB , 2, 0.25, 0.
82, 0, 0, 0, 0, 0, 0, 0, 0
7, COMPOSITE , CONCIO C1 , CT, 0, 0, 0, 0, 0, 0, YES, NO, I
  1.945, 0.02, 0.7, 0.02, 0.9, 0.035
  0, 0, 0, 0, 0, 0
  0
  0
  0
  3.34, 1, 3.34, 3.34, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
8, COMPOSITE , CONCIO C2 , CT, 0, 0, 0, 0, 0, 0, YES, NO, I
  1.935, 0.02, 0.7, 0.025, 1.2, 0.04
  0, 0, 0, 0, 0, 0
  0
  0
  0
  3.34, 1, 3.34, 3.34, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
9, COMPOSITE , CONCIO C3 , CT, 0, 0, 0, 0, 0, 0, YES, NO, I
  1.915, 0.02, 0.7, 0.03, 1.2, 0.055
  0, 0, 0, 0, 0, 0
  0
  0
  0
  3.34, 1, 3.34, 3.34, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,


```

```

*STLDCASE      ; Static Load Cases
; LCNAME, LCTYPE, DESC
  PESO PROPRIO ACCIAIO, CS, PESO PROPRIO ACCIAIO
  PESO PROPRIO SOLETTA, CS, PESO PROPRIO SOLETTA
  PERMANENTE PORTATO, CS, PERMANENTE PORTATO
  VENTO PONTE SCARICO, W , VENTO PONTE SCARICO
  VENTO PONTE CARICO, WL, VENTO PONTE CARICO
  TERMICA UNIFORME +, T , TERMICA UNIFORME +

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TERMICA UNIFORME -, T , TERMICA UNIFORME -  
 TERMICA GRADIENTE +, TPG, TERMICA GRADIENTE +  
 TERMICA GRADIENTE -, TPG, TERMICA GRADIENTE -  
 FRENATURA, BRK, FRENATURA

```
*SPRING      ; Point Spring Supports
; NODE_LIST, Type, SDx, SDy, SDz, SRx, SRy, SRz, GROUP, FROMTYPE, EFFAREA, Kx, Ky, Kz
; LINEAR
; NODE_LIST, Type, Direction, Vx, Vy, Vz, Stiffness, GROUP, FROMTYPE, EFFAREA
; COMP, TENS
; NODE_LIST, Type, Multi-Linear Type, Direction, Vx, Vy, Vz, ax, ay, bx, by, cx, cy, d
x, dy, ex, ey, fx, fy, GROUP, FROMTYPE, EFFAREA ; MULTI
1to4, LINEAR, 4220, 4220, 4.33e+006, 0, 0, 0, NO, 0, 0, 0, 0, 0, 0, 0, FASE 1, 0, 0, 0
, 0, 0
```

```
*RIGIDLINK   ; Rigid Link
; KEY, M-NODE, DOF, S-NODE LIST, GROUP
32, 1, 111111, 9 93, FASE 1
31, 2, 111111, 31 195, FASE 1
1, 3, 111111, 19 143, FASE 1
2, 4, 111111, 41 245, FASE 1
6, 10, 111111, 98, FASE 1
9, 11, 111111, 103, FASE 1
12, 12, 111111, 108, FASE 1
15, 13, 111111, 113, FASE 1
17, 14, 111111, 118, FASE 1
20, 15, 111111, 123, FASE 1
24, 16, 111111, 128, FASE 1
27, 17, 111111, 133, FASE 1
30, 18, 111111, 138, FASE 1
3, 20, 111111, 144, FASE 1
5, 21, 111111, 149, FASE 1
8, 22, 111111, 154, FASE 1
11, 23, 111111, 159, FASE 1
14, 24, 111111, 164, FASE 1
18, 25, 111111, 169, FASE 1
21, 26, 111111, 174, FASE 1
23, 27, 111111, 179, FASE 1
26, 28, 111111, 184, FASE 1
29, 29, 111111, 189, FASE 1
33, 30, 111111, 194, FASE 1
4, 32, 111111, 200, FASE 1
7, 33, 111111, 205, FASE 1
10, 34, 111111, 210, FASE 1
13, 35, 111111, 215, FASE 1
16, 36, 111111, 220, FASE 1
19, 37, 111111, 225, FASE 1
22, 38, 111111, 230, FASE 1
25, 39, 111111, 235, FASE 1
28, 40, 111111, 240, FASE 1
```

```
; *LOADTOMASS, DIR, bNODAL, bBEAM, bFLOOR, bPRES, GRAV
; LCNAME1, FACTOR1, LCNAME2, FACTOR2, ... ; from line 1
```

```
*LOADTOMASS, XYZ, YES, YES, YES, YES, 9.806
PESO PROPRIO SOLETTA, 1, PERMANENTE PORTATO, 1
```

```
*USE-STLD, PESO PROPRIO ACCIAIO
```

```
; *SELFWEIGHT, X, Y, Z, GROUP
*SELFWEIGHT, 0, 0, -1.12, FASE 1
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; End of data for load case [PESO PROPRIO ACCIAIO] -----

\*USE-STLD, PESO PROPRIO SOLETTA


```

*BEAMLOAD      ; Element Beam Loads
; ELEM_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM_LIST, CMD, TYPE, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE]      : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN]      : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
  74, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  75, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  76, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  77, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  78, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  79, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  80, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  81, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  82, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  83, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  84, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  85, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  86, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  87, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  88, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  89, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  90, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  91, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  92, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  93, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  94, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  95, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  96, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  97, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  98, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
  99, BEAM      , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,

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100, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
101, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
102, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
103, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
104, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
105, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
106, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
107, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
108, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
109, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
110, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
111, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
112, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
113, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
114, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
115, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
116, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
117, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
118, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
119, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -23.516, 1, -23.516, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
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183, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -26.844, 1, -26.844, 0, 0, 0,
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PROJECT TITLE :

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184, BEAM , UNILoad, GZ, NO , NO, aDir[1], , , , 0, -26.844, 1, -26.844, 0, 0, 0,
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229, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -26.844, 1, -26.844, 0, 0, 0,
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; End of data for load case [PESO PROPRIO SOLETTA] -----

\*USE-STLD, PERMANENTE PORTATO


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*BEAMLOAD ; Element Beam Loads
; ELEM_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM_LIST, CMD, TYPE, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
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; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
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, FASE 3, NO, 0, 0, NO,

; End of data for load case [PERMANENTE PORTATO] -----

\*USE-STLD, VENTO PONTE SCARICO


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*BEAMLOAD ; Element Beam Loads
; ELEM_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM_LIST, CMD, TYPE, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE] : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
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99, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.592, 1, -0.592, 0, 0, 0, 0
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299, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.592, 1, 0.592, 0, 0, 0, 0,
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314, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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331, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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334, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.592, 1, 0.592, 0, 0, 0, 0,
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334, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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336, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.592, 1, 0.592, 0, 0, 0, 0,
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336, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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338, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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345, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0  
, , NO, 0, 0, NO,

; End of data for load case [VENTO PONTE SCARICO] -----

\*USE-STLD, VENTO PONTE CARICO

\*BEAMLOAD ; Element Beam Loads

; ELEM\_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP

; ELEM\_LIST, CMD, TYPE, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP

; [VALUE] : D1, P1, D2, P2, D3, P3, D4, P4

; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END

; [ADDITIONAL] : bADDITIONAL, ADDITIONAL\_I-END, ADDITIONAL\_J-END, bADDITIONAL\_J-END

74, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.304, 1, -0.304, 0, 0, 0, 0  
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76, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.304, 1, -0.304, 0, 0, 0, 0  
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91, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.304, 1, -0.304, 0, 0, 0, 0  
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98, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.304, 1, -0.304, 0, 0, 0, 0



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113, BEAM , UNILoad, LZ, NO , NO, aDir[1], , , , 0, -0.304, 1, -0.304, 0, 0, 0, 0
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298, BEAM , UNILoad, LZ, NO , NO, aDir[1], , , , 0, 0.304, 1, 0.304, 0, 0, 0, 0

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314, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -10.142, 1, -10.142, 0, 0, 0,
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# MIDAS

PROJECT TITLE :

	Company	Client
	Author	File Name
		L2-CV01_REVISIONE_2


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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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; End of data for load case [VENTO PONTE CARICO] -----

\*USE-STLD, TERMICA UNIFORME +

\*ELTEMPER ; Element Temperatures

; ELEM\_LIST, TEMPER, GROUP

- 1, 49,
- 2, 49,
- 3, 49,
- 4, 49,
- 5, 49,
- 6, 49,
- 7, 49,
- 8, 49,
- 9, 49,
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- 11, 49,
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- 13, 49,
- 14, 49,
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- 102, 49,
- 103, 49,

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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# MIDAS

PROJECT TITLE :


	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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; End of data for load case [TERMICA UNIFORME +] -----

\*USE-STLD, TERMICA UNIFORME -

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2


\*ELTEMPER ; Element Temperatures

; ELEM LIST, TEMPER, GROUP

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PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2


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 345, -11,

; End of data for load case [TERMICA UNIFORME -] -----

\*USE-STLD, TERMICA GRADIENTE +

\*THERGRAD ; Temperature Gradient  
 ; ELEM\_LIST, iETYP, TZ, bUSEHZ, HZ, TY, bUSEHY, HY, GROUP  
 ; ELEM\_LIST, iETYP, TZ, bUSEHZ, HZ, GROUP  
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	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2


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PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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344, 1, 15, YES, 0, 0, YES, 0,
345, 1, 15, YES, 0, 0, YES, 0,

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; End of data for load case [TERMICA GRADIENTE +] -----

\*USE-STLD, TERMICA GRADIENTE -

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*THERGRAD ; Temperature Gradient
; ELEM_LIST, iETYP, TZ, bUSEHZ, HZ, TY, bUSEHY, HY, GROUP
; ELEM_LIST, iETYP, TZ, bUSEHZ, HZ, GROUP
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	Company		Client	
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PROJECT TITLE :


	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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335, 1, -18, YES, 0, 0, YES, 0,
336, 1, -18, YES, 0, 0, YES, 0,
337, 1, -18, YES, 0, 0, YES, 0,
338, 1, -18, YES, 0, 0, YES, 0,
339, 1, -18, YES, 0, 0, YES, 0,
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344, 1, -18, YES, 0, 0, YES, 0,
345, 1, -18, YES, 0, 0, YES, 0,

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; End of data for load case [TERMICA GRADIENTE -] -----

\*USE-STLD, FRENATURA

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*BEAMLOAD ; Element Beam Loads
; ELEM_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM_LIST, CMD, TYPE, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE] : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
74, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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76, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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77, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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78, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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79, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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PROJECT TITLE :

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      80, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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      82, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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      83, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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177, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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, NO, 0, 0, NO,

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201, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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202, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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243, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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244, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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296, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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326, BEAM , UNILOAD, LX, NO , NO, aDir[1], , , , 0, 3.724, 1, 3.724, 0, 0, 0, 0,
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, NO, 0, 0, NO,

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; End of data for load case [FRENATURA] -----

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*STAGE ; Define Construction Stage
; NAME=NAME, DURATION, bSAVESTAGE, bSAVESTAGE, bINCRESTEP, INCRESTEP ; line 1
; STEP=DAY1, DAY2, ... ; line 2
; AELEM=GROUP1, AGE1, GROUP2, AGE2, ... ; line 3
; DELEM=GROUP1, REDIST1, GROUP2, REDIST2, ... ; line 4
; ABNDR=BGROUP1, POS1, BGROUP2, POS2, ... ; line 5
; DBNDR=BGROUP1, BGROUP2, ... ; line 6
; ALOAD=LGROUP1, DAY1, LGROUP2, DAY2, ... ; line 7
; DLOAD=LGROUP1, DAY1, LGROUP2, DAY2, ... ; line 8
NAME=FASE 1, 2, YES, NO, NO, 5
AELEM=FASE 1, 0
ABNDR=FASE 1, DEFORMED
ALOAD=FASE 1, FIRST
NAME=FASE 2, 28, YES, NO, NO, 5
AELEM=FASE 2, 0
ALOAD=FASE 2, FIRST
NAME=FASE 3, 10000, YES, NO, NO, 5
ALOAD=FASE 3, FIRST

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*SFUNCTION      ; Spectrum Function
; FUNC=NAME, iTYPE, iMETHOD, SCALE/MAX, GRAV, DRATIO, DESC, RMF      ; line 1
; SPEC CODE, [CODE DATA]                                          ; line 2
; PERIOD1, VALUE1, PERIOD2, VALUE2, ...                            ; from line 3
; [CODE_DATA] : NSC, SFI, SC, EQ, TG, DP, MaxEQ
; CH2001
; [CODE_DATA] : NSC, SFI, SC, EQ, TG, DP, MaxEQ, nLForce           ; CH2010
; [CODE_DATA] : SFI, SC, EQ, TG, DP, MaxEQ                         ; CHSH2003
; [CODE_DATA] : DIV, SC, SFI, EQ, TG, G                           ; GB50111_2006
; [CODE_DATA] : BT, ZM, ST, SI, SC, TG, CI, CS, CD, EPA, SMAX, PERIOD ; JTG/T B02-01-20
08
; [CODE_DATA] : iSPE, SParam, TB, TC, TD, AG, Q, IF, FPX, FPY      ; P100-1(2013)
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  USER
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    0.010641,    0.547205658,    0.011324,    0.551018111
    0.012049,    0.555074745,    0.012822,    0.559391763
    0.013644,    0.56398537,    0.014519,    0.568873445
    0.015450,    0.574075547,    0.016441,    0.579610673
    0.017495,    0.585500614,    0.018617,    0.59176884
    0.019810,    0.598438258,    0.021080,    0.60553569
    0.022432,    0.613087958,    0.023870,    0.621124674
    0.025401,    0.629676572,    0.027029,    0.638776618
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    0.032569,    0.66973052,    0.034657,    0.68139865
    0.036879,    0.693815525,    0.039244,    0.707028081
    0.041760,    0.721088283,    0.044438,    0.736049215
    0.047287,    0.751970105,    0.050319,    0.768911299
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    0.068656,    0.871373706,    0.073058,    0.895971098
    0.077743,    0.922145883,    0.082727,    0.94999864
    0.088032,    0.979637209,    0.093676,    1.011176139
    0.099682,    1.044737238,    0.106074,    1.0804507
    0.112875,    1.118452305,    0.120112,    1.158890125
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    0.185573,    1.408697568,    0.197472,    1.408697568
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    0.269435,    1.408697568,    0.286711,    1.408697568
    0.305094,    1.408697568,    0.324656,    1.408697568
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    0.391194,    1.408697568,    0.416277,    1.408697568
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    0.501593,    1.388657081,    0.533754,    1.304984452
    0.567977,    1.226353658,    0.604394,    1.152461261
    0.643146,    1.083021074,    0.684384,    1.017762939
    0.728265,    0.956438482,    0.774959,    0.898809707
    0.824648,    0.844652108,    0.877523,    0.793757738
    0.933788,    0.745930202,    0.993660,    0.700984916
    1.057370,    0.658748282,    1.125170,    0.619053718
    1.197310,    0.58175466,    1.274080,    0.546700891
    1.355770,    0.513760204,    1.442700,    0.482803543
    1.535200,    0.453713309,    1.633640,    0.426373419
    1.738380,    0.400683781,    1.849840,    0.376541037
    1.968450,    0.353852357,    2.094660,    0.332531614
    2.228970,    0.312494413,    2.371880,    0.293666067
    2.523960,    0.275971359,    2.685800,    0.259341973
    2.858000,    0.24371612,    3.041250,    0.229031047
    3.236250,    0.215230798,    3.443750,    0.202262264

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PROJECT TITLE :


	Company	Client	
	Author	File Name	L2-CV01_REVISIONE_2

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3.664560, 0.184183915, 3.899520, 0.162657116
4.149550, 0.143645944, 4.415620, 0.126856285
4.698740, 0.112029538, 5.000000, 0.098936231
5.320590, 0.087372706, 5.661740, 0.077160588
6.024750, 0.068142382, 6.411040, 0.06017809
6.822120, 0.053144297, 7.259530, 0.046933005
7.724990, 0.041447613, 8.220300, 0.036603283
8.747380, 0.032325063, 9.308210, 0.028547168
9.905010, 0.025210737, 10.000000, 0.024734058
FUNC=SLV-V, 1, 0, 1, 9.806, 0.05, , 1.000000
USER
0.000000, 0.245499002, 0.010000, 0.273623505
0.010641, 0.275426848, 0.011324, 0.277345783
0.012049, 0.279387622, 0.012822, 0.281560521
0.013644, 0.283872636, 0.014519, 0.286332968
0.015450, 0.288951359, 0.016441, 0.291737372
0.017495, 0.294701976, 0.018617, 0.297856983
0.019810, 0.301213923, 0.021080, 0.304786298
0.022432, 0.308587605, 0.023870, 0.312632753
0.025401, 0.316937208, 0.027029, 0.321517564
0.028762, 0.326391822, 0.030607, 0.331578543
0.032569, 0.337097695, 0.034657, 0.342970654
0.036879, 0.349220481, 0.039244, 0.355870801
0.041760, 0.362947769, 0.044438, 0.370478105
0.047287, 0.378491619, 0.050319, 0.387018687
0.053545, 0.396092777, 0.056979, 0.405748481
0.060632, 0.416023206, 0.064519, 0.426956888
0.068656, 0.438591432, 0.073058, 0.450972119
0.077743, 0.464146761, 0.082727, 0.478165982
0.088032, 0.493084062, 0.093676, 0.508958656
0.099682, 0.525851077, 0.106074, 0.543826852
0.112875, 0.562954327, 0.120112, 0.583308029
0.127814, 0.604969521, 0.136009, 0.628017551
0.144730, 0.65254493, 0.154009, 0.678641656
0.163884, 0.706414603, 0.174392, 0.709044442
0.185573, 0.709044442, 0.197472, 0.709044442
0.210134, 0.709044442, 0.223607, 0.709044442
0.237944, 0.709044442, 0.253200, 0.709044442
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0.345472, 0.709044442, 0.367623, 0.709044442
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0.442967, 0.709044442, 0.471369, 0.709044442
0.501593, 0.698957398, 0.533754, 0.656842174
0.567977, 0.617264674, 0.604394, 0.580072168
0.643146, 0.545120607, 0.684384, 0.512274013
0.728265, 0.48140737, 0.774959, 0.452400886
0.824648, 0.425141561, 0.877523, 0.399524728
0.933788, 0.375451535, 0.993660, 0.352829074
1.057370, 0.331569969, 1.125170, 0.311590371
1.197310, 0.292816512, 1.274080, 0.275172782
1.355770, 0.258592636, 1.442700, 0.243011117
1.535200, 0.228369032, 1.633640, 0.214607954
1.738380, 0.201677503, 1.849840, 0.189525655
1.968450, 0.178105686, 2.094660, 0.167374246
2.228970, 0.157288854, 2.371880, 0.14781192
2.523960, 0.138905584, 2.685800, 0.13053546
2.858000, 0.122670447, 3.041250, 0.11527896
3.236250, 0.108332835, 3.443750, 0.10180534
3.664560, 0.092705904, 3.899520, 0.081870749
4.149550, 0.072301792, 4.415620, 0.063850997
4.698740, 0.056388201, 5.000000, 0.049797903

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PROJECT TITLE :

	Company	Client	
	Author	File Name	L2-CV01_REVISIONE_2


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5.320590, 0.043977595, 5.661740, 0.038837496
6.024750, 0.034298332, 6.411040, 0.030289639
6.822120, 0.026749296, 7.259530, 0.023622946
7.724990, 0.020861965, 8.220300, 0.018423652
8.747380, 0.016270282, 9.308210, 0.014368741
9.905010, 0.012689404, 10.000000, 0.012449476
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0.010641, 0.151868152, 0.011324, 0.153035479
0.012049, 0.15427757, 0.012822, 0.155599388
0.013644, 0.157005894, 0.014519, 0.158502562
0.015450, 0.160095382, 0.016441, 0.161790169
0.017495, 0.163593597, 0.018617, 0.16551285
0.019810, 0.167554944, 0.021080, 0.16972809
0.022432, 0.172040502, 0.023870, 0.174501245
0.025401, 0.177119731, 0.027029, 0.179906053
0.028762, 0.182871161, 0.030607, 0.186026347
0.032569, 0.189383757, 0.034657, 0.192956395
0.036879, 0.196758289, 0.039244, 0.200803811
0.041760, 0.205108872, 0.044438, 0.209689725
0.047287, 0.214564506, 0.050319, 0.219751691
0.053545, 0.22527164, 0.056979, 0.231145398
0.060632, 0.237395717, 0.064519, 0.244046894
0.068656, 0.251124419, 0.073058, 0.258655837
0.077743, 0.266670235, 0.082727, 0.275198406
0.088032, 0.284273371, 0.093676, 0.293930203
0.099682, 0.304206199, 0.106074, 0.315141221
0.112875, 0.326776845, 0.120112, 0.339158408
0.127814, 0.349222131, 0.136009, 0.349222131
0.144730, 0.349222131, 0.154009, 0.349222131
0.163884, 0.349222131, 0.174392, 0.349222131
0.185573, 0.349222131, 0.197472, 0.349222131
0.210134, 0.349222131, 0.223607, 0.349222131
0.237944, 0.349222131, 0.253200, 0.349222131
0.269435, 0.349222131, 0.286711, 0.349222131
0.305094, 0.349222131, 0.324656, 0.349222131
0.345472, 0.349222131, 0.367623, 0.349222131
0.391194, 0.337428277, 0.416277, 0.31709635
0.442967, 0.297990408, 0.471369, 0.280035211
0.501593, 0.263161402, 0.533754, 0.247304783
0.567977, 0.232403631, 0.604394, 0.218400443
0.643146, 0.205240983, 0.684384, 0.192874055
0.728265, 0.18125259, 0.774959, 0.170331485
0.824648, 0.160068195, 0.877523, 0.150423313
0.933788, 0.14135962, 0.993660, 0.132842136
1.057370, 0.124837963, 1.125170, 0.117315532
1.197310, 0.110247068, 1.274080, 0.103604104
1.355770, 0.097361586, 1.442700, 0.091495056
1.535200, 0.085982229, 1.633640, 0.080801105
1.738380, 0.075932717, 1.849840, 0.071357478
1.968450, 0.067057795, 2.094660, 0.063017348
2.228970, 0.056714205, 2.371880, 0.050085829
2.523960, 0.044231876, 2.685800, 0.039061863
2.858000, 0.034496564, 3.041250, 0.03046464
3.236250, 0.026903957, 3.443750, 0.023759487
3.664560, 0.02098247, 3.899520, 0.01853011
4.149550, 0.016364332, 4.415620, 0.014451633
4.698740, 0.01276255, 5.000000, 0.011270944
5.320590, 0.009953612, 5.661740, 0.008790234
6.024750, 0.007762869, 6.411040, 0.006855566
6.822120, 0.006054267, 7.259530, 0.005346669

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PROJECT TITLE :


	Company	Client	
	Author	File Name	L2-CV01_REVISIONE_2

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7.724990, 0.004721766, 8.220300, 0.004169893
8.747380, 0.003682513, 9.308210, 0.00325213
9.905010, 0.00287204, 10.000000, 0.002817736
FUNC=SLC-H, 1, 0, 1, 9.806, 0.05, , 1.000000
USER
0.000000, 0.615325955, 0.010000, 0.687708286
0.010641, 0.692349441, 0.011324, 0.697288087
0.012049, 0.702543044, 0.012822, 0.708135303
0.013644, 0.714085855, 0.014519, 0.720417861
0.015450, 0.727156656, 0.016441, 0.73432685
0.017495, 0.741956671, 0.018617, 0.750076521
0.019810, 0.758716076, 0.021080, 0.76791008
0.022432, 0.777693275, 0.023870, 0.788104026
0.025401, 0.799182142, 0.027029, 0.810970328
0.028762, 0.82351491, 0.030607, 0.836863659
0.032569, 0.851067968, 0.034657, 0.866182846
0.036879, 0.882267648, 0.039244, 0.899383174
0.041760, 0.917596739, 0.044438, 0.936977108
0.047287, 0.957601006, 0.050319, 0.979546605
0.053545, 1.00290004, 0.056979, 1.027750342
0.060632, 1.054193779, 0.064519, 1.082333133
0.068656, 1.112276256, 0.073058, 1.144139682
0.077743, 1.178046461, 0.082727, 1.214126881
0.088032, 1.252520641, 0.093676, 1.293376124
0.099682, 1.336851123, 0.106074, 1.383114289
0.112875, 1.432341513, 0.120112, 1.484724605
0.127814, 1.540473476, 0.136009, 1.599790796
0.144730, 1.662915427, 0.154009, 1.730078992
0.163884, 1.801556543, 0.174392, 1.877615896
0.185573, 1.911192325, 0.197472, 1.911192325
0.210134, 1.911192325, 0.223607, 1.911192325
0.237944, 1.911192325, 0.253200, 1.911192325
0.269435, 1.911192325, 0.286711, 1.911192325
0.305094, 1.911192325, 0.324656, 1.911192325
0.345472, 1.911192325, 0.367623, 1.911192325
0.391194, 1.911192325, 0.416277, 1.911192325
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0.501593, 1.911192325, 0.533754, 1.911192325
0.567977, 1.80726798, 0.604394, 1.698373322
0.643146, 1.59603985, 0.684384, 1.499869438
0.728265, 1.409496056, 0.774959, 1.324568971
0.824648, 1.244757333, 0.877523, 1.169754691
0.933788, 1.099271618, 0.993660, 1.033036094
1.057370, 0.970792292, 1.125170, 0.912294716
1.197310, 0.85732738, 1.274080, 0.80566891
1.355770, 0.757124472, 1.442700, 0.711503878
1.535200, 0.668633823, 1.633640, 0.628343237
1.738380, 0.590484615, 1.849840, 0.554905638
1.968450, 0.521469504, 2.094660, 0.49004929
2.228970, 0.46052062, 2.371880, 0.432773431
2.523960, 0.406696875, 2.685800, 0.382190277
2.858000, 0.359162577, 3.041250, 0.337521297
3.236250, 0.317183977, 3.443750, 0.298072347
3.664560, 0.28011184, 3.899520, 0.263234102
4.149550, 0.242112259, 4.415620, 0.213813636
4.698740, 0.188823461, 5.000000, 0.166754965
5.320590, 0.147264883, 5.661740, 0.13005257
6.024750, 0.114852571, 6.411040, 0.101428922
6.822120, 0.089573609, 7.259530, 0.079104607
7.724990, 0.069859091, 8.220300, 0.061694074
8.747380, 0.054483224, 9.308210, 0.048115659
9.905010, 0.042492174, 10.000000, 0.041688741
    
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV01_REVISIONE_2

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*SPLDCASE      ; Spectrum Load Cases
; TYPE, bADDSIGN, iSIGNTYPE
; NAME=NAME, DIR, ANGLE, SCALE, PMFT, bDAMP, bECC, INTERP, DESC,           ; line 1
; COMTYPE, bADDSIGN, iSIGNTYPE, bMODE                                     ; line 2
; FUNC1, FUNC2, FUNC3, ...                                              ; line 3
; bUSE1, dFACTOR1, bUSE2, dFACTOR2, ..., bUSEn, dFACTORn                ; line 4 (bMODE
=YES)
; bCDR, [DR-DC]                                                         ; line 5 (bDAMP
=YES)
; [DR-DC] : iMDTYPE, DALL, iMODE1, DAMPING1, iMODE2, DAMPING2, ...      ; iMDTYPE=1
;          : iMDTYPE, iCOEF, bMASSP, MASSC, bSTIFFP, STIFFC             ; iMDTYPE=2, iC
OEF=1
;          : iMDTYPE, iCOEF, iCALC, bMASSP, FP1, DR1, bSTIFFP, FP2, DR2 ; iMDTYPE=2, iC
OEF=2
  NAME=EX, XY, 0, 1, 1, NO, NO, LOG, EX
    CQC, NO, 0, YES
    SLV-H
    YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1
  NAME=EY, XY, 90, 1, 1, NO, NO, LOG, EY
    CQC, NO, 0, YES
    SLV-H
    YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1
  NAME=EZ, Z, 0, 1, 1, NO, NO, LOG, EZ
    CQC, NO, 0, YES
    SLV-V
    YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1
  NAME=DX, XY, 0, 1, 1, NO, NO, LOG, DX
    CQC, NO, 0, YES
    SLD-H
    YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1
  NAME=CX, XY, 0, 1, 1, NO, NO, LOG, CX
    CQC, NO, 0, YES
    SLC-H
    YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1
  NAME=CY, XY, 90, 1, 1, NO, NO, LOG, CY
    CQC, NO, 0, YES
    SLC-H
    YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1, YES, 1

*MVLDCODE      ; Moving Load Code
; CODE=CODE
  CODE=EUROCODE

*LINELANE      ; Traffic Line Lanes
; NAME=NAME, LDIST, GROUP, SKEWS, SKEWE, MOVING, LW, WS, bLANEOPT, ALLOWWIDTH ; line
1
; iELEM1, ECC1, FACT1, bSPAN1, ECCVL... ; from
line 2
  NAME=LC01, CROSS, FASE 2, 0, 0, BOTH, 3, 2, NO, 3
    74, -1, 0, NO, 0, 75, -1, 0, NO, 0, 76, -1, 0, NO, 0
    77, -1, 0, NO, 0, 78, -1, 0, NO, 0, 79, -1, 0, NO, 0
    80, -1, 0, NO, 0, 81, -1, 0, NO, 0, 82, -1, 0, NO, 0
    83, -1, 0, NO, 0, 84, -1, 0, NO, 0, 85, -1, 0, NO, 0
    86, -1, 0, NO, 0, 87, -1, 0, NO, 0, 88, -1, 0, NO, 0
    89, -1, 0, NO, 0, 90, -1, 0, NO, 0, 91, -1, 0, NO, 0
    92, -1, 0, NO, 0, 93, -1, 0, NO, 0, 94, -1, 0, NO, 0
    95, -1, 0, NO, 0, 96, -1, 0, NO, 0, 97, -1, 0, NO, 0
    98, -1, 0, NO, 0, 99, -1, 0, NO, 0, 100, -1, 0, NO, 0
    101, -1, 0, NO, 0, 102, -1, 0, NO, 0, 103, -1, 0, NO, 0
    104, -1, 0, NO, 0, 105, -1, 0, NO, 0, 106, -1, 0, NO, 0
    107, -1, 0, NO, 0, 108, -1, 0, NO, 0, 109, -1, 0, NO, 0

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PROJECT TITLE :

MIDAS	Company	Client
	Author	File Name
		L2-CV01_REVISIONE_2

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110, -1, 0, NO, 0, 111, -1, 0, NO, 0, 112, -1, 0, NO, 0
113, -1, 0, NO, 0, 114, -1, 0, NO, 0, 115, -1, 0, NO, 0
116, -1, 0, NO, 0, 117, -1, 0, NO, 0, 118, -1, 0, NO, 0
119, -1, 0, NO, 0, 120, -1, 0, NO, 0, 121, -1, 0, NO, 0
122, -1, 0, NO, 0, 123, -1, 0, NO, 0
NAME=LC02, CROSS, FASE 2, 0, 0, BOTH, 3, 2, NO, 3
74, -4, 0, NO, 0, 75, -4, 0, NO, 0, 76, -4, 0, NO, 0
77, -4, 0, NO, 0, 78, -4, 0, NO, 0, 79, -4, 0, NO, 0
80, -4, 0, NO, 0, 81, -4, 0, NO, 0, 82, -4, 0, NO, 0
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86, -4, 0, NO, 0, 87, -4, 0, NO, 0, 88, -4, 0, NO, 0
89, -4, 0, NO, 0, 90, -4, 0, NO, 0, 91, -4, 0, NO, 0
92, -4, 0, NO, 0, 93, -4, 0, NO, 0, 94, -4, 0, NO, 0
95, -4, 0, NO, 0, 96, -4, 0, NO, 0, 97, -4, 0, NO, 0
98, -4, 0, NO, 0, 99, -4, 0, NO, 0, 100, -4, 0, NO, 0
101, -4, 0, NO, 0, 102, -4, 0, NO, 0, 103, -4, 0, NO, 0
104, -4, 0, NO, 0, 105, -4, 0, NO, 0, 106, -4, 0, NO, 0
107, -4, 0, NO, 0, 108, -4, 0, NO, 0, 109, -4, 0, NO, 0
110, -4, 0, NO, 0, 111, -4, 0, NO, 0, 112, -4, 0, NO, 0
113, -4, 0, NO, 0, 114, -4, 0, NO, 0, 115, -4, 0, NO, 0
116, -4, 0, NO, 0, 117, -4, 0, NO, 0, 118, -4, 0, NO, 0
119, -4, 0, NO, 0, 120, -4, 0, NO, 0, 121, -4, 0, NO, 0
122, -4, 0, NO, 0, 123, -4, 0, NO, 0
NAME=LC03, CROSS, FASE 2, 0, 0, BOTH, 3, 2, NO, 3
74, -7, 0, NO, 0, 75, -7, 0, NO, 0, 76, -7, 0, NO, 0
77, -7, 0, NO, 0, 78, -7, 0, NO, 0, 79, -7, 0, NO, 0
80, -7, 0, NO, 0, 81, -7, 0, NO, 0, 82, -7, 0, NO, 0
83, -7, 0, NO, 0, 84, -7, 0, NO, 0, 85, -7, 0, NO, 0
86, -7, 0, NO, 0, 87, -7, 0, NO, 0, 88, -7, 0, NO, 0
89, -7, 0, NO, 0, 90, -7, 0, NO, 0, 91, -7, 0, NO, 0
92, -7, 0, NO, 0, 93, -7, 0, NO, 0, 94, -7, 0, NO, 0
95, -7, 0, NO, 0, 96, -7, 0, NO, 0, 97, -7, 0, NO, 0
98, -7, 0, NO, 0, 99, -7, 0, NO, 0, 100, -7, 0, NO, 0
101, -7, 0, NO, 0, 102, -7, 0, NO, 0, 103, -7, 0, NO, 0
104, -7, 0, NO, 0, 105, -7, 0, NO, 0, 106, -7, 0, NO, 0
107, -7, 0, NO, 0, 108, -7, 0, NO, 0, 109, -7, 0, NO, 0
110, -7, 0, NO, 0, 111, -7, 0, NO, 0, 112, -7, 0, NO, 0
113, -7, 0, NO, 0, 114, -7, 0, NO, 0, 115, -7, 0, NO, 0
116, -7, 0, NO, 0, 117, -7, 0, NO, 0, 118, -7, 0, NO, 0
119, -7, 0, NO, 0, 120, -7, 0, NO, 0, 121, -7, 0, NO, 0
122, -7, 0, NO, 0, 123, -7, 0, NO, 0

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*VEHICLE ; Vehicles
; if Moving Load Code is China
; NAME=NAME, 1, TYPE-NAME, CODE
; standard
; NAME=NAME, 2, LTYPE, [TRUCK/LANE] or [TRAIN/SUBWAY] or [CROWD]
; user: line 1
; LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2
; [TRUCK/LANE] : 1, P, Qm, Qq
; truck(JTG)
; [TRUCK/LANE] : 2, P, Qm, Qq
; lane load1
; [TRUCK/LANE] : 3, Qk, Pk1, L1, Pk2, L2
; lane load2
; [TRUCK/LANE] : 4, dW, dD
; crawler type
; [TRUCK/LANE] : 5
; GC type load
; [TRAIN/SUBWAY] : iTYPE, W1, D1, W2, D2
; train-typel,3

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# MIDAS

PROJECT TITLE :

	Company		Client	
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; [TRAIN/SUBWAY] : iTYPE, DD, FD, BD, MAINCOUNT
; train-type2
; [TRAIN/SUBWAY] : 4, P1, D1, P2, D2, P3, D3, P4, dD, Po, n, IFR
; subway
; [CROWD] : 1, dw1
; crowd-type1
; [CROWD] : 2, dw1, dL1, dw2, dL2, WIDTH
; crowd-type2
; if Moving Load Type is India
; NAME=NAME, 1, TYPE-NAME, CODE
; standard
; NAME=NAME, 2, bWTB, P, D, Pb, Db, dD1, dD2, NDIST
; user: line 1
; NAME=NAME, 2, bWTB, dD1, dD2, NDIST
; user: line 1
;     LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2
; if Moving Load Code is CANADA
; NAME=NAME, 1, TYPE-NAME, DLA, CODE, [DYNA]
; standard
; NAME=NAME, 2, bTRAIN, W(W1), PL(D1), PLM(W2), PLV(D2), NDIST, [DYNA]
; user: line 1
;     LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2
; [DYNA] : nDYNA, FACT1AXLE, FACT2AXLE, FACT3AXLE
; Dynamic Load Allowance
; if Moving Load Code is BS
; NAME=NAME, 1, TYPE-NAME, CODE, UNITNUM
; standard
; NAME=nLane, FACTOR1, FACTOR2, FACTOR3, FACTOR4, ADDDATA, AL, CA, LL
; HA, HA & HB, HA & HB(Auto)
; NAME=NAME, 2, iSTYPE, W1, W2, W3, L, Pa, Pb, D1, D2, d, UNITNUM
; user(BS 5400)
; NAME=NAME, 2, iSTYPE, [BD37/01-HA], [BS-DATA-LF]
; user(HA)
; NAME=NAME, 2, iSTYPE, [BS-DATA-HB]
; user(HB)
; NAME=NAME, 2, iSTYPE, [BD37/01-HA2], [BS-DATA-HB2], [BS-DATA-LF]
; user(HA&HB)
; NAME=NAME, 2, iSTYPE, [BD37/01-HA], [BD37/01-HB], [BS-DATA-LF]
; user(HA&HB(AUTO))
; NAME=NAME, 2, iSTYPE, W, L
; user(Pedestrian)
; NAME=NAME, 2, iSTYPE, V, AN, MINS, MAXS, P1, D1, P2, D2, ...
; user(Special Vehicle)
; [BS-DATA-HA] : W1, W2, W3, EXP, EXP2, L1, L2, Pa
; [BS-DATA-HA2] : W1, W2, W3, EXP, EXP2, L1, L2
; [BS-DATA-HB] : Pb, D1, D2, d, UNITNUM
; [BS-DATA-HB2] : Pb, D1, D2, d, dd, UNITNUM
; [BS-DATA-LF] : nLT, LF1, LF2, LF3, LF4
; if Moving Load Code is EUROCODE
; NAME=NAME, 1, iTYPE, TYPE-NAME, PSY1, PSY2, PHI, [AF7]
; standard (LM1, FLM1)
; NAME=NAME, 1, iTYPE, TYPE-NAME, bDF, bU, PHI, PSY, ADJ, IN
; standard (others)
; NAME=NAME, 2, 1, [AF7]
; user(Type 1)
;     [LOAD7], D, PHI, TPSY, UPSY
; user(Type 1): line 2
; NAME=NAME, 2, 2, ALP, TPSY, W, BET, LPSY, P1, D1, P2, D2, ...
; user(Type 2)
; NAME=NAME, 2, 3

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; user(Type 3)
; [LOADCASE1]
; user(Type 3): line 2
; [LOADCASE2]
; user(Type 3): line 3
; [LOADCASE3]
; user(Type 3): line 4
; NAME=NAME, 2, 4, WS, V, AN, MINS, MAXS, DYF, UI, F, P1, D1, P2, D2, ...
; user(Type 4)
; NAME=NAME, 2, 5, INT, bPHI1, PHI1, bPHI2, PHI2
; user(Type 5)
; [VEHICLE1]
; user(Type 5): line 2
; [VEHICLE2]
; user(Type 5): line 3
; [VEHICLE3]
; user(Type 5): line 4
; [AF7] : TF1, TF2, TF3, UF1, UF2, UF3, UF4
; adjustment factor
; [LOAD7] : TL1, TL2, TL3, UL1, UL2, UL3, UL4
; tandem/udl loads
; [LOADCASE] : bUSE, N, bDF, bUI, PHI, P1, L1, P2, L2, ...
; load case
; [VEHICLE] : bUSE, N, P1, L1, P2, L2, ...
; vehicle
; if Moving Load Code is RUSSIA
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; standard (SK)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; standard (SK FATIGUE)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, dDYNAFAC_UDL
;
; bFATI, nLOADFAC, dLOADFAC, dLOADFAC_UDL, s1[3], s1_UDL[3]
; standard (AK)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC,
;
; bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; standard (N14)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC,
;
; bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; standard (N11)
; NAME=NAME, 1, iTYPE, nDYNAFAC, dDYNAFAC, dEMPTYCAR
;
; bFATI, nLOADFAC, dLOADFAC,
; standard (SUBWAY TRAINS)
; NAME=NAME, 1, iTYPE, VARIABLE, nDYNAFAC, dDYNAFAC, dEMPTYCAR
;
; bFATI, nLOADFAC, dLOADFAC,
; standard (TRAMCARS)
; NAME=NAME, 1, iTYPE, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; standard (NK-80)
; NAME=NAME, 1, iTYPE, W, D, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; standard (NG-60)
; NAME=NAME, 1, iTYPE, BRIDGETYPE, W, bFATI, nLOADFAC, dLOADFAC
; standard (UNIFORM LOAD)
; NAME=NAME, 1, iTYPE, BRIDGETYPE, W, bFATI, nLOADFAC, dLOADFAC
; standard (UNIFORM LOAD (W/O OTHER LOADS))
; NAME=NAME, 1, iTYPE, BRIDGETYPE, P
; standard (CONCENTRATED LOAD (W/O OTHER LOADS))
; NAME=NAME, 2, iTYPE, W, nDYNAFAC, dDYNAFAC, dDYNAFAC_UDL
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;          bFATI, nLOADFAC, dLOADFAC, dLOADFAC_UDL, s1[3], s1_UDL[3]
; user (Type 1)
; NAME=NAME, 2, iTYPE, nDYNAFAC, dDYNAFAC, dEMPTYCAR

;          bFATI, nLOADFAC, dLOADFAC,
; user (Type 2)
; NAME=NAME, 2, iTYPE, Variable, nDYNAFAC, dDYNAFAC, dEMPTYCAR

;          bFATI, nLOADFAC, dLOADFAC,
; user (Type 3)
; NAME=NAME, 2, iTYPE, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC

;          bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; user (Type 4)
; NAME=NAME, 2, iTYPE, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC

;          bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; user (Type 5)
; NAME=NAME, 2, iTYPE, W, D, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; user (Type 6)
; NAME=NAME, 2, iTYPE, P, W, bFATI, nLOADFAC, dLOADFAC
; user (Type 7)
; if Moving Load Code is KSCE-LSD15
; NAME=NAME, 1, TYPE-NAME, nLANETYPE, dDYNAFAC, CODE, nLANELOAD, L, CONVERTDIST
; standard
; NAME=NAME, 2, 8, L1, W1, W2, EXP, dDYNAFAC, nLANELOAD, L, CONVERTDIST
; user: line 1 (Type 1)
; NAME=NAME, 2, 1, W1, W2, D1, D2, 0, 0, 0, NO
; user: line 1 (Type 2)
; NAME=NAME, 2, 6, LOADNUM, DIST, W, L, 0, 0, 0, NO
; user: line 1 (Type 3)
;     LOAD1, DIST1, (DIST2_1), LOAD2, DIST2, (DIST2_2), ...
; user: from line 2
; if Moving Load Code is South Africa
; NAME=NAME, 1, TYPE-NAME, CODE, bINCREL, dINCREL
; standard NA
; NAME=NAME, 1, TYPE-NAME, CODE, UNITNUM
; standard NB
; NAME=NAME, 1, TYPE-NAME, CODE, OPPOSITE
; standard NC
; NAME=NAME, 2, TYPE, W1, L, W2, W3, PA, bINCREL, dINCREL
; user NA
; NAME=NAME, 2, TYPE, PB, UNITNUM, DELTA, D1, D2
; user NB
; NAME=NAME, 2, TYPE, P, OPPOSITE, NUM1, NUM2, NUM3 [DIST1], [DIST2], [DIST3]
; user NC
; if Load Type is Permit Truck
; NAME=NAME, 3, AXLE-TYPE-NUM, IMP-FACTOR
; user(Permit Truck)
;     AXLE-TYPE-NAME1, bEDWL1, bSV1, P1, D1, P2, D2, ..., Pn, Dn
; user(from line 2)
;     ...
;     AXLE-TYPE-NAMEn, bEDWLn, bSVn, P1, D1, P2, D2, ..., Pn, Dn
;     AXLE-TYPE1, SPACING1, bVS1, AXLE-TYPE2, SPACING2, bVS2 ...
; line 2+AXLE-TYPE-NUM
; if Moving Load Code is not one of those specified above.
; NAME=NAME, 1, TYPE-NAME, DLA, CODE
; standard
; NAME=NAME, 2, bTRAIN, W(W1), PL(D1), PLM(W2), PLV(D2), NDIST
; user: line 1
;     LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2

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NAME=Q1A_q1a, 2, 1, 1, 0, 0, 1, 0, 0, 0
      300, 0, 0, 9, 0, 0, 0, 1.2, 1, 1, 1
NAME=Q1B_q1b, 2, 1, 1, 0, 0, 1, 0, 0, 0
      200, 0, 0, 2.5, 0, 0, 0, 1.2, 1, 1, 1
NAME=Q1C_q1c, 2, 1, 1, 0, 0, 1, 0, 0, 0
      100, 0, 0, 2.5, 0, 0, 0, 1.2, 1, 1, 1
  
```

```

*MVLDCase (EURO) ; Moving Load Cases
; NAME=NAME, bOPTIM, iTYPE, DESC, VHL1, VHL2, bLEAD, SERIAL
;                                     ; type 1, 3
;      nType, SLN1, SLN2, SLN3, ...
;                                     ; line 2 (nType=1)
;      nType, SRA1, SRA2, SRA3, ...
;                                     ; line 2/3 (nType=2)
;      nType, FLN1, FLN2, FLN3, ...
;                                     ; line 4 (nType=3)
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, SERIAL
;                                     ; type 2
;      [SUB1]
;                                     ; line 2
;      ...
;                                     ; ...
;      [SUBn]
;                                     ; line n+1
; [SUB] : VCLA, SCA, MIN, MAX, SLN1, SLN2, ...
;                                     ; sub-loadcase
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, SERIAL
;                                     ; type 4
;      0, SLN1, SLN2, SLN3, ...
;                                     ; line 2
;      1, SRA1, SRA2, SRA3, ...
;                                     ; line 3
;      2, STRADD1, STRADD2, STRADD3, ...
;                                     ; line 4
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, SF1, SF2, SF3, bPSI1, MULF1, MULF2, MULF3, SERIAL
;                                     ; type 5
;      [SUB1]
;                                     ; line 2
;      ...
;                                     ; ...
;      [SUBn]
;                                     ; line n+1
; //////////////////////////////////////
; //////////////////////////////////////
; Moving Load Optimization
;
; NAME=NAME, bOPTIM, iTYPE, DESC, VHL1, VHL2, bLEAD, MINVEHL, LANEOP, LOADEDLANE, SERIAL
;                                     ; type 1, 3
;      nType, SRA1, SRA2, SRA3, ...
;                                     ; line 2 (nType=2)
;      nType, FLN1, FLN2, FLN3, ...
;                                     ; line 3 (nType=3)
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, MINVEHL, LANEOP, MIN, MAX, SERIAL
;                                     ; type 2
;      [ASSGNVEHL1]
;                                     ; line 2
;      ...
;                                     ; ...
;      [ASSGNVEHLn]
;                                     ; line n+1
; [ASSGNVEHL] : VC, VCLA, SCA ...
;                                     ; Assignment-Vehicle
  
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; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, MINVEHL, LANEOP, LOADEDLANE, SERIAL
; type 4
; 1, SRA1, SRA2, SRA3, ...
; line 3
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, SF1, SF2, SF3, bPSI1, MULF1, MULF2, MULF3, MIN
VEHL, LANEOP, MIN, MAX, SERIAL ; type 5
; [ASSGNVEHL1]
; line 2
; ...
; ...
; [ASSGNVEHLn]
; line n+1
NAME=LM01, NO, 1, , Q1A_q1a, , YES, 1
1, LC01
NAME=LM02, NO, 1, , Q1B_q1b, , YES, 2
1, LC02
NAME=LM03, NO, 1, , Q1C_q1c, , YES, 3
1, LC03

*CPOSECT4CS ; Composite Section for Construction Stage
; SEC=SEC, ASTAGE, TYPE, bTAP //
line 1
; [PART-INFO]-1 //
from line 2
; ...
; [PART-INFO]-n
; [PART-INFO] : [COMMON], [SCALE], H, VS //
TYPE=A,B,NORMAL
; [COMMON], [SCALE], CY, CZ, [STIFF], H, VS //
TYPE=USER
; [COMMON], [SCALE], CYI, CZI, CYJ, CZJ, [STIFF]-I, [STIFF]-J, H, VS //
TYPE=USER, bTAP=YES
; [COMMON]: PART, MTYPE, MAT, CSTAGE, AGE
; [SCALE] : AREA, ASY, ASZ, IXX, IYY, IZZ, WAREA
; [STIFF] : AREA, ASY, ASZ, IXX, IYY, CYP, CYM, CZP, CZM, QYB, QZB, \
; X1, X2, X3, X4, Y1, Y2, Y3, Y4
SEC=7, FASE 1, NORMAL, NO
1, ELEM, , , 0, 1, 1, 1, 1, 1, 1, 1, 0.0235754, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 1, 0.227273, 0
SEC=9, FASE 1, NORMAL, NO
1, ELEM, , , 0, 1, 1, 1, 1, 1, 1, 1, 0.0322938, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 1, 0.227273, 0
SEC=8, FASE 1, NORMAL, NO
1, ELEM, , , 0, 1, 1, 1, 1, 1, 1, 1, 0.0268557, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 1, 0.227273, 0
SEC=3, FASE 1, NORMAL, NO
1, ELEM, , , 0, 1, 1, 1, 1, 1, 1, 1, 0.0187086, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 1, 0.234568, 0


*STAGE-COLOR ; Diagram Color for Construction Stage
; STAGENAME, iR(COLOR), iG(COLOR), iB(COLOR)
FASE 1, 210, 210, 210
FASE 2, 0, 128, 128
FASE 3, 0, 192, 192

*LOADCOMB ; Combinations
; NAME=NAME, KIND, ACTIVE, bES, iTYPE, DESC, iSERV-TYPE, nLCOMTYPE ; line 1
; ANAL1, LCNAME1, FACT1, ... ; from line 2
NAME=SLU1, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35

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NAME=SLU2, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU3, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU4, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU5, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU6, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU7, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU8, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU9, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU10, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU11, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU12, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU13, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU14, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU15, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU16, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU17, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU18, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU19, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9

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ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU20, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU21, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU22, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU23, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU24, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLV1, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV2, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV3, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV4, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV5, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV6, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV7, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV8, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV9, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV10, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV11, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV12, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV13, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV14, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV15, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV16, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV17, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV18, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV19, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV20, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, -0.3

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CS, Summation, 1
NAME=SLV21, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV22, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV23, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV24, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -1
CS, Summation, 1
NAME=SLV25, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV26, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV27, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV28, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV29, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV30, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV31, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV32, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV33, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV34, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV35, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV36, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV37, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV38, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV39, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV40, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV41, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV42, GEN, ACTIVE, 0, 0, , 0, 0

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ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV43, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV44, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV45, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV46, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV47, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV48, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV49, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV50, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV51, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV52, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV53, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV54, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV55, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV56, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=RARA_1, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_2, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_3, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_4, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_5, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_6, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1

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NAME=RARA_7, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1, ST, TERMICA GRADIENTE +, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_8, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1, ST, TERMICA GRADIENTE -, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_9, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_10, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_11, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_12, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_13, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_14, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_15, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_16, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=FREQ_1, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_2, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_3, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_4, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_5, GEN, ACTIVE, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
  CS, Summation, 1
NAME=FREQ_6, GEN, ACTIVE, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
  CS, Summation, 1
NAME=FREQ_7, GEN, ACTIVE, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
  CS, Summation, 1
NAME=FREQ_8, GEN, ACTIVE, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
  CS, Summation, 1
NAME=FREQ_9, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_10, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_11, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5

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ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_12, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_13, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_14, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_15, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_16, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=QP_1, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, CS, Summation, 1
NAME=QP_2, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, CS, Summation, 1
NAME=QP_3, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, CS, Summation, 1
NAME=QP_4, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, CS, Summation, 1
NAME=SLD1, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD2, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=SLD3, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD4, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=ENVESLU-SLV, GEN, ACTIVE, 0, 1, , 0, 0
CB, SLU1, 1, CB, SLU2, 1, CB, SLU3, 1, CB, SLU4, 1, CB, SLU5, 1
CB, SLU6, 1, CB, SLU7, 1, CB, SLU8, 1, CB, SLU9, 1, CB, SLU10, 1
CB, SLU11, 1, CB, SLU12, 1, CB, SLU13, 1, CB, SLU14, 1, CB, SLU15, 1
CB, SLU16, 1, CB, SLU17, 1, CB, SLU18, 1, CB, SLU19, 1, CB, SLU20, 1
CB, SLU21, 1, CB, SLU22, 1, CB, SLU23, 1, CB, SLU24, 1, CB, SLV1, 1
CB, SLV2, 1, CB, SLV3, 1, CB, SLV4, 1, CB, SLV5, 1, CB, SLV6, 1
CB, SLV7, 1, CB, SLV8, 1, CB, SLV9, 1, CB, SLV10, 1, CB, SLV11, 1
CB, SLV12, 1, CB, SLV13, 1, CB, SLV14, 1, CB, SLV15, 1, CB, SLV16, 1
CB, SLV17, 1, CB, SLV18, 1, CB, SLV19, 1, CB, SLV20, 1, CB, SLV21, 1
CB, SLV22, 1, CB, SLV23, 1, CB, SLV24, 1, CB, SLV25, 1, CB, SLV26, 1
CB, SLV27, 1, CB, SLV28, 1, CB, SLV29, 1, CB, SLV30, 1, CB, SLV31, 1
CB, SLV32, 1, CB, SLV33, 1, CB, SLV34, 1, CB, SLV35, 1, CB, SLV36, 1
CB, SLV37, 1, CB, SLV38, 1, CB, SLV39, 1, CB, SLV40, 1, CB, SLV41, 1
CB, SLV42, 1, CB, SLV43, 1, CB, SLV44, 1, CB, SLV45, 1, CB, SLV46, 1
CB, SLV47, 1, CB, SLV48, 1, CB, SLV49, 1, CB, SLV50, 1, CB, SLV51, 1
CB, SLV52, 1, CB, SLV53, 1, CB, SLV54, 1, CB, SLV55, 1, CB, SLV56, 1
NAME=ENVE-RARA, GEN, ACTIVE, 0, 1, , 0, 0
CB, RARA_1, 1, CB, RARA_2, 1, CB, RARA_3, 1, CB, RARA_4, 1
CB, RARA_5, 1, CB, RARA_6, 1, CB, RARA_7, 1, CB, RARA_8, 1
CB, RARA_9, 1, CB, RARA_10, 1, CB, RARA_11, 1, CB, RARA_12, 1
CB, RARA_13, 1, CB, RARA_14, 1, CB, RARA_15, 1, CB, RARA_16, 1
NAME=ENVE-FREQ, GEN, ACTIVE, 0, 1, , 0, 0
CB, FREQ_1, 1, CB, FREQ_2, 1, CB, FREQ_3, 1, CB, FREQ_4, 1
CB, FREQ_5, 1, CB, FREQ_6, 1, CB, FREQ_7, 1, CB, FREQ_8, 1
CB, FREQ_9, 1, CB, FREQ_10, 1, CB, FREQ_11, 1, CB, FREQ_12, 1
CB, FREQ_13, 1, CB, FREQ_14, 1, CB, FREQ_15, 1, CB, FREQ_16, 1
NAME=ENVE-QP, GEN, ACTIVE, 0, 1, , 0, 0
CB, QP_1, 1, CB, QP_2, 1, CB, QP_3, 1, CB, QP_4, 1
NAME=ENVE-SLD, GEN, ACTIVE, 0, 1, , 0, 0

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CB, SLD1, 1, CB, SLD2, 1, CB, SLD3, 1, CB, SLD4, 1
NAME=LCB126, GEN, ACTIVE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLC X, GEN, ACTIVE, 0, 0, , 0, 0
  RS, CX, 1, RS, CY, 0.3
NAME=SLC_Y, GEN, ACTIVE, 0, 0, , 0, 0
  RS, CY, 1, RS, CX, 0.3
NAME=SLU1, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU2, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU3, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU4, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU5, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU6, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU7, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU8, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
  MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU9, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU10, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU11, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU12, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU13, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU14, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU15, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35

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NAME=SLU16, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU17, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU18, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU19, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU20, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU21, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU22, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU23, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLU24, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
  ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
  CS, Summation, 1.35
NAME=SLV1, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV2, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV3, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV4, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV5, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV6, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV7, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV8, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV9, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV10, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV11, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV12, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV13, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV14, STEEL, STRENGTH, 0, 0, , 0, 0
  
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ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV15, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV16, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV17, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV18, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV19, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV20, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, -0.3
CS, Summation, 1
NAME=SLV21, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV22, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV23, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV24, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -1
CS, Summation, 1
NAME=SLV25, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV26, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV27, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV28, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV29, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV30, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV31, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV32, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV33, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV34, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV35, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV36, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV37, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1

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NAME=SLV38, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
  CS, Summation, 1
NAME=SLV39, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
  CS, Summation, 1
NAME=SLV40, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
  CS, Summation, 1
NAME=SLV41, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
  CS, Summation, 1
NAME=SLV42, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
  CS, Summation, 1
NAME=SLV43, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
  CS, Summation, 1
NAME=SLV44, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
  CS, Summation, 1
NAME=SLV45, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV46, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV47, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV48, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV49, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV50, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV51, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV52, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV53, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV54, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV55, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
  CS, Summation, 1
NAME=SLV56, STEEL, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
  CS, Summation, 1
NAME=RARA_1, STEEL, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1, ST, TERMICA UNIFORME +, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_2, STEEL, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1, ST, TERMICA UNIFORME -, 0.6
    
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ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_3, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_4, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_5, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_6, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_7, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_8, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_9, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_10, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_11, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_12, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_13, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_14, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_15, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_16, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=FREQ_1, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_2, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_3, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_4, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_5, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_6, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_7, STEEL, SERVICE, 0, 0, , 0, 0

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ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_8, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_9, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_10, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_11, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_12, STEEL, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_13, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_14, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_15, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_16, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=QP_1, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, CS, Summation, 1
NAME=QP_2, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, CS, Summation, 1
NAME=QP_3, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, CS, Summation, 1
NAME=QP_4, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, CS, Summation, 1
NAME=SLD1, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD2, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=SLD3, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD4, STEEL, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=ENVESLU-SLV, STEEL, STRENGTH, 0, 1, , 0, 0
CBS, SLU1, 1, CBS, SLU2, 1, CBS, SLU3, 1, CBS, SLU4, 1, CBS, SLU5, 1
CBS, SLU6, 1, CBS, SLU7, 1, CBS, SLU8, 1, CBS, SLU9, 1, CBS, SLU10, 1
CBS, SLU11, 1, CBS, SLU12, 1, CBS, SLU13, 1, CBS, SLU14, 1
CBS, SLU15, 1, CBS, SLU16, 1, CBS, SLU17, 1, CBS, SLU18, 1
CBS, SLU19, 1, CBS, SLU20, 1, CBS, SLU21, 1, CBS, SLU22, 1
CBS, SLU23, 1, CBS, SLU24, 1, CBS, SLV1, 1, CBS, SLV2, 1, CBS, SLV3, 1
CBS, SLV4, 1, CBS, SLV5, 1, CBS, SLV6, 1, CBS, SLV7, 1, CBS, SLV8, 1
CBS, SLV9, 1, CBS, SLV10, 1, CBS, SLV11, 1, CBS, SLV12, 1
CBS, SLV13, 1, CBS, SLV14, 1, CBS, SLV15, 1, CBS, SLV16, 1
CBS, SLV17, 1, CBS, SLV18, 1, CBS, SLV19, 1, CBS, SLV20, 1
CBS, SLV21, 1, CBS, SLV22, 1, CBS, SLV23, 1, CBS, SLV24, 1
CBS, SLV25, 1, CBS, SLV26, 1, CBS, SLV27, 1, CBS, SLV28, 1
CBS, SLV29, 1, CBS, SLV30, 1, CBS, SLV31, 1, CBS, SLV32, 1
CBS, SLV33, 1, CBS, SLV34, 1, CBS, SLV35, 1, CBS, SLV36, 1
CBS, SLV37, 1, CBS, SLV38, 1, CBS, SLV39, 1, CBS, SLV40, 1
CBS, SLV41, 1, CBS, SLV42, 1, CBS, SLV43, 1, CBS, SLV44, 1
    
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CBS, SLV45, 1, CBS, SLV46, 1, CBS, SLV47, 1, CBS, SLV48, 1
CBS, SLV49, 1, CBS, SLV50, 1, CBS, SLV51, 1, CBS, SLV52, 1
CBS, SLV53, 1, CBS, SLV54, 1, CBS, SLV55, 1, CBS, SLV56, 1
NAME=ENVE-RARA, STEEL, SERVICE, 0, 1, , 0, 0
CBS, RARA_1, 1, CBS, RARA_2, 1, CBS, RARA_3, 1, CBS, RARA_4, 1
CBS, RARA_5, 1, CBS, RARA_6, 1, CBS, RARA_7, 1, CBS, RARA_8, 1
CBS, RARA_9, 1, CBS, RARA_10, 1, CBS, RARA_11, 1, CBS, RARA_12, 1
CBS, RARA_13, 1, CBS, RARA_14, 1, CBS, RARA_15, 1, CBS, RARA_16, 1
NAME=ENVE-FREQ, STEEL, SERVICE, 0, 1, , 0, 0
CBS, FREQ_1, 1, CBS, FREQ_2, 1, CBS, FREQ_3, 1, CBS, FREQ_4, 1
CBS, FREQ_5, 1, CBS, FREQ_6, 1, CBS, FREQ_7, 1, CBS, FREQ_8, 1
CBS, FREQ_9, 1, CBS, FREQ_10, 1, CBS, FREQ_11, 1, CBS, FREQ_12, 1
CBS, FREQ_13, 1, CBS, FREQ_14, 1, CBS, FREQ_15, 1, CBS, FREQ_16, 1
NAME=ENVE-QP, STEEL, SERVICE, 0, 1, , 0, 0
CBS, QP_1, 1, CBS, QP_2, 1, CBS, QP_3, 1, CBS, QP_4, 1
NAME=ENVE-SLD, STEEL, SERVICE, 0, 1, , 0, 0
CBS, SLD1, 1, CBS, SLD2, 1, CBS, SLD3, 1, CBS, SLD4, 1
NAME=SLU1, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU2, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU3, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU4, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU5, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU6, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU7, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU8, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU9, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU10, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU11, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU12, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU13, STLCOMP, STRENGTH, 0, 0, , 0, 0
    
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ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU14, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU15, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU16, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU17, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU18, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU19, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU20, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU21, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU22, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU23, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU24, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, VENTO PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLV1, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV2, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV3, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV4, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV5, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV6, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV7, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV8, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV9, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV10, STLCOMP, STRENGTH, 0, 0, , 0, 0

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ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV11, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV12, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV13, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV14, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV15, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV16, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV17, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV18, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV19, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV20, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, -0.3
CS, Summation, 1
NAME=SLV21, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV22, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV23, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV24, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -1
CS, Summation, 1
NAME=SLV25, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV26, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV27, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV28, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV29, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV30, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV31, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV32, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV33, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV34, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV35, STLCOMP, STRENGTH, 0, 0, , 0, 0

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ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV36, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV37, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV38, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV39, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV40, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV41, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV42, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV43, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV44, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV45, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV46, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV47, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV48, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV49, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV50, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV51, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV52, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV53, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV54, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV55, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1

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NAME=SLV56, STLCOMP, STRENGTH, 0, 0, , 0, 0
  ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
  CS, Summation, 1
NAME=RARA_1, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1, ST, TERMICA UNIFORME +, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_2, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1, ST, TERMICA UNIFORME -, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_3, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1, ST, TERMICA GRADIENTE +, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_4, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 1, ST, TERMICA GRADIENTE -, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_5, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1, ST, TERMICA UNIFORME +, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_6, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1, ST, TERMICA UNIFORME -, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_7, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1, ST, TERMICA GRADIENTE +, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_8, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 1, ST, TERMICA GRADIENTE -, 0.6
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_9, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_10, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_11, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_12, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_13, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_14, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_15, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_16, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE CARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
  ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=FREQ_1, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_2, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_3, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
  ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_4, STLCOMP, SERVICE, 0, 0, , 0, 0
  ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5

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ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_5, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_6, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_7, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_8, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_9, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_10, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_11, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_12, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_13, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_14, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_15, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_16, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
MV, LM02, 0.75, CS, Summation, 1
NAME=QP_1, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, CS, Summation, 1
NAME=QP_2, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, CS, Summation, 1
NAME=QP_3, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, CS, Summation, 1
NAME=QP_4, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, CS, Summation, 1
NAME=SLD1, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD2, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=SLD3, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD4, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=ENVESLU-SLV, STLCOMP, STRENGTH, 0, 1, , 0, 0
CBSC, SLU1, 1, CBSC, SLU2, 1, CBSC, SLU3, 1, CBSC, SLU4, 1
CBSC, SLU5, 1, CBSC, SLU6, 1, CBSC, SLU7, 1, CBSC, SLU8, 1
CBSC, SLU9, 1, CBSC, SLU10, 1, CBSC, SLU11, 1, CBSC, SLU12, 1
CBSC, SLU13, 1, CBSC, SLU14, 1, CBSC, SLU15, 1, CBSC, SLU16, 1
CBSC, SLU17, 1, CBSC, SLU18, 1, CBSC, SLU19, 1, CBSC, SLU20, 1
CBSC, SLU21, 1, CBSC, SLU22, 1, CBSC, SLU23, 1, CBSC, SLU24, 1
CBSC, SLV1, 1, CBSC, SLV2, 1, CBSC, SLV3, 1, CBSC, SLV4, 1
CBSC, SLV5, 1, CBSC, SLV6, 1, CBSC, SLV7, 1, CBSC, SLV8, 1

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CBSC, SLV9, 1, CBSC, SLV10, 1, CBSC, SLV11, 1, CBSC, SLV12, 1
CBSC, SLV13, 1, CBSC, SLV14, 1, CBSC, SLV15, 1, CBSC, SLV16, 1
CBSC, SLV17, 1, CBSC, SLV18, 1, CBSC, SLV19, 1, CBSC, SLV20, 1
CBSC, SLV21, 1, CBSC, SLV22, 1, CBSC, SLV23, 1, CBSC, SLV24, 1
CBSC, SLV25, 1, CBSC, SLV26, 1, CBSC, SLV27, 1, CBSC, SLV28, 1
CBSC, SLV29, 1, CBSC, SLV30, 1, CBSC, SLV31, 1, CBSC, SLV32, 1
CBSC, SLV33, 1, CBSC, SLV34, 1, CBSC, SLV35, 1, CBSC, SLV36, 1
CBSC, SLV37, 1, CBSC, SLV38, 1, CBSC, SLV39, 1, CBSC, SLV40, 1
CBSC, SLV41, 1, CBSC, SLV42, 1, CBSC, SLV43, 1, CBSC, SLV44, 1
CBSC, SLV45, 1, CBSC, SLV46, 1, CBSC, SLV47, 1, CBSC, SLV48, 1
CBSC, SLV49, 1, CBSC, SLV50, 1, CBSC, SLV51, 1, CBSC, SLV52, 1
CBSC, SLV53, 1, CBSC, SLV54, 1, CBSC, SLV55, 1, CBSC, SLV56, 1
NAME=ENVE-RARA, STLCOMP, SERVICE, 0, 1, , 0, 0
CBSC, RARA_1, 1, CBSC, RARA_2, 1, CBSC, RARA_3, 1, CBSC, RARA_4, 1
CBSC, RARA_5, 1, CBSC, RARA_6, 1, CBSC, RARA_7, 1, CBSC, RARA_8, 1
CBSC, RARA_9, 1, CBSC, RARA_10, 1, CBSC, RARA_11, 1, CBSC, RARA_12, 1
CBSC, RARA_13, 1, CBSC, RARA_14, 1, CBSC, RARA_15, 1, CBSC, RARA_16, 1
NAME=ENVE-FREQ, STLCOMP, SERVICE, 0, 1, , 0, 0
CBSC, FREQ_1, 1, CBSC, FREQ_2, 1, CBSC, FREQ_3, 1, CBSC, FREQ_4, 1
CBSC, FREQ_5, 1, CBSC, FREQ_6, 1, CBSC, FREQ_7, 1, CBSC, FREQ_8, 1
CBSC, FREQ_9, 1, CBSC, FREQ_10, 1, CBSC, FREQ_11, 1, CBSC, FREQ_12, 1
CBSC, FREQ_13, 1, CBSC, FREQ_14, 1, CBSC, FREQ_15, 1, CBSC, FREQ_16, 1
NAME=ENVE-QP, STLCOMP, SERVICE, 0, 1, , 0, 0
CBSC, QP_1, 1, CBSC, QP_2, 1, CBSC, QP_3, 1, CBSC, QP_4, 1
NAME=ENVE-SLD, STLCOMP, SERVICE, 0, 1, , 0, 0
CBSC, SLD1, 1, CBSC, SLD2, 1, CBSC, SLD3, 1, CBSC, SLD4, 1
    
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\*LC-COLOR ; Diagram Color for Load Case

; ANAL, LCNAME, iR1(ALL), iG1(ALL), iB1(ALL), iR2(MIN), iG2(MIN), iB2(MIN), iR3(MAX), iG2(MAX), iB2(MAX)

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CS, Dead Load, 255, 87, 87, 255, 0, 192, 0, 128, 192
CS, Erection Load 1, 0, 128, 57, 255, 87, 87, 160, 192, 255
CS, Erection Load 2, 0, 157, 192, 212, 160, 255, 192, 72, 0
CS, Erection Load 3, 212, 160, 255, 128, 192, 0, 192, 128, 0
CS, Tendon Primary, 192, 0, 192, 192, 72, 0, 128, 192, 0
CS, Tendon Secondary, 163, 160, 255, 0, 128, 192, 255, 255, 255
CS, Creep Primary, 192, 72, 0, 212, 160, 255, 85, 0, 192
CS, Creep Secondary, 0, 192, 192, 192, 128, 0, 0, 128, 57
CS, Shrinkage Primary, 128, 192, 0, 192, 192, 192, 210, 210, 210
CS, Shrinkage Secondary, 0, 192, 192, 255, 0, 192, 255, 255, 87
CS, Summation, 192, 128, 0, 85, 192, 0, 0, 192, 128
ST, PESO PROPRIO ACCIAIO, 192, 72, 0, 0, 128, 255, 255, 160, 255
ST, PESO PROPRIO SOLETTA, 192, 192, 0, 212, 160, 255, 0, 192, 128
ST, PERMANENTE PORTATO, 255, 87, 87, 212, 160, 255, 85, 192, 0
ST, VENTO PONTE SCARICO, 160, 192, 255, 255, 0, 128, 255, 192, 160
ST, VENTO PONTE CARICO, 148, 87, 255, 255, 0, 128, 255, 0, 128
ST, TERMICA UNIFORME +, 255, 255, 87, 160, 192, 255, 146, 0, 255
ST, TERMICA UNIFORME -, 192, 0, 128, 148, 87, 255, 0, 128, 128
ST, TERMICA GRADIENTE +, 255, 87, 87, 255, 160, 255, 255, 192, 87
ST, TERMICA GRADIENTE -, 192, 0, 192, 255, 128, 0, 160, 192, 255
ST, FRENATURA, 0, 157, 192, 163, 160, 255, 255, 255, 255
MV, LM01, 192, 192, 0, 128, 192, 0, 255, 0, 128
MV, LM02, 160, 192, 255, 146, 0, 255, 192, 72, 0
MV, LM03, 85, 192, 0, 128, 192, 0, 255, 192, 160
RS, EX, 255, 0, 192, 255, 0, 192, 0, 128, 255
RS, EY, 128, 192, 0, 0, 128, 128, 212, 160, 255
RS, EZ, 85, 0, 192, 160, 255, 255, 128, 192, 0
RS, DX, 210, 210, 210, 148, 87, 255, 148, 87, 255
RS, CX, 192, 128, 0, 255, 160, 255, 210, 210, 210
CB, SLU1, 0, 128, 57, 0, 157, 192, 163, 160, 255
CB, SLU2, 85, 0, 192, 93, 255, 87, 85, 0, 192
CB, SLU3, 0, 128, 128, 163, 255, 160, 0, 128, 255
    
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CB, SLU4, 0, 128, 57, 0, 128, 128, 255, 0, 128  
 CB, SLU5, 255, 255, 87, 255, 0, 192, 0, 157, 192  
 CB, SLU6, 0, 128, 192, 255, 255, 255, 212, 160, 255  
 CB, SLU7, 255, 255, 255, 163, 160, 255, 255, 128, 0  
 CB, SLU8, 0, 128, 255, 85, 0, 192, 255, 192, 160  
 CB, SLU9, 148, 87, 255, 0, 128, 192, 0, 128, 128  
 CB, SLU10, 255, 128, 0, 0, 128, 192, 0, 128, 57  
 CB, SLU11, 78, 0, 255, 148, 87, 255, 93, 255, 87  
 CB, SLU12, 192, 0, 128, 0, 128, 255, 255, 255, 87  
 CB, SLU13, 0, 128, 57, 0, 192, 192, 0, 128, 255  
 CB, SLU14, 192, 192, 192, 255, 255, 255, 192, 128, 0  
 CB, SLU15, 78, 0, 255, 192, 0, 128, 0, 128, 192  
 CB, SLU16, 93, 255, 87, 255, 255, 255, 85, 192, 0  
 CB, SLU17, 255, 255, 87, 78, 0, 255, 192, 192, 0  
 CB, SLU18, 163, 160, 255, 0, 128, 255, 0, 128, 255  
 CB, SLU19, 255, 255, 255, 255, 160, 255, 192, 192, 0  
 CB, SLU20, 255, 255, 87, 212, 160, 255, 0, 192, 128  
 CB, SLU21, 0, 157, 192, 255, 255, 255, 93, 255, 87  
 CB, SLU22, 255, 192, 87, 160, 192, 255, 255, 255, 255  
 CB, SLU23, 146, 0, 255, 255, 0, 192, 163, 160, 255  
 CB, SLU24, 255, 255, 87, 0, 192, 128, 0, 192, 128  
 CB, SLV1, 212, 160, 255, 255, 128, 0, 0, 192, 128  
 CB, SLV2, 210, 210, 210, 255, 255, 255, 0, 128, 255  
 CB, SLV3, 0, 128, 57, 0, 128, 57, 163, 255, 160  
 CB, SLV4, 255, 192, 87, 128, 192, 0, 148, 87, 255  
 CB, SLV5, 255, 255, 87, 128, 192, 0, 0, 192, 128  
 CB, SLV6, 255, 255, 255, 146, 0, 255, 85, 0, 192  
 CB, SLV7, 192, 0, 128, 210, 210, 210, 255, 128, 0  
 CB, SLV8, 85, 0, 192, 163, 160, 255, 160, 192, 255  
 CB, SLV9, 0, 128, 255, 255, 255, 255, 0, 128, 128  
 CB, SLV10, 192, 72, 0, 0, 128, 192, 212, 160, 255  
 CB, SLV11, 255, 192, 160, 93, 255, 87, 148, 87, 255  
 CB, SLV12, 192, 192, 192, 255, 0, 192, 255, 160, 255  
 CB, SLV13, 255, 160, 255, 160, 192, 255, 163, 255, 160  
 CB, SLV14, 192, 128, 0, 0, 128, 57, 93, 255, 87  
 CB, SLV15, 85, 192, 0, 255, 0, 192, 255, 255, 87  
 CB, SLV16, 160, 192, 255, 255, 192, 160, 85, 0, 192  
 CB, SLV17, 210, 210, 210, 0, 128, 255, 255, 192, 87  
 CB, SLV18, 255, 0, 192, 192, 192, 0, 93, 255, 87  
 CB, SLV19, 212, 160, 255, 0, 192, 128, 255, 128, 0  
 CB, SLV20, 192, 192, 192, 160, 255, 255, 255, 192, 87  
 CB, SLV21, 255, 255, 255, 255, 87, 87, 255, 255, 255  
 CB, SLV22, 78, 0, 255, 148, 87, 255, 148, 87, 255  
 CB, SLV23, 163, 255, 160, 255, 0, 192, 0, 192, 128  
 CB, SLV24, 192, 192, 0, 255, 255, 87, 192, 192, 192  
 CB, SLV25, 210, 210, 210, 255, 255, 255, 78, 0, 255  
 CB, SLV26, 146, 0, 255, 146, 0, 255, 128, 192, 0  
 CB, SLV27, 192, 128, 0, 212, 160, 255, 255, 160, 255  
 CB, SLV28, 146, 0, 255, 192, 0, 128, 146, 0, 255  
 CB, SLV29, 210, 210, 210, 163, 255, 160, 255, 255, 255  
 CB, SLV30, 210, 210, 210, 192, 192, 192, 146, 0, 255  
 CB, SLV31, 192, 128, 0, 192, 192, 192, 255, 255, 87  
 CB, SLV32, 255, 192, 87, 255, 87, 87, 0, 128, 255  
 CB, SLV33, 163, 160, 255, 255, 0, 192, 212, 160, 255  
 CB, SLV34, 78, 0, 255, 192, 0, 192, 192, 0, 128  
 CB, SLV35, 85, 192, 0, 255, 192, 87, 210, 210, 210  
 CB, SLV36, 0, 128, 57, 255, 192, 87, 212, 160, 255  
 CB, SLV37, 192, 72, 0, 255, 192, 160, 210, 210, 210  
 CB, SLV38, 128, 192, 0, 146, 0, 255, 255, 87, 128  
 CB, SLV39, 192, 192, 0, 210, 210, 210, 210, 210, 210  
 CB, SLV40, 146, 0, 255, 160, 192, 255, 210, 210, 210  
 CB, SLV41, 255, 0, 192, 255, 255, 255, 160, 192, 255

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CB, SLV42, 148, 87, 255, 192, 72, 0, 148, 87, 255  
 CB, SLV43, 255, 87, 128, 255, 0, 128, 255, 128, 0  
 CB, SLV44, 0, 157, 192, 93, 255, 87, 255, 0, 192  
 CB, SLV45, 148, 87, 255, 255, 0, 192, 0, 128, 255  
 CB, SLV46, 192, 0, 192, 85, 192, 0, 255, 192, 87  
 CB, SLV47, 160, 255, 255, 78, 0, 255, 192, 192, 0  
 CB, SLV48, 0, 157, 192, 192, 0, 192, 0, 128, 255  
 CB, SLV49, 0, 192, 128, 93, 255, 87, 146, 0, 255  
 CB, SLV50, 93, 255, 87, 212, 160, 255, 163, 255, 160  
 CB, SLV51, 163, 255, 160, 0, 192, 192, 192, 192, 192  
 CB, SLV52, 148, 87, 255, 128, 192, 0, 192, 72, 0  
 CB, SLV53, 78, 0, 255, 192, 192, 192, 78, 0, 255  
 CB, SLV54, 160, 192, 255, 255, 192, 87, 160, 255, 255  
 CB, SLV55, 0, 192, 128, 255, 255, 255, 255, 87, 128  
 CB, SLV56, 192, 192, 0, 128, 192, 0, 0, 192, 128  
 CB, RARA\_1, 255, 192, 160, 255, 255, 87, 0, 192, 128  
 CB, RARA\_2, 163, 255, 160, 192, 72, 0, 85, 192, 0  
 CB, RARA\_3, 192, 0, 128, 255, 192, 160, 0, 157, 192  
 CB, RARA\_4, 0, 128, 255, 0, 128, 192, 255, 0, 128  
 CB, RARA\_5, 0, 128, 57, 255, 0, 128, 255, 0, 192  
 CB, RARA\_6, 163, 255, 160, 0, 192, 128, 192, 192, 192  
 CB, RARA\_7, 160, 255, 255, 148, 87, 255, 255, 0, 192  
 CB, RARA\_8, 192, 128, 0, 255, 255, 255, 255, 0, 192  
 CB, RARA\_9, 0, 157, 192, 160, 255, 255, 212, 160, 255  
 CB, RARA\_10, 85, 192, 0, 0, 192, 128, 255, 255, 255  
 CB, RARA\_11, 255, 128, 0, 93, 255, 87, 146, 0, 255  
 CB, RARA\_12, 0, 128, 255, 255, 0, 128, 192, 0, 128  
 CB, RARA\_13, 0, 128, 57, 210, 210, 210, 210, 212, 160, 255  
 CB, RARA\_14, 210, 210, 210, 85, 192, 0, 255, 128, 0  
 CB, RARA\_15, 255, 0, 128, 255, 192, 87, 85, 0, 192  
 CB, RARA\_16, 0, 128, 128, 255, 0, 128, 255, 192, 160  
 CB, FREQ\_1, 163, 160, 255, 212, 160, 255, 192, 192, 0  
 CB, FREQ\_2, 212, 160, 255, 255, 128, 0, 255, 160, 255  
 CB, FREQ\_3, 0, 128, 192, 192, 192, 0, 93, 255, 87  
 CB, FREQ\_4, 0, 192, 128, 93, 255, 87, 0, 128, 128  
 CB, FREQ\_5, 255, 192, 87, 192, 0, 128, 255, 255, 255  
 CB, FREQ\_6, 255, 0, 128, 163, 160, 255, 160, 255, 255  
 CB, FREQ\_7, 255, 128, 0, 85, 192, 0, 160, 255, 255  
 CB, FREQ\_8, 163, 255, 160, 255, 128, 0, 146, 0, 255  
 CB, FREQ\_9, 192, 192, 0, 192, 192, 192, 255, 87, 87  
 CB, FREQ\_10, 0, 157, 192, 0, 128, 192, 93, 255, 87  
 CB, FREQ\_11, 146, 0, 255, 255, 0, 192, 192, 72, 0  
 CB, FREQ\_12, 0, 192, 128, 255, 0, 192, 255, 87, 87  
 CB, FREQ\_13, 146, 0, 255, 255, 0, 192, 255, 160, 255  
 CB, FREQ\_14, 148, 87, 255, 192, 0, 128, 0, 128, 57  
 CB, FREQ\_15, 85, 0, 192, 0, 128, 128, 160, 192, 255  
 CB, FREQ\_16, 192, 0, 192, 255, 192, 87, 192, 192, 0  
 CB, QP\_1, 210, 210, 210, 0, 192, 192, 255, 192, 87  
 CB, QP\_2, 210, 210, 210, 0, 128, 255, 163, 160, 255  
 CB, QP\_3, 192, 192, 192, 255, 192, 87, 160, 192, 255  
 CB, QP\_4, 160, 192, 255, 255, 160, 255, 163, 255, 160  
 CB, SLD1, 0, 128, 192, 78, 0, 255, 192, 128, 0  
 CB, SLD2, 148, 87, 255, 163, 255, 160, 255, 87, 128  
 CB, SLD3, 0, 157, 192, 160, 255, 255, 128, 192, 0  
 CB, SLD4, 192, 0, 192, 255, 192, 160, 85, 192, 0  
 CBS, SLU1, 255, 87, 128, 255, 0, 192, 192, 192, 0  
 CBS, SLU2, 192, 192, 192, 0, 157, 192, 0, 192, 128  
 CBS, SLU3, 0, 192, 192, 160, 255, 255, 148, 87, 255  
 CBS, SLU4, 255, 87, 87, 192, 72, 0, 255, 87, 87  
 CBS, SLU5, 255, 87, 87, 0, 128, 57, 255, 128, 0  
 CBS, SLU6, 0, 128, 255, 0, 128, 128, 78, 0, 255  
 CBS, SLU7, 210, 210, 210, 255, 255, 87, 78, 0, 255

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CBS, SLU8, 0, 128, 255, 0, 157, 192, 192, 128, 0  
 CBS, SLU9, 192, 192, 192, 160, 255, 255, 85, 0, 192  
 CBS, SLU10, 163, 255, 160, 0, 128, 57, 210, 210, 210  
 CBS, SLU11, 163, 160, 255, 0, 157, 192, 163, 160, 255  
 CBS, SLU12, 255, 87, 128, 255, 0, 192, 210, 210, 210  
 CBS, SLU13, 255, 255, 87, 0, 192, 128, 0, 128, 128  
 CBS, SLU14, 0, 128, 255, 255, 87, 87, 192, 72, 0  
 CBS, SLU15, 160, 255, 255, 255, 192, 160, 0, 192, 128  
 CBS, SLU16, 163, 255, 160, 212, 160, 255, 163, 255, 160  
 CBS, SLU17, 255, 192, 87, 255, 128, 0, 163, 255, 160  
 CBS, SLU18, 210, 210, 210, 0, 128, 128, 0, 128, 57  
 CBS, SLU19, 255, 87, 87, 210, 210, 210, 255, 255, 87  
 CBS, SLU20, 192, 192, 0, 85, 0, 192, 0, 128, 192  
 CBS, SLU21, 0, 128, 57, 255, 87, 87, 255, 160, 255  
 CBS, SLU22, 0, 192, 192, 192, 72, 0, 255, 0, 192  
 CBS, SLU23, 210, 210, 210, 255, 87, 87, 128, 192, 0  
 CBS, SLU24, 146, 0, 255, 0, 192, 128, 210, 210, 210  
 CBS, SLV1, 192, 72, 0, 192, 0, 128, 255, 192, 160  
 CBS, SLV2, 192, 72, 0, 0, 128, 128, 210, 210, 210  
 CBS, SLV3, 212, 160, 255, 255, 128, 0, 255, 0, 192  
 CBS, SLV4, 192, 128, 0, 85, 0, 192, 255, 192, 87  
 CBS, SLV5, 163, 255, 160, 255, 192, 160, 0, 157, 192  
 CBS, SLV6, 255, 192, 160, 255, 192, 87, 255, 160, 255  
 CBS, SLV7, 255, 87, 87, 163, 160, 255, 192, 0, 192  
 CBS, SLV8, 146, 0, 255, 0, 157, 192, 0, 192, 128  
 CBS, SLV9, 192, 0, 128, 0, 128, 57, 0, 128, 57  
 CBS, SLV10, 192, 0, 192, 0, 157, 192, 163, 255, 160  
 CBS, SLV11, 0, 157, 192, 255, 87, 128, 212, 160, 255  
 CBS, SLV12, 163, 255, 160, 0, 128, 57, 0, 192, 128  
 CBS, SLV13, 192, 0, 192, 0, 157, 192, 0, 192, 192  
 CBS, SLV14, 192, 192, 0, 160, 255, 255, 163, 160, 255  
 CBS, SLV15, 0, 128, 128, 192, 72, 0, 0, 128, 128  
 CBS, SLV16, 0, 192, 128, 93, 255, 87, 255, 0, 192  
 CBS, SLV17, 192, 192, 192, 93, 255, 87, 255, 0, 192  
 CBS, SLV18, 0, 192, 128, 0, 128, 255, 93, 255, 87  
 CBS, SLV19, 0, 128, 192, 128, 192, 0, 163, 160, 255  
 CBS, SLV20, 146, 0, 255, 255, 87, 87, 85, 0, 192  
 CBS, SLV21, 192, 128, 0, 255, 0, 128, 255, 0, 192  
 CBS, SLV22, 160, 255, 255, 192, 0, 128, 255, 160, 255  
 CBS, SLV23, 160, 255, 255, 192, 192, 0, 255, 0, 192  
 CBS, SLV24, 192, 0, 192, 192, 0, 128, 255, 128, 0  
 CBS, SLV25, 78, 0, 255, 192, 192, 0, 192, 0, 128  
 CBS, SLV26, 0, 128, 57, 210, 210, 210, 192, 128, 0  
 CBS, SLV27, 128, 192, 0, 163, 255, 160, 93, 255, 87  
 CBS, SLV28, 163, 160, 255, 78, 0, 255, 255, 87, 128  
 CBS, SLV29, 160, 192, 255, 0, 128, 192, 192, 72, 0  
 CBS, SLV30, 192, 72, 0, 163, 160, 255, 192, 0, 128  
 CBS, SLV31, 255, 0, 192, 85, 192, 0, 0, 192, 128  
 CBS, SLV32, 0, 128, 128, 255, 87, 128, 148, 87, 255  
 CBS, SLV33, 85, 192, 0, 148, 87, 255, 192, 192, 0  
 CBS, SLV34, 255, 0, 192, 255, 128, 0, 255, 128, 0  
 CBS, SLV35, 192, 128, 0, 0, 128, 128, 255, 0, 128  
 CBS, SLV36, 192, 72, 0, 93, 255, 87, 85, 192, 0  
 CBS, SLV37, 255, 192, 87, 255, 0, 128, 148, 87, 255  
 CBS, SLV38, 0, 192, 192, 85, 0, 192, 85, 192, 0  
 CBS, SLV39, 128, 192, 0, 192, 192, 0, 255, 128, 0  
 CBS, SLV40, 0, 128, 57, 160, 192, 255, 255, 0, 128  
 CBS, SLV41, 255, 255, 87, 85, 192, 0, 0, 157, 192  
 CBS, SLV42, 210, 210, 210, 0, 192, 128, 212, 160, 255  
 CBS, SLV43, 0, 192, 192, 255, 0, 128, 0, 192, 128  
 CBS, SLV44, 212, 160, 255, 78, 0, 255, 160, 255, 255  
 CBS, SLV45, 210, 210, 210, 255, 0, 192, 255, 0, 128

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CBS, SLV46, 255, 0, 128, 93, 255, 87, 255, 160, 255  
 CBS, SLV47, 146, 0, 255, 255, 192, 87, 128, 192, 0  
 CBS, SLV48, 85, 0, 192, 212, 160, 255, 85, 192, 0  
 CBS, SLV49, 163, 160, 255, 192, 0, 192, 85, 0, 192  
 CBS, SLV50, 212, 160, 255, 0, 157, 192, 163, 160, 255  
 CBS, SLV51, 160, 192, 255, 163, 160, 255, 128, 192, 0  
 CBS, SLV52, 163, 160, 255, 192, 0, 192, 163, 255, 160  
 CBS, SLV53, 146, 0, 255, 255, 255, 255, 128, 192, 0  
 CBS, SLV54, 160, 192, 255, 163, 255, 160, 192, 0, 192  
 CBS, SLV55, 255, 255, 255, 160, 255, 255, 210, 210, 210  
 CBS, SLV56, 192, 72, 0, 255, 192, 160, 255, 128, 0  
 CBS, RARA\_1, 0, 128, 57, 192, 0, 192, 192, 0, 128  
 CBS, RARA\_2, 0, 128, 57, 85, 0, 192, 146, 0, 255  
 CBS, RARA\_3, 255, 255, 87, 148, 87, 255, 148, 87, 255  
 CBS, RARA\_4, 0, 157, 192, 255, 255, 87, 255, 87, 128  
 CBS, RARA\_5, 210, 210, 210, 255, 0, 128, 0, 157, 192  
 CBS, RARA\_6, 0, 128, 255, 192, 0, 192, 0, 128, 128  
 CBS, RARA\_7, 85, 192, 0, 160, 255, 255, 128, 192, 0  
 CBS, RARA\_8, 93, 255, 87, 160, 192, 255, 255, 87, 128  
 CBS, RARA\_9, 0, 128, 255, 210, 210, 210, 85, 0, 192  
 CBS, RARA\_10, 255, 0, 192, 163, 160, 255, 0, 128, 192  
 CBS, RARA\_11, 192, 72, 0, 192, 128, 0, 255, 87, 128  
 CBS, RARA\_12, 255, 0, 192, 85, 0, 192, 192, 192, 192  
 CBS, RARA\_13, 255, 128, 0, 93, 255, 87, 0, 128, 192  
 CBS, RARA\_14, 146, 0, 255, 255, 255, 87, 128, 192, 0  
 CBS, RARA\_15, 0, 128, 255, 0, 128, 128, 192, 0, 128  
 CBS, RARA\_16, 192, 192, 192, 192, 0, 192, 0, 192, 192  
 CBS, FREQ\_1, 163, 255, 160, 255, 0, 192, 0, 128, 255  
 CBS, FREQ\_2, 0, 128, 192, 0, 128, 57, 0, 128, 192  
 CBS, FREQ\_3, 85, 0, 192, 85, 0, 192, 192, 72, 0  
 CBS, FREQ\_4, 255, 0, 128, 192, 192, 192, 212, 160, 255  
 CBS, FREQ\_5, 255, 87, 87, 255, 87, 128, 0, 157, 192  
 CBS, FREQ\_6, 255, 192, 87, 148, 87, 255, 0, 192, 128  
 CBS, FREQ\_7, 192, 0, 192, 160, 255, 255, 255, 255, 87  
 CBS, FREQ\_8, 255, 87, 87, 210, 210, 210, 255, 160, 255  
 CBS, FREQ\_9, 0, 128, 192, 255, 128, 0, 255, 87, 87  
 CBS, FREQ\_10, 192, 192, 192, 163, 255, 160, 192, 192, 0  
 CBS, FREQ\_11, 255, 0, 192, 192, 0, 192, 192, 128, 0  
 CBS, FREQ\_12, 163, 255, 160, 255, 87, 128, 192, 0, 192  
 CBS, FREQ\_13, 146, 0, 255, 192, 192, 0, 255, 87, 87  
 CBS, FREQ\_14, 0, 128, 128, 212, 160, 255, 255, 87, 87  
 CBS, FREQ\_15, 163, 255, 160, 255, 0, 192, 192, 192, 0  
 CBS, FREQ\_16, 146, 0, 255, 0, 157, 192, 128, 192, 0  
 CBS, QP\_1, 210, 210, 210, 192, 72, 0, 163, 255, 160  
 CBS, QP\_2, 128, 192, 0, 0, 192, 192, 192, 72, 0  
 CBS, QP\_3, 255, 192, 160, 160, 192, 255, 255, 87, 128  
 CBS, QP\_4, 192, 0, 192, 163, 255, 160, 160, 192, 255  
 CBS, SLD1, 192, 0, 128, 255, 0, 192, 255, 87, 87  
 CBS, SLD2, 255, 128, 0, 146, 0, 255, 192, 192, 0  
 CBS, SLD3, 93, 255, 87, 0, 192, 192, 163, 160, 255  
 CBS, SLD4, 128, 192, 0, 255, 128, 0, 0, 128, 57  
 CBSC, SLU1, 85, 192, 0, 255, 0, 192, 255, 128, 0  
 CBSC, SLU2, 0, 128, 255, 210, 210, 210, 0, 157, 192  
 CBSC, SLU3, 192, 128, 0, 255, 87, 87, 85, 192, 0  
 CBSC, SLU4, 255, 192, 160, 192, 0, 192, 255, 87, 87  
 CBSC, SLU5, 255, 87, 87, 0, 157, 192, 0, 192, 192  
 CBSC, SLU6, 192, 72, 0, 146, 0, 255, 85, 0, 192  
 CBSC, SLU7, 160, 192, 255, 146, 0, 255, 0, 157, 192  
 CBSC, SLU8, 163, 255, 160, 0, 192, 192, 255, 255, 255  
 CBSC, SLU9, 93, 255, 87, 0, 192, 128, 255, 192, 160  
 CBSC, SLU10, 128, 192, 0, 192, 72, 0, 0, 128, 128  
 CBSC, SLU11, 0, 128, 57, 212, 160, 255, 85, 192, 0

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CBSC, SLU12, 0, 128, 255, 163, 160, 255, 192, 0, 128  
 CBSC, SLU13, 255, 87, 128, 210, 210, 210, 192, 128, 0  
 CBSC, SLU14, 192, 192, 192, 255, 255, 87, 212, 160, 255  
 CBSC, SLU15, 93, 255, 87, 210, 210, 210, 93, 255, 87  
 CBSC, SLU16, 192, 72, 0, 255, 87, 128, 0, 128, 192  
 CBSC, SLU17, 148, 87, 255, 212, 160, 255, 128, 192, 0  
 CBSC, SLU18, 192, 0, 128, 146, 0, 255, 128, 192, 0  
 CBSC, SLU19, 163, 160, 255, 192, 72, 0, 160, 255, 255  
 CBSC, SLU20, 255, 192, 160, 0, 128, 57, 93, 255, 87  
 CBSC, SLU21, 163, 160, 255, 192, 72, 0, 192, 0, 192  
 CBSC, SLU22, 163, 160, 255, 210, 210, 210, 192, 0, 192  
 CBSC, SLU23, 146, 0, 255, 146, 0, 255, 255, 87, 87  
 CBSC, SLU24, 255, 192, 87, 160, 192, 255, 128, 192, 0  
 CBSC, SLV1, 0, 192, 128, 163, 160, 255, 210, 210, 210  
 CBSC, SLV2, 163, 160, 255, 255, 0, 128, 192, 192, 0  
 CBSC, SLV3, 255, 0, 192, 255, 0, 128, 192, 72, 0  
 CBSC, SLV4, 255, 255, 87, 128, 192, 0, 212, 160, 255  
 CBSC, SLV5, 160, 192, 255, 0, 128, 128, 210, 210, 210  
 CBSC, SLV6, 0, 128, 128, 0, 128, 192, 160, 192, 255  
 CBSC, SLV7, 192, 0, 192, 128, 192, 0, 0, 157, 192  
 CBSC, SLV8, 93, 255, 87, 192, 192, 192, 0, 128, 57  
 CBSC, SLV9, 255, 87, 87, 0, 192, 128, 255, 192, 160  
 CBSC, SLV10, 192, 0, 192, 255, 255, 255, 160, 192, 255  
 CBSC, SLV11, 212, 160, 255, 163, 255, 160, 0, 157, 192  
 CBSC, SLV12, 0, 192, 128, 212, 160, 255, 255, 0, 128  
 CBSC, SLV13, 192, 0, 128, 160, 192, 255, 0, 128, 192  
 CBSC, SLV14, 255, 87, 87, 128, 192, 0, 0, 128, 255  
 CBSC, SLV15, 0, 128, 255, 255, 160, 255, 255, 192, 160  
 CBSC, SLV16, 192, 192, 192, 0, 192, 192, 0, 192, 128  
 CBSC, SLV17, 212, 160, 255, 0, 128, 57, 163, 160, 255  
 CBSC, SLV18, 93, 255, 87, 0, 192, 128, 0, 157, 192  
 CBSC, SLV19, 93, 255, 87, 128, 192, 0, 146, 0, 255  
 CBSC, SLV20, 85, 192, 0, 128, 192, 0, 78, 0, 255  
 CBSC, SLV21, 255, 87, 128, 0, 157, 192, 192, 128, 0  
 CBSC, SLV22, 192, 128, 0, 0, 128, 128, 160, 255, 255  
 CBSC, SLV23, 255, 255, 255, 192, 192, 0, 192, 0, 128  
 CBSC, SLV24, 0, 128, 255, 255, 0, 128, 192, 0, 128  
 CBSC, SLV25, 255, 192, 160, 212, 160, 255, 0, 192, 192  
 CBSC, SLV26, 85, 0, 192, 192, 0, 192, 85, 192, 0  
 CBSC, SLV27, 255, 0, 192, 85, 192, 0, 255, 192, 87  
 CBSC, SLV28, 0, 128, 255, 85, 192, 0, 0, 192, 192  
 CBSC, SLV29, 0, 192, 192, 210, 210, 210, 255, 192, 87  
 CBSC, SLV30, 0, 128, 128, 192, 0, 192, 192, 128, 0  
 CBSC, SLV31, 163, 160, 255, 255, 160, 255, 0, 128, 255  
 CBSC, SLV32, 255, 192, 87, 255, 160, 255, 255, 160, 255  
 CBSC, SLV33, 255, 0, 192, 0, 192, 192, 255, 160, 255  
 CBSC, SLV34, 0, 128, 57, 128, 192, 0, 192, 192, 0  
 CBSC, SLV35, 146, 0, 255, 0, 128, 255, 255, 0, 128  
 CBSC, SLV36, 0, 128, 255, 255, 128, 0, 160, 192, 255  
 CBSC, SLV37, 160, 255, 255, 0, 192, 192, 255, 192, 160  
 CBSC, SLV38, 0, 128, 255, 93, 255, 87, 93, 255, 87  
 CBSC, SLV39, 255, 255, 87, 255, 87, 87, 255, 0, 192  
 CBSC, SLV40, 0, 128, 128, 255, 87, 87, 163, 160, 255  
 CBSC, SLV41, 255, 0, 128, 255, 160, 255, 85, 192, 0  
 CBSC, SLV42, 255, 87, 87, 255, 160, 255, 255, 255, 255  
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 CBSC, SLV44, 163, 255, 160, 0, 192, 192, 85, 192, 0  
 CBSC, SLV45, 0, 128, 128, 255, 192, 160, 0, 192, 192  
 CBSC, SLV46, 255, 160, 255, 0, 157, 192, 85, 192, 0  
 CBSC, SLV47, 255, 192, 87, 0, 192, 128, 148, 87, 255  
 CBSC, SLV48, 255, 87, 128, 192, 72, 0, 163, 255, 160  
 CBSC, SLV49, 163, 255, 160, 163, 160, 255, 255, 192, 160



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CBSC, SLV50, 0, 192, 128, 212, 160, 255, 0, 128, 57  
 CBSC, SLV51, 255, 192, 87, 93, 255, 87, 0, 192, 128  
 CBSC, SLV52, 146, 0, 255, 0, 192, 128, 146, 0, 255  
 CBSC, SLV53, 160, 192, 255, 0, 128, 255, 0, 192, 128  
 CBSC, SLV54, 0, 128, 57, 0, 128, 192, 255, 0, 128  
 CBSC, SLV55, 160, 192, 255, 163, 160, 255, 255, 0, 128  
 CBSC, SLV56, 148, 87, 255, 192, 192, 0, 0, 192, 192  
 CBSC, RARA\_1, 146, 0, 255, 0, 157, 192, 163, 160, 255  
 CBSC, RARA\_2, 255, 0, 128, 85, 0, 192, 192, 72, 0  
 CBSC, RARA\_3, 255, 0, 128, 210, 210, 210, 255, 192, 160  
 CBSC, RARA\_4, 255, 160, 255, 78, 0, 255, 85, 192, 0  
 CBSC, RARA\_5, 85, 192, 0, 128, 192, 0, 192, 72, 0  
 CBSC, RARA\_6, 192, 0, 192, 93, 255, 87, 0, 192, 128  
 CBSC, RARA\_7, 163, 255, 160, 146, 0, 255, 192, 192, 0  
 CBSC, RARA\_8, 255, 192, 87, 160, 255, 255, 78, 0, 255  
 CBSC, RARA\_9, 255, 160, 255, 192, 0, 128, 255, 128, 0  
 CBSC, RARA\_10, 192, 192, 192, 0, 128, 255, 255, 160, 255  
 CBSC, RARA\_11, 160, 192, 255, 78, 0, 255, 160, 192, 255  
 CBSC, RARA\_12, 163, 160, 255, 192, 72, 0, 148, 87, 255  
 CBSC, RARA\_13, 255, 255, 255, 0, 157, 192, 255, 0, 192  
 CBSC, RARA\_14, 0, 128, 192, 0, 128, 57, 255, 192, 87  
 CBSC, RARA\_15, 160, 255, 255, 0, 128, 128, 255, 192, 160  
 CBSC, RARA\_16, 210, 210, 210, 0, 192, 192, 192, 0, 192  
 CBSC, FREQ\_1, 192, 128, 0, 85, 0, 192, 255, 0, 192  
 CBSC, FREQ\_2, 212, 160, 255, 146, 0, 255, 255, 192, 160  
 CBSC, FREQ\_3, 78, 0, 255, 192, 192, 0, 0, 192, 128  
 CBSC, FREQ\_4, 85, 192, 0, 212, 160, 255, 192, 72, 0  
 CBSC, FREQ\_5, 93, 255, 87, 192, 0, 192, 255, 192, 87  
 CBSC, FREQ\_6, 255, 192, 87, 146, 0, 255, 255, 87, 87  
 CBSC, FREQ\_7, 85, 192, 0, 255, 255, 255, 210, 210, 210  
 CBSC, FREQ\_8, 192, 0, 192, 192, 128, 0, 85, 192, 0  
 CBSC, FREQ\_9, 192, 0, 128, 255, 160, 255, 0, 128, 255  
 CBSC, FREQ\_10, 0, 128, 255, 146, 0, 255, 78, 0, 255  
 CBSC, FREQ\_11, 160, 255, 255, 255, 0, 128, 163, 160, 255  
 CBSC, FREQ\_12, 255, 192, 160, 192, 0, 128, 210, 210, 210  
 CBSC, FREQ\_13, 210, 210, 210, 93, 255, 87, 163, 255, 160  
 CBSC, FREQ\_14, 0, 128, 57, 255, 160, 255, 192, 192, 192  
 CBSC, FREQ\_15, 160, 192, 255, 255, 192, 87, 255, 160, 255  
 CBSC, FREQ\_16, 93, 255, 87, 255, 255, 255, 255, 255, 255  
 CBSC, QP\_1, 160, 192, 255, 192, 192, 0, 0, 128, 255  
 CBSC, QP\_2, 192, 192, 0, 146, 0, 255, 85, 0, 192  
 CBSC, QP\_3, 93, 255, 87, 255, 255, 255, 255, 0, 128  
 CBSC, QP\_4, 192, 72, 0, 0, 128, 57, 192, 0, 192  
 CBSC, SLD1, 0, 128, 192, 192, 72, 0, 78, 0, 255  
 CBSC, SLD2, 163, 160, 255, 0, 128, 192, 192, 0, 192  
 CBSC, SLD3, 148, 87, 255, 0, 192, 128, 255, 255, 255  
 CBSC, SLD4, 255, 160, 255, 255, 87, 128, 85, 0, 192  
 CB, ENVESLU-SLV, 255, 192, 87, 0, 128, 255, 210, 210, 210  
 CB, ENVE-RARA, 255, 0, 128, 212, 160, 255, 0, 128, 57  
 CB, ENVE-FREQ, 0, 192, 192, 255, 160, 255, 255, 87, 128  
 CB, ENVE-QP, 0, 192, 128, 93, 255, 87, 255, 87, 87  
 CB, ENVE-SLD, 128, 192, 0, 0, 128, 255, 210, 210, 210  
 CBS, ENVESLU-SLV, 255, 87, 87, 93, 255, 87, 192, 0, 192  
 CBS, ENVE-RARA, 210, 210, 210, 255, 255, 255, 255, 192, 160  
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# MIDAS

PROJECT TITLE :

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