

## **ITINERARIO RAGUSA-CATANIA**

Collegamento viario compreso tra lo Svincolo della S.S. 514 "di Chiaramonte"  
con la S.S. 115 e lo Svincolo della S.S. 194 "Ragusana"

LOTTO 1 - Dallo svincolo n. 1 sulla S.S. 115 (compreso) allo svincolo n. 3 sulla S.P. 5 (escluso)

## **PROGETTO ESECUTIVO**

COD. **PA895**

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DELLA PROVINCIA DI REGGIO CALABRIA

## **CAVALCAVIA** **CAVALCAVIA AL KM 15+818** **Relazione di calcolo impalcato**

CODICE PROGETTO			NOME FILE	REVISIONE	SCALA:
PROGETTO	LIV. PROG.	N. PROG.	T01CV06STRRE01B		
LO408Z	E	2101	CODICE ELAB. T01 CV06 STR RE01	B	-
D					
C					
B	REVISIONE A SEGUITO DI RAPPORTO DI VERIFICA		NOVEMBRE 2021	RAGNACCI	PELLE
A	EMISSIONE		GIUGNO 2021	RAGNACCI	PELLE
REV.	DESCRIZIONE		DATA	REDATTO	VERIFICATO
					APPROVATO

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**INDICE**

<b>1 DESCRIZIONE DELLE OPERE .....</b>	<b>4</b>
<b>2 NORMATIVA DI RIFERIMENTO E RIFERIMENTI TECNICI .....</b>	<b>7</b>
<b>2.1 NORME TECNICHE.....</b>	<b>7</b>
<b>2.2 RIFERIMENTI TECNICI .....</b>	<b>7</b>
2.2.1 CNR e UNI.....	7
2.2.2 EUROCODICI.....	7
<b>3 METODO DI CALCOLO .....</b>	<b>8</b>
<b>3.1 CODICI DI CALCOLO.....</b>	<b>8</b>
3.1.1 Programmi di Verifica .....	8
<b>4 MATERIALI.....</b>	<b>9</b>
<b>4.1 CONGLOMERATO CEMENTIZIO.....</b>	<b>9</b>
4.1.1 Calcestruzzo - solette .....	9
4.1.2 Calcestruzzo – pile e spalle .....	9
<b>4.2 ACCIAIO.....</b>	<b>9</b>
4.2.1 Acciaio per cemento armato tipo B450C.....	9
4.2.2 Acciaio per carpenteria tipo S355.....	9
4.2.3 Acciaio in reti e tralicci elettrosaldati ad aderenza migliorata per c.a. .....	10
<b>4.3 BULLONI AD ALTA RESISTENZA – CLASSE 10.9 .....</b>	<b>10</b>
<b>4.4 PIOLI CON TESTA.....</b>	<b>10</b>
<b>4.5 SALDATURE DI TESTA O A T A COMPLETA PENETRAZIONE .....</b>	<b>10</b>
<b>4.6 SALDATURE A CORDONE D'ANGOLO .....</b>	<b>10</b>
<b>5 ANALISI GENERALE DEI CARICHI .....</b>	<b>11</b>
<b>5.1 PESI PROPRI STRUTTURALI .....</b>	<b>11</b>
<b>5.2 PESI PROPRI PORTATI.....</b>	<b>11</b>

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

<b>5.2.1 PESO PROPRIO SOLETTA E CORDOLI .....</b>	<b>11</b>
<b>5.2.2 PESO PERMANENTE PORTATO.....</b>	<b>12</b>
<b>5.3 AZIONI ACCIDENTALI.....</b>	<b>13</b>
5.3.1 Carichi mobili .....	13
5.3.2 Frenamento .....	15
<b>5.4 CALCOLO SEZIONE COLLABORANTE .....</b>	<b>16</b>
<b>5.5 AZIONI METEORICHE.....</b>	<b>17</b>
5.5.1 Azioni dovute alla neve.....	17
5.5.2 Azioni dovute al vento .....	17
5.5.3 Azioni dovute alla temperatura .....	19
<b>5.6 AZIONE SISMICA .....</b>	<b>20</b>
<b>6 MODELLAZIONE STRUTTURALE .....</b>	<b>24</b>
<b>6.1 COMBINAZIONI DI CARICO .....</b>	<b>27</b>
<b>6.2 VERIFICHE STRUTTURALI.....</b>	<b>31</b>
<b>6.2.1 VERIFICA ELEMENTI DI IMPALCATO .....</b>	<b>31</b>
6.2.1.1 TRAVE PRINCIPALE CONCIO 1.....	44
6.2.1.2 TRAVE PRINCIPALE CONCIO 2.....	63
6.2.1.3 TRAVE PRINCIPALE CONCIO 3.....	83
<b>6.2.2 VERIFICA A FATICA .....</b>	<b>102</b>
6.2.3 TRAVE DI SPINA .....	105
6.2.4 TRAVERSO .....	108
6.2.4.1 VERIFICA TRAVERSO DI CAMPATA.....	109
6.2.4.2 VERIFICA TRAVERSO DI TESTATA.....	111
6.2.5 VERIFICA PIOLI .....	113
6.2.6 GIUNTI BULLONATI .....	115

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

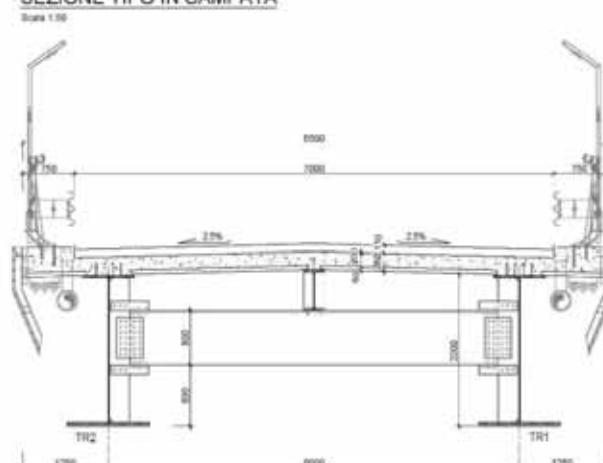
6.2.6.1	VERIFICA GIUNTO TRAVERSO DI CAMPATA .....	116
6.2.6.2	VERIFICA GIUNTO TRAVERSO DI TESTATA .....	118
6.2.7	VERIFICA VELETTA METALLICA.....	120
7	VERIFICA SOLETTA .....	128
7.1	VERIFICA SOLETTA FASE TRANSITORIA.....	128
7.1.1	ANALISI DEI CARICHI .....	128
7.1.2	VERIFICA A MOMENTO POSITIVO - CAMPATA.....	128
7.1.3	VERIFICA A MOMENTO NEGATIVO APPOGGIO .....	134
7.2	VERIFICA SOLETTA FASE DEFINITIVA .....	135
8	VERIFICA ELEMENTI DI APPOGGIO .....	153
8.1	VERIFICA BAGGIOLI .....	155
8.2	VERIFICA ISOLATORI.....	156
8.3	VERIFICA GIUNTI.....	156
8.4	VERIFICA RITEGNI SISMICI .....	156
8.5	VERIFICA DEFORMAZIONI .....	159
9	VALIDAZIONE CODICE DI CALCOLO .....	161
10	ALLEGATO 1 - TABULATO MIDAS.....	162

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

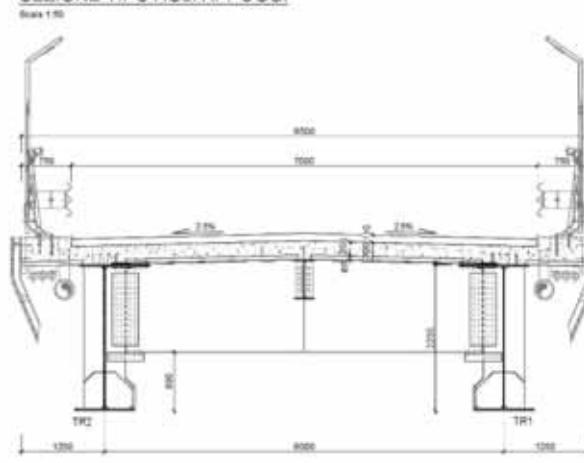
## 1 DESCRIZIONE DELLE OPERE

La presente relazione di calcolo riguarda l'impalcato posizionato alla progressiva 15+518 del Lotto 1. L'impalcato è realizzato in struttura mista acciaio-calcestruzzo e presenta una larghezza complessiva di 8,50 metri e luce di 45,50 metri, con cordoli laterali da 75cm. La soletta ha uno spessore totale medio di 25cm. Le travi in acciaio sono poste ad interasse pari a 3m e presentano un'altezza totale di 2100mm. Le piattabande hanno larghezze e spessori variabili. I traversi sono costituiti da profili a doppio T saldati di altezza pari a 800mm e posti ad interasse 4400mm. L'impalcato viene realizzato a mezzo di 3 conci diversi (Concio C1, C2 e C3) che vengono saldati in opera, collegati agli elementi trasversali e poi il tutto viene sollevato da autogru di grossa portata posizionate in corrispondenza dei rilevati delle spalle. Per dettagli circa le geometrie delle travi fare riferimento agli elaborati specifici, comunque nel proseguito vengono riportate delle immagini rappresentative delle opere in oggetto

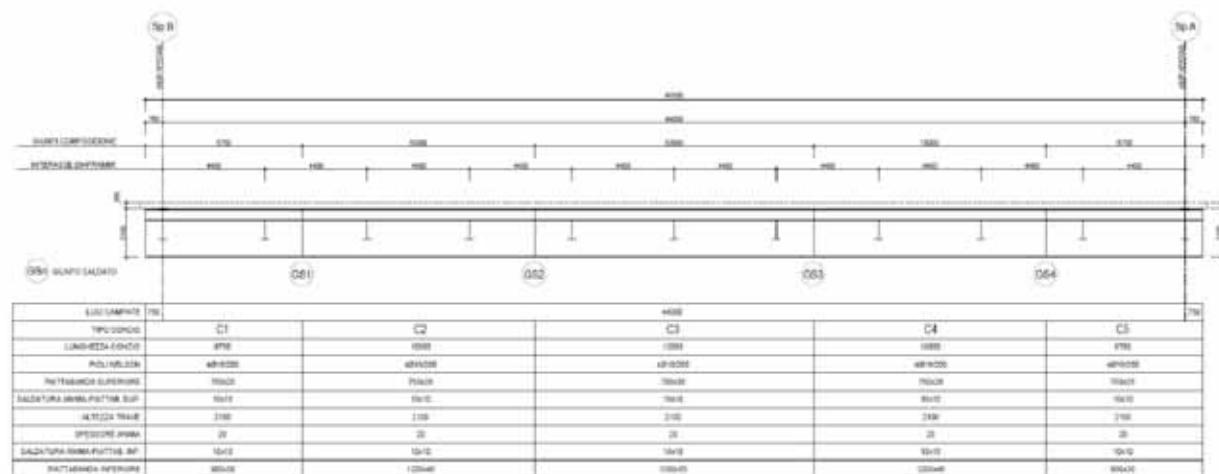
**SEZIONE TIPO IN CAMPATA**



**SEZIONE TIPO AGLI APPOGGI**



**Figura 1 - Sezioni trasversali dell'impalcato**



**Figura 2 - Pianta Conci**

## LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO

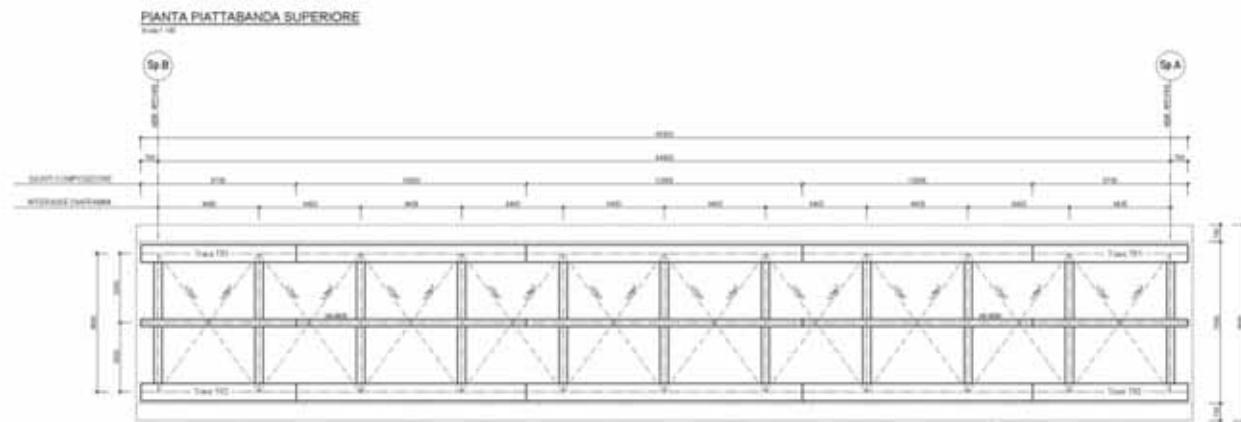


Figura 3 - Pianta conci

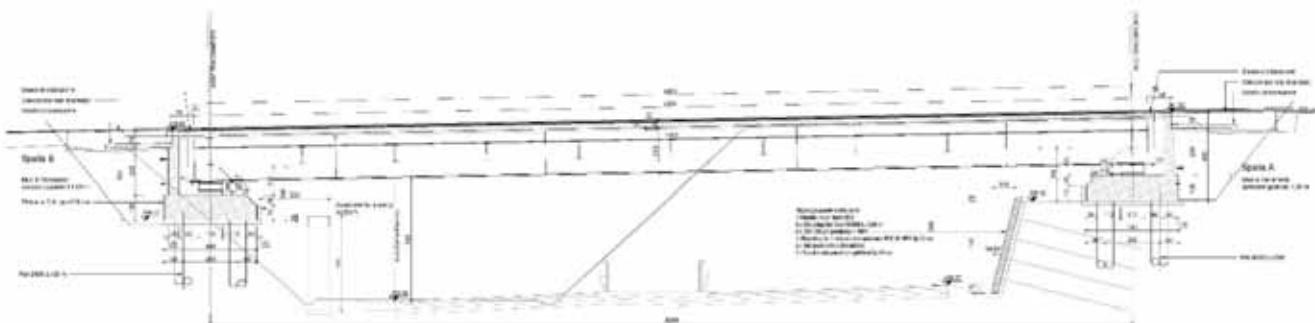


Figura 4 - Sezione prospetto

Lo schema di vincolo prevede l'utilizzo di 4 dispositivi di appoggio tipo isolatori elastomerici con nucleo di piombo che consentono uno spostamento di  $\pm 250\text{mm}$  e di conseguenza un giunto di dilatazione di spalla che permetta il corrispondente spostamento. Si riporta di seguito le caratteristiche che deve avere il dispositivo di appoggio.

V	Fzd	Ke	ξe	Kv	Dg	te	h	H	z
kN	kN	kN/mm	%	kN/mm	mm	mm	mm	mm	mm
23710	37010	4,49	21	4330	1100	190	322	402	1150

<b>V</b>	Carico verticale massimo agente in presenza del sisma allo SLC
<b>Fzd</b>	Carico verticale massimo in assenza di SISMA e spostamento 10mm
<b>Ke</b>	Rigidezza orizzontale equivalente
<b>ξe</b>	Coefficiente di smorzamento viscoso equivalente
<b>Kv</b>	Rigidezza verticale
<b>Dg</b>	Diametro elastomero
<b>te</b>	Spessore totale gomma
<b>h</b>	Altezza escluse piastre di ancoraggio
<b>H</b>	Altezza totale incluse piastre di ancoraggio
<b>Z</b>	Lato piastre di ancoraggio

#### **MANDATORIA:**

**MANDANTI:**

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

SCHEMA APPOGGI



Figura 5 - Schema di vincolo

## 2 NORMATIVA DI RIFERIMENTO E RIFERIMENTI TECNICI

Le strutture sono state verificate con il criterio degli stati limite (SL). I calcoli sono stati eseguiti in osservanza alle seguenti disposizioni normative e regole tecniche:

### 2.1 NORME TECNICHE

- Ministero delle infrastrutture - D.M. 14/01/2008. Norme tecniche per le costruzioni.
- Consiglio superiore dei lavori pubblici. Istruzioni per l'applicazione delle "Norme tecniche per le costruzioni" di cui al D.M. 14/01/2008.

### 2.2 RIFERIMENTI TECNICI

#### 2.2.1 CNR e UNI

- Norma UNI EN 11104:2004
- Calcestruzzo – Specificazione, prestazione, produzione e conformità – Istruzioni complementari per l'applicazione della EN 206-1.
- Norma UNI EN 13369:2013
- Regole comuni per i prodotti prefabbricati di calcestruzzo.
- Norma UNI EN 15050:2012
- Prodotti prefabbricati di calcestruzzo. Elementi da ponte.

#### 2.2.2 EUROCODICI

- UNI EN 1993-1-1: Eurocodice 3 – Progettazione delle strutture in acciaio. Parte 1-1: Regole generali e regole per gli edifici.
- UNI EN 1993-1-5: Eurocodice 3 – Progettazione delle strutture in acciaio. Parte 1-5: Regole generali – Regole supplementari per lastre ortotrope in assenza di carichi trasversali.
- UNI EN 1993-1-9: Eurocodice 3 - Progettazione delle strutture in acciaio. Parte 1-9: Fatica.
- UNI EN 1993-1-10: Eurocodice 3 - Progettazione delle strutture in acciaio. Parte 1-10: Resilienza del materiale e proprietà attraverso lo spessore.
- UNI EN 1993-2: Eurocodice 3 – Progettazione delle strutture in acciaio. Parte 2: Ponti in acciaio.
- UNI EN 1998-2: Eurocodice 8 - Indicazioni progettuali per la resistenza sismica delle strutture. Parte 2: Ponti.

### 3 METODO DI CALCOLO

Lo studio delle strutture è stato condotto secondo i metodi della scienza delle costruzioni supponendo i materiali elastici, omogenei ed isotropi.

La ricerca dei parametri di sollecitazione è stata fatta secondo le disposizioni di carico più gravose avvalendosi di codici di calcolo automatico per l'analisi strutturale.

Le verifiche di resistenza delle sezioni sono state eseguite secondo il metodo degli stati limite.

#### 3.1 CODICI DI CALCOLO

Tutti i codici di calcolo automatico utilizzati per il calcolo e la verifica delle strutture e la redazione della presente relazione di calcolo sono di sicura ed accertata validità e sono stati impiegati conformemente alle loro caratteristiche. Tale affermazione è suffragata dai seguenti elementi:

- grande diffusione del codice di calcolo sul mercato;
- storia consolidata del codice di calcolo (svariati anni di utilizzo);
- utilizzo delle versioni più aggiornate (dopo test);
- pratica d'uso frequente in studio.

In considerazione dei casi-studio, caratterizzati da piccoli spostamenti e tensioni inferiori ai limiti elastici dei materiali, si è ritenuto sufficiente adottare una schematizzazione della geometria e dei materiali di tipo lineare con leggi elastiche e isotrope ed omogenee.

##### 3.1.1 Programmi di Verifica

Per la verifica delle sezioni in acciaio e composite è stato utilizzato un foglio di excel opportunamente testato.

## 4 MATERIALI

### 4.1 CONGLOMERATO CEMENTIZIO

#### 4.1.1 Calcestruzzo - solette

Classe di resistenza	C32/40	fck/Rck = 32/40 MPa
Resistenza di calcolo (SLU)	$f_{cd} = \frac{\alpha_{cc} \cdot f_{ck}}{\gamma_c} = 18.81 \text{ MPa}$	
Resistenza a compressione media	$f_{cm} = f_{ck} + 8 = 41.20 \text{ MPa}$	
Resistenza a trazione semplice	$f_{csm} = 0,3 \cdot f_{ck}^{2/3} = 3.10 \text{ MPa}$	
Modulo elastico	$E_{cm} = 22000 \cdot [f_{cm}/10]^{0.3} = 33642.78 \text{ MPa}$	

#### 4.1.2 Calcestruzzo – pile e spalle

Classe di resistenza	C32/40	fck/Rck = 32/40 MPa
Resistenza di calcolo (SLU)	$f_{cd} = \frac{\alpha_{cc} \cdot f_{ck}}{\gamma_c} = 18.81 \text{ MPa}$	
Resistenza a compressione media	$f_{cm} = f_{ck} + 8 = 41.20 \text{ MPa}$	
Resistenza a trazione semplice	$f_{csm} = 0,3 \cdot f_{ck}^{2/3} = 3.10 \text{ MPa}$	
Modulo elastico	$E_{cm} = 22000 \cdot [f_{cm}/10]^{0.3} = 33642.78 \text{ MPa}$	

## 4.2 ACCIAIO

### 4.2.1 Acciaio per cemento armato tipo B450C

Tensione caratteristica di snervamento	$f_{yk} \geq 450 \text{ MPa}$
Tensione caratteristica di rottura	$f_{rk} \geq 540 \text{ MPa}$
Resistenza di calcolo	$f_{yd} = \frac{f_{yk}}{1,15} = 391 \text{ MPa}$

### 4.2.2 Acciaio per carpenteria tipo S355

Tensione caratteristica di rottura	$f_{rk} = 470 \text{ MPa} (\text{per } s > 40\text{mm})$
	$f_{rk} = 510 \text{ MPa} (\text{per } s < 40\text{mm})$
Resistenza caratteristica di snervamento	$f_{yk} = 335 \text{ MPa} (\text{per } s > 40\text{mm})$
	$f_{yk} = 355 \text{ MPa} (\text{per } s < 40\text{mm})$
Coefficiente di sicurezza	$\gamma_{M0} = 1,05$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Coefficiente di sicurezza per verifiche a fatica

$$\gamma_{M,F} = 1,35$$

#### 4.2.3 Acciaio in reti e tralicci elettrosaldati ad aderenza migliorata per c.a.

Tipo di acciaio: B450C ad aderenza migliorata, controllato in stabilimento

Tensione caratteristica di snervamento:  $f_{yk} \geq 450 \text{ MPa}$

Tensione caratteristica di rottura:  $f_{tk} \geq 540 \text{ MPa}$

Allungamento percentuale:  $A_{gik} \geq 7.5\%$

#### 4.3 BULLONI AD ALTA RESISTENZA – CLASSE 10.9

Vite: classe 10.9 (UNI EN 14399:2005)

Tensione di rottura a trazione  $f_{tk} \geq 1000 \text{ MPa}$

Tensione di snervamento  $f_{yk} \geq 900 \text{ MPa}$

#### 4.4 PIOLI CON TESTA

Acciaio: S235J2G3+C450

Tensione di rottura a trazione  $f_{tk} \geq 450 \text{ MPa}$

Tensione di snervamento  $f_{yk} \geq 370 \text{ MPa}$

Allungamento  $A_S \geq 15\%$

#### 4.5 SALDATURE DI TESTA O A T A COMPLETA PENETRAZIONE

Giunto di prima classe:  $f_{d,S355} = 338 \text{ MPa}$

Giunto di seconda classe:  $0.85 f_{d,S355} = 287.38 \text{ MPa}$

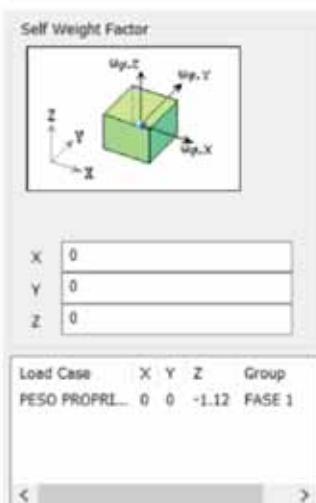
#### 4.6 SALDATURE A CORDONE D'ANGOLO

Per S355:  $|\sigma_{\perp}|, |\tau_{\perp}|, |\tau_{//}| \leq 0.70 f_{d,S355} = 236.6 \text{ MPa}$

## 5 ANALISI GENERALE DEI CARICHI

### 5.1 PESI PROPRI STRUTTURALI

I pesi strutturali delle opere in acciaio comprensive di travi principali, trave di spina, traversi e controventi, vengono considerati in automatico nel programma di calcolo. Per considerare l'effetto del "piastrame", è stato scelto di aumentare il peso proprio del 12%, come riportato nell'immagine che segue.



### 5.2 PESI PROPRI PORTATI

I pesi proprio portati sono stati presi in conto nel modello di calcolo utilizzato, come carichi lineari direttamente applicati alle singole travi portanti. Si riporta di seguito la definizione dei carichi applicati.

#### 5.2.1 PESO PROPRIO SOLETTA E CORDOLI

Per la determinazione delle azioni da applicare alle singole travi è stata schematizzata una trave su più appoggi a cui è stata applicato il carico distribuito della soletta e dei cordoli laterali. Le reazioni vincolari risultanti sono poi state applicate alle travi del modello.

<b>PESO SOLETTA</b>		
Hsolella =	<b>0,26</b>	m
$\gamma$ =	25	kN/mc
peso =	6,5	kN/mq
Carico trave SX =	<b>17,976</b>	
Carico trave Centrale =	<b>19,299</b>	
Carico trave DX =	<b>17,976</b>	

<b>CORDOLI</b>		
Hcordoli =	<b>0,15</b>	m
$\gamma$ =	25	kN/mc
peso =	3,75	kN/mq
Carico trave SX =	<b>4,037</b>	
Carico trave Centrale =	<b>-2,449</b>	
Carico trave DX =	<b>4,037</b>	

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

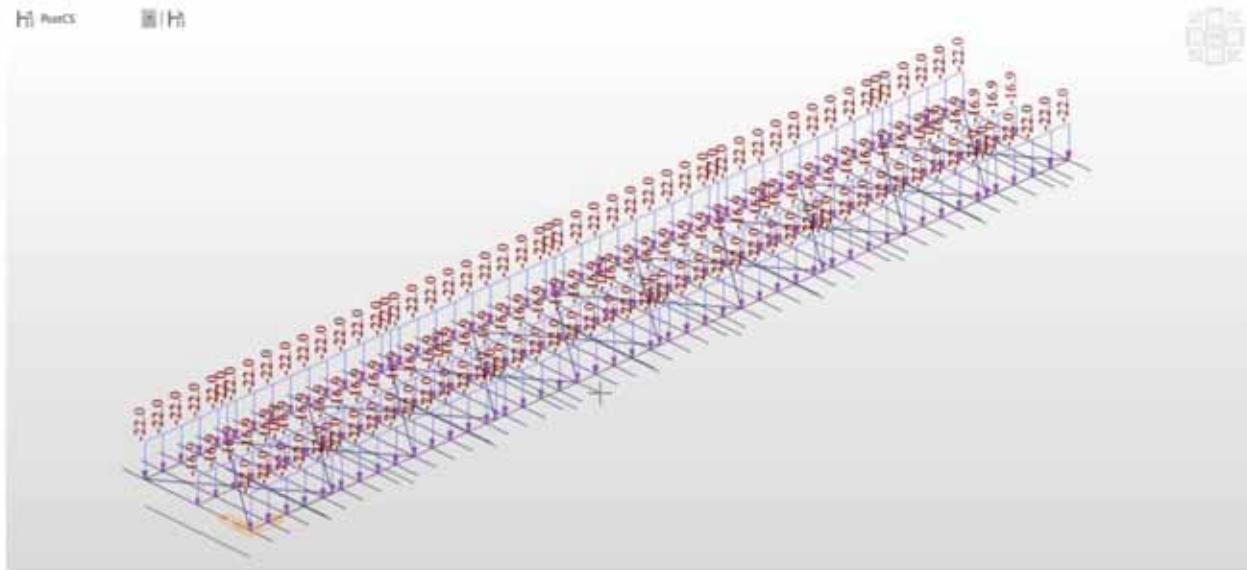


Figura 6 – Pesi propri soletta e cordoli

### 5.2.2 PESO PERMANENTE PORTATO

Per la determinazione delle azioni da applicare alle singole travi è stata schematizzata una trave su più appoggi a cui è stata applicato il carico del pacchetto stradale e delle barriere bordo ponte. Le reazioni vincolari risultanti sono poi state applicate alle travi del modello.

#### PERMANENTE PORTATO

Hsoletta =	<b>0,11</b>	cm
$\gamma$ =	<b>24</b>	kN/mc
peso =	<b>2,64</b>	kN/mq
Carico trave SX =	<b>4,459</b>	
Carico trave Centrale =	<b>9,562</b>	
Carico trave DX =	<b>4,459</b>	

#### BARRIERE+VELETTE E TUBAZIONI

Barriere =	<b>1.5</b>	kN/ml
Tubazioni =	<b>0.1</b>	kN/ml
Velette =	<b>0.4</b>	kN/ml
Totale	<b>2.000</b>	kN/ml
Carico trave SX =	<b>2.498</b>	
Carico trave Centrale =	<b>-0.995</b>	
Carico trave DX =	<b>2.498</b>	

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

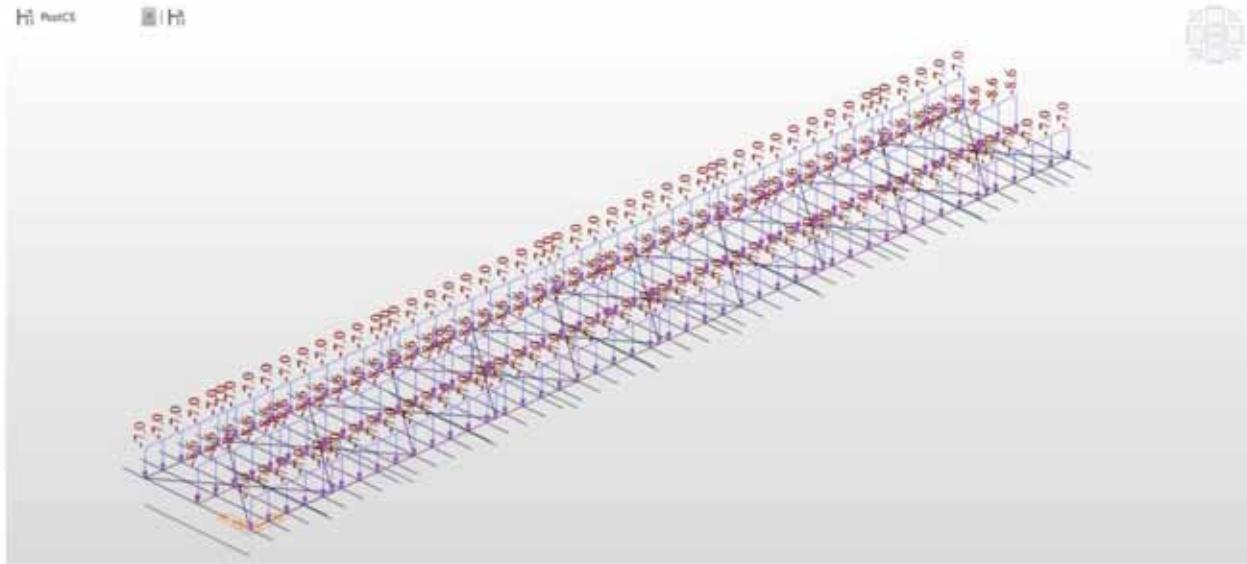


Figura 7 - Peso permanente portato

## 5.3 AZIONI ACCIDENTALI

### 5.3.1 Carichi mobili

Le azioni accidentali considerate nei calcoli sono quelle previste dall'attuale D.M. 14.1.2008 per i ponti classificati di prima categoria e calcolate in relazione alla larghezza dell'impalcato.

La carreggiata tra i cordoli ha larghezza di 7,00 m, pertanto sono state considerate ai fini del calcolo 2 corsie da 3,00 m più una parte rimanente di 1,00 m.

Si riporta lo schema di carico 1:

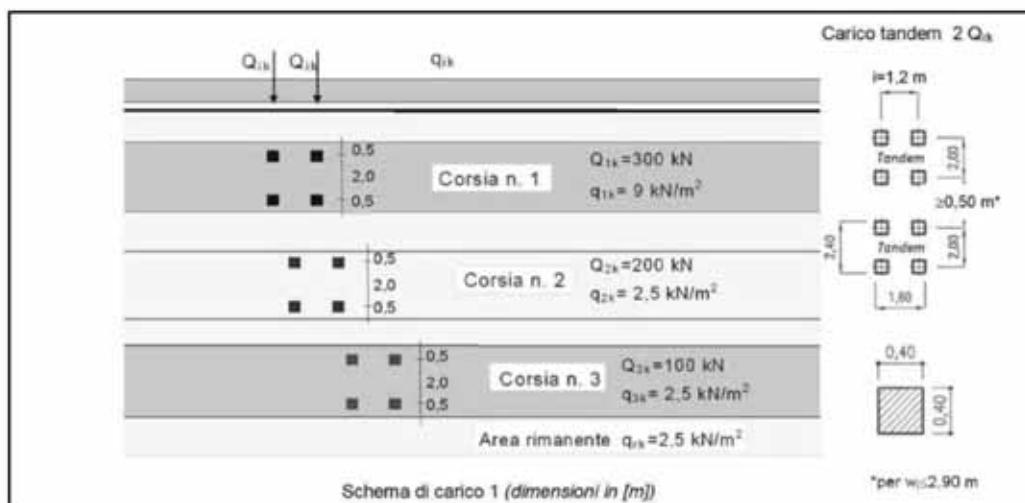


Figura 8 - Schema di carico 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

L'intensità dei carichi da applicare è riportata nella seguente tabella:

**Tabella 5.1.II - Intensità dei carichi  $Q_{ik}$  e  $q_{ik}$  per le diverse corsie**

Posizione	Carico asse $Q_{ik}$ [kN]	$q_{ik}$ [kN/m <sup>2</sup> ]
Corsia Numero 1	300	9,00
Corsia Numero 2	200	2,50
Corsia Numero 3	100	2,50
Altre corsie	0,00	2,50

Le suddette colonne di carico sono state disposte secondo lo schema longitudinale che produce le azioni accidentali più gravose per la struttura in esame.

Si riportano di seguito alcune immagini dei carichi considerati.

Define User Defined Vehicular Load

Standard Name: EN 1991-2:2003 - RoadBridge/Footway and Footbridge

Load Type:  Load Model 1 / Fatigue Load Model 1

Vehicular Load Properties: Vehicular Load Name: Q1A\_q1a

$\alpha_QiQi_k$  : Tandem System,  $Q_{ik}$   
 $\alpha_{qi}q_{ik}$  : UDL System,  $q_{ik}$

Dynamic amplification factor included

Location	Tandem System		UDL System	
	Adjustment Factor	Axle Loads (kN)	Adjustment Factor	Uniformly Dist. Loads (kN/m <sup>2</sup> )
Lane Number1	1	300	1	9
Lane Number2	0	0	0	0
Lane Number3	0	0	0	0
Other Lanes & Remaining Area	0	0	0	0

D :  m

Phi :

Psi factor for Tandem System:

Psi factor for UDL System:

OK Cancel Apply

Figura 9 - Colonna di carico 1

Define User Defined Vehicular Load

Standard Name: EN 1991-2:2003 - RoadBridge/Footway and Footbridge

Load Type:  Load Model 1 / Fatigue Load Model 1

Vehicular Load Properties: Vehicular Load Name: Q1B\_q1b

$\alpha_QiQi_k$  : Tandem System,  $Q_{ik}$   
 $\alpha_{qi}q_{ik}$  : UDL System,  $q_{ik}$

Dynamic amplification factor included

Location	Tandem System		UDL System	
	Adjustment Factor	Axle Loads (kN)	Adjustment Factor	Uniformly Dist. Loads (kN/m <sup>2</sup> )
Lane Number1	1	200	1	2,5
Lane Number2	0	0	0	0
Lane Number3	0	0	0	0
Other Lanes & Remaining Area	0	0	0	0

D :  m

Phi :

Psi factor for Tandem System:

Psi factor for UDL System:

OK Cancel Apply

Figura 10 - Colonna di carico 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

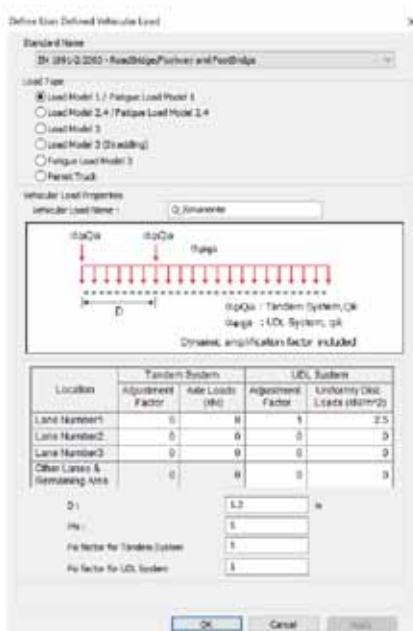


Figura 11 - Colonna di carico 3

### 5.3.2 Frenamento

#### FRENATURA

Si considera un'azione longitudinale funzione del carico verticale agente sulla corsia convenzionale n°1

$$Q_{1k} = 0.6 \times (2 \times Q_{1k}) + 0.10 \times q_{1k} \times W_1 \times L$$

$Q_{1k} =$	482,850	kN
$q_{1k} =$	3,537	kN

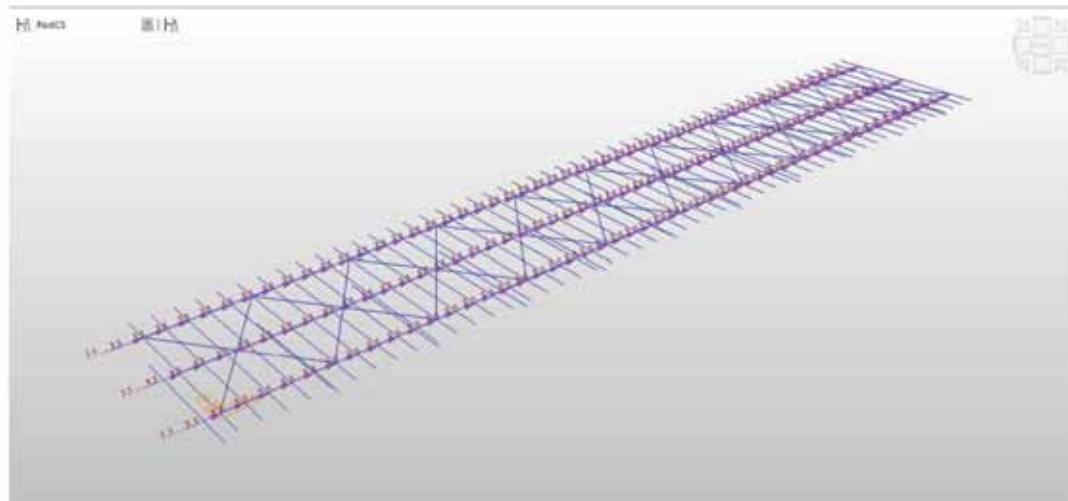
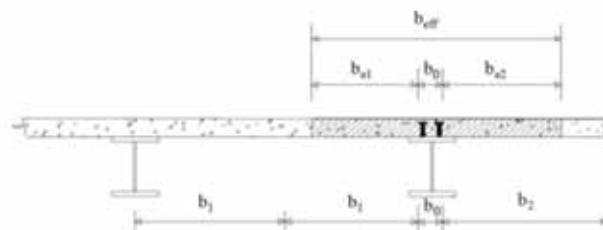


Figura 12 - Frenamento

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

#### 5.4 CALCOLO SEZIONE COLLABORANTE

La distribuzione delle tensioni normali negli elementi composti è stata determinata mediante un modello che tiene conto della diffusione degli sforzi nelle ali della trave metallica e nella soletta in calcestruzzo. La larghezza efficace,  $b_{eff}$ , della soletta in calcestruzzo è stata determinata, in conformità a quanto prescritto dalla normativa, mediante l'espressione:  $b_{eff} = b_0 + b_{e1} + b_{e2}$ , in cui  $b_0$  è la distanza tra gli assi dei connettori esterni e  $b_{e1} = \min(L_e/8, b_i)$  è il valore della larghezza collaborante da ciascun lato della sezione composta.



$L_e$ , nelle travate in semplice appoggio, indica la luce della trave.

Per il caso in esame si ha:

$L_e =$	44,00 m	
$L_e/8 =$	5,5 m	
$b_0 =$	0,382 m	
$b_{e2} =$	1,059 m	SBALZO
$b_{e1} =$	1,402 m	DISTANZA
$b_{e2} =$	1,059	
$b_{e1} =$	1,402	
 		<b>TRAVE PRINCIPALE</b>
$b_{eff} =$	2,843 m	
 		<b>TRAVE DI SPINA</b>

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

## 5.5 AZIONI METEORICHE

### 5.5.1 Azioni dovute alla neve

Il carico dovuto alla neve, non risulta dimensionante per il tipo di struttura

### 5.5.2 Azioni dovute al vento

Viene di seguito riportata una tabella di calcolo per la determinazione della pressione del vento da applicare alla struttura

AZIONE DEL VENTO				
(Inserire i dati necessari nelle celle campite)	simbolo	valore	unità	formula
Altitudine sul livello del mare	$a_s =$	630	[m]	
Regione	Sicilia e Provincia di Reggio Calabria			
	Zona =	4		
Parametri tabella 3.3.1	$v_{b0} =$	28	[m/s]	
Parametri tabella 3.3.1	$a_0 =$	500	[m]	
Parametri tabella 3.3.1	$k_s =$	0.360	[-]	
Coefficiente di altitudine	$c_a =$	1.094	[-]	
Velocità di riferimento	$v_b =$	30.6	[m/s]	
Periodo di ritorno	$T_R =$	50	[anni]	
Velocità di riferimento associata a $T_R = 50$	$v_b =$	30.6	[m/s]	
Velocità di riferimento	$v_r =$	30.6	[m/s]	$v_r = v_b \times c_r$
<b>CALCOLO PRESSIONE DEL VENTO</b>				
Densità dell'aria	$\rho =$	1.25	[kg/m <sup>3</sup> ]	
Pressione cinetica di riferimento	$q_r =$	586.9	[N/m <sup>2</sup> ]	$1/2 \times \rho v_r^2$
Categoria di esposizione del terreno	II			
Parametri per la definizione del coefficiente di esposizione	$k_r =$	0.19	[-]	
	$z_0 =$	0.05	[m]	
	$z_{max} =$	4.00	[m]	
Quota struttura	$z =$	9	[m]	
Coefficiente di topografia	$c_t =$	1		
Classe di rugosità	D			
Coefficiente di esposizione	$c_e =$	2.29		
Coefficiente di forma	$c_p =$	1.40		
Coefficiente dinamico	$c_d =$	1		
Pressione del vento	$p =$	1.88	[kN/m <sup>2</sup> ]	$p = q_r \times c_e \times c_p \times c_d$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Sulla base delle caratteristiche geometriche dell'impalcato, sono state determinate le azioni da applicare alle strutture in caso di ponte scarico e ponte carico.

**VENTO A PONTE SCARICO**

Fh =	4.507	kN/ml
Pv =	0.789	kN/ml

**VENTO A PONTE CARICO**

Fh =	10.142	kN/ml
Pv =	0.406	kN/ml

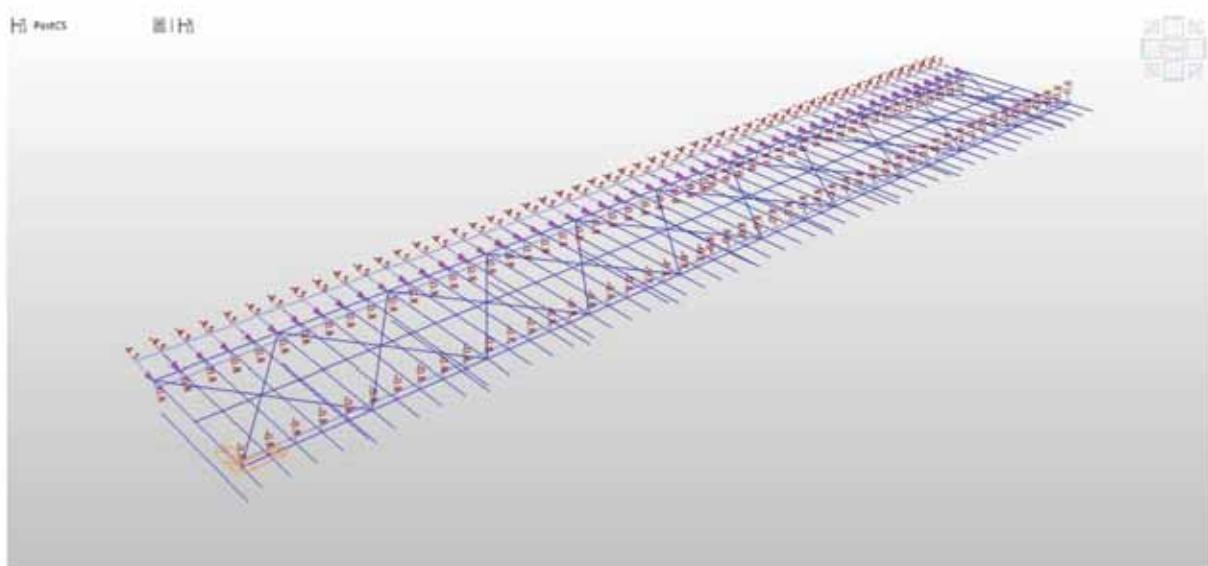


Figura 13 - Vento a ponte scarico

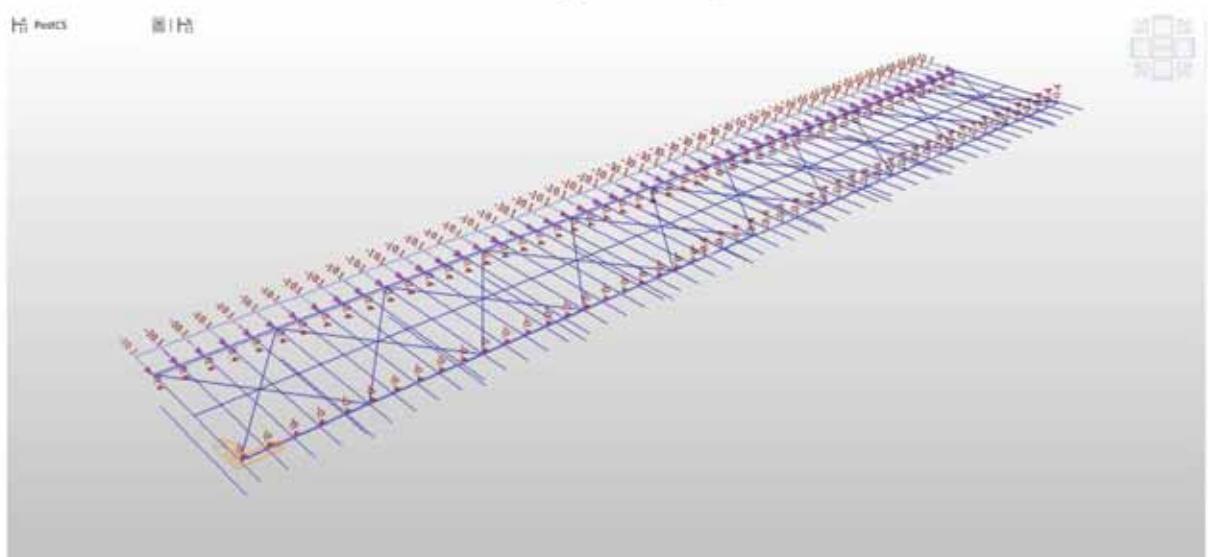
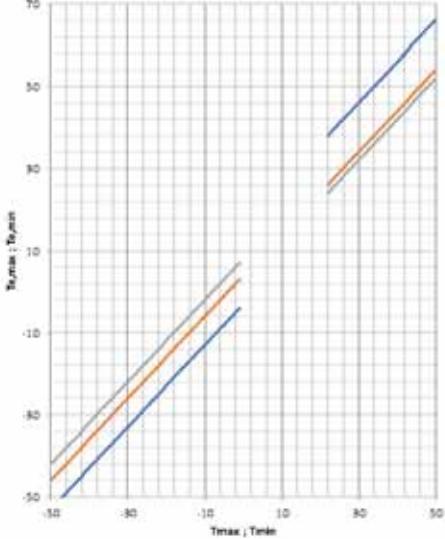


Figura 14 - Vento a ponte carico

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 – RELAZIONE DI CALCOLO IMPALCATO**

### 5.5.3 Azioni dovute alla temperatura

L'azione termica, sia uniforme che a forma di gradiente, è stata applicata in base a quanto previsto dalla normativa vigente.

AZIONE DELLA TEMPERATURA				
(Inserire i dati necessari nelle celle campate)				
	simbolo	valore	unità	formula
Altitudine di riferimento	$a_s =$	630	[m]	
Individuazione della zona :		Zona IV		
<i>Calabria, Sicilia</i>				
Temperatura massima estiva dell'aria	$T_{max} =$	45	[°C]	Valeure raccomandato in assenza di studi specifici
Temperatura minima invernale dell'aria	$T_{min} =$	-15	[°C]	Valeure raccomandato in assenza di studi specifici
<i>I valori di <math>T_{max}</math> e <math>T_{min}</math> sono riferiti ad un periodo di ritorno di 50 anni</i>				
<i>Per la valutazione della temperatura uniforme minima/ massima del ponte, si considera il grafico riportato sull'eurocodice nella sezione Ponti</i>				
Tipologia di impalcato	Tipo 2			
	Impalcati a struttura mista			
	$T_0 =$	15	[°C]	Annesso Nazionale A.1(3)
				
<b>Legenda</b> 1 - Tipo 1 $T_{var,max} = T_{max} + 16$ 2 - Tipo 2 $T_{var,max} = T_{max} + 4$ 3 - Tipo 3 $T_{var,max} = T_{max} + 2$ 4 - Tipo 1 $T_{var,min} = T_{min} - 3$ 5 - Tipo 2 $T_{var,min} = T_{min} - 4$ 6 - Tipo 3 $T_{var,min} = T_{min} - 8$				
<b>Calcolo variazione termica uniforme</b> Temperatura uniforme del ponte massima $T_{e,max} = -49$ [°C] Temperatura uniforme del ponte minima $T_{e,min} = -11$ [°C] Variazione termica di espansione $\Delta T_{N,exp} = 34$ [°C] Variazione termica di contrazione $\Delta T_{N,contr} = -26$ [°C]				
<b>Calcolo variazione termica lineare</b> Spessore della superficie $100$ [mm] Coefficiente riduttivo heat $K_{surf,heat} = 1.00$ [-] Coefficiente riduttivo cool $K_{surf,cool} = 1.00$ [-] Variazione termica lineare (heat) $\Delta T_{M,heat} = 15$ [°C] Variazione termica lineare (cool) $\Delta T_{M,cool} = -18$ [°C]				

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

## 5.6 AZIONE SISMICA

Con riferimento al DM 14.01.2008, sono stati presi in considerazione i seguenti parametri, per il sito in esame:

Vita nominale della costruzione – VN = 50 anni

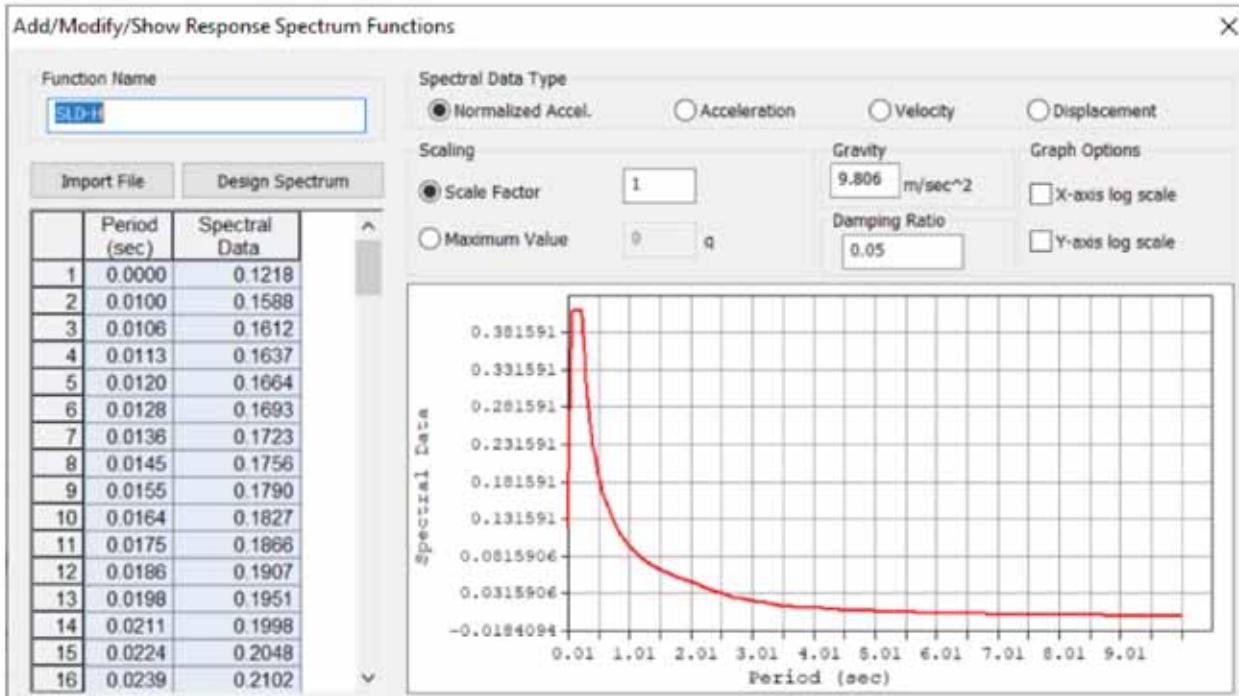
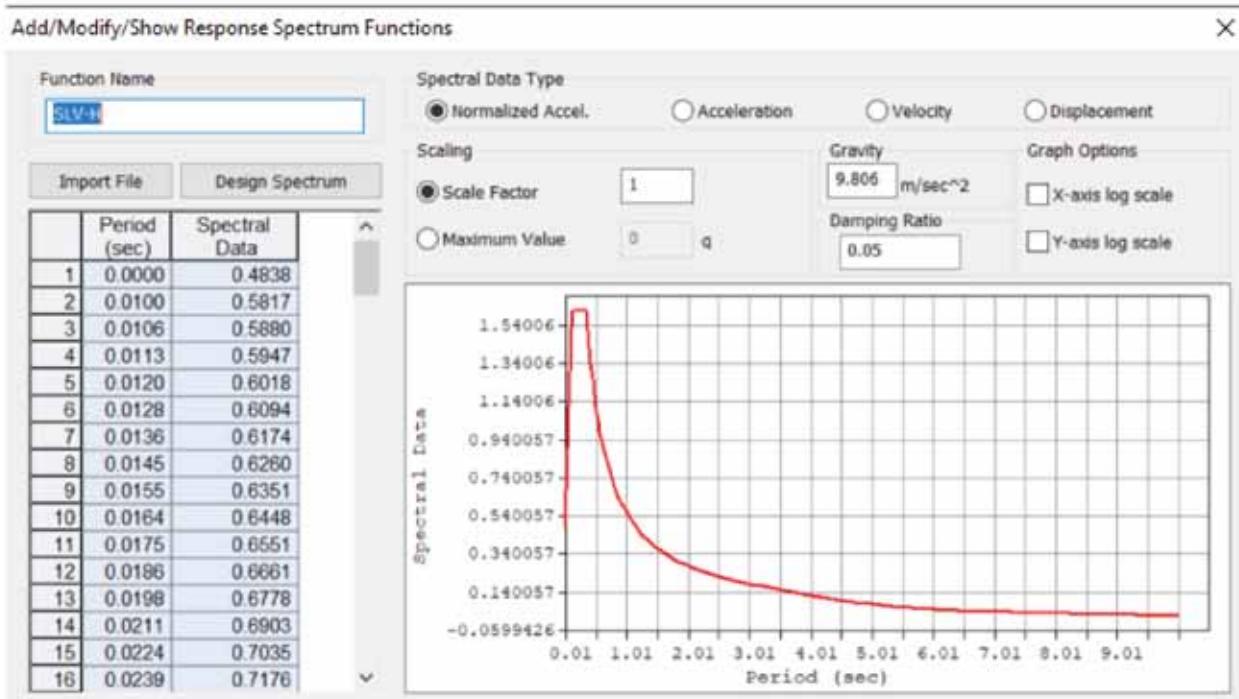
Coefficiente d'uso della costruzione – cu = 2.0

Ne deriva un periodo di riferimento per la costruzione VR=100 anni.

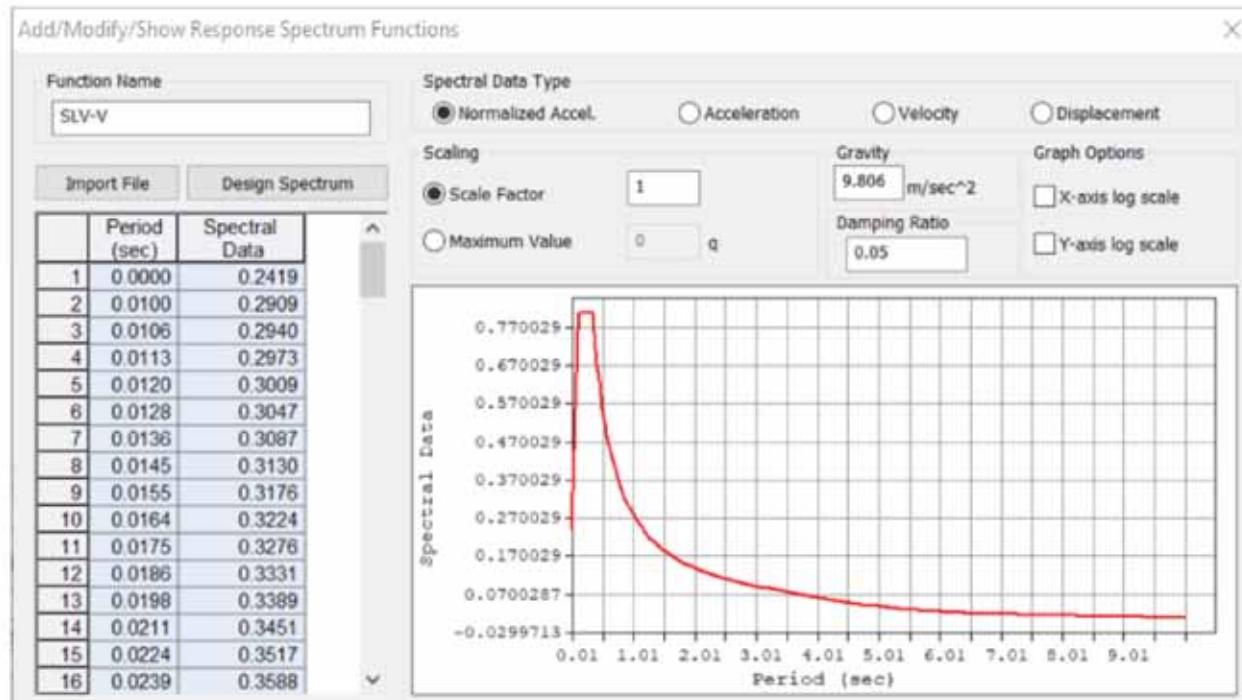
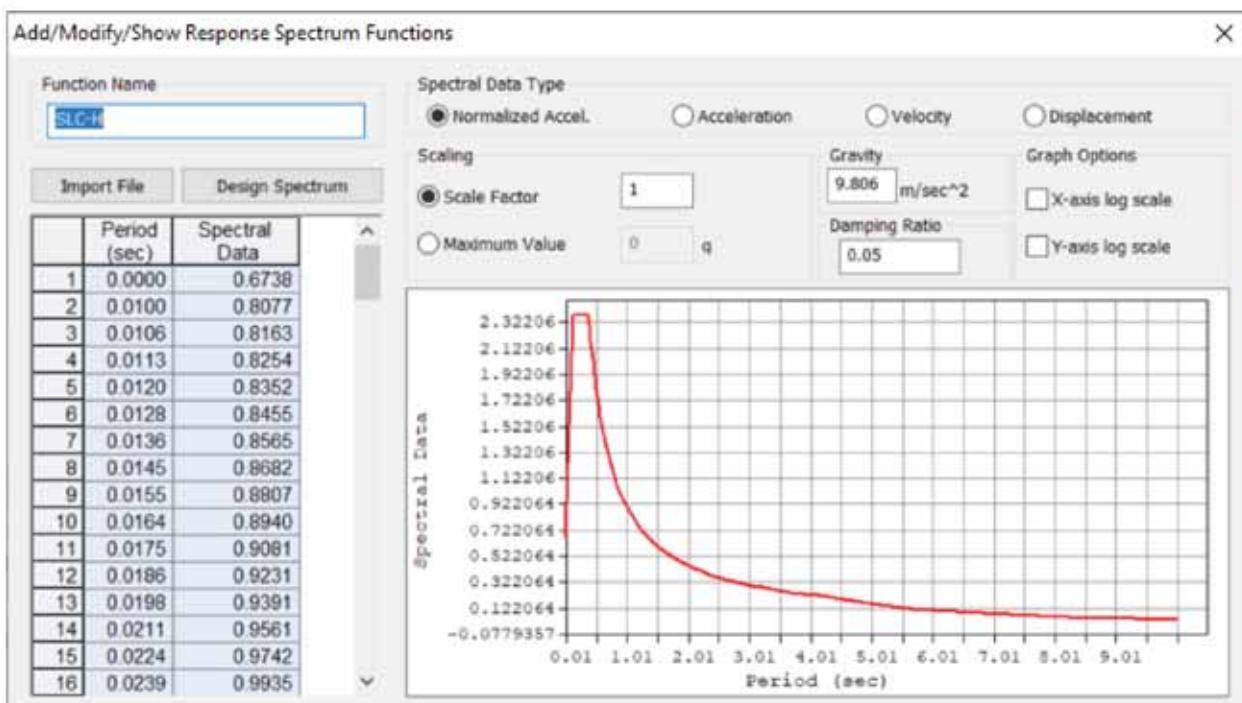
Chilometrica	Categoria di sottosuolo NTC	Categoria topografica	S <sub>T</sub>	S <sub>b</sub>	ag	F0	Tc*	Latitudine	Longitudine
1+459	RSL	T1							
3+005	B	T1	1,00	1,116	0,301	2,361	0,454	36,959319	14,669724
5+204	B	T1	1,00	1,105	0,312	2,36	0,455	36,975768	14,67698
12+235	RSL	T1							
15+818	RSL	T1							
0+553	RSL	T1							
1+259	B	T1	1,00	1,087	0,332	2,355	0,456	37,091473	14,63782
2+065	B	T1	1,00	1,086	0,333	2,356	0,457	37,098526	14,640441
2+781	B	T1	1,00	1,085	0,334	2,357	0,457	37,104181	14,643716
4+182	B	T1	1,00	1,081	0,337	2,36	0,458	37,114961	14,650551
8+318	B	T1	1,00	1,075	0,344	2,364	0,461	37,147444	14,671512
8+746	B	T1	1,00	1,075	0,344	2,363	0,462	37,151077	14,673177

Gli spettri di risposta utilizzati nel programma di calcolo sono rappresentati nelle figure seguenti e sono stati determinati da un analisi sismica locale del sito:

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Node	Mode	UX	UY	UZ	RX	RY	RZ
<b>EIGENVALUE ANALYSIS</b>							
	Mode No	Frequency (rad/sec)	Frequency (cycle/sec)	Period (sec)	Tolerance		
1	6.005995	0.955884	1.046152	0.0000e+000			
2	6.129563	0.975550	1.025063	0.0000e+000			
3	10.475246	1.667187	0.599813	0.0000e+000			
4	14.718550	2.342530	0.426889	0.0000e+000			
5	17.977806	2.861225	0.349501	0.0000e+000			
6	50.076717	7.965957	0.125471	0.0000e+000			
7	58.521622	9.314005	0.107385	0.0000e+000			
8	68.042501	10.829300	0.092342	0.0000e+000			
9	95.765787	15.241596	0.065610	6.2767e-056			
10	113.897373	18.127330	0.055185	2.8109e-047			
<b>MODAL PARTICIPATION MASSES PRINTOUT</b>							
	Mode No	TRAN-X	TRAN-Y	TRAN-Z	ROTN-X	ROTN-Y	ROTN-Z
		MASS(%) SUM(%)	MASS(%) SUM(%)	MASS(%) SUM(%)	MASS(%) SUM(%)	MASS(%) SUM(%)	MASS(%) SUM(%)
1	0.00	0.00 99.94	99.94 99.94	0.00 0.00	0.03 0.03	0.00 0.00	0.00 0.00
2	100.00	100.00 0.00	0.00 99.94	0.00 0.00	0.00 0.03	0.00 0.00	0.00 0.00
3	0.00	100.00 0.00	99.94 0.00	0.00 0.00	0.00 0.03	0.00 0.00	99.99 99.99
4	0.00	100.00 0.00	99.94 81.58	81.58 81.58	0.00 0.03	0.00 0.00	0.00 99.99
5	0.00	100.00 0.04	99.98 0.00	81.58 79.44	79.47 79.47	0.00 0.00	0.00 99.99
6	0.00	100.00 0.00	99.98 0.00	81.58 79.47	79.47 61.47	61.47 61.47	0.00 99.99
7	0.00	100.00 0.00	99.98 81.58	81.58 79.47	79.47 0.00	61.47 61.47	0.00 99.99
8	0.00	100.00 0.02	100.00 0.00	81.58 80.45	80.45 0.00	61.47 61.47	0.00 99.99
9	0.00	100.00 0.00	100.00 8.58	8.58 90.16	90.16 0.00	80.45 80.45	0.00 99.99
10	0.00	100.00 0.00	100.00 0.00	90.16 8.41	8.41 88.85	88.85 0.00	61.47 61.47
	Mode No	TRAN-X	TRAN-Y	TRAN-Z	ROTN-X	ROTN-Y	ROTN-Z
		MASS SUM	MASS SUM	MASS SUM	MASS SUM	MASS SUM	MASS SUM
1	0.00	0.00 475.82	475.82 0.00	0.00 0.90	0.90 0.90	0.00 0.00	0.00 0.00
2	476.11	476.11 0.00	475.82 0.00	0.00 0.90	0.90 0.97	0.97 0.97	0.00 0.00
3	0.00	476.11 0.00	475.82 0.00	0.00 0.90	0.90 0.00	0.97 0.97	79054.83 79054.83
4	0.00	476.11 0.00	475.82 388.44	388.44 0.00	0.90 0.90	0.97 0.00	79054.83 79054.83
5	0.00	476.11 0.20	476.02 0.00	388.44 2724.18	2724.18 2725.09	0.00 0.97	0.00 0.97
6	0.01	476.12 0.00	476.02 0.00	388.44 0.00	2725.09 46884.35	46884.35 46885.32	0.00 0.00
7	0.00	476.12 0.00	476.02 0.00	388.44 0.00	2725.09 46885.32	46885.32 0.05	46885.32 79054.83
8	0.00	476.12 0.10	476.12 0.00	388.44 33.50	33.50 2758.59	2758.59 0.00	46885.32 0.00
9	0.00	476.12 0.00	476.12 40.86	40.86 429.29	429.29 0.00	2758.59 46885.32	46885.32 0.00
10	0.00	476.12 0.00	476.12 0.00	429.29 288.28	288.28 3046.85	3046.85 0.00	46885.32 79054.83

E' stata eseguita un'analisi modale a spettro di risposta. Come si nota dall'immagine sopra riportata, avendo introdotto come appoggi degli isolatori, le azioni sismiche orizzontali si dimostrano praticamente disaccoppiate. In altre parole, andando ad analizzare i modi di vibrare che "muovono" maggior quantitativo di masse, si nota che il modo di vibrare numero 1 e 2 muovono massa in direzione rispettivamente Y ed X in modo disaccoppiato.

## 6 MODELLAZIONE STRUTTURALE

La modellazione strutturale è stata effettuata tramite il programma di calcolo Midas-Civil, schematizzando l'opera come un graticcio equivalente.

Trattandosi di un impalcato a sezione mista acciaio-cls, nell'ambito della modellazione, sono state introdotte le fasi di maturazione del cls necessarie per le verifiche degli elementi portanti e nel dettaglio:

- ✓ Fase1 : Presenza della sola struttura in acciaio soggetta al solo peso proprio strutturale (durata per la sola fase di varo)

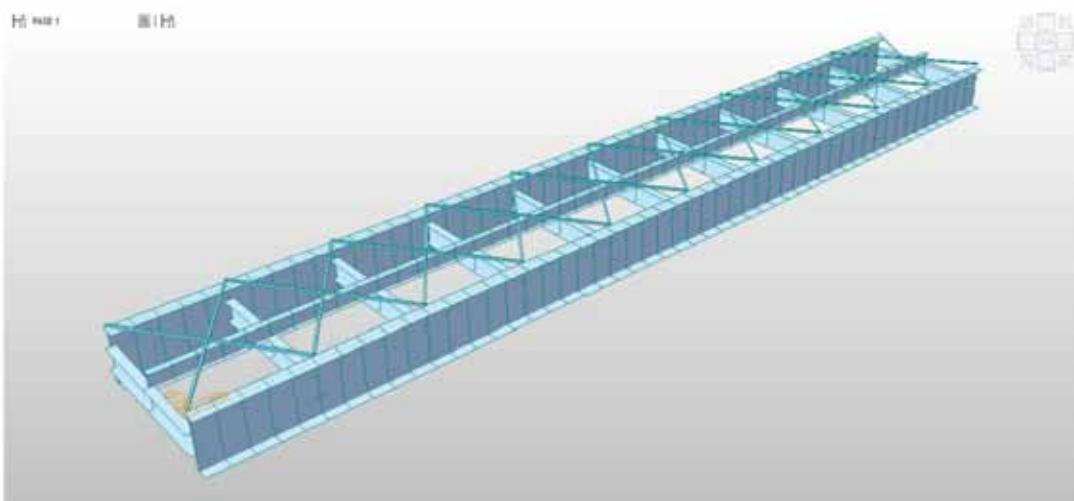


Figura 15 - Schema struttura in Fase 1

- ✓ Fase2: Presenza della struttura in acciaio e della soletta (non ancora collaborante) ma che interviene solo come peso portato (durata della fase 28giorni).
- ✓ Fase 3: Fase in cui hanno corso gli effetti reologici del calcestruzzo e in cui viene posato anche il carico permanente portato (durata giorni – fino alla fine degli effetti reologici).

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

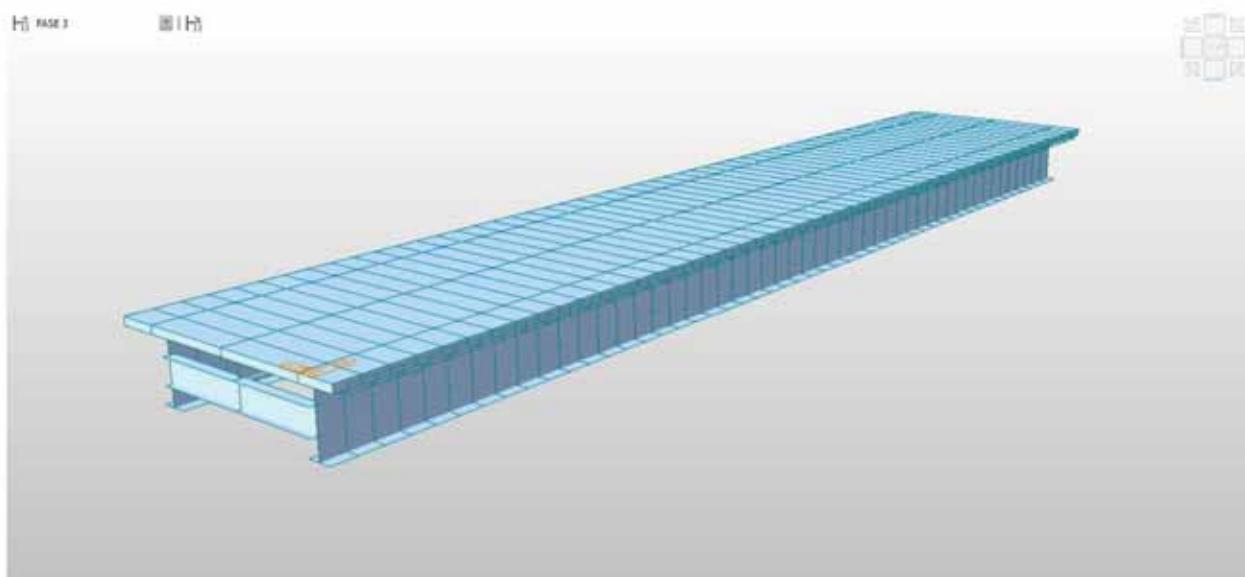


Figura 16 - Schema struttura in Fase 2-3

Inserendo in modo opportuno le varie curve di maturazione e ritiro del calcestruzzo, il programma di calcolo tiene in conto in automatico degli effetti reologici nel tempo. Pertanto, nelle varie fasi di carico, e di maturazione del getto, si instaurano effetti dovuti a ritiro e viscosità che vengono tenuti in conto in automatico dal programma di calcolo nell'ambito delle singole fasi introdotte. La fase 3, che ha una durata effettiva di circa 10000 giorni, ha lo scopo di esaurire tutti gli effetti reologici del calcestruzzo e precede la successiva fase di applicazione dei carichi variabili da traffico e meteorologici.

Construction Stage

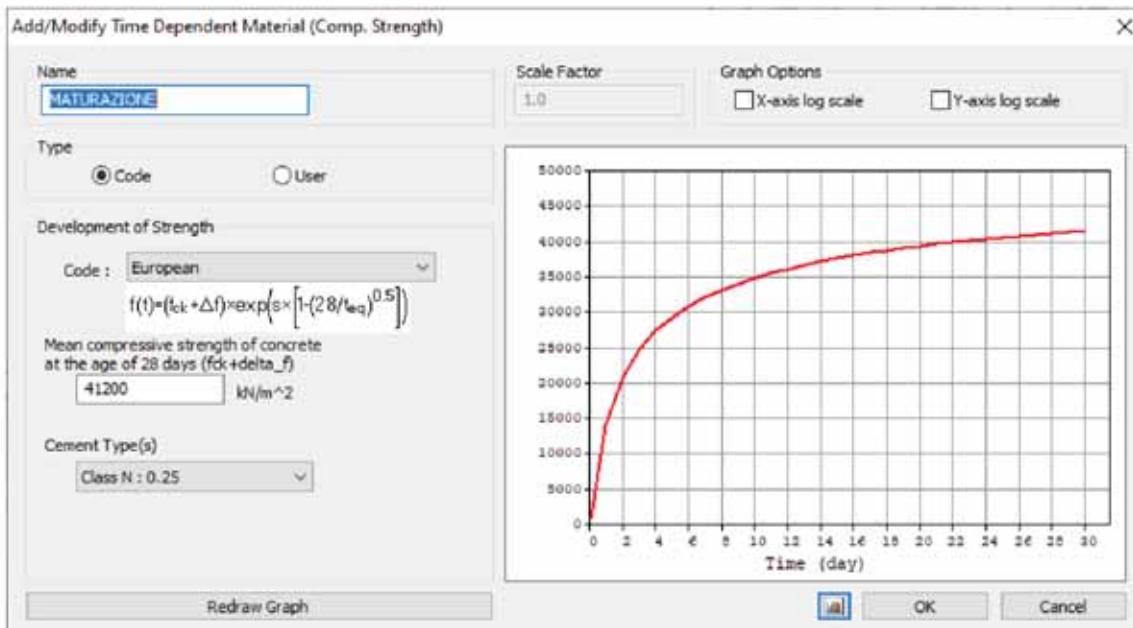
Name	Duration	Date	Step	Result
FASE 1	2	2	0	Stage
FASE 2	28	30	0	Stage
FASE 3	10000	10030	0	Stage

X

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Add/Modify Time Dependent Material (Creep / Shrinkage)

Name :	C+S	Code :	European
European			
Characteristic compressive cylinder strength of concrete at the age of 28 days ( $f_{ck}$ ) :		32000	kN/m <sup>2</sup>
Relative Humidity of ambient environment (40 - 99) :		70	%
Nominal size of member :		0.25	m
$h = 2 * A_c / u$ ( $A_c$ : Section Area, $u$ : Perimeter in contact with atmosphere)			
Type of cement	<input type="radio"/> Class S	<input checked="" type="radio"/> Class N	<input type="radio"/> Class R
Type of code	<input type="radio"/> EN 1992-1 (General Structure)	<input checked="" type="radio"/> EN 1992-2 (Concrete Bridge)	<input type="checkbox"/> Use of silica-fume
Age of concrete at the beginning of shrinkage :	3	day	
<input type="button" value="Show Result..."/> <input type="button" value="OK"/> <input type="button" value="Cancel"/> <input type="button" value="Apply"/>			



Per quanto riguarda la soletta, la stessa viene modellata con elementi tipo frame di dimensioni pari all'interasse previsto, e viene utilizzata per l'applicazione in senso trasversale dei carichi mobili. In altre parole, i carichi mobili vengono ripartiti lungo gli elementi principali tramite la soletta.

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

## 6.1 COMBINAZIONI DI CARICO

Le combinazioni di carico considerate sono quelle desunte secondo lo schema seguente.

Gruppo di azioni	Carichi sulla carreggiata						Carichi su marciapiedi e piste ciclabili
	Carichi verticali			Carichi orizzontali		Carichi verticali	
	Modello principale (Schema di carico 1, 2, 3, 4, 6)	Veicoli speciali	Folla (Schema di carico 5)	Frenatura $q_3$	Forza centrifuga $q_4$	Carico uniformemente distribuito	
1	Valore caratteristico						Schema di carico 5 con valore di combinazione 2,5 kN/m <sup>2</sup>
2 a	Valore frequente			Valore caratteristico			
2 b	Valore frequente				Valore caratteristico		
3 (*)							Schema di carico 5 con valore caratteristico 5,0 kN/m <sup>2</sup>
4 (**)			Schema di carico 5 con valore caratteristico 5,0 kN/m <sup>2</sup>				Schema di carico 5 con valore caratteristico 5,0 kN/m <sup>2</sup>
5 (***)	Da definirsi per il singolo progetto	Valore caratteristico o nominale					

(\*) Punti di 3<sup>a</sup> categoria  
(\*\*) Da considerare solo se richiesto dal particolare progetto (ad es. ponti in zona urbana)  
(\*\*\*) Da considerare solo se si considerano veicoli speciali

I coefficienti parziali di sicurezza per le combinazioni di carico SLU sono i seguenti.

		Coefficiente	EQU <sup>(1)</sup>	A1 STR	A2 GEO
Carichi permanenti	favorevoli sfavorevoli	$\gamma_{G1}$	0,90 1,10	1,00 1,35	1,00 1,00
Carichi permanenti non strutturali <sup>(2)</sup>	favorevoli sfavorevoli	$\gamma_{G2}$	0,00 1,50	0,00 1,50	0,00 1,30
Carichi variabili da traffico	favorevoli sfavorevoli	$\gamma_Q$	0,00 1,35	0,00 1,35	0,00 1,15
Carichi variabili	favorevoli sfavorevoli	$\gamma_{Q1}$	0,00 1,50	0,00 1,50	0,00 1,30
Distorsioni e presollecitazioni di progetto	favorevoli sfavorevoli	$\gamma_{\epsilon 1}$	0,90 1,00 <sup>(3)</sup>	1,00 1,00 <sup>(4)</sup>	1,00 1,00
Ritiro e viscosità, Variazioni termiche, Cedimenti vincolari	favorevoli sfavorevoli	$\gamma_{\epsilon 2}, \gamma_{\epsilon 3}, \gamma_{\epsilon 4}$	0,00 1,20	0,00 1,20	0,00 1,00

(1) Equilibrio che non coinvolga i parametri di deformabilità e resistenza del terreno; altrimenti si applicano i valori di GEO.  
(2) Nel caso in cui i carichi permanenti non strutturali (ad es. carichi permanenti portati) siano compiutamente definiti si potranno adottare gli stessi coefficienti validi per le azioni permanenti.  
(3) 1,30 per instabilità in strutture con precompressione esterna.  
(4) 1,20 per effetti locali

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

I coefficienti  $\psi$  per le azioni variabili per i ponti stradali sono i seguenti.

Azioni	Gruppo di azioni (Tabella 5.IV)	Coefficiente $\Psi_0$ di combinazione	Coefficiente $\Psi_1$ (valori frequenti)	Coefficiente $\Psi_2$ (valori quasi permanenti)
Azioni da traffico (Tabella 5.IV)	Schema 1 (Carichi tandem)	0,75	0,75	0,0
	Schemi 1, 5 e 6 (Carichi distribuiti)	0,40	0,40	0,0
	Schemi 3 e 4 (carichi concentrati)	0,40	0,40	0,0
	Schema 2	0,0	0,75	0,0
	2	0,0	0,0	0,0
	3	0,0	0,0	0,0
	4 (folla)	----	0,75	0,0
	5	0,0	0,0	0,0
	Vento a ponte scarico			
	SLU e SLE	0,6	0,2	0,0
Vento $q_3$	Esecuzione	0,8	----	0,0
	Vento a ponte carico	0,6		
Neve $q_3$	SLU e SLE	0,0	0,0	0,0
	esecuzione	0,8	0,6	0,5
Temperatura	T <sub>k</sub>	0,6	0,6	0,5

Si riporta di seguito una tabella raffigurante tutte le combinazioni di carico utilizzate.

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

		PESO PROPRIO ACCIAIO	PESO PROPRIO SOLLETTA	PERMANENTE PORTATO	VENTO PONTE SCARICO	VENTO PONTE CARICO	TERMINA UNIFORME +	TERMINA UNIFORME -	TERMINA GRADIENTE +	TERMINA GRADIENTE -	FRENATURA	LM01	LM02	LM03	SISMA X	SISMA Y	SISMA Z
SLU	SLU1	1.35	1.35	1.35	1.5	0	0.9	0	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU2	1.35	1.35	1.35	1.5	0	0	0.9	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU3	1.35	1.35	1.35	1.5	0	0	0	0.9	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU4	1.35	1.35	1.35	1.5	0	0	0	0	0.9	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU5	1.35	1.35	1.35	0	1.5	0.9	0	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU6	1.35	1.35	1.35	0	1.5	0	0.9	0	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU7	1.35	1.35	1.35	0	1.5	0	0	0.9	0	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU8	1.35	1.35	1.35	0	1.5	0	0	0	0.9	1.0125	1.0125	1.0125	1.0125	0	0	0
	SLU9	1.35	1.35	1.35	0.9	0	0.9	0	0	0	1.35	1.35	0	0	0	0	0
	SLU10	1.35	1.35	1.35	0.9	0	0	0.9	0	0	1.35	1.35	0	0	0	0	0
	SLU11	1.35	1.35	1.35	0.9	0	0	0	0.9	0	1.35	1.35	0	0	0	0	0
	SLU12	1.35	1.35	1.35	0.9	0	0	0	0	0.9	1.35	1.35	0	0	0	0	0
	SLU13	1.35	1.35	1.35	0	0.9	0.9	0	0	0	1.35	1.35	0	0	0	0	0
	SLU14	1.35	1.35	1.35	0	0.9	0	0.9	0	0	1.35	1.35	0	0	0	0	0
	SLU15	1.35	1.35	1.35	0	0.9	0	0	0.9	0	1.35	1.35	0	0	0	0	0
	SLU16	1.35	1.35	1.35	0	0.9	0	0	0	0.9	1.35	1.35	0	0	0	0	0
	SLU17	1.35	1.35	1.35	0.9	0	0.9	0	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU18	1.35	1.35	1.35	0.9	0	0	0.9	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU19	1.35	1.35	1.35	0.9	0	0	0	0.9	0	1.35	1.35	1.35	1.35	0	0	0
	SLU20	1.35	1.35	1.35	0.9	0	0	0	0	0.9	1.35	1.35	1.35	1.35	0	0	0
	SLU21	1.35	1.35	1.35	0	0.9	0.9	0	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU22	1.35	1.35	1.35	0	0.9	0	0.9	0	0	1.35	1.35	1.35	1.35	0	0	0
	SLU23	1.35	1.35	1.35	0	0.9	0	0	0.9	0	1.35	1.35	1.35	1.35	0	0	0
	SLU24	1.35	1.35	1.35	0	0.9	0	0	0	0.9	1.35	1.35	1.35	1.35	0	0	0
RARA	RARA_1	1	1	1	1	0	0.6	0	0	0	0.75	0.75	0	0	0	0	0
	RARA_2	1	1	1	1	0	0	0.6	0	0	0.75	0.75	0	0	0	0	0
	RARA_3	1	1	1	1	0	0	0	0.6	0	0.75	0.75	0	0	0	0	0
	RARA_4	1	1	1	1	0	0	0	0	0.6	0.75	0.75	0	0	0	0	0
	RARA_5	1	1	1	0	1	0.6	0	0	0	0.75	0.75	0	0	0	0	0
	RARA_6	1	1	1	0	1	0	0.6	0	0	0.75	0.75	0	0	0	0	0
	RARA_7	1	1	1	0	1	0	0	0.6	0	0.75	0.75	0	0	0	0	0
	RARA_8	1	1	1	0	1	0	0	0	0.6	0.75	0.75	0	0	0	0	0
	RARA_9	1	1	1	0.6	0	0.6	0	0	0	1	1	0	0	0	0	0
	RARA_10	1	1	1	0.6	0	0	0.6	0	0	1	1	0	0	0	0	0
	RARA_11	1	1	1	0.6	0	0	0	0.6	0	1	1	0	0	0	0	0
	RARA_12	1	1	1	0.6	0	0	0	0	0.6	1	1	0	0	0	0	0
	RARA_13	1	1	1	0	0.6	0.6	0	0	0	1	1	0	0	0	0	0
	RARA_14	1	1	1	0	0.6	0	0.6	0	0	1	1	0	0	0	0	0
	RARA_15	1	1	1	0	0.6	0	0	0.6	0	1	1	0	0	0	0	0
	RARA_16	1	1	1	0	0.6	0	0	0	0.6	1	1	0	0	0	0	0
FREQ	FREQ_1	1	1	1	0.2	0	0.5	0	0	0	0.75	0.75	0	0	0	0	0
	FREQ_2	1	1	1	0.2	0	0	0.5	0	0	0.75	0.75	0	0	0	0	0
	FREQ_3	1	1	1	0.2	0	0	0	0.5	0	0.75	0.75	0	0	0	0	0
	FREQ_4	1	1	1	0.2	0	0	0	0	0.5	0.75	0.75	0	0	0	0	0
	FREQ_5	1	1	1	0	0	0.5	0	0	0	0.75	0.75	0	0	0	0	0
	FREQ_6	1	1	1	0	0	0	0.5	0	0	0.75	0.75	0	0	0	0	0
	FREQ_7	1	1	1	0	0	0	0	0.5	0	0.75	0.75	0	0	0	0	0
	FREQ_8	1	1	1	0	0	0	0	0	0.5	0.75	0.75	0	0	0	0	0
	FREQ_9	1	1	1	0.2	0	0.5	0	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_10	1	1	1	0.2	0	0	0.5	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_11	1	1	1	0.2	0	0	0	0.5	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_12	1	1	1	0.2	0	0	0	0	0.5	0.75	0.75	0.75	0.75	0	0	0
	FREQ_13	1	1	1	0	0	0.5	0	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_14	1	1	1	0	0	0	0.5	0	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_15	1	1	1	0	0	0	0	0.5	0	0.75	0.75	0.75	0.75	0	0	0
	FREQ_16	1	1	1	0	0	0	0	0	0.5	0.75	0.75	0.75	0.75	0	0	0

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**GPI INGEGNERIA**  
BENTONIETTA INGENIERIA



**ICARIA**  
società di ingegneria



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

SISMA SVA	QP_1	1	1	1	0	0	0.5	0	0	0	0	0	0	0	0	0	0
	QP_2	1	1	1	0	0	0	0.5	0	0	0	0	0	0	0	0	0
	QP_3	1	1	1	0	0	0	0	0.5	0	0	0	0	0	0	0	0
	QP_4	1	1	1	0	0	0	0	0	0.5	0	0	0	0	0	0	0
	SLV1	1	1	1	0	0	0.5	0	0	0	0	0	0	1	0.3	0	0
	SLV2	1	1	1	0	0	0.5	0	0	0	0	0	0	-1	0.3	0	0
	SLV3	1	1	1	0	0	0.5	0	0	0	0	0	0	1	-0.3	0	0
	SLV4	1	1	1	0	0	0.5	0	0	0	0	0	0	-1	-0.3	0	0
	SLV5	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	1	0	0
	SLV6	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	-1	0	0
	SLV7	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	1	0	0
	SLV8	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	-1	0	0
	SLV9	1	1	1	0	0	0	0.5	0	0	0	0	0	1	0.3	0	0
	SLV10	1	1	1	0	0	0	0.5	0	0	0	0	0	-1	0.3	0	0
	SLV11	1	1	1	0	0	0	0.5	0	0	0	0	0	1	-0.3	0	0
	SLV12	1	1	1	0	0	0	0.5	0	0	0	0	0	-1	-0.3	0	0
	SLV13	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	1	0	0
	SLV14	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	-1	0	0
	SLV15	1	1	1	0	0	0	0.5	0	0	0	0	0	-0.3	1	0	0
	SLV16	1	1	1	0	0	0	0.5	0	0	0	0	0	-0.3	-1	0	0
	SLV17	1	1	1	0	0	0	0	0.5	0	0	0	0	1	0.3	0	0
	SLV18	1	1	1	0	0	0	0	0.5	0	0	0	0	-1	0.3	0	0
	SLV19	1	1	1	0	0	0	0	0.5	0	0	0	0	1	-0.3	0	0
	SLV20	1	1	1	0	0	0	0	0.5	0	0	0	0	-1	-0.3	0	0
	SLV21	1	1	1	0	0	0	0	0.5	0	0	0	0	0.3	1	0	0
	SLV22	1	1	1	0	0	0	0	0.5	0	0	0	0	0.3	-1	0	0
	SLV23	1	1	1	0	0	0	0	0.5	0	0	0	0	-0.3	1	0	0
	SLV24	1	1	1	0	0	0	0	0.5	0	0	0	0	-0.3	-1	0	0
	SLV25	1	1	1	0	0	0	0	0	0.5	0	0	0	1	0.3	0	0
	SLV26	1	1	1	0	0	0	0	0	0.5	0	0	0	-1	0.3	0	0
	SLV27	1	1	1	0	0	0	0	0	0.5	0	0	0	1	-0.3	0	0
	SLV28	1	1	1	0	0	0	0	0	0.5	0	0	0	-1	-0.3	0	0
	SLV29	1	1	1	0	0	0	0	0	0.5	0	0	0	0.3	1	0	0
	SLV30	1	1	1	0	0	0	0	0	0.5	0	0	0	0.3	-1	0	0
	SLV31	1	1	1	0	0	0	0	0	0.5	0	0	0	-0.3	1	0	0
	SLV32	1	1	1	0	0	0	0	0	0.5	0	0	0	-0.3	-1	0	0
	SLV33	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	0.3	1	0
	SLV34	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	0.3	1	0
	SLV35	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	-0.3	1	0
	SLV36	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	-0.3	1	0
	SLV37	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	0.3	1	0
	SLV38	1	1	1	0	0	0	0.5	0	0	0	0	0	-0.3	0.3	1	0
	SLV39	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	-0.3	1	0
	SLV40	1	1	1	0	0	0	0.5	0	0	0	0	0	-0.3	-0.3	1	0
	SLV41	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	0.3	1	0
	SLV42	1	1	1	0	0	0	0.5	0	0	0	0	0	-0.3	0.3	1	0
	SLV43	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	-0.3	1	0
	SLV44	1	1	1	0	0	0	0.5	0	0	0	0	0	-0.3	-0.3	1	0
	SLV45	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	0.3	-1	0
	SLV46	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	0.3	-1	0
	SLV47	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	-0.3	-1	0
	SLV48	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	-0.3	-1	0
	SLV49	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	0.3	-1	0
	SLV50	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	0.3	-1	0
	SLV51	1	1	1	0	0	0.5	0	0	0	0	0	0	0.3	-0.3	-1	0
	SLV52	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	-0.3	-1	0
	SLV53	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	0.3	-1	0
	SLV54	1	1	1	0	0	0	0.5	0	0	0	0	0	-0.3	0.3	-1	0
	SLV55	1	1	1	0	0	0	0.5	0	0	0	0	0	0.3	-0.3	-1	0
	SLV56	1	1	1	0	0	0.5	0	0	0	0	0	0	-0.3	-0.3	-1	0
SISMA SLD	SLD1	1	1	1	0	0	0.5	0	0	0	0	0	0	1	0	0	0
	SLD2	1	1	1	0	0	0.5	0	0	0	0	0	0	-1	0	0	0
	SLD3	1	1	1	0	0	0.5	0	0	0	0	0	0	1	0	0	0
	SLD4	1	1	1	0	0	0.5	0	0	0	0	0	0	-1	0	0	0

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## 6.2 VERIFICHE STRUTTURALI

### 6.2.1 VERIFICA ELEMENTI DI IMPALCATO

Si riportano di seguito alcune immagini del modello 3D effettuato con la suddivisione in conci delle travi principali.

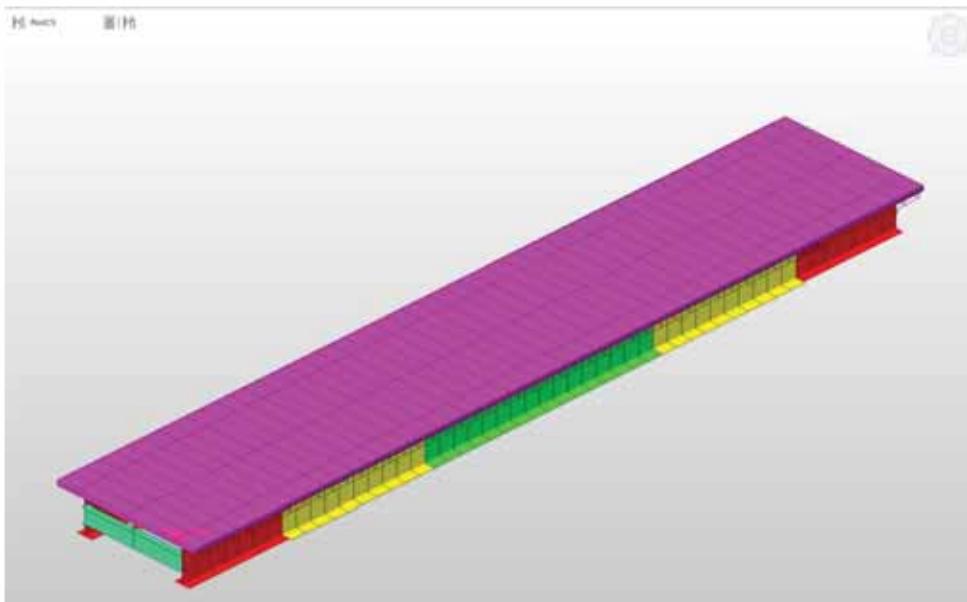


Figura 17 - Immagine modello 3D

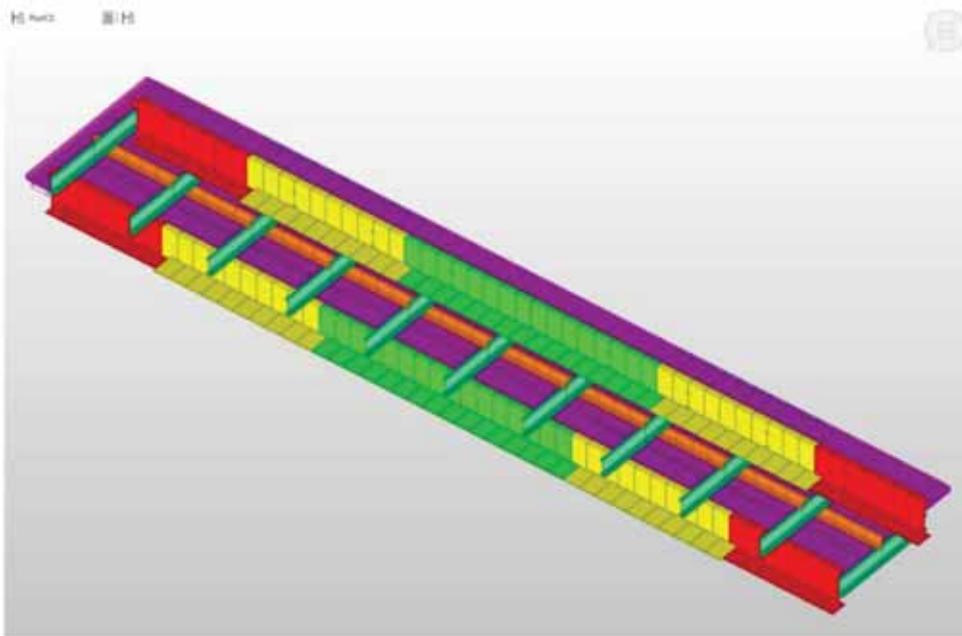


Figura 18 - Immagine modello 3D

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Si riportano inoltre i diagrammi delle sollecitazioni M, N e T nelle varie fasi operative.

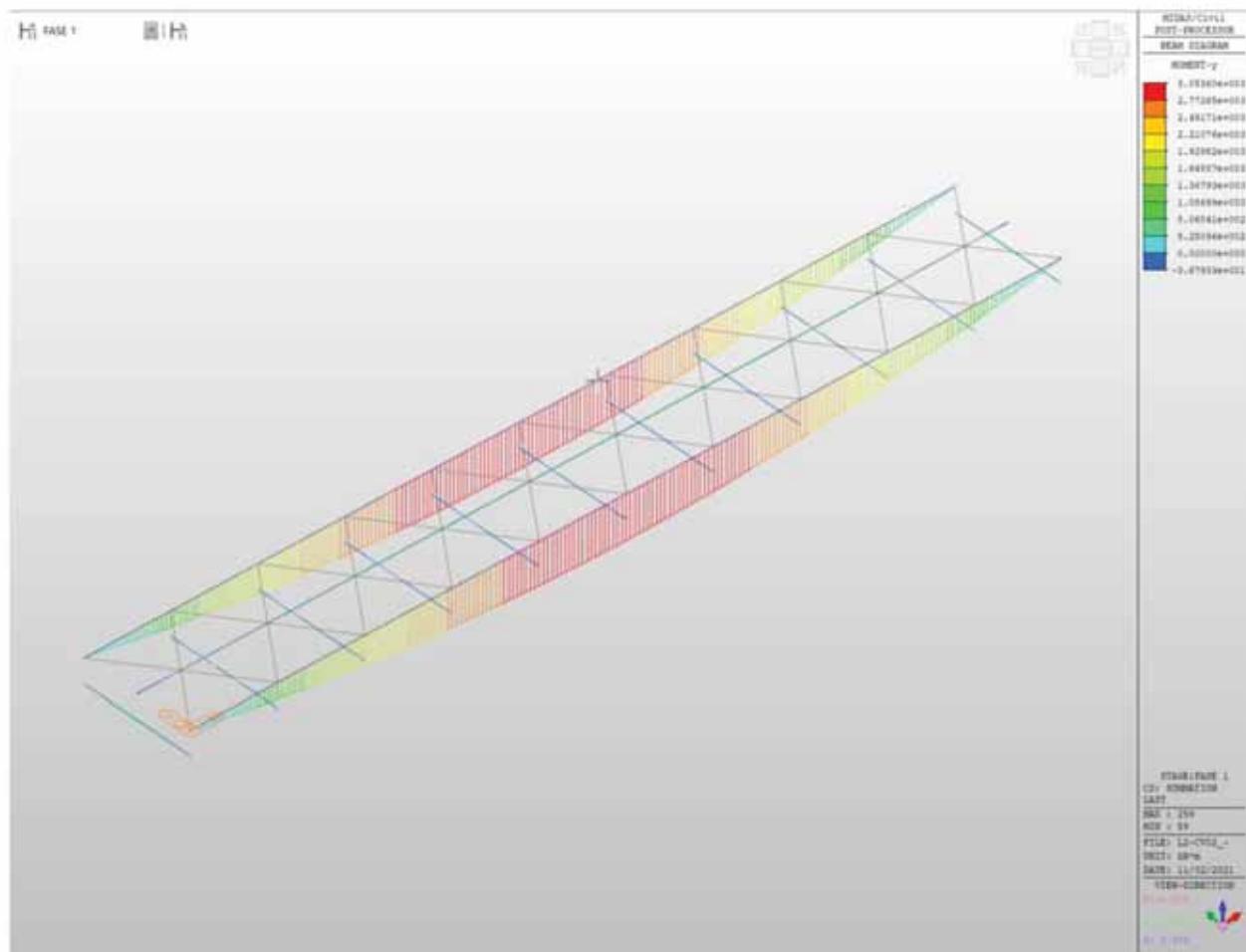
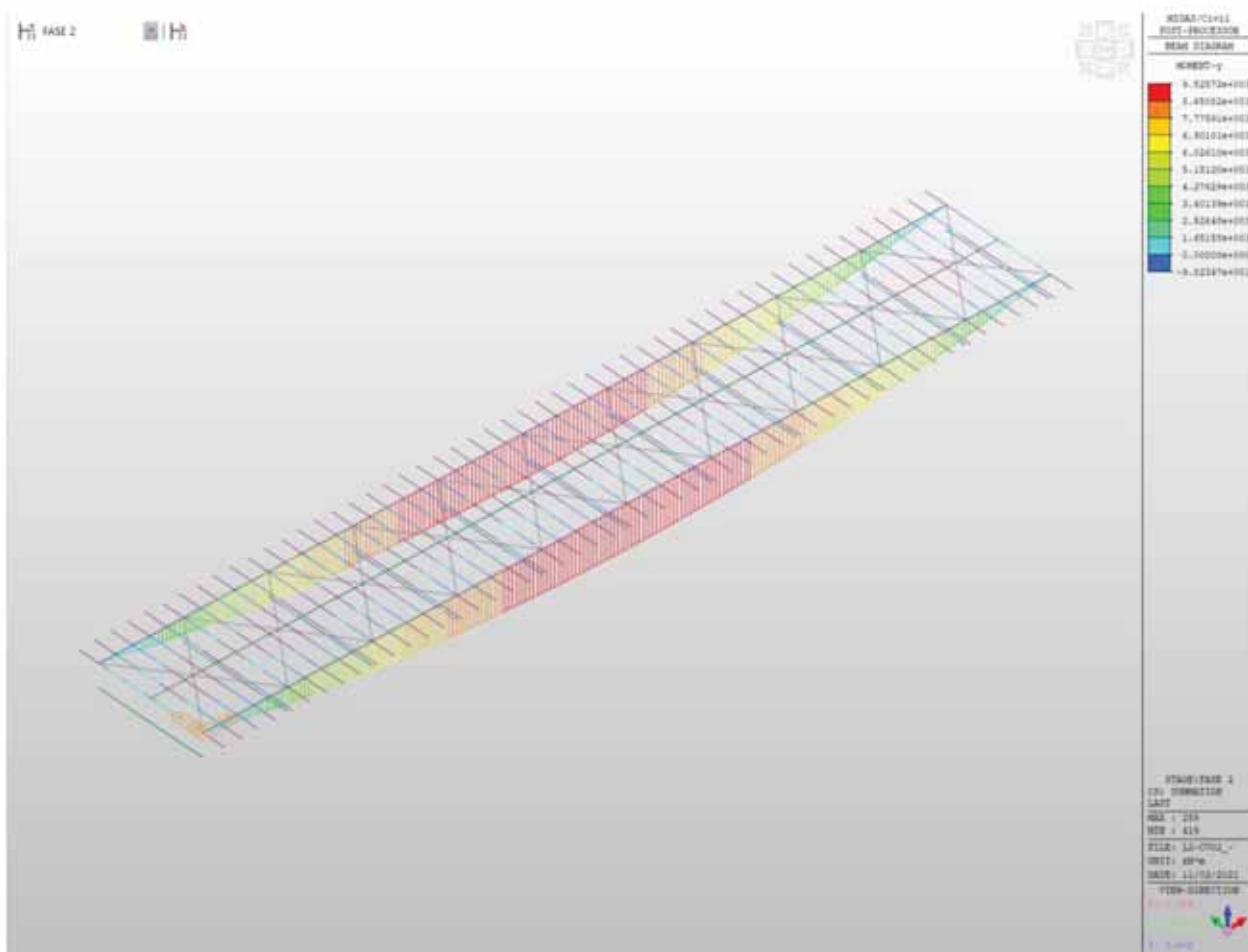


Figura 19 - Diagramma My - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

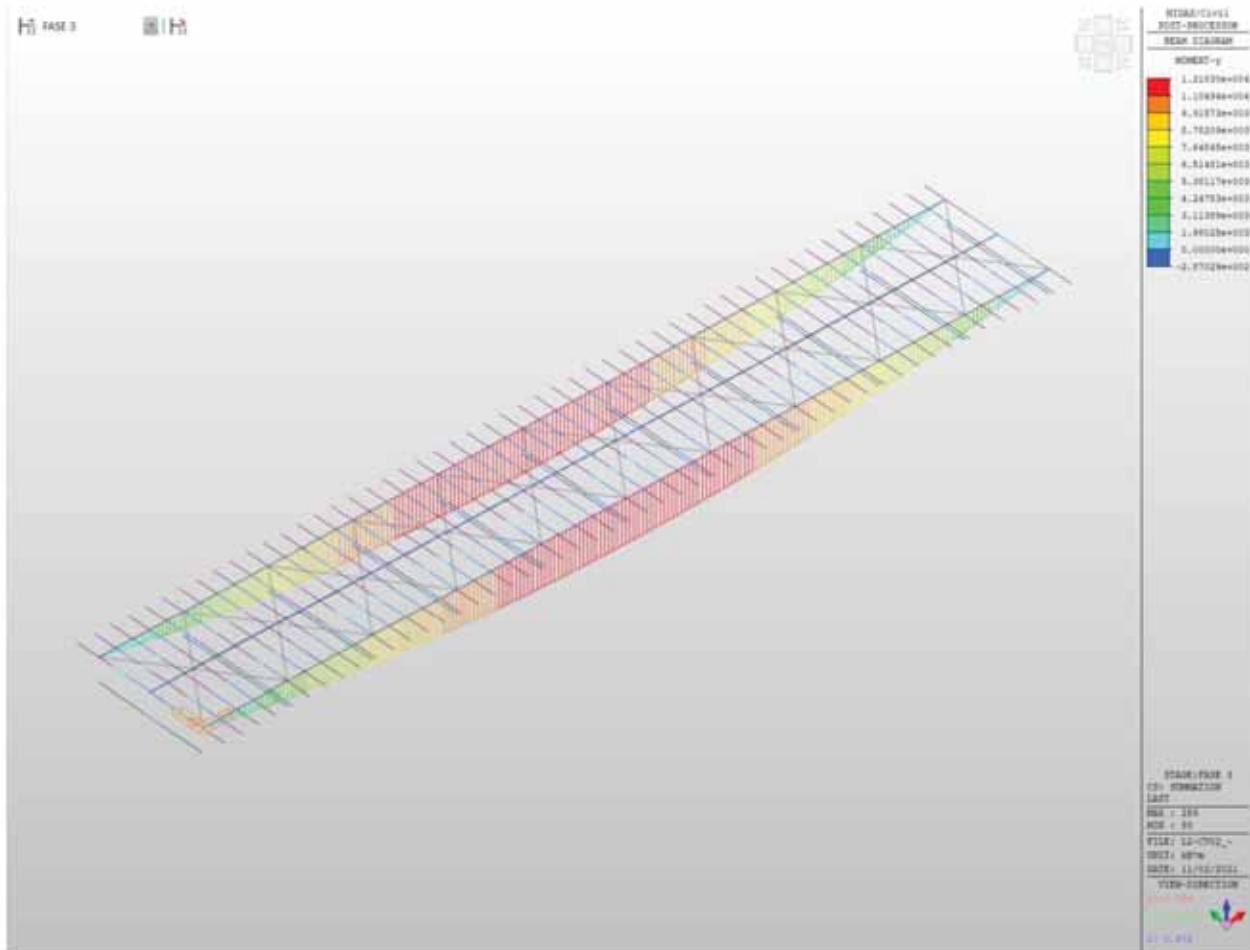


Figura 21 - Diagramma My - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

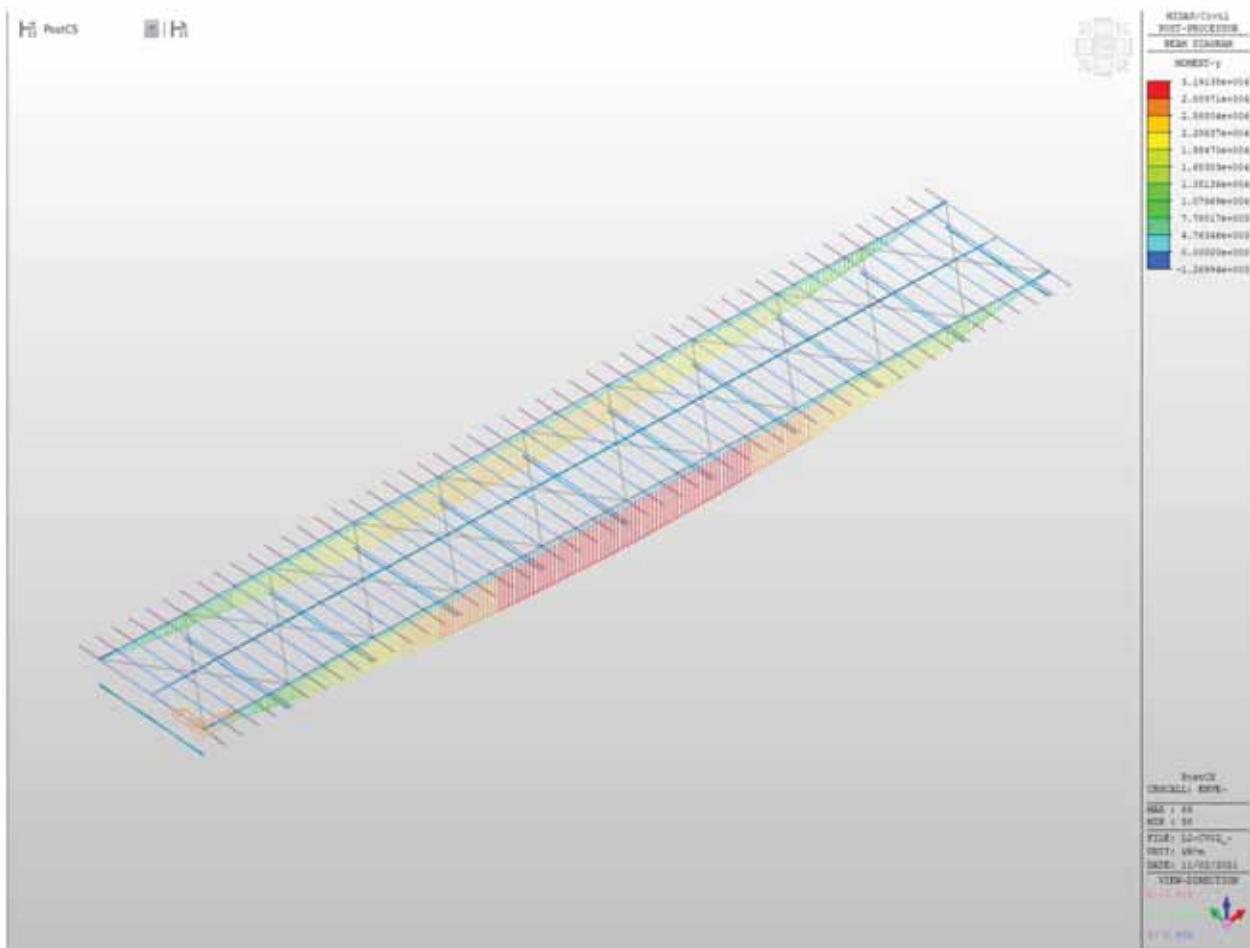
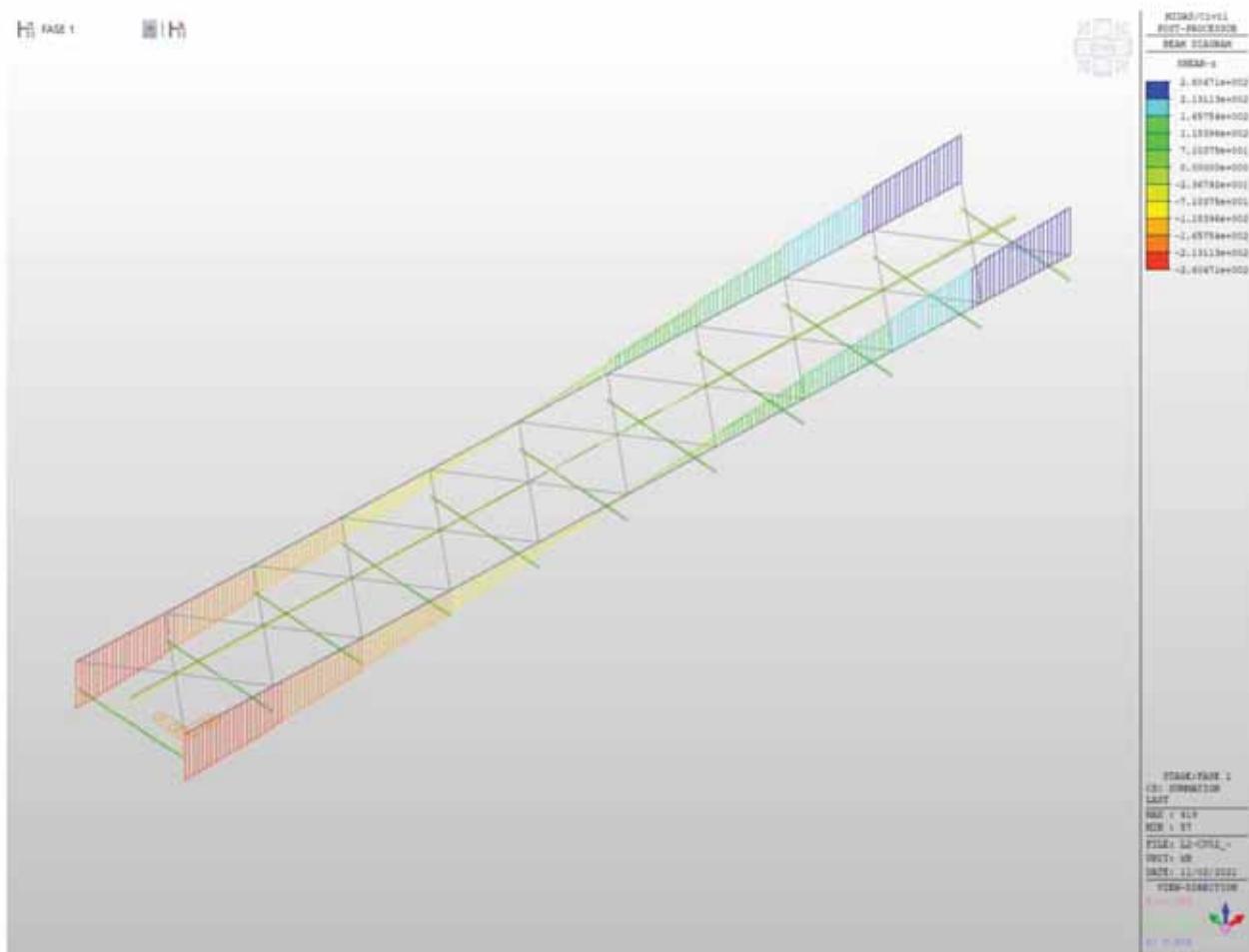


Figura 22 - Diagramma My – Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

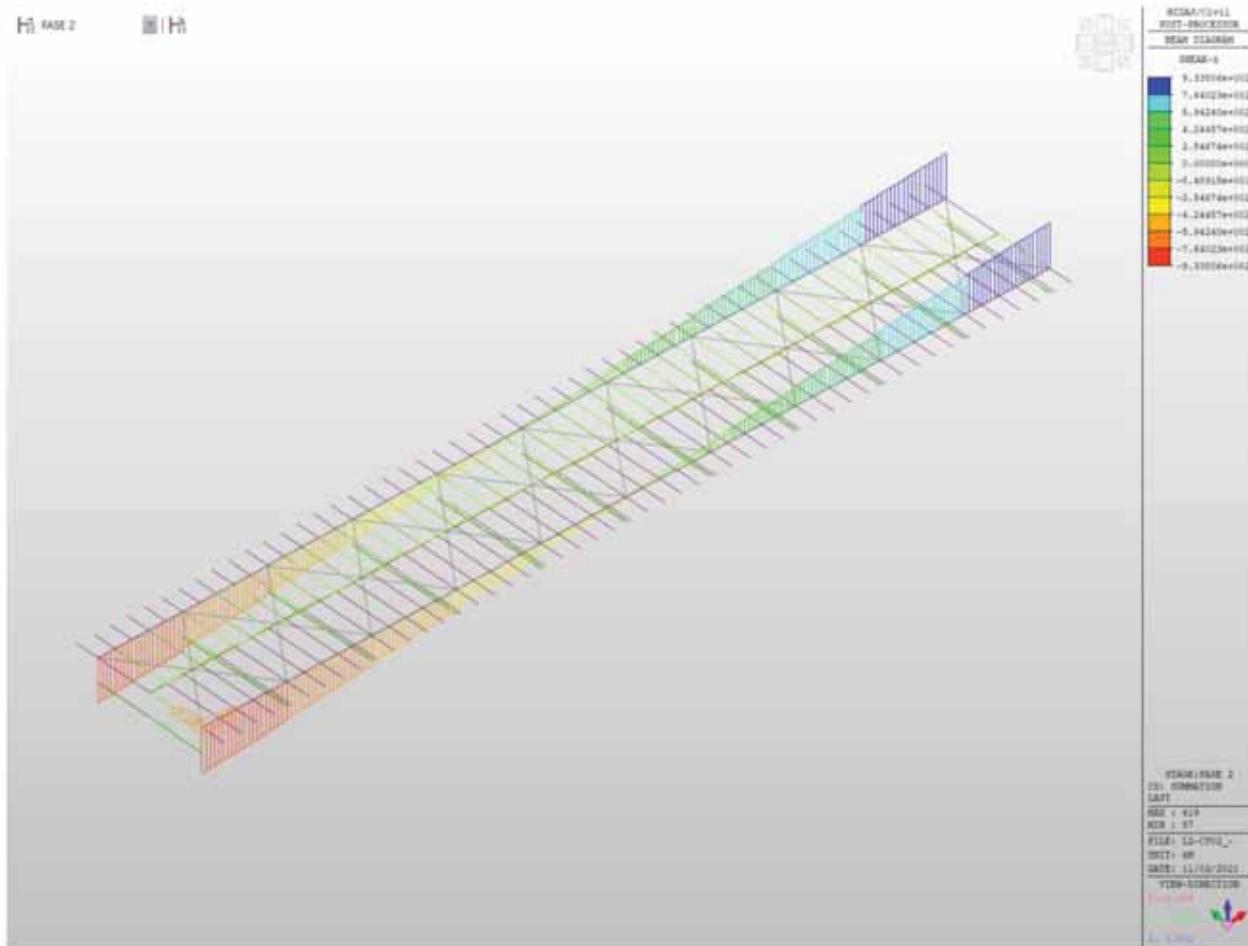
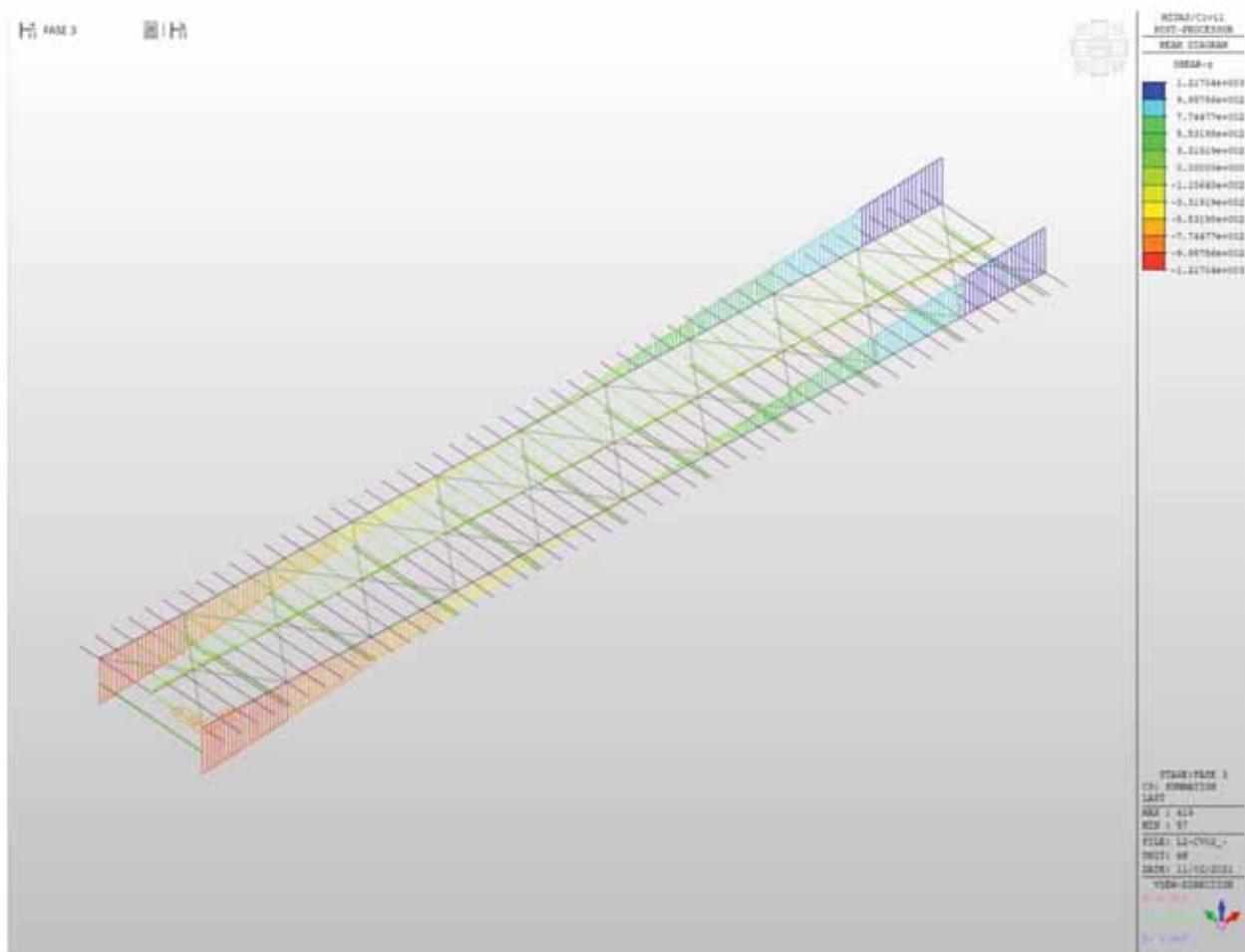


Figura 24 - Diagramma Tz - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



## LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO



Figura 26 - Diagramma Tz - Inviluppo SLU-SLV

## LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO

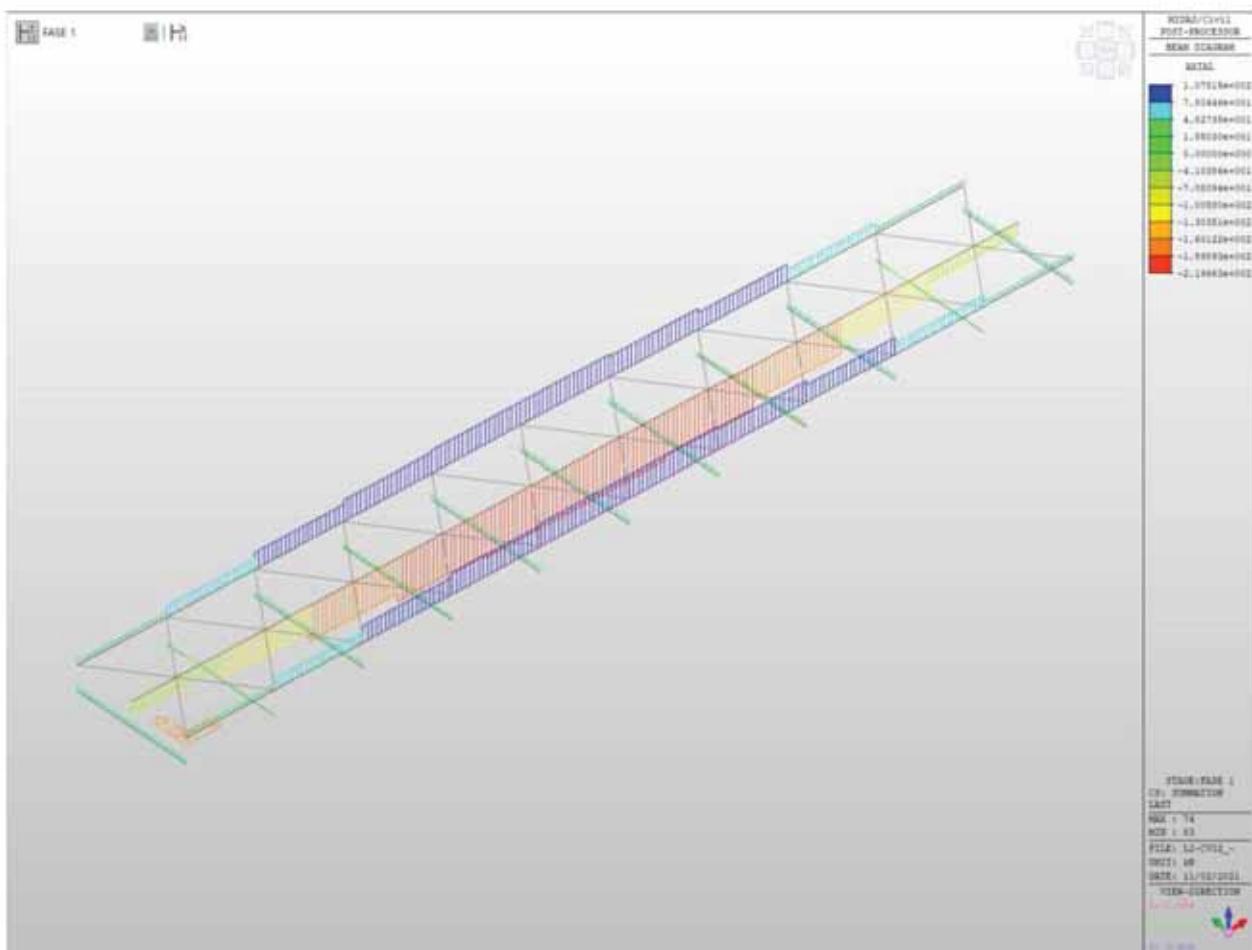


Figura 27 - Diagramma N - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

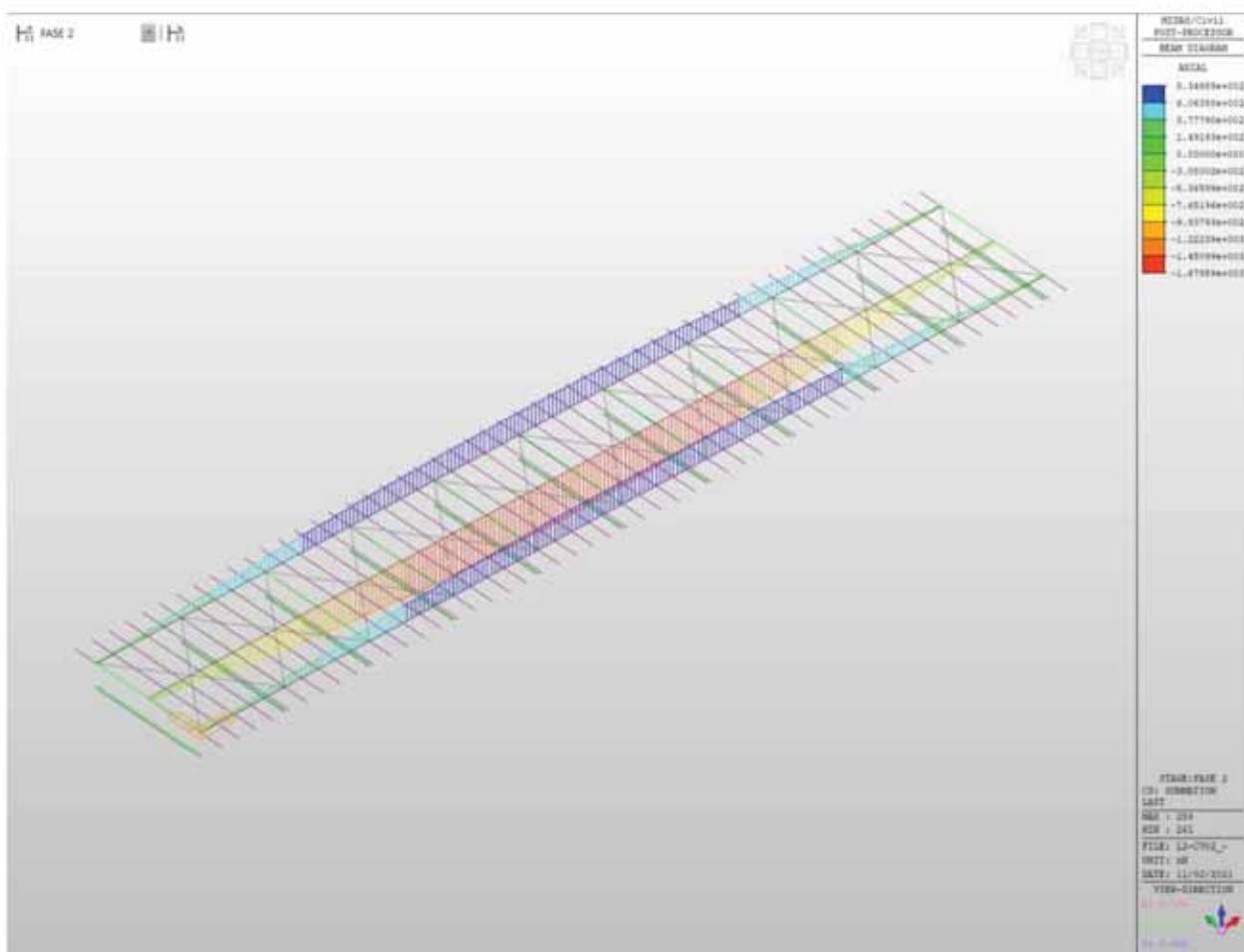
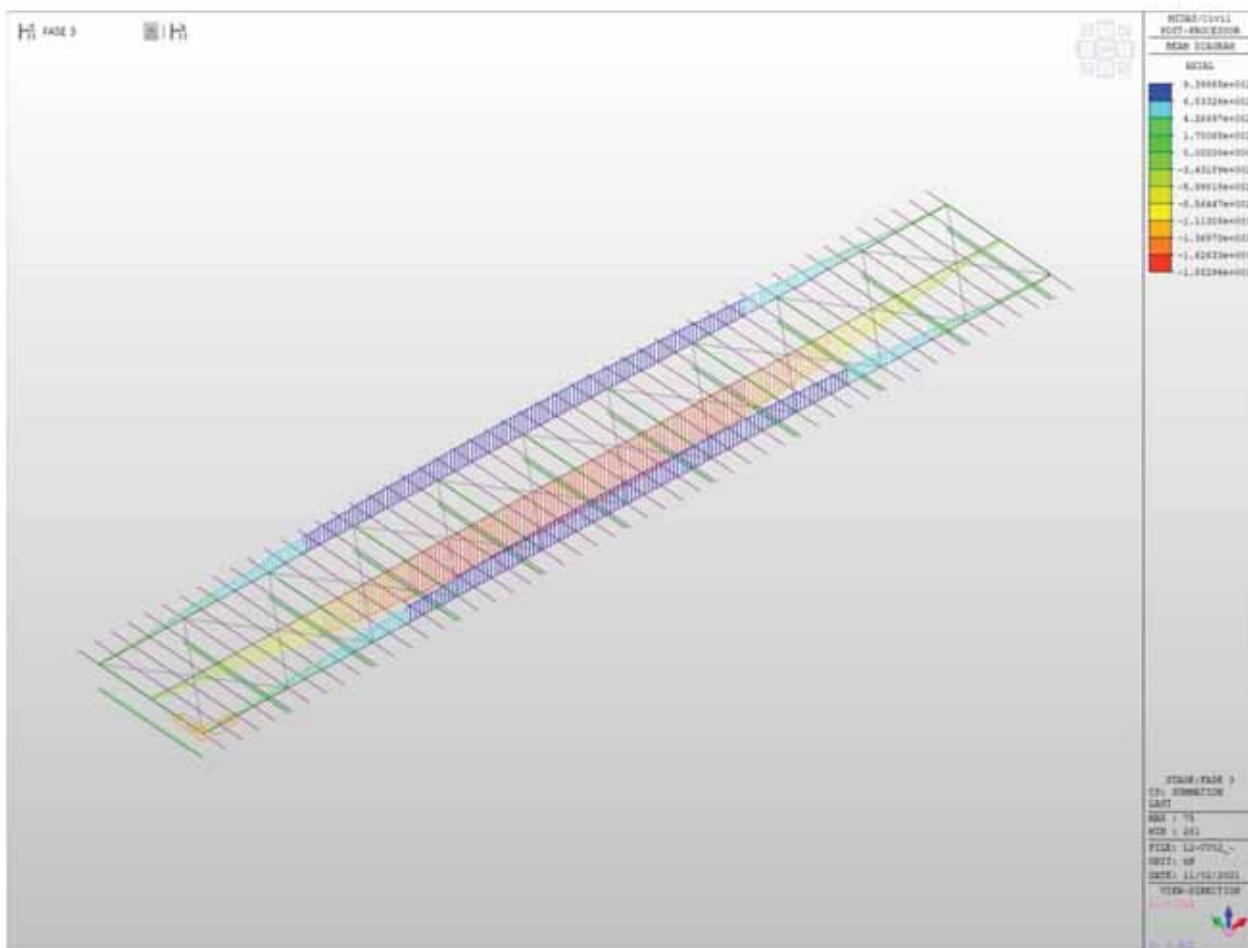


Figura 28 - Diagramma N - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

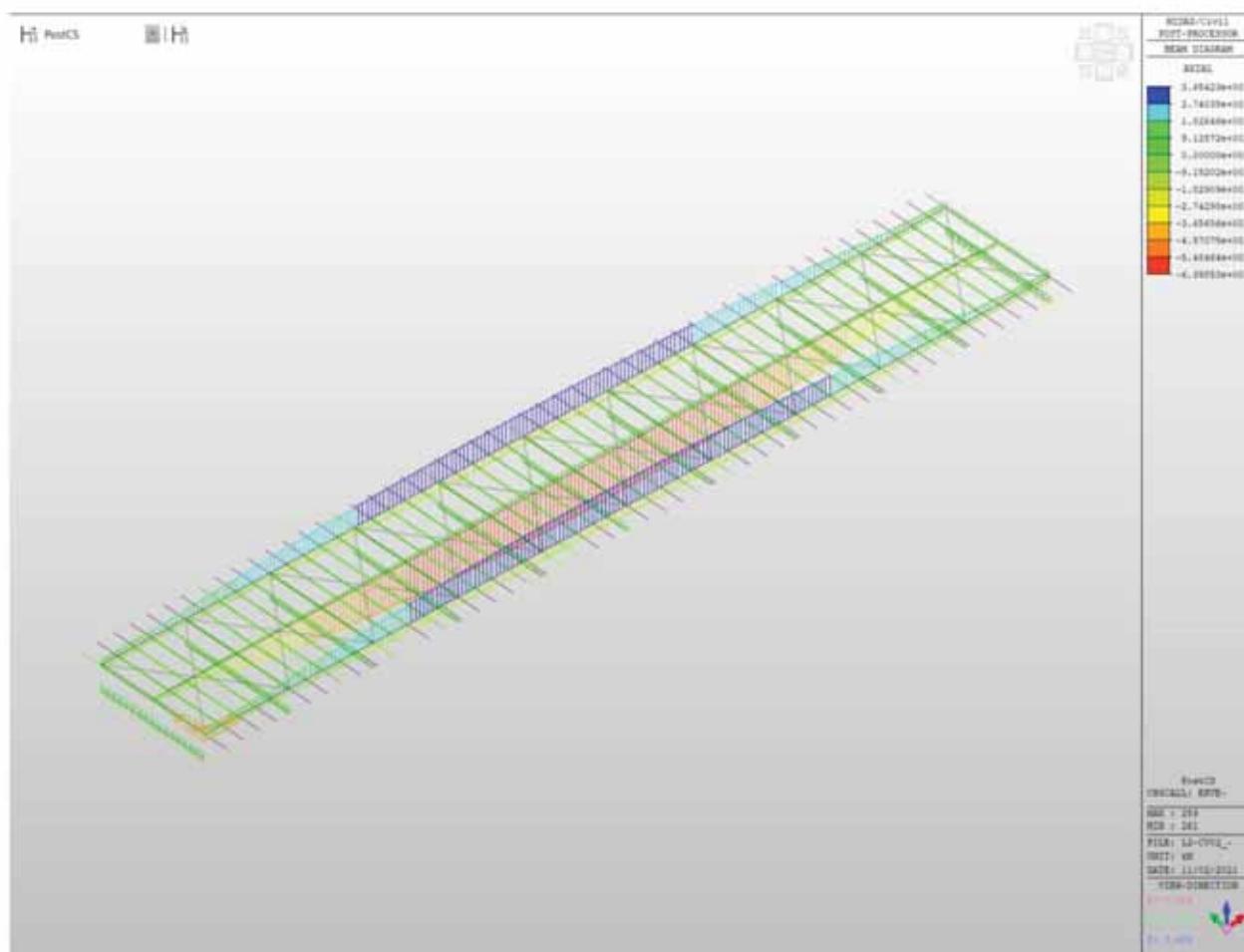


Figura 30 - Diagramma N - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**6.2.1.1 TRAVE PRINCIPALE CONCIO 1**



Figura 31 - Concio 1 - Diagramma My - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

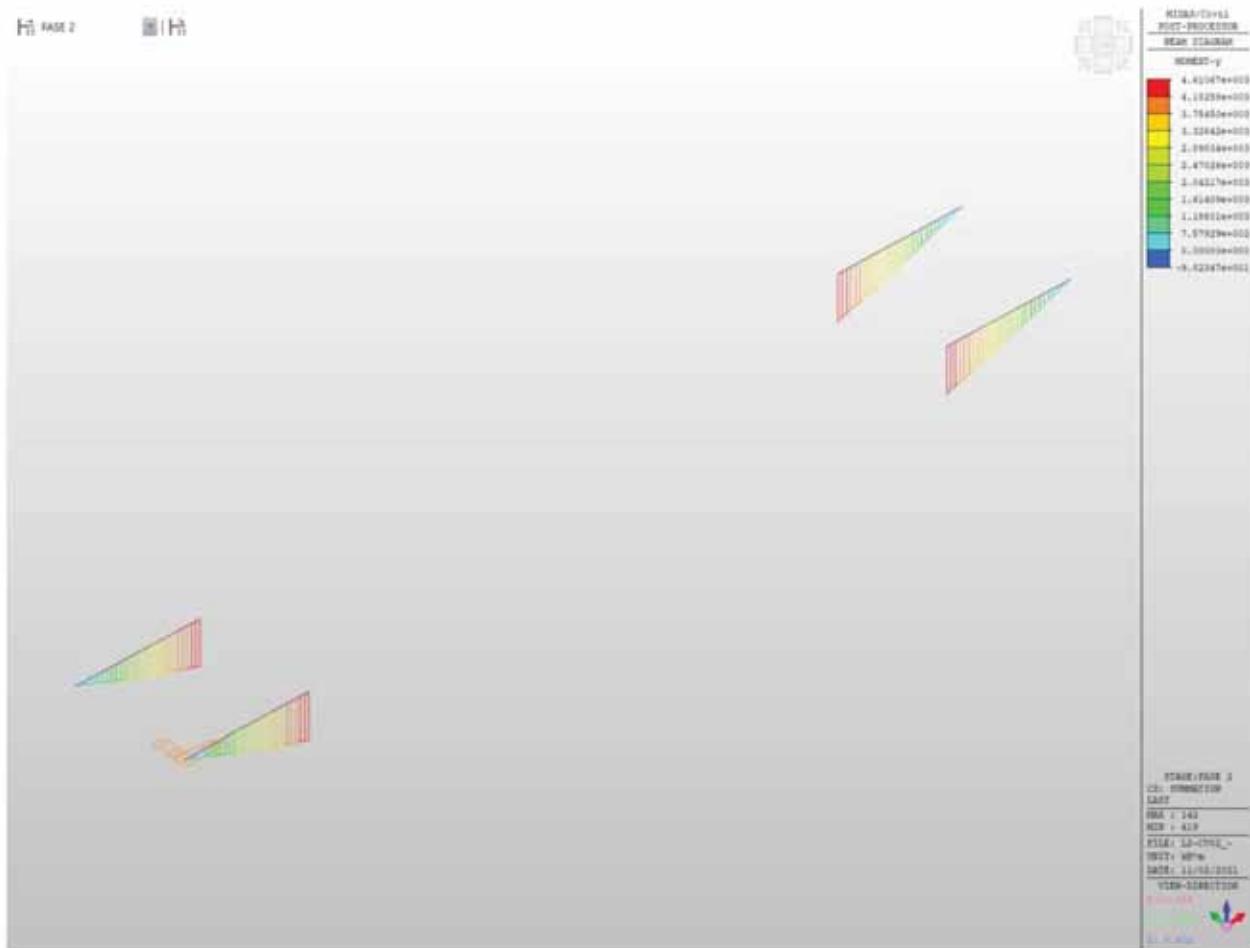


Figura 32 - Concilio 1 - Diagramma My - Fase 2

## LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO

Figura 33 - Concio 1 - Diagramma My - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

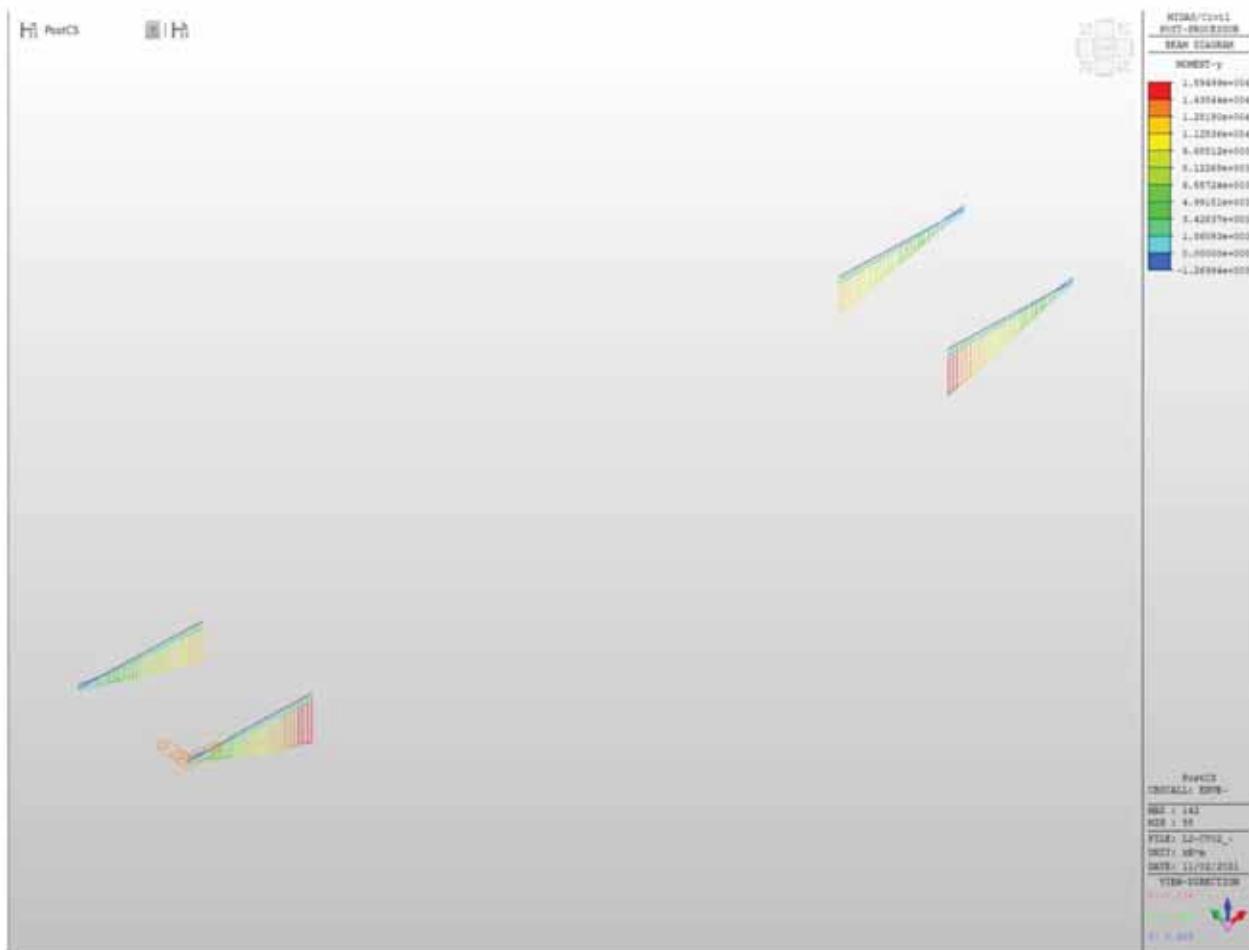


Figura 34 - Concio 1 - Diagramma My - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

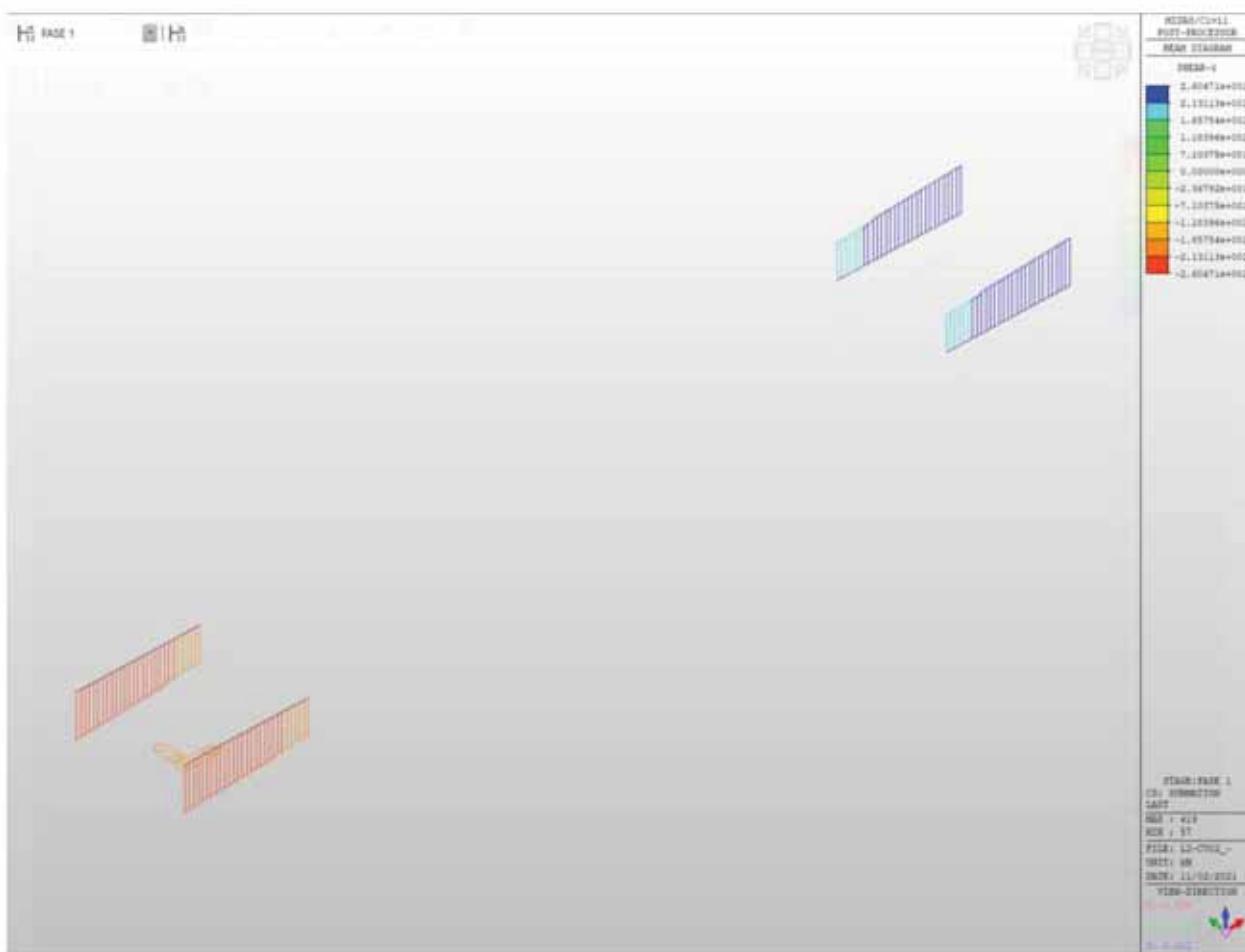


Figura 35 - Concio 1 - Diagramma Tz - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

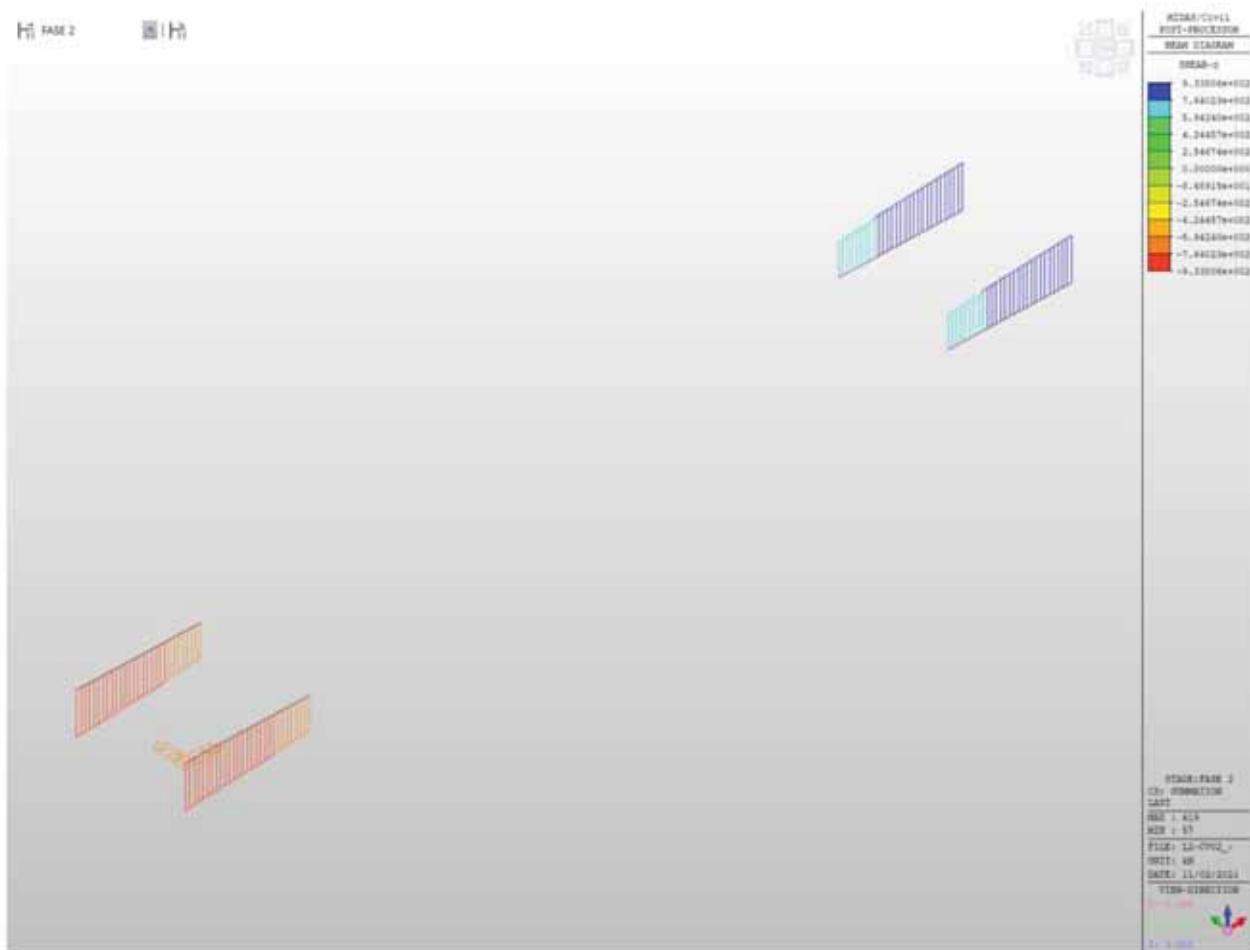
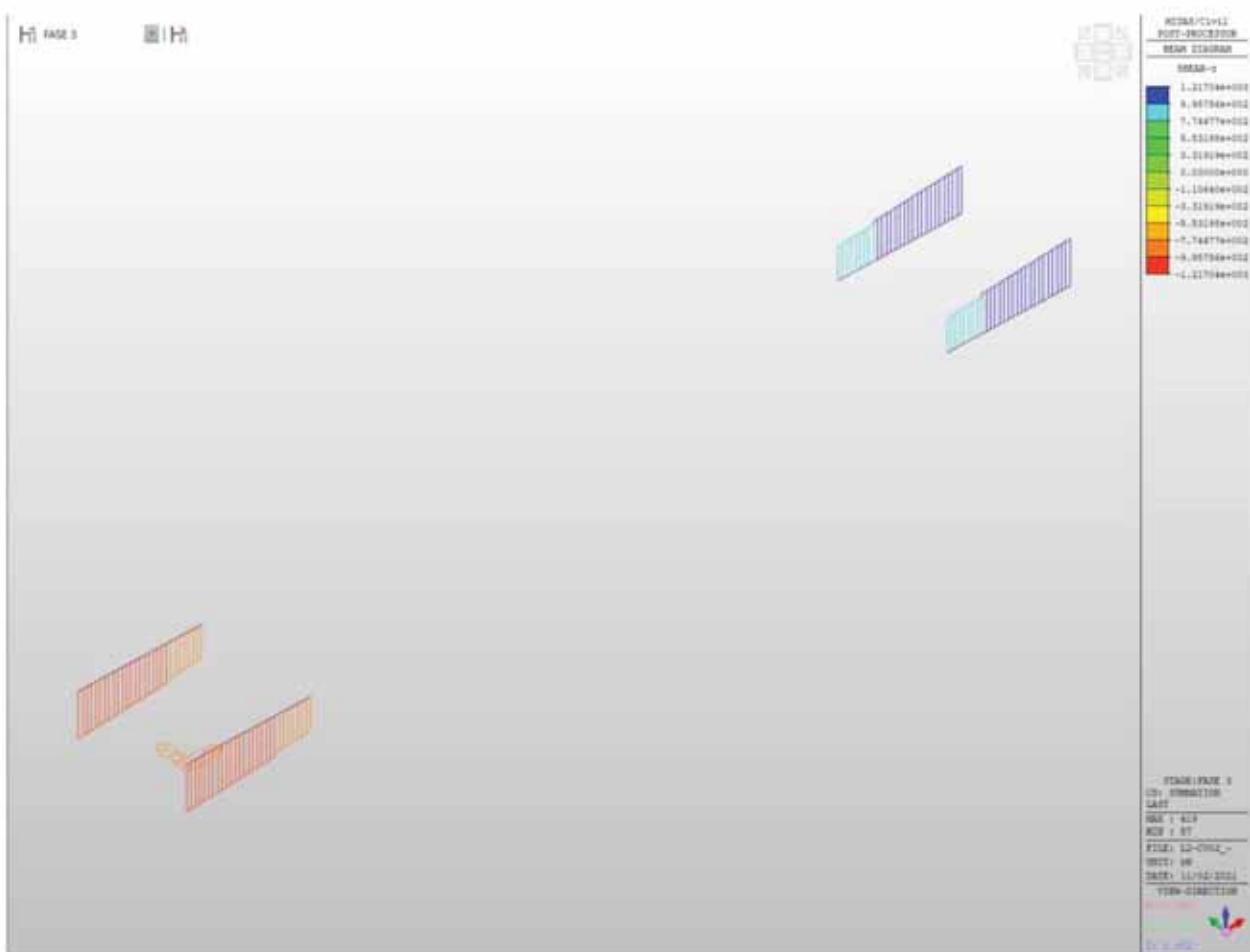


Figura 36 - Concio 1 - Diagramma Tz - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

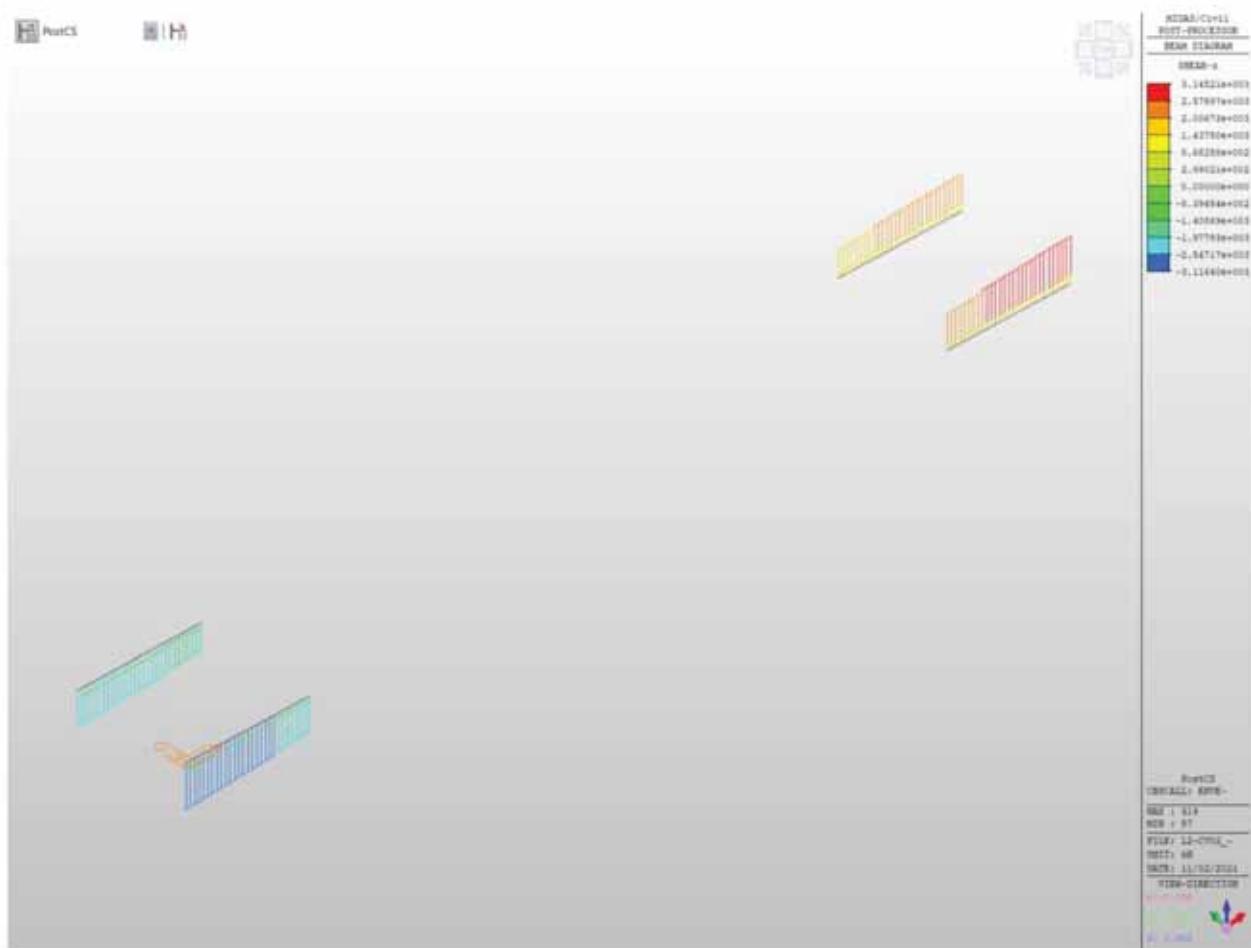


Figura 38 - Concio 1 - Diagramma Tz - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

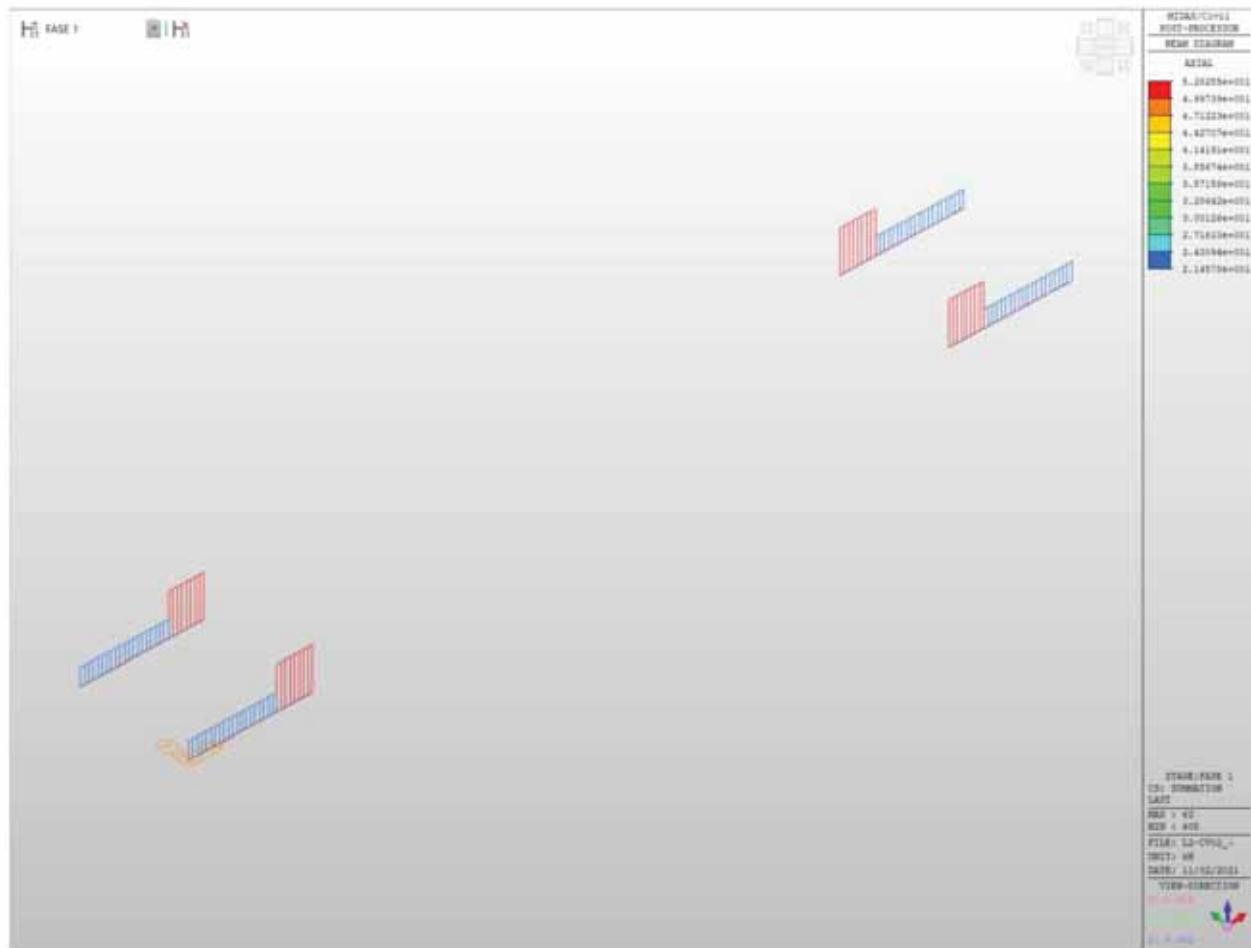


Figura 39 - Concio 1 - Diagramma N - Fase 1

## LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO

Figura 40 - Concio 1 - Diagramma N - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

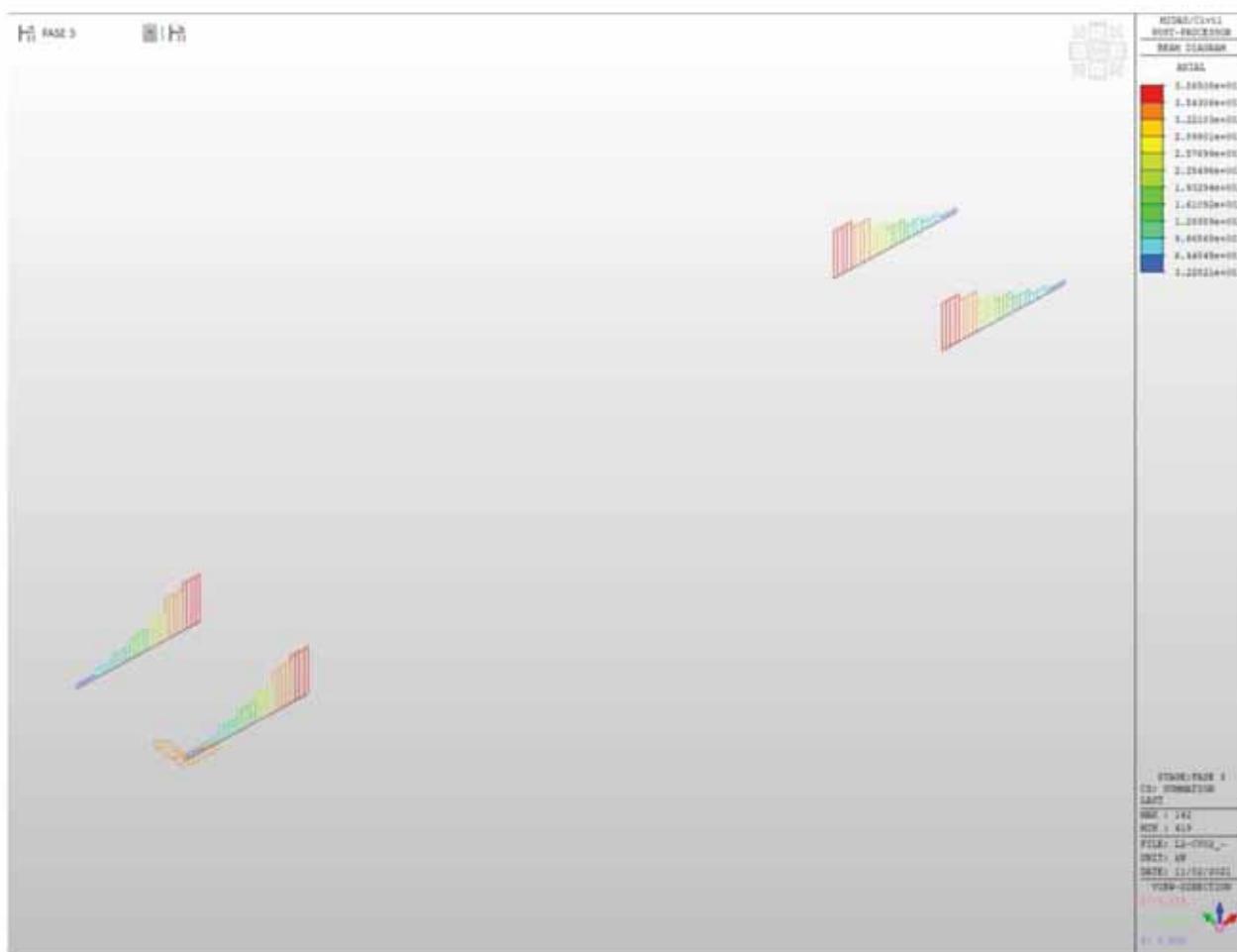


Figura 41 - Concio 1 - Diagramma N - Fase 3

## LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO

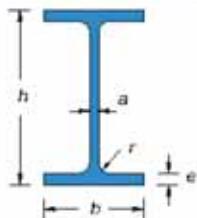
Figura 42 - Concio 1 - Diagramma N - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**VERIFICA SEZIONE IN ACCIAIO COMPOSTA**

**CARATTERISTICHE PROFILO:**

Tipo:	S355	▼
$B_{S,up}$ =	750	mm
$t_{t,S,up}$ =	25.0	mm
$B_{I,up}$ =	900	mm
$t_{t,I,up}$ =	30.0	mm
$H$ =	2100	mm
$t_w = a$ =	20.0	mm
$L$ =	4500	mm
$A_{tot}$ =	86650.00	mm <sup>2</sup>



$$Y_{G,inf} = 953.18 \text{ mm} \quad Y_{G,up} = 1146.82 \text{ mm}$$

$$J_{yy} = 6.26E+10 \text{ mm}^4 \quad J_z = 17459583 \text{ mm}^4$$

$$J_{zz} = 2702806250 \text{ mm}^4 \quad J_e = 1.069E+09 \text{ mm}^6$$

$$W_{el,yy,inf} = 65622983 \text{ mm}^3 \quad W_{el,yy,up} = 54542366 \text{ mm}^3$$

$$W_{pl,yy} = 67706766 \text{ mm}^3$$

$$W_{pl,xx} = 2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$$

$$\rho_{yy} = 849.63 \text{ mm} \quad \rho_{zz} = \sqrt{\frac{J_{zz}}{A}}$$

$$\rho_{zz} = 176.61 \text{ mm}$$

$$\text{Peso} = 680.20 \text{ kg/m}$$

$$E = 210000 \text{ N/mm}^2 \quad \text{modulo elastico}$$

$$\nu = 0.3 \quad \text{coeff. poisson}$$

$$G = 80769 \text{ N/mm}^2 \quad G = \frac{E}{2(1+\nu)} \quad \text{modulo elasticità trasversale}$$

$$\alpha = 0.000012 \text{ } ^\circ\text{C}^{-1} \quad \text{coeff. espansione termica lineare}$$

$$f_{yk} = 355 \text{ N/mm}^2 \quad \text{tensione snervamento caratteristica}$$

$$f_u = 510 \text{ N/mm}^2 \quad \text{tensione rottura caratteristica}$$

$$\gamma_{M0} = 1.05 \quad \text{per sezioni classe 1,2,3 e 4}$$

$$\gamma_{M1} = 1.05 \quad \text{per instabilità membrature}$$

$$\gamma_{M2} = 1.10 \quad \text{per instabilità membrature ponti stradali e ferroviari}$$

$$\gamma_{M3} = 1.25 \quad \text{per sezioni tese indebolite dai fori nei riguardi della frattura}$$

**SOLLECITAZIONI:**

$$N_{sd} = 0 \text{ N} \quad \text{caso per trave tesa}$$

$$N_{cd} = 373000 \text{ N} \quad \text{caso per trave compressa}$$

$$M_{sd} = 4511000000 \text{ Nmm} \quad \text{caso per trave inflessa in una direzione}$$

$$V_{sd} = 934000 \text{ N} \quad \text{taglio}$$

Caso per trave a flessione deviata (Momenti flettenti agenti alle estremità  $|M_x| < |M_z|$ ):

$$M_x = 368800000 \text{ Nmm} \quad M_z = 4511000000 \text{ Nmm}$$

Valori del momento massimo e minimo dell'asta:

$$M_{max,y,ed} = 0 \text{ Nmm} \quad M_{max,y,id} = 0 \text{ Nmm}$$

$$M_{max,z,ed} = 0 \text{ Nmm} \quad M_{max,z,id} = 0 \text{ Nmm}$$

**CLASSE APPARTENENZA PROFILO (NTC2006, 4.2.3.1):**

**ALA SUPERIORE**

$$C/t_{t,up} = 14.60$$

$$c = 0.81$$

**ALA INFERIORE**

$$C/t_{t,inf} = 14.67$$

$$c = 0.81$$

**ANIMA**

$$C/t_w = 102.25$$

$$c = 0.81$$

3

Curva di instabilità "b" (Asse yy):

Fattore di imperfezione  $\alpha$ : 0.34

Curva di instabilità "c" (Asse zz):

Fattore di imperfezione  $\alpha$ : 0.49

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{sd}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:  $A_v = 41450 \text{ mm}^2$

$$V_{c,Rd} = \frac{A_v f_y k}{\sqrt{3} \gamma_M 0} \quad V_{c,Rd} = 8091014 \quad N \Rightarrow 0.12 \quad OK!(<1)$$

Verifica in presenza di torsione uniforme:

$$\tau_{t,Rd} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,Rd}}{f_y k} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 8091014 \quad N \Rightarrow 0.12 \quad OK!(<1)$$

Verifica in termini tensionali nel punto più sollecitato:

$$\frac{\tau_{sd}}{f_y k} \leq 1 \Rightarrow 0.00 \quad OK!(<1)$$

**SOLLECITAZIONE DELL'ASTA A COMPRESSIONE SEMPLICE (NTC2008, 4.2.4.1.2):**

$$\frac{N_{sd}}{N_{c,Rd}} \leq 1 \quad N_{cr} = \frac{\pi^2 E J}{(\eta l)^2} = 276636147 \quad N \quad N_{c,Rd} < 0,04 N_{cr}$$

$$0.04 N_{cr} = 11065446 \text{ N} \quad \text{Instabilità trascurabile se } \lambda < 0.2$$

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$$\mu = 1.00 \quad l_0 = \mu l = 4500 \text{ mm}$$

$$\lambda = l_0 / \rho = 5$$

Calcolo resistenza per sezioni in classe 1,2 e 3:

$$N_{c,Rd} = \frac{A f_y k}{\gamma_M 0} \quad N_{c,Rd} = 29295952 \quad N \Rightarrow 0.01 \quad OK!(<1)$$

**STABILITÀ DELLE MEMBRATURE (NTC2008, 4.2.4.1.3):**

Curva di instabilità "b" (Asse yy):

Fattore di imperfezione  $\alpha$ : 0.49

Verifica per sezioni classe 1,2 e 3:

$$\bar{\lambda}_c > 0.2 \quad \text{Serve verifica instabilità!}$$

$$\Phi = 0.5 [1 + \alpha (\bar{\lambda}_c - 0.2) + \bar{\lambda}_c^2] = 0.59$$

$$\chi = \frac{1}{\Phi + \sqrt{\Phi^2 - \bar{\lambda}_c^2}} = 0.93$$

$$N_{b,Rd} = \frac{\gamma A f_y k}{\gamma_M 1} = 26062867 \quad N$$

$$\frac{N_{c,Rd}}{N_{b,Rd}} \leq 1 \Rightarrow 0.01 \quad OK!(<1)$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):**

$$\frac{M_{sd}}{M_{c,Rd}} \leq 1$$

$$V_{sd} < 0,5 V_{c,Rd}$$

*Il taglio non influisce sulla verifica a flessione!*

$$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2 \quad 0.59156$$

$$f_{y,red} = (1 - \rho)f_{yk} \quad 145 \quad N/mm^2$$

$$f_{yk} = \quad 355 \quad N/mm^2$$

Calcolo resistenza elastica sezione linda, classe 3:

$$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}} \quad M_{c,Rd} = \quad 22186817978 \quad Nmm \quad \Rightarrow \quad 0.21 \quad **OK!(<1)**$$

$$\text{n. fori} = \quad \text{diametro} = \quad \text{mm}$$

$$\frac{0,9 A_f,net f_{ck}}{\gamma_{M2}} \geq \frac{A_f f_{yk}}{\gamma_{M0}} \quad **Non serve la verifica per la presenza dei fori!**$$

**TRAVI INFLESSE (NTC2008, 4.2.4.1.3.2):**

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$$\mu = \quad 0.70 \quad L_{cr} = L / \mu = \quad 3150 \quad \text{mm}$$

$$W_{yy} = \quad 65622983 \quad \text{mm}^3 \quad W_{yy} = \quad 54542366 \quad \text{mm}^3$$

$$\psi = 1.75 - 1.05 \frac{M_B}{M_A} + 0.3 \left( \frac{M_B}{M_A} \right)^2 \quad 1.10$$

$$M_{cr} = \psi \frac{\pi}{L_{cr}} \sqrt{EJ_y G J_T} \sqrt{1 + \left( \frac{\pi}{L_{cr}} \right)^2 \frac{EJ_w}{G J_T}} \quad 1.496E+11 \quad Nmm \quad 0.031$$

$$M_{sd}/M_{cr} = \quad 0.031 \quad **Risulta < 0.16 Non serve la verifica instabilità**$$

$$\tilde{\lambda}_{LT} = \sqrt{\frac{W_y f_{yk}}{M_{cr}}} \quad 0.36 \quad **Risulta < 0.4 Non serve la verifica instabilità**$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Stabilità dei pannelli soggetti a taglio (NTC2008, 4.2.4.1.3.4.1):**

Se irrigiditori trasversali rigido o no,

$$I_{st} = 45000000 \text{ mm}^4 \quad 0.75hw^3 = 5488000 \text{ mm}^4 \quad \textcolor{red}{\text{Irrigiditore Trasversali rigidi}}$$

Coefficiente minimo di instabilità per taglio del panello (In assenza di irrigiditori longitudinali),

$$a/hw = 2.14 \quad k_t = 7.21$$

Coefficiente minimo di instabilità per taglio del panello (irrigiditori longitudinali più di due o  $a/hw > 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad k_{tl} = 0.00$$

$$a/hw = 2.14 \quad k_t = 7.21$$

Coefficiente minimo di instabilità per taglio del panello (irrigiditori longitudinali più di due o  $a/hw < 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad a/hw = 2.14 \quad k_t = 5.47$$

$$k_t = 7.21$$

$$\eta = 1.2 \quad hw/t = 102.25 \quad \textcolor{red}{\text{Serve la verifica di instabilità}}$$

$$\sigma_E = 17.23356009 \text{ MPa} \quad \tau_{cr} = 124.20 \quad \lambda_w = 1.28$$

$$\text{Montanti di appoggio rigidi, } X_w = 0.69$$

$$\text{Gli altri casi, } X_w = 0.65$$

$$V_{bw,Rd} = 5295814 \text{ N}$$

Momento resistente sole piattabande,

$$y_{G,inf} = 864.39 \text{ mm} \quad y_{G,sup} = 1235.61 \text{ mm}$$

$$J_{yy} = 47532476460 \text{ mm}^4$$

$$W_{el,yy,inf} = 54989921 \text{ mm}^3 \quad W_{el,yy,sup} = 38468686 \text{ mm}^3$$

$$M_{t,red} = 12692444270 \text{ Nmm}$$

$$bf = 303.75 \text{ mm} \quad V_{bf,Rd} = 35833 \text{ N}$$

$$V_{b,Rd} = 5331648 \text{ N} \quad 0.175 \quad \textcolor{red}{OK!(<1)}$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Stabilità dei pannelli soggetti a Compressione (NTC2008, 4.2.4.1.3.4.1):**

Stress,

$\sigma_{\text{sup}} = 84.54 \text{ MPa}$

$\sigma_{\text{inf}} = -70.27 \text{ MPa}$

$\sigma_{\text{irr.}} = 46.94 \text{ MPa}$

Irrigidati su entrambi lati (Anima),

$b = 2045.0 \text{ mm}$  (Web bw)  $a = 4500 \text{ mm}$

$\sigma_1 = 84.5 \text{ (Max)}$   $\sigma_2 = -70.27 \text{ (Min)}$

$\psi = -0.8$   $k\sigma = 19.79$

$\lambda_p = 1.0$   $\rho = 1.00$

$\sigma_{cr,p} = 359.7 \text{ MPa}$   $\sigma_{cr,c} = 3.749 \text{ MPa}$

$\zeta = 1.0$   $\lambda_c = 9.731$

$\alpha = 0.34$   $\Phi = 153.918$

$x_c = 0.003$

$p_c = 1.00$

$b_{eff} = 1116.79 \text{ mm}$   $b_{e1} = 446.71 \text{ mm}$   $b_{e2} = 670.07 \text{ mm}$

$b_t = 928.21 \text{ mm}$

Irrigidati a un lato (Piattebande),

Piattebanda Superiore,

$b = 365 \text{ mm}$  (Flange Sup)

$\sigma (\text{Irr.}) = 84.5$   $\sigma (\text{Lib.}) = 84.54$

$k\sigma = 0.43$

$\lambda_p = 1.2$   $\rho = 0.70$

$b_{eff} = 254.81 \text{ mm}$   $b_t = 0 \text{ mm}$

Effective Section Properties,

$A_{c,eff} (\text{Piattebanda Sup}) = 12740.7 \text{ mm}^2$

$A_{c,eff} (\text{Piattebanda Inf}) = 27000.0 \text{ mm}^2$

$A_{c,eff} (\text{Anima}) = 22335.7 \text{ mm}^2$

$A_c (\text{Anima tensione}) = 18564.3 \text{ mm}^2$

$A_{c,eff} (\text{Total}) = 80640.7 \text{ mm}^2$

$y_{g,inf} = 868.6 \text{ mm}$   $y_{g,sup} = 1231.4 \text{ mm}$

$l_{eff} = 5.62E+10 \text{ mm}^4$

$W_{eff} (\text{inf}) = 64663484 \text{ mm}^3$   $W_{eff} (\text{sup}) = 45616407 \text{ mm}^3$

$e = 84.53 \text{ mm}$

$\eta = 0.30$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Fase 2

**Soletta c.a.collaborante**

$b_{eff}$ =	2.84	m
Altezza soletta:	0.26	m
Area soletta:	0.74	$m^2$
$J_{yy}$ =	0.00416	$m^4$
Peso =	18.46	kN/m

**Trave acciaio**

Area trave acciaio:	0.08665	$m^2$
H trave acciaio:	2.10	m
$y_c$ =	0.953	m
$J_{yy}$ =	0.062550	$m^4$
Peso =	6.80	kN/m

**Barre armatura soletta**

<b>1° strato barre (superiore)</b>	<b>2° strato barre (inferiore)</b>
n. Barre: 14	n. Barre: 14
Interasse: 0.20 m	Interasse: 0.20 m
Diametro: 0.02 m	Diametro: 0.02 m
Area: 0.004398 $m^2$	Area: 0.004398 m
$y_1$ = 0.05 m	$y_2$ = 0.21 m

n = 18.0

Coeff. omogeneizzazione con viscosità in atto

**Materiali**

cls	Barre armature		Acciaio	
$R_{ck}$ = 40 MPa	$f_{yk}$ = 450 MPa	$f_{pk}$ = 355 MPa		
$f_{ck}$ = 33.20 MPa	$\gamma_M$ = 1.15	$\gamma_{M0}$ = 1.05		
$f_{cd}$ = 18.81 MPa	$f_{yd,1}$ = 391.30 MPa	$f_{yd,2}$ = 338.10 MPa		
$f_{cm}$ = 41.2 MPa				
$f_{cmn}$ = 3.10 MPa				
$E_{cn}$ = 33643 MPa				

**Caratteristiche geometriche**

$A_{sd}$ = 0.1365 $m^2$	Area sezione omogeneizzata in area di acciaio
$A_v$ = 41450 $mm^2$	Area taglio sezione di acciaio
$y_{C,inf,d}$ = 1.42 m	Asse neutro rispetto lembo inferiore:
$y_{C,sup,d}$ = 0.94 m	Asse neutro rispetto lembo superiore
$J_{xx,d}$ = 0.114407 $m^4$	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{hd,d}$ = 0.080608 $m^3$	Modulo elastico sezione omogeneizzata ad acciaio
$W_{wd,d}$ = 0.121617 $m^3$	Modulo elastico sezione omogeneizzata ad acciaio

**Sollecitazioni**

$M_{sd}$ = 5984.00 kNm	(SLU)		
$M_{rd}$ = 27253.34 kNm	=>	0.22	ok!
$T_{sd}$ = 1217.00 kN	(SLU)		
$T_{rd}$ = 7705.73 kN	=>	0.16	ok!

**Tensioni**

$\sigma_c$ = 2.73 MPa	=>	0.15	ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,sup}$ = 48.59 MPa	=>	0.12	ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf}$ = 38.22 MPa	=>	0.10	ok!(<1)	Tensione barre inferiori soletta
$\sigma_{a,sup}$ = 35.60 MPa	=>	0.11	ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{a,inf}$ = 74.24 MPa	=>	0.22	ok!(<1)	Tensione acciaio lembo inferiore

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Fase 3

**Soletta c.a.collaborante**

$b_{eff} =$	2.84	m
Altezza soletta:	0.26	m
Area soletta:	0.74	$m^2$
$J_{xx} =$	0.00416	$m^4$

Peso = 18.46 kN/m

**Trave acciaio**

Area trave acciaio:	0.08665	$m^2$
H trave acciaio:	2.10	m
$y_c =$	0.953	m
$J_{xx} =$	0.062550	$m^4$

Peso = 6.80 kN/m

**Barre armatura soletta**

1° strato barre (superiore)		2° strato barre (inferiore)	
n. Barre:	14	n. Barre:	14
Interasse:	0.20	Interasse:	0.20
Diametro:	0.02	Diametro:	0.02
Area:	0.00439823	$m^2$	Area: 0.004398 m $m^2$
$y_1 =$	0.05	$y_2 =$	0.21

$n =$  6.2 Coeff. omogeneizzazione a tempo infinito

**Materiali**

cls		Barre armature		Acciaio	
$R_{ck} =$	40 MPa	$f_{vk} =$	450 MPa	$f_{vk} =$	355 MPa
$f_{ck} =$	33.20 MPa	$\gamma_M =$	1.15	$\gamma_{M0} =$	1.05
$f_{ct} =$	18.81 MPa	$f_{vdk,s} =$	391.30 MPa	$f_{vdk,s} =$	338.10 MPa
$f_{cm} =$	41.2 MPa				
$f_{dm} =$	3.10 MPa				
$E_{cm} =$	33643 MPa				

**Caratteristiche geometriche**

$A_{sl} =$	0.2145 $m^2$	Area sezione omogeneizzata in area di acciaio
$A_s =$	41450 $mm^2$	Area taglio sezione di acciaio
$y_{G,inf,d} =$	1.71 m	Asse neutro rispetto lembo inferiore
$y_{G,sup,d} =$	0.65 m	Asse neutro rispetto lembo superiore
$J_{ox,d} =$	0.147487274 $m^4$	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{ref,d} =$	0.086032749 $m^3$	Modulo elastico sezione omogeneizzata ad acciaio
$W_{sup,d} =$	0.228420023 $m^3$	Modulo elastico sezione omogeneizzata ad acciaio

**Sollecitazioni**

$M_{sd} =$	15950.00	kNm	(SLU)		
$M_{ed} =$	29087.26	kNm	=>	0.55	ok!
$T_{sd} =$	3145.00	kN	(SLU)		
$T_{ed} =$	7705.73	kN	=>	0.41	ok!

**Tensioni**

$\sigma_c =$	11.26 MPa	=>	0.60	ok!(<1)	Tensione al lembo superiore soletta
$\sigma_{s,sup} =$	64.42 MPa	=>	0.16	ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf} =$	47.12 MPa	=>	0.12	ok!(<1)	Tensione barre inferiori soletta
$\sigma_{s,sup} =$	41.71 MPa	=>	0.12	ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{s,inf} =$	185.39 MPa	=>	0.55	ok!(<1)	Tensione acciaio lembo inferiore

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**6.2.1.2 TRAVE PRINCIPALE CONCIO 2**

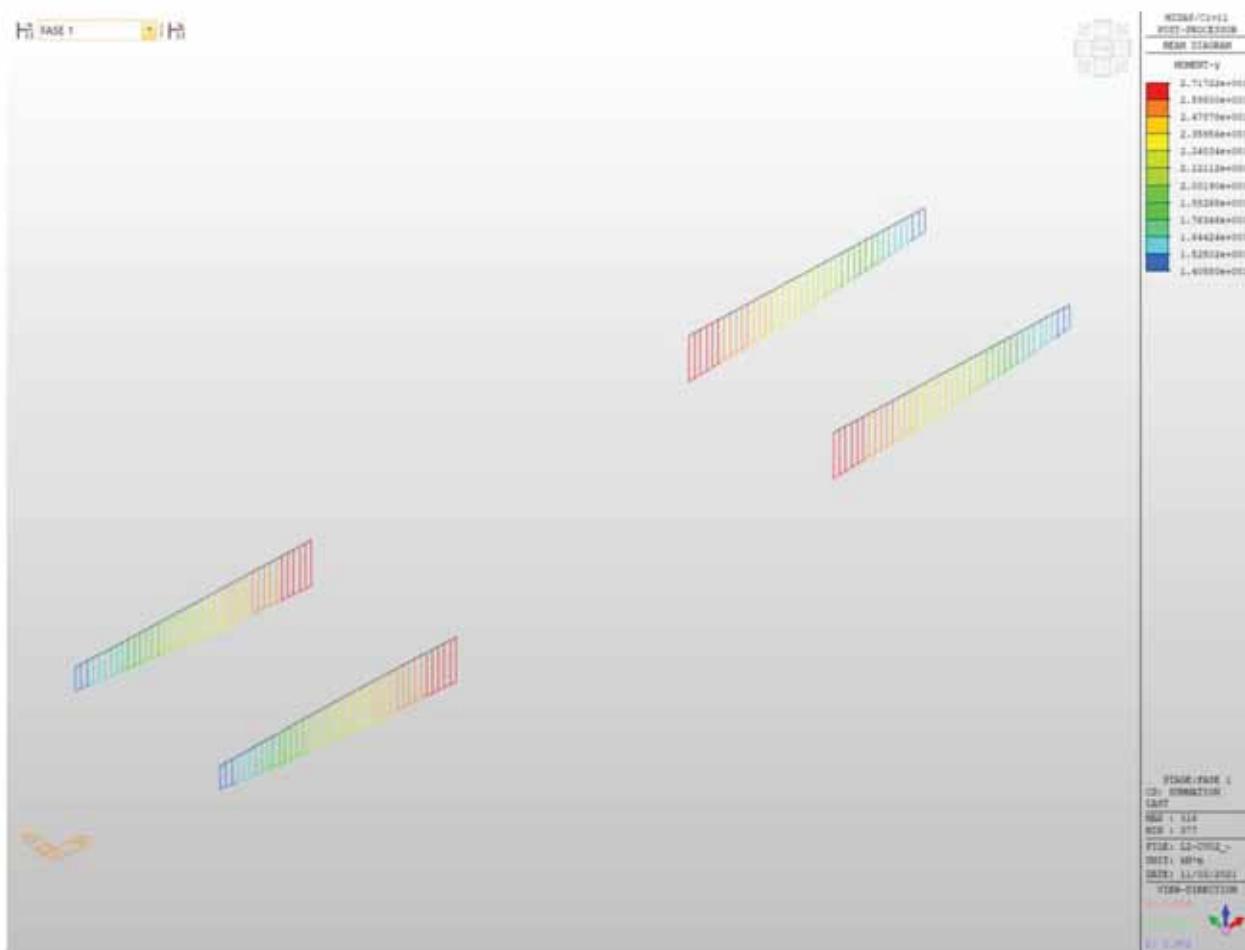


Figura 43 - Concio 2 - Diagramma My - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

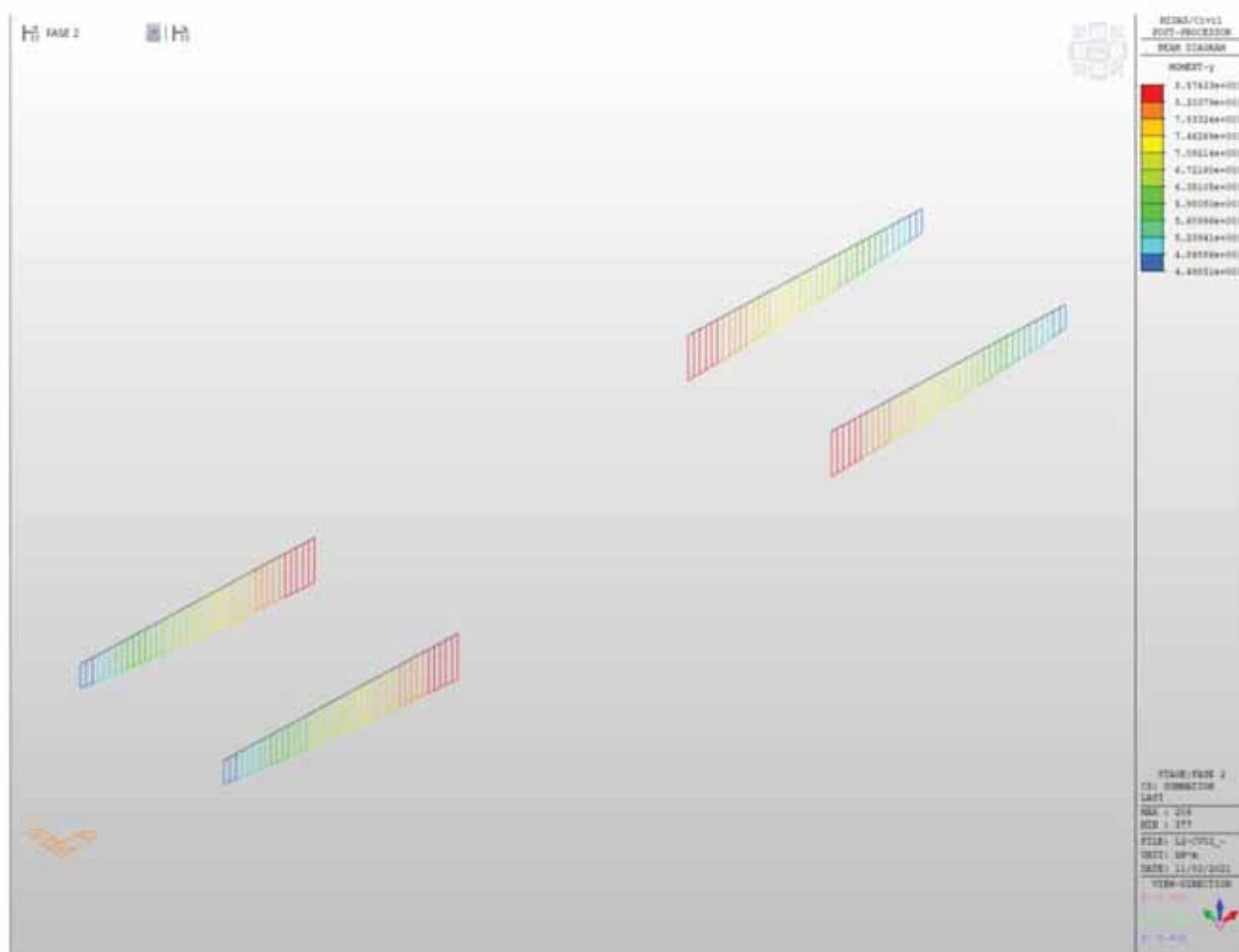


Figura 44 - Concio 2 - Diagramma My - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

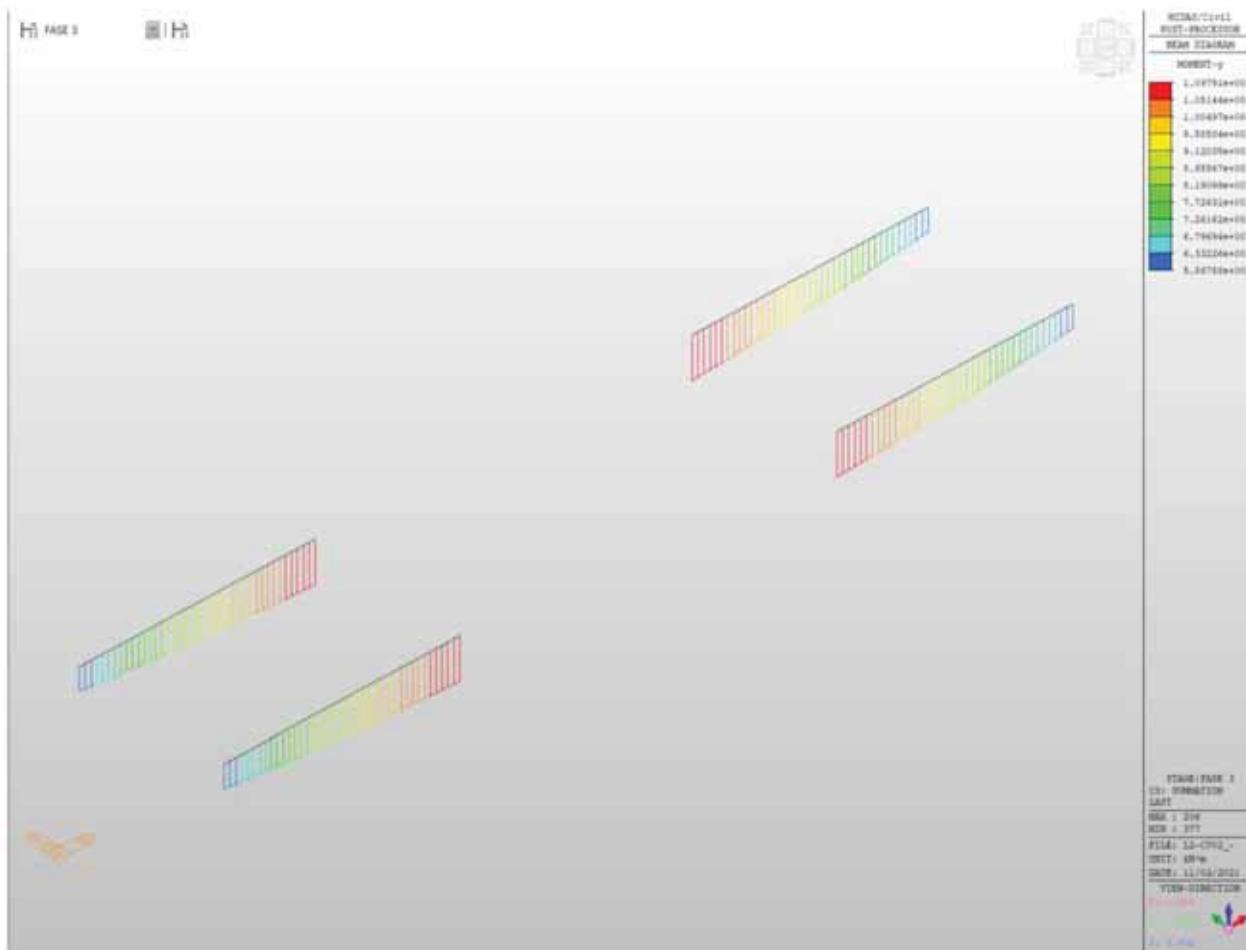


Figura 45 - Concio 2 - Diagramma My - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

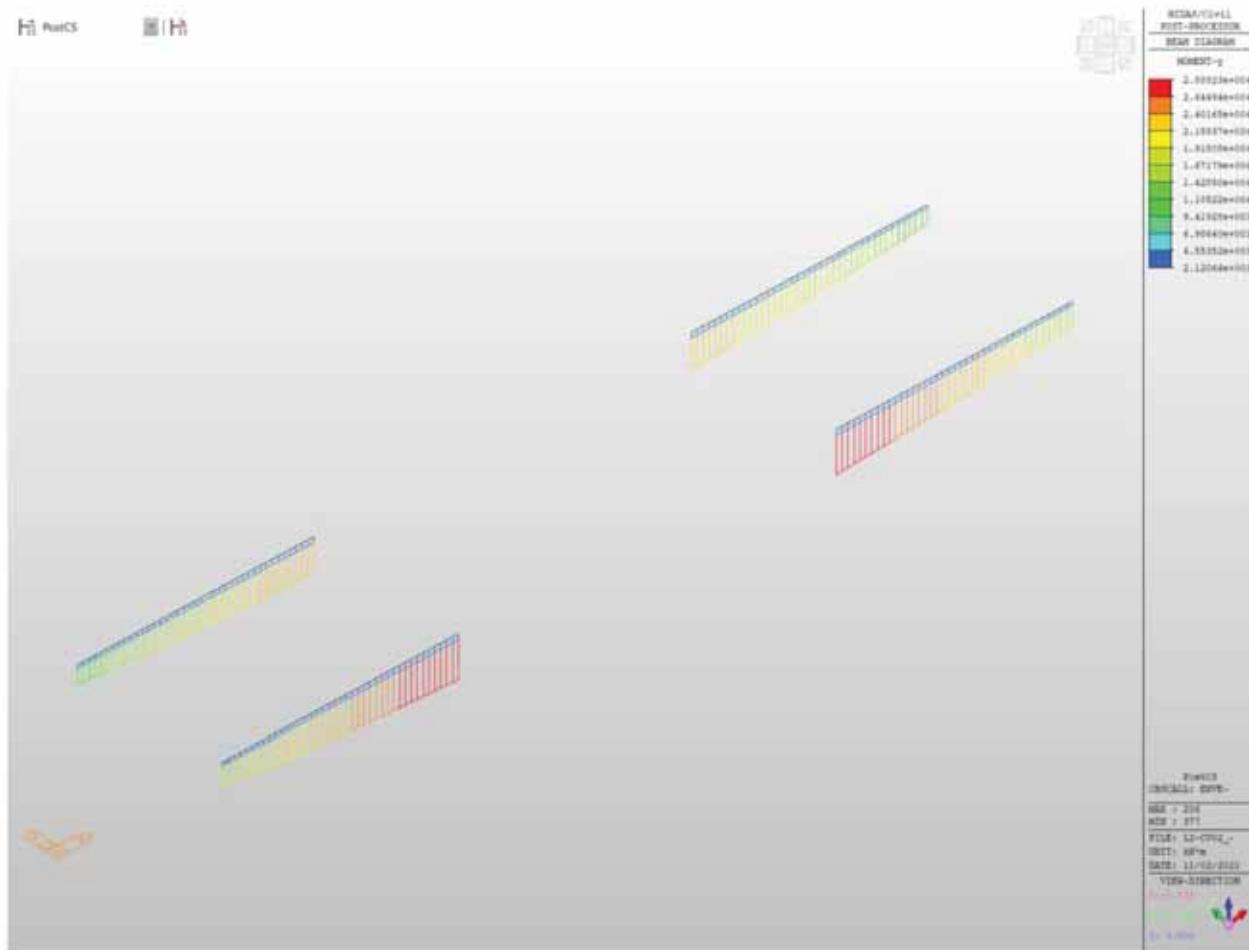


Figura 46 - Concio 2 - Diagramma My - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

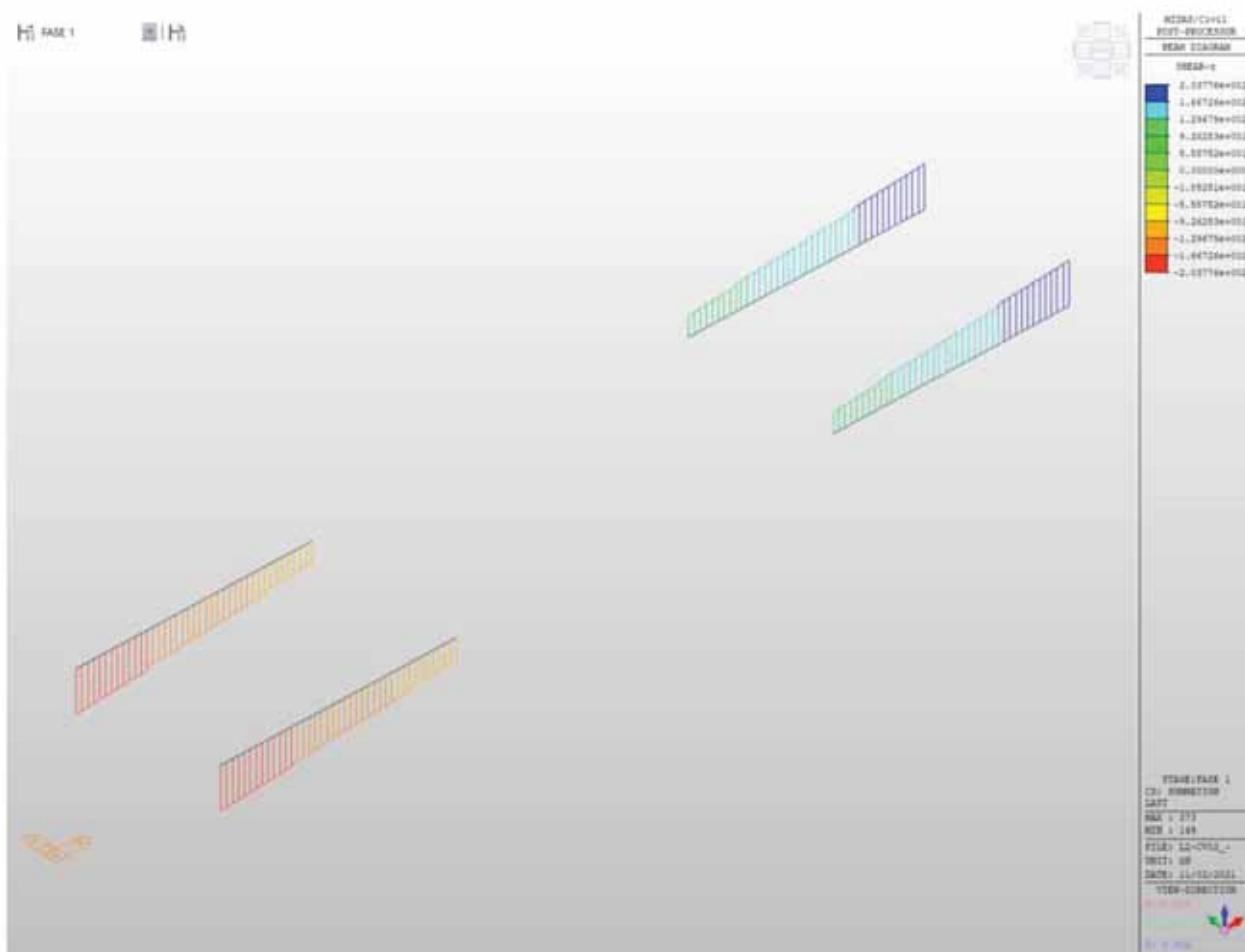


Figura 47 - Concio 2 - Diagramma Tz - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

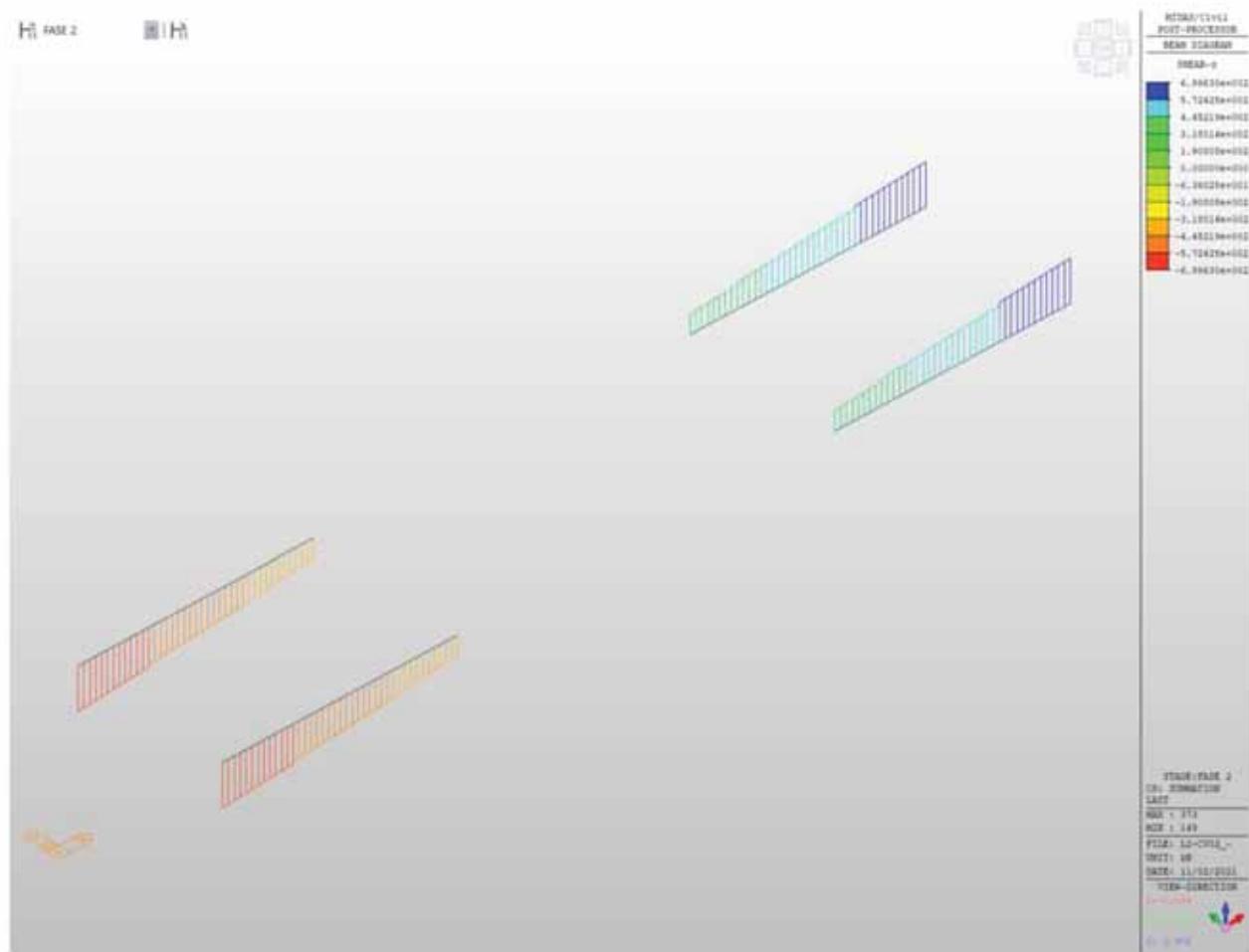


Figura 48 - Concio 2 - Diagramma Tz - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

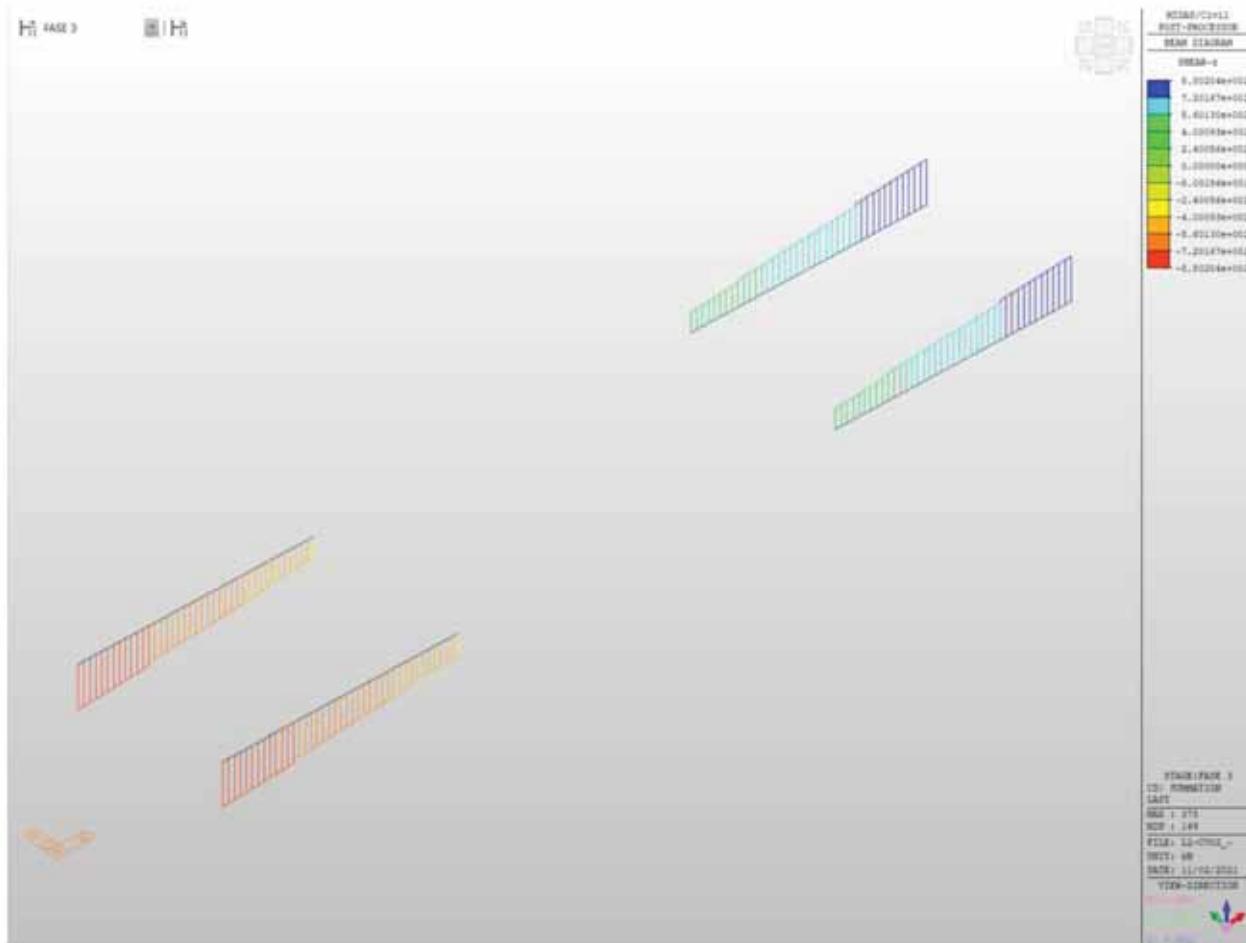


Figura 49 - Concio 2 - Diagramma Tz - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

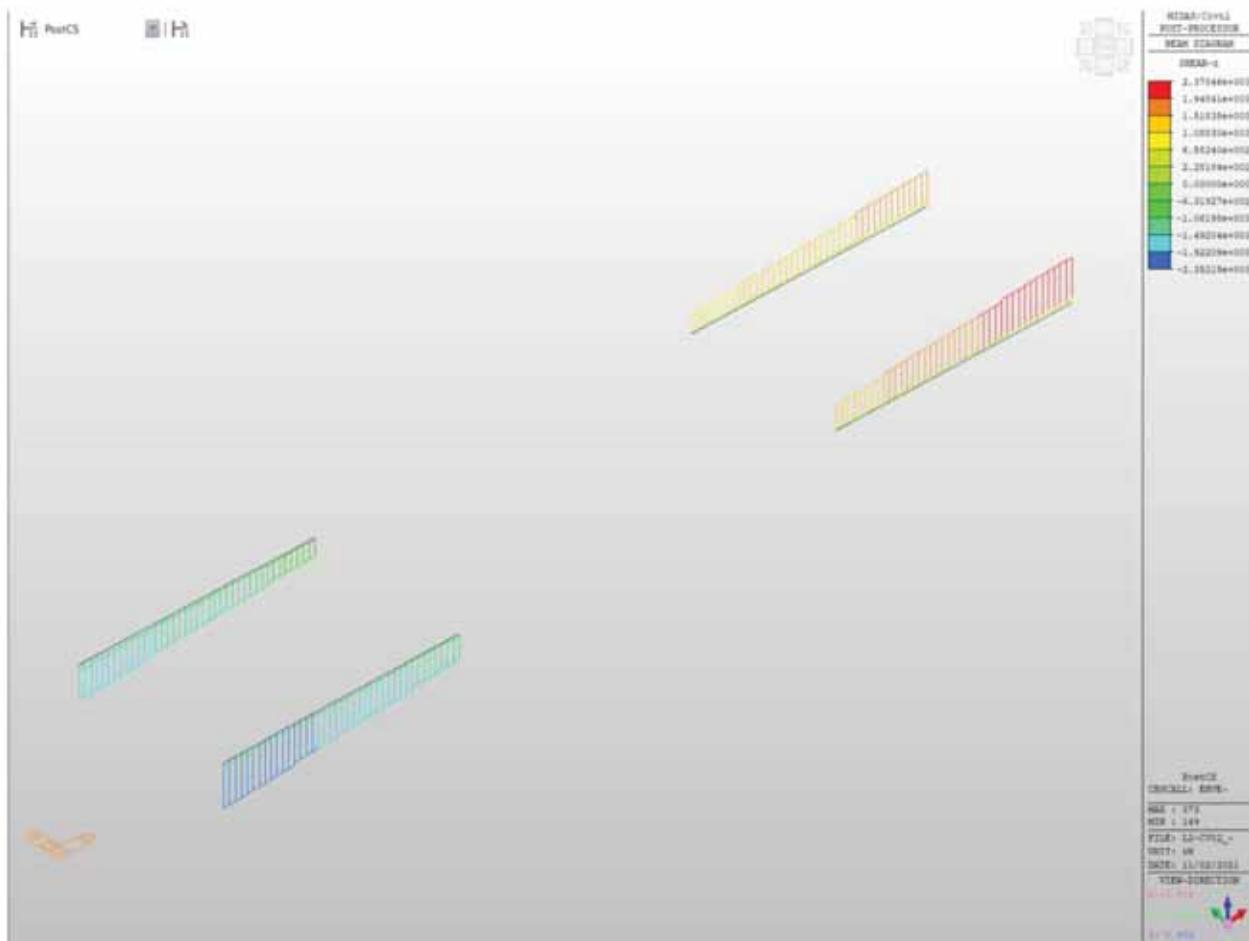


Figura 50 - Concio 2 - Diagramma Tz - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

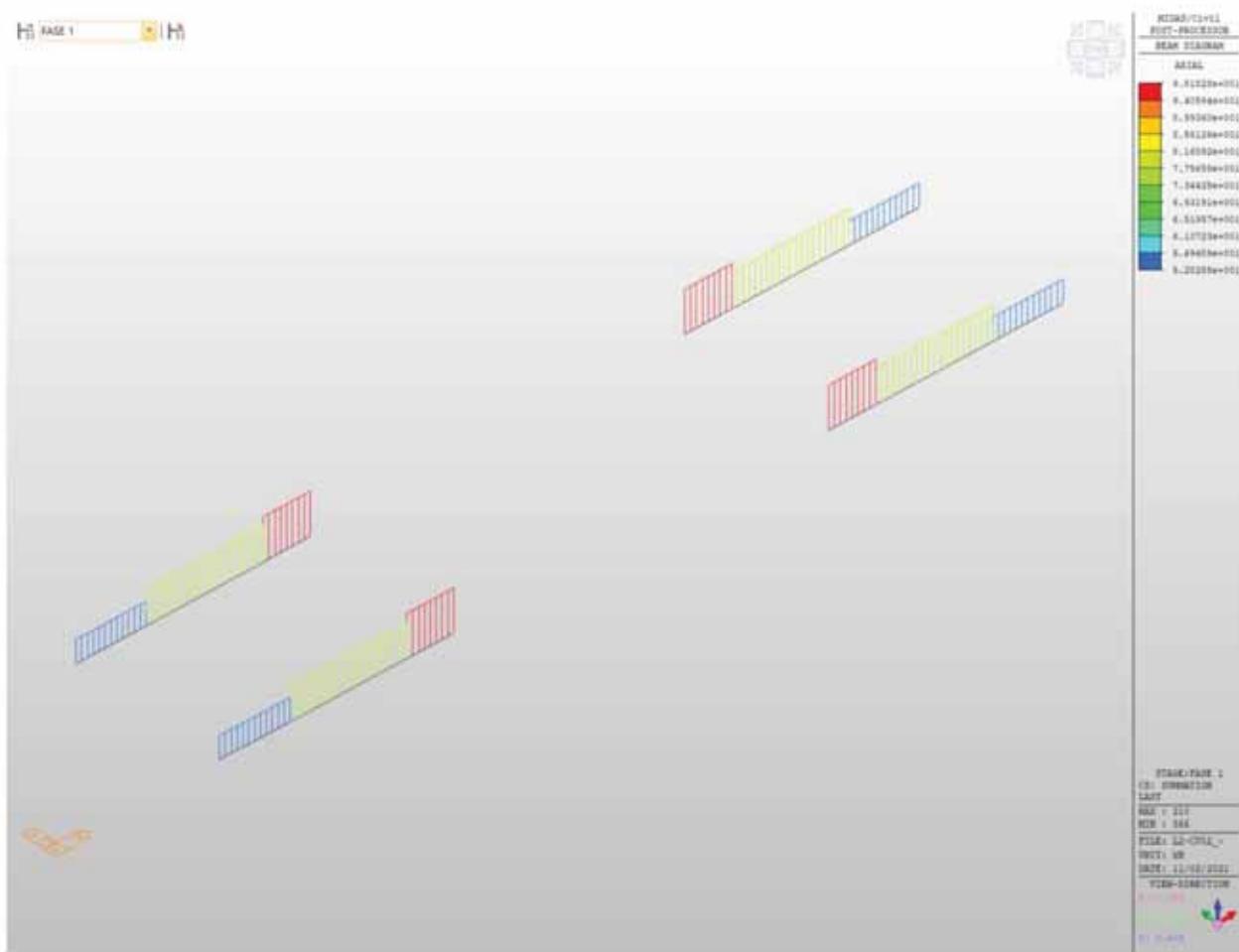
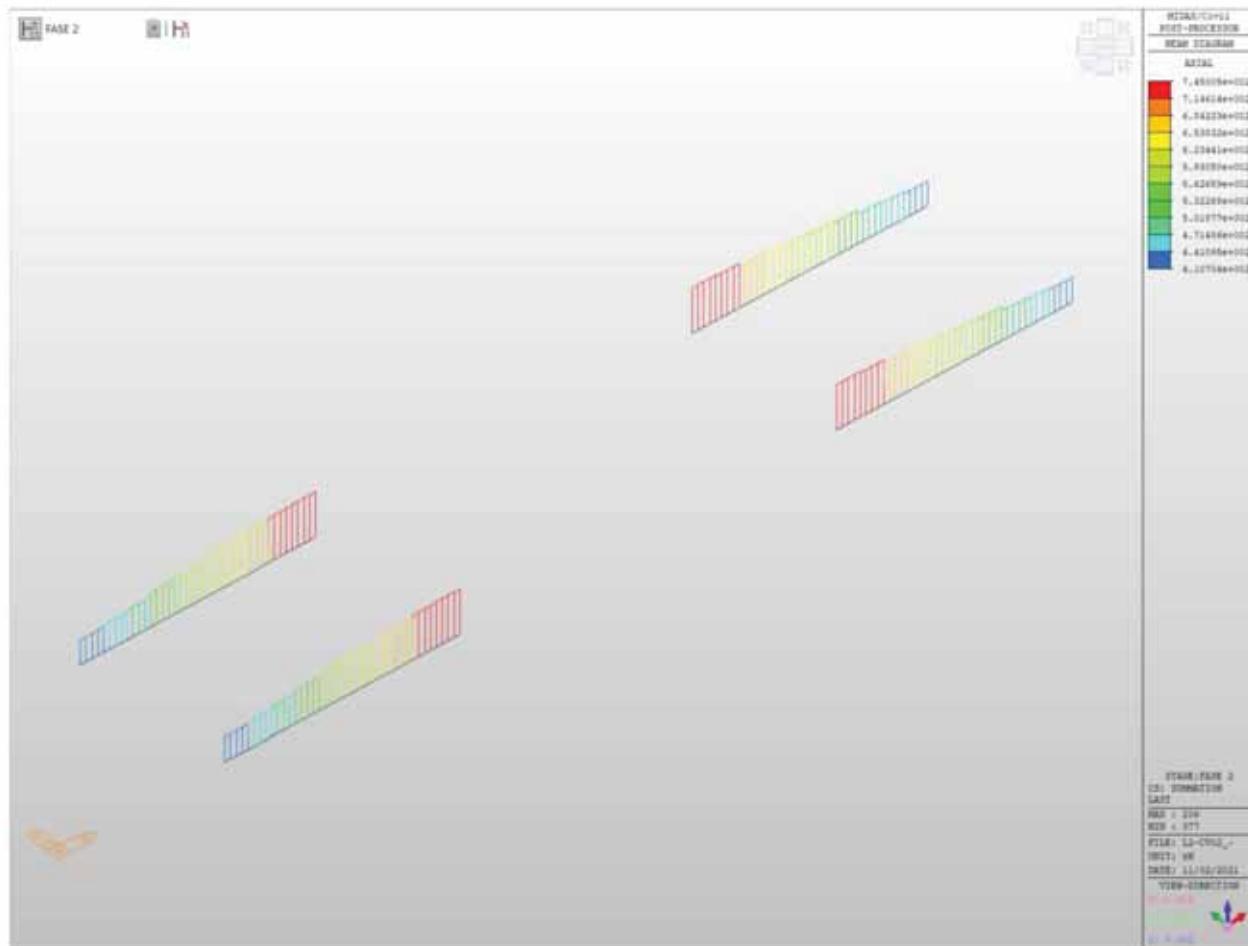


Figura 51 - Concio 2 - Diagramma N - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

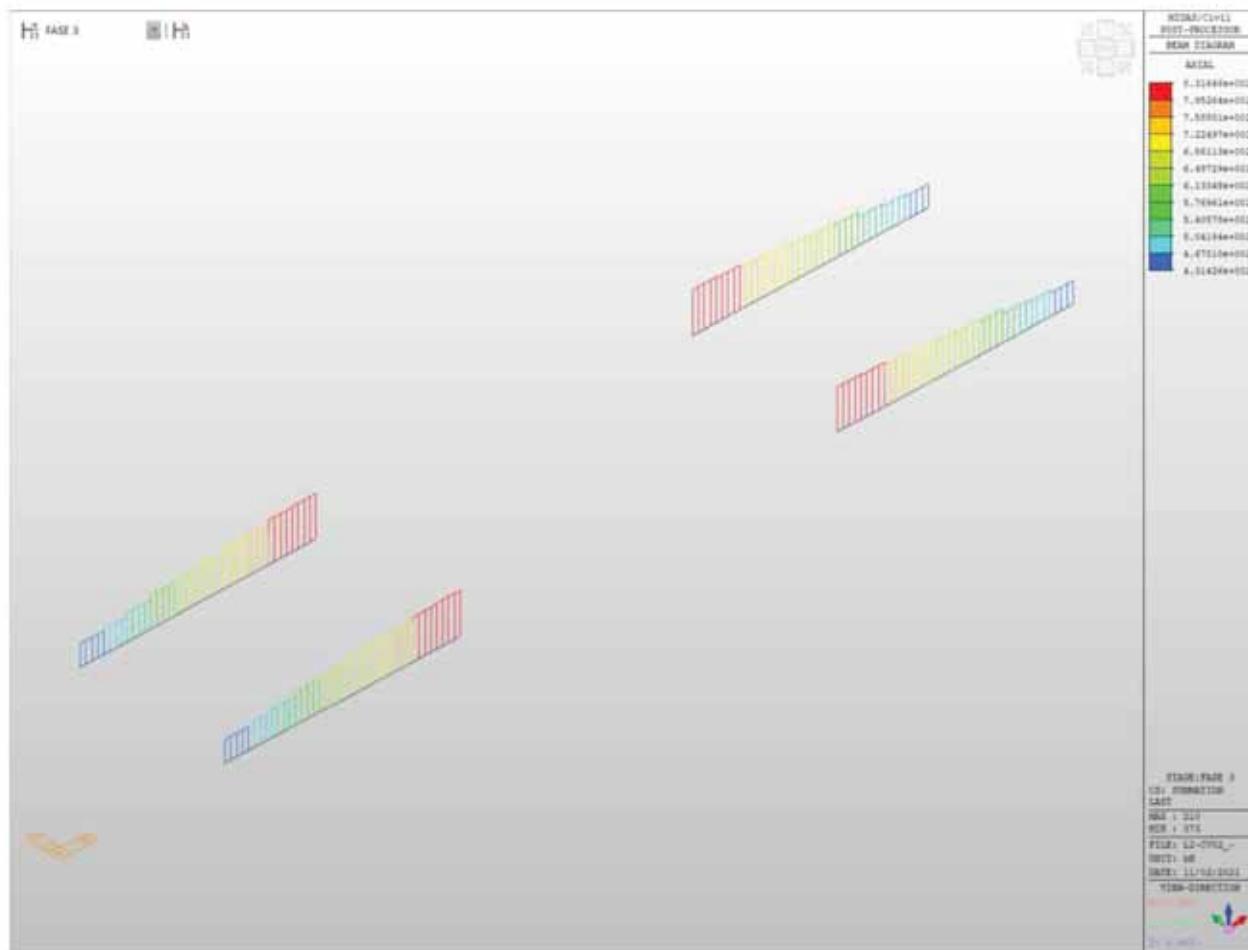


Figura 53 - Concio 2 - Diagramma N - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



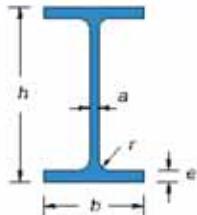
Figura 54 - Concio 2 - Diagramma N - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**VERIFICA SEZIONE IN ACCIAIO COMPOSTA**

**CARATTERISTICHE PROFILO:**

Tipo: **S355**  
 $B_{S,up} = 750$  mm  
 $t_{t,S,up} = 28.0$  mm       $A_{t,up} = 21000.00$  mm<sup>2</sup>  
 $B_{I,up} = 1200$  mm  
 $t_{t,I,up} = 40.0$  mm       $A_{t,I,up} = 48000.00$  mm<sup>2</sup>  
 $H = 2100$  mm  
 $t_w = a = 20.0$  mm       $A_w = 40640.00$  mm<sup>2</sup>  
 $L = 4400$  mm      n. traversi: **1**  
 $A_{tot} = 109640.00$  mm<sup>2</sup>



$y_{G,inf} = 799.72$  mm       $y_{G,sup} = 1300.28$  mm

$J_{yy} = 8.06E+10$  mm<sup>4</sup>       $J_z = 36506667$  mm<sup>4</sup>  
 $J_{zz} = 6745775000$  mm<sup>4</sup>       $J_w = 1.069E+09$  mm<sup>6</sup>

$W_{el,yz,inf} = 100769263$  mm<sup>3</sup>       $W_{el,yw,sup} = 61977417$  mm<sup>3</sup>

$W_{pl,yy} = 86397231$  mm<sup>3</sup>

$$W_{pl,xx} = 2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$$

$\rho_{yy} = 857.33$  mm       $\rho_{zz} = \sqrt{\frac{J_{zz}}{A}}$   
 $\rho_{zz} = 248.05$  mm

Peso = 860.67 kg/m

E = 210000 N/mm<sup>2</sup> modulo elastico

v = 0.3 coeff. poisson

G = 80769 N/mm<sup>2</sup>       $G = \frac{E}{2(1+\nu)}$  modulo elasticità trasversale

$\alpha = 0.000012$  °C<sup>-1</sup> coeff. espansione termica lineare

$f_yk = 355$  N/mm<sup>2</sup> tensione snervamento caratteristica

$f_u = 510$  N/mm<sup>2</sup> tensione rottura caratteristica

$\gamma_{M0} = 1.05$  per sezioni classe 1,2,3 e 4

$\gamma_{M1} = 1.05$  per instabilità membrature

$\gamma_{M2} = 1.10$  per instabilità membrature ponti stradali e ferroviari

$\gamma_{M3} = 1.25$  per sezioni tese indebolite dai fori nei riguardi della frattura

**SOLLECITAZIONI:**

$N_{sd} = 0$  N caso per trave tesa  
 $N_{cd} = 745000$  N caso per trave compressa  
 $M_{sd} = 8574300000$  Nmm caso per trave inflessa in una direzione  
 $V_{sd} = 699630$  N taglio

Caso per trave a flessione deviata (Momenti flettenti agenti alle estremità  $|M_L| < |M_u|$ ):

$M_L = 6859440000$  Nmm       $M_A = 8574300000$  Nmm

Valori del momento massimo e minimo dell'asta:

$M_{max,y,ed} = 0$  Nmm       $M_{max,y,ud} = 0$  Nmm  
 $M_{max,z,ed} = 0$  Nmm       $M_{max,z,ud} = 0$  Nmm

**CLASSE APPARTENENZA PROFILO (NTC2006, 4.2.3.1):**

ALA SUPERIORE	ALA INFERIORE	ANIMA	
$C/t_{t,up} = 13.04$	$C/t_{t,inf} = 14.75$	$C/t_w = 101.60$	3
$c = 0.81$	$c = 0.81$	$c = 0.81$	

Curva di instabilità "b" (Asse yy):

Fattore di imperfezione  $\alpha$ : 0.34

Curva di instabilità "c" (Asse zz):

Fattore di imperfezione  $\alpha$ : 0.49

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{sd}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:  $A_v = 41320 \text{ mm}^2$

$$V_{c,Rd} = \frac{A_v f_y k}{\sqrt{3} \gamma_M 0} \quad V_{c,Rd} = 8065638 \quad N \Rightarrow 0.09 \quad OK!(<1)$$

Verifica in presenza di torsione uniforme:

$$\tau_{t,rd} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,rd}}{f_y k} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 8065638 \quad N \Rightarrow 0.09 \quad OK!(<1)$$

Verifica in termini tensionali nel punto più sollecitato:

$$\frac{\tau_{rd}}{f_y k} \leq 1 \Rightarrow 0.00 \quad OK!(<1)$$

**SOLLECITAZIONE DELL'ASTA A COMPRESSIONE SEMPLICE (NTC2008, 4.2.4.1.2):**

$$\frac{N_{sd}}{N_{c,Rd}} \leq 1 \quad N_{cr} = \frac{\pi^2 E J}{(\eta l)^2} \quad 722180136 \quad N \quad N_{c,rd} < 0,04 N_{cr}$$

$$0.04 N_{cr} = 28887205 \text{ N} \quad \text{Instabilità trascurabile se } \lambda < 0.2$$

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$$\mu = 1.00 \quad I_0 = \mu l = 4400 \text{ mm}$$

$$\lambda = I_0 / p = 5$$

Calcolo resistenza per sezioni in classe 1,2 e 3:

$$N_{c,Rd} = \frac{A f_y k}{\gamma_M 0} \quad N_{c,Rd} = 37068762 \quad N \Rightarrow 0.02 \quad OK!(<1)$$

**STABILITÀ DELLE MEMBRATURE (NTC2008, 4.2.4.1.3):**

Curva di instabilità "b" (Asse yy):

Fattore di imperfezione  $\alpha$ : 0.49

Verifica per sezioni classe 1,2 e 3:

$$\bar{\lambda} = \sqrt{\frac{A f_y k}{N_{cr}}} \quad 0.23 \quad \bar{\lambda} > 0.2 \quad \text{Serve verifica instabilità!}$$

$$\Phi = 0.5 [1 + \alpha (\bar{\lambda} - 0.2) + \bar{\lambda}^2] \quad 0.53$$

$$\chi = \frac{1}{\Phi + \sqrt{\Phi^2 - \bar{\lambda}^2}} \quad 0.98$$

$$N_{b,Rd} = \frac{\chi A f_y k}{\gamma_M 1} \quad 34804758 \quad N$$

$$\frac{N_{c,rd}}{N_{b,Rd}} \leq 1 \Rightarrow 0.02 \quad OK!(<1)$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):**

$$\frac{M_{sd}}{M_{c,Rd}} \leq 1$$

$$V_{sd} < 0.5 V_{c,Rd}$$

*Il taglio non influisce sulla verifica a flessione!*

$$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2 \quad 0.68313$$

$$f_{y,red} = (1 - \rho)f_{yk} \quad 112 \quad N/mm^2$$

$$f_{yk} = \quad 355 \quad N/mm^2$$

Calcolo resistenza elastica sezione linda, classe 3:

$$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}} \quad M_{c,Rd} = \quad 34069608013 \quad Nmm \quad \Rightarrow \quad 0.25 \quad **OK!(<1)**$$

$$\text{n. fori} = \quad \text{diametro} = \quad \text{mm}$$

$$\frac{0.9 A_{f,net} f_{ck}}{\gamma_{M2}} \geq \frac{A_f f_{yk}}{\gamma_{M0}} \quad **Non serve la verifica per la presenza dei fori!**$$

**TRAVI INFLESSE (NTC2008, 4.2.4.1.3.2):**

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$$\mu = \quad 0.70 \quad L_{cr} = L / \mu = \quad 3080 \quad \text{mm}$$

$$W_{yy} = \quad 100769263 \quad \text{mm}^3 \quad W_{yy} = \quad 61977417 \quad \text{mm}^3$$

$$\psi = 1.75 - 1.05 \frac{M_B}{M_A} + 0.3 \left( \frac{M_B}{M_A} \right)^2 \quad 1.10$$

$$M_{cr} = \psi \frac{\pi}{L_{cr}} \sqrt{EJ_y G J_T} \sqrt{1 + \left( \frac{\pi}{L_{cr}} \right)^2 \frac{EJ_w}{G J_T}} \quad 2.511E+11 \quad Nmm \quad 0.034$$

$$M_{sd}/M_{cr} = \quad 0.034 \quad **Risulta < 0.16 Non serve la verifica instabilità**$$

$$\tilde{\lambda}_{LT} = \sqrt{\frac{W_y f_{yk}}{M_{cr}}} \quad 0.30 \quad **Risulta < 0.4 Non serve la verifica instabilità**$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Stabilità dei pannelli soggetti a taglio (NTC2008, 4.2.4.1.3.4.1):**

Se irrigiditori trasversali rigido o no,

$$I_{st} = 45000000 \text{ mm}^4 \quad 0.75hw^3 = 5740289 \text{ mm}^4 \quad \text{Irregiditore Trasversali rigidi}$$

Coefficiente minimo di instabilità per taglio del panello (In assenza di irrigiditori longitudinali),

$$a/hw = 2.10 \quad k_t = 7.25$$

Coefficiente minimo di instabilità per taglio del panello (irrigiditori longitudinali più di due o  $a/hw > 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad k_{tl} = 0.00$$

$$a/hw = 2.10 \quad k_t = 7.25$$

Coefficiente minimo di instabilità per taglio del panello (irrigiditori longitudinali più di due o  $a/hw < 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad a/hw = 2.10 \quad k_t = 5.54$$

$$k_t = 7.25$$

$$\eta = 1.2 \quad hw/t = 101.60 \quad \text{Serve la verifica di instabilità}$$

$$\sigma_E = 17.23356009 \text{ MPa} \quad \tau_{cr} = 124.93 \quad \lambda_w = 1.28$$

$$\text{Montanti di appoggio rigidi, } X_w = 0.69$$

$$\text{Gli altri casi, } X_w = 0.65$$

$$V_{bw,Rd} = 5311379 \text{ N}$$

Momento resistente sole piattabande,

$$y_{G,inf} = 648.78 \text{ mm} \quad y_{G,sup} = 1451.22 \text{ mm}$$

$$J_{yy} = 62362885739 \text{ mm}^4$$

$$W_{el,yy,inf} = 96122931 \text{ mm}^3 \quad W_{el,yy,sup} = 42972808 \text{ mm}^3$$

$$M_{t,red} = 14064919853 \text{ Nmm}$$

$$bf = 340.2 \text{ mm} \quad V_{bf,Rd} = 31395 \text{ N}$$

$$V_{b,Rd} = 5342774 \text{ N} \quad 0.131 \quad \text{OK!(<1)}$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Stabilità dei pannelli soggetti a Compressione (NTC2008, 4.2.4.1.3.4.1):**

Stress,

$\sigma_{\text{sup}} = 138.35 \text{ MPa}$

$\sigma_{\text{inf}} = -85.09 \text{ MPa}$

$\sigma_{\text{irr.}} = 84.08 \text{ MPa}$

Irrigidati su entrambi lati (Anima),

$b = 2032.0 \text{ mm}$  (Web bw)  $a = 4400 \text{ mm}$

$\alpha_1 = 138.3 \text{ (Max)}$   $\alpha_2 = -85.09 \text{ (Min)}$

$\psi = -0.6$   $k\sigma = 15.38$

$\lambda_p = 1.1$   $\rho = 0.89$

$\sigma_{cr,p} = 283.1 \text{ MPa}$   $\sigma_{cr,c} = 3.921 \text{ MPa}$

$\zeta = 1.0$   $\lambda_c = 9.515$

$\alpha = 0.34$   $\Phi = 143.847$

$\chi_c = 0.003$

$\rho_c = 0.89$

$b_{eff} = 1117.12 \text{ mm}$   $b_{e1} = 446.85 \text{ mm}$   $b_{e2} = 670.27 \text{ mm}$

$b_t = 773.83 \text{ mm}$

Irrigidati a un lato (Piattebande),

Piattabanda Superiore,

$b = 365 \text{ mm}$  (Flange Sup)

$\sigma (\text{Irri.}) = 138.3$   $\sigma (\text{Lib.}) = 138.35$

$k\sigma = 0.43$

$\lambda_p = 1.2$   $\rho = 0.70$

$b_{eff} = 254.81 \text{ mm}$   $b_t = 0 \text{ mm}$

Effective Section Properties,

$A_{c,eff} (\text{Piattabanda Sup}) = 14269.5 \text{ mm}^2$

$A_{c,eff} (\text{Piattabanda Inf}) = 48000.0 \text{ mm}^2$

$A_{c,eff} (\text{Anima}) = 22342.5 \text{ mm}^2$

$A_c (\text{Anima tensione}) = 15476.6 \text{ mm}^2$

$A_{c,eff} (\text{Total}) = 100088.6 \text{ mm}^2$

$y_{g,inf} = 692.0 \text{ mm}$   $y_{g,sup} = 1408.0 \text{ mm}$

$I_{eff} = 7.04E+10 \text{ mm}^4$

$W_{eff} (\text{inf}) = 101748797 \text{ mm}^3$   $W_{eff} (\text{sup}) = 50002161 \text{ mm}^3$

$e = 107.77 \text{ mm}$

$\eta = 0.51$



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Fase 2

**Soletta c.o.collaborante**

b <sub>eff</sub> =	2.84	m
Altezza soletta:	0.26	m
Area soletta:	0.74	m <sup>2</sup>
J <sub>yy</sub> =	0.00416	m <sup>4</sup>
Peso =	18.46	kN/m

**Trave acciaio**

Area trave acciaio:	0.10964	m <sup>2</sup>
H trave acciaio:	2.10	m
y <sub>0</sub> =	0.800	m
J <sub>yy</sub> =	0.080588	m <sup>4</sup>
Peso =	8.61	kN/m

**Barre armatura soletta**

1° strato barre (superiore)	2° strato barre (inferiore)
n. Barre: 14	n. Barre: 14
Interasse: 0.20 m	Interasse: 0.20 m
Diametro: 0.024 m	Diametro: 0.024 m
Area: 0.006333 m <sup>2</sup>	Area: 0.006333 m <sup>2</sup>
y <sub>1</sub> = 0.05 m	y <sub>2</sub> = 0.21 m

1° strato barre (superiore)	2° strato barre (inferiore)
n. Barre: 14	n. Barre: 14
Interasse: 0.20 m	Interasse: 0.20 m
Diametro: 0.024 m	Diametro: 0.024 m
Area: 0.006333 m <sup>2</sup>	Area: 0.006333 m <sup>2</sup>
y <sub>1</sub> = 0.05 m	y <sub>2</sub> = 0.21 m

n = 18.0 Coeff. omogeneizzazione con viscosità in atto

**Materiali**

cls	Barre armature		Acciaio	
R <sub>ex</sub> = 40 MPa	f <sub>yk</sub> = 450 MPa	γ <sub>M</sub> = 1.15	f <sub>yk</sub> = 355 MPa	γ <sub>M0</sub> = 1.05
f <sub>ck</sub> = 33.20 MPa	γ <sub>M</sub> = 1.15	f <sub>yd,0</sub> = 391.30 MPa	f <sub>yd,0</sub> = 338.10 MPa	
f <sub>ct</sub> = 18.81 MPa				
f <sub>cn</sub> = 41.2 MPa				
f <sub>dm</sub> = 3.10 MPa				
E <sub>cm</sub> = 33643 MPa				

**Caratteristiche geometriche**

A <sub>s</sub> = 0.1633 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
A <sub>v</sub> = 41320 mm <sup>2</sup>	Area taglio sezione di acciaio
y <sub>0,int,v</sub> = 1.27 m	Asse neutro rispetto lembo inferiore:
y <sub>0,int,q,0</sub> = 1.09 m	Asse neutro rispetto lembo superiore
J <sub>xx,0</sub> = 0.154628 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
W <sub>int,0</sub> = 0.121765 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
W <sub>int,0,d</sub> = 0.141845 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

**Sollecitazioni**

M <sub>sd</sub> = 10979.10 kNm	(SLU)		
M <sub>d</sub> = 41168.30 kNm	⇒	0.27	ok!
T <sub>sd</sub> = 880.21 kN	(SLU)		
T <sub>d</sub> = 7681.56 kN	⇒	0.11	ok!

**Tensioni**

σ <sub>c</sub> = 4.30 MPa	⇒	0.23	ok!{<1}	Tensione al lembo superiore soletta
σ <sub>s,sup</sub> = 73.85 MPa	⇒	0.19	ok!{<1}	Tensione barre superiori soletta
σ <sub>s,inf</sub> = 62.49 MPa	⇒	0.16	ok!{<1}	Tensione barre inferiori soletta
σ <sub>s,up</sub> = 58.94 MPa	⇒	0.17	ok!{<1}	Tensione acciaio lembo superiore
σ <sub>s,inf</sub> = 90.17 MPa	⇒	0.27	ok!{<1}	Tensione acciaio lembo inferiore

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Fase 3

Soletta c.a.collaborante

b <sub>eff</sub> =	2.84	m
Altezza soletta:	0.26	m
Area soletta:	0.74	m <sup>2</sup>
J <sub>xx</sub> =	0.00416	m <sup>4</sup>
Peso =	18.46	kN/m

Trave acciaio

Area trave acciaio:	0.10964	m <sup>2</sup>
H trave acciaio:	2.10	m
y <sub>0</sub> =	0.800	m
J <sub>xx</sub> =	0.080588	m <sup>4</sup>
Peso =	8.61	kN/m

Barre armatura soletta

1° strato barre (superiore)

n. Barre:	14	
Interasse:	0.20	m
Diametro:	0.024	m
Area:	0.006333	m <sup>2</sup>
y <sub>1</sub> =	0.05	m

2° strato barre (inferiore)

n. Barre:	14	
Interasse:	0.20	m
Diametro:	0.024	m
Area:	0.006333	m <sup>2</sup>
y <sub>2</sub> =	0.21	m

n = 6.2 Coeff. omogeneizzazione a tempo infinito

Materiali

cls	Barre armature		Acciaio	
R <sub>ck</sub> = 40 MPa	f <sub>yk</sub> = 450 MPa		f <sub>vk</sub> = 355 MPa	
f <sub>ck</sub> = 33.20 MPa	y <sub>M</sub> = 1.15		y <sub>M0</sub> = 1.05	
f <sub>ct</sub> = 18.81 MPa	f <sub>vd,s</sub> = 391.30 MPa		f <sub>vd,a</sub> = 338.10 MPa	
f <sub>cm</sub> = 41.2 MPa				
f <sub>csm</sub> = 3.10 MPa				
E <sub>cm</sub> = 33643 MPa				

Caratteristiche geometriche

A <sub>id</sub> = 0.2414 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
A <sub>i</sub> = 41320 mm <sup>2</sup>	Area taglio sezione di acciaio
y <sub>0,inferior</sub> = 1.58 m	Asse neutro rispetto lembo inferiore:
y <sub>0,superior</sub> = 0.78 m	Asse neutro rispetto lembo superiore
J <sub>xx,id</sub> = 0.203762 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
W <sub>inf,id</sub> = 0.12893 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
W <sub>sup,id</sub> = 0.261368 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

Sollecitazioni

M <sub>id</sub> = 28882.30 kNm	(SLU)		
M <sub>id</sub> = 43590.75 kNm		=>	0.66 ok!
T <sub>id</sub> = 2378.50 kN	(SLU)		
T <sub>id</sub> = 7681.56 kN		=>	0.31 ok!

Tensioni

σ <sub>z</sub> = 17.82 MPa	=>	0.95	ok!(<1)	Tensione al lembo superiore soletta
σ <sub>x,sup</sub> = 103.42 MPa	=>	0.26	ok!(<1)	Tensione barre superiori soletta
σ <sub>x,inf</sub> = 80.74 MPa	=>	0.21	ok!(<1)	Tensione barre inferiori soletta
σ <sub>a,sup</sub> = 73.65 MPa	=>	0.22	ok!(<1)	Tensione acciaio lembo superiore
σ <sub>a,inf</sub> = 224.01 MPa	=>	0.66	ok!(<1)	Tensione acciaio lembo inferiore

### 6.2.1.3 TRAVE PRINCIPALE CONCIO 3

Figura 55 - Concio 3 - Diagramma My - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

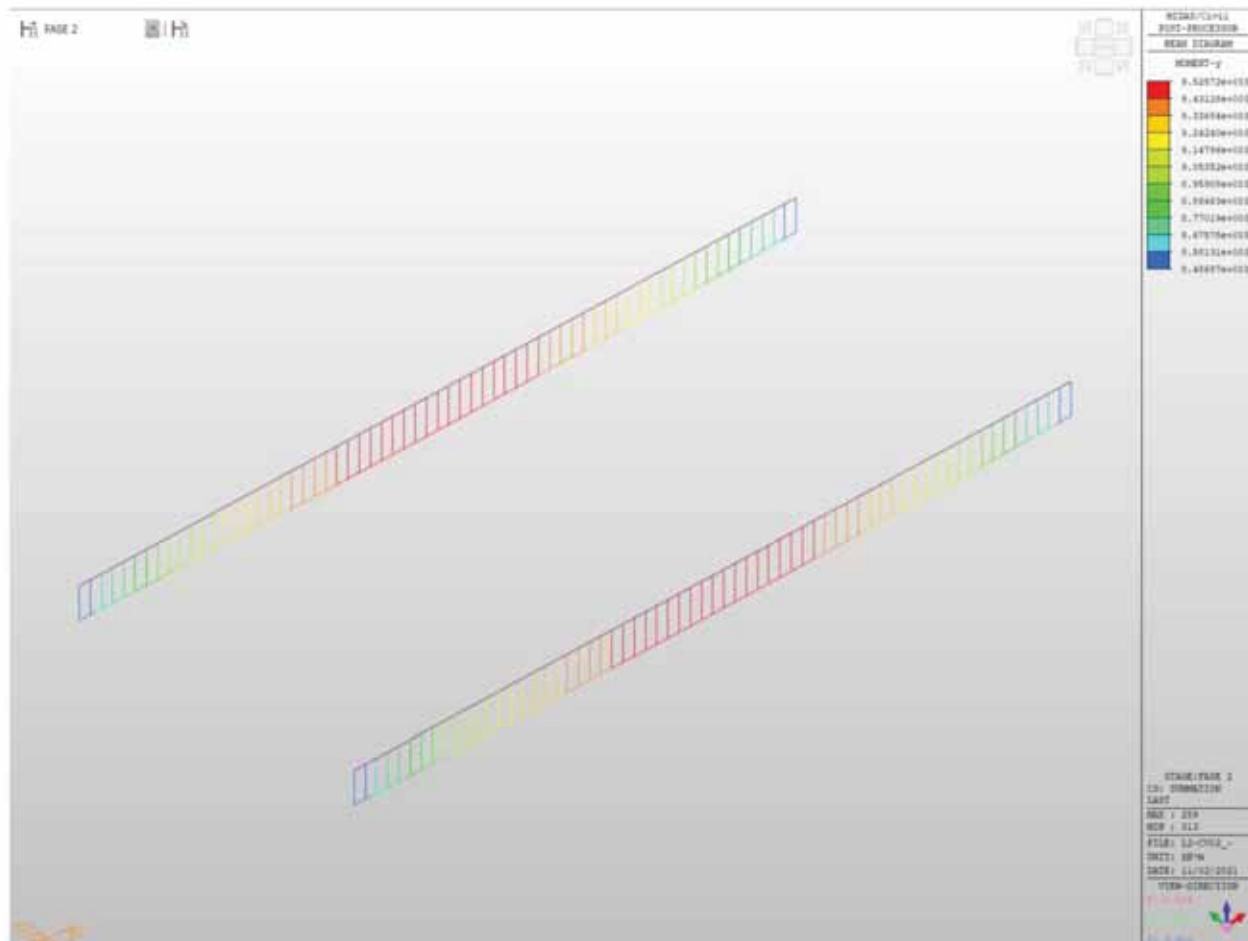


Figura 56 - Concio 3 - Diagramma My - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

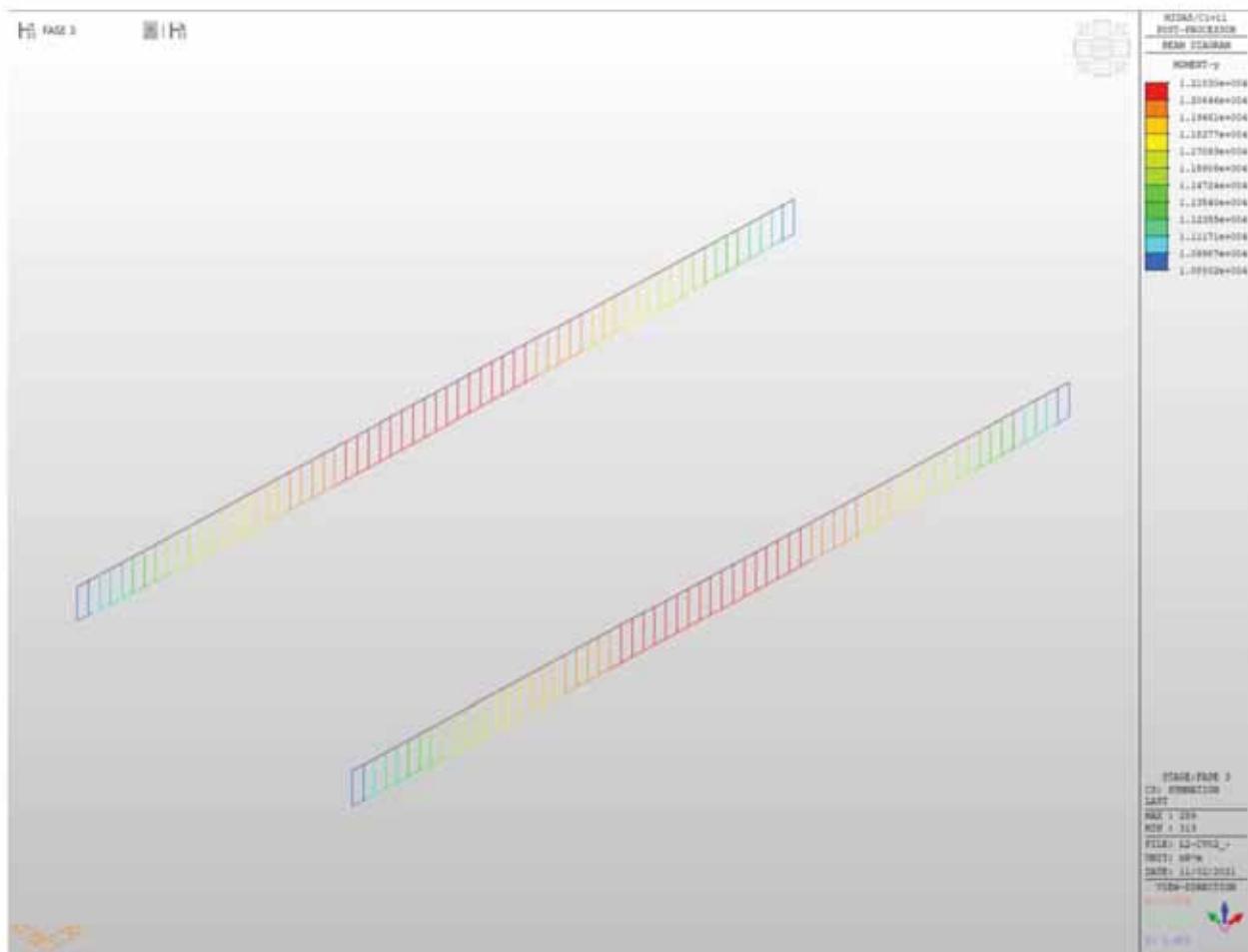


Figura 57 - Concio 3 - Diagramma My - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

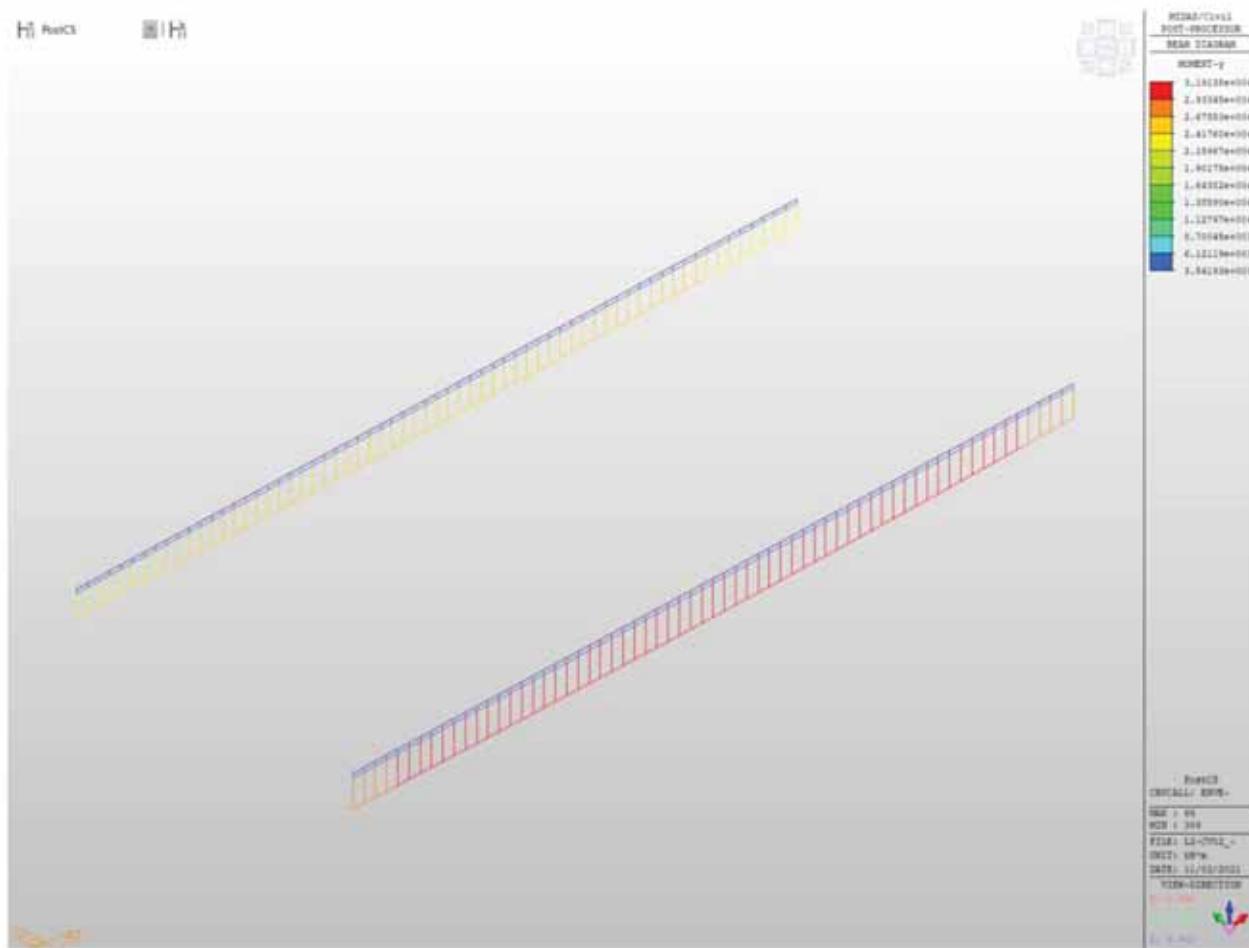


Figura 58 - Concio 3 - Diagramma My - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

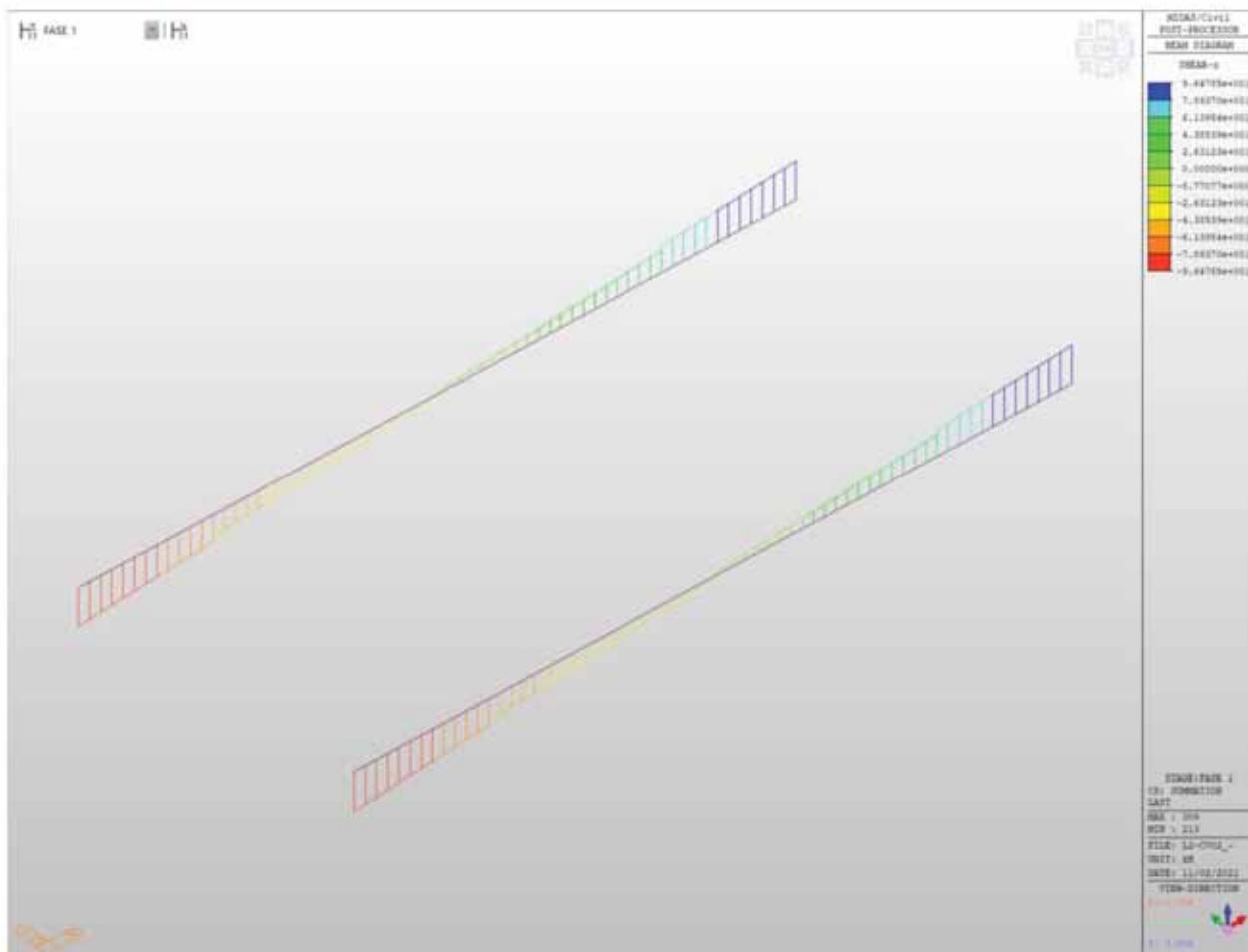


Figura 59 - Concio 3 - Diagramma Tz - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

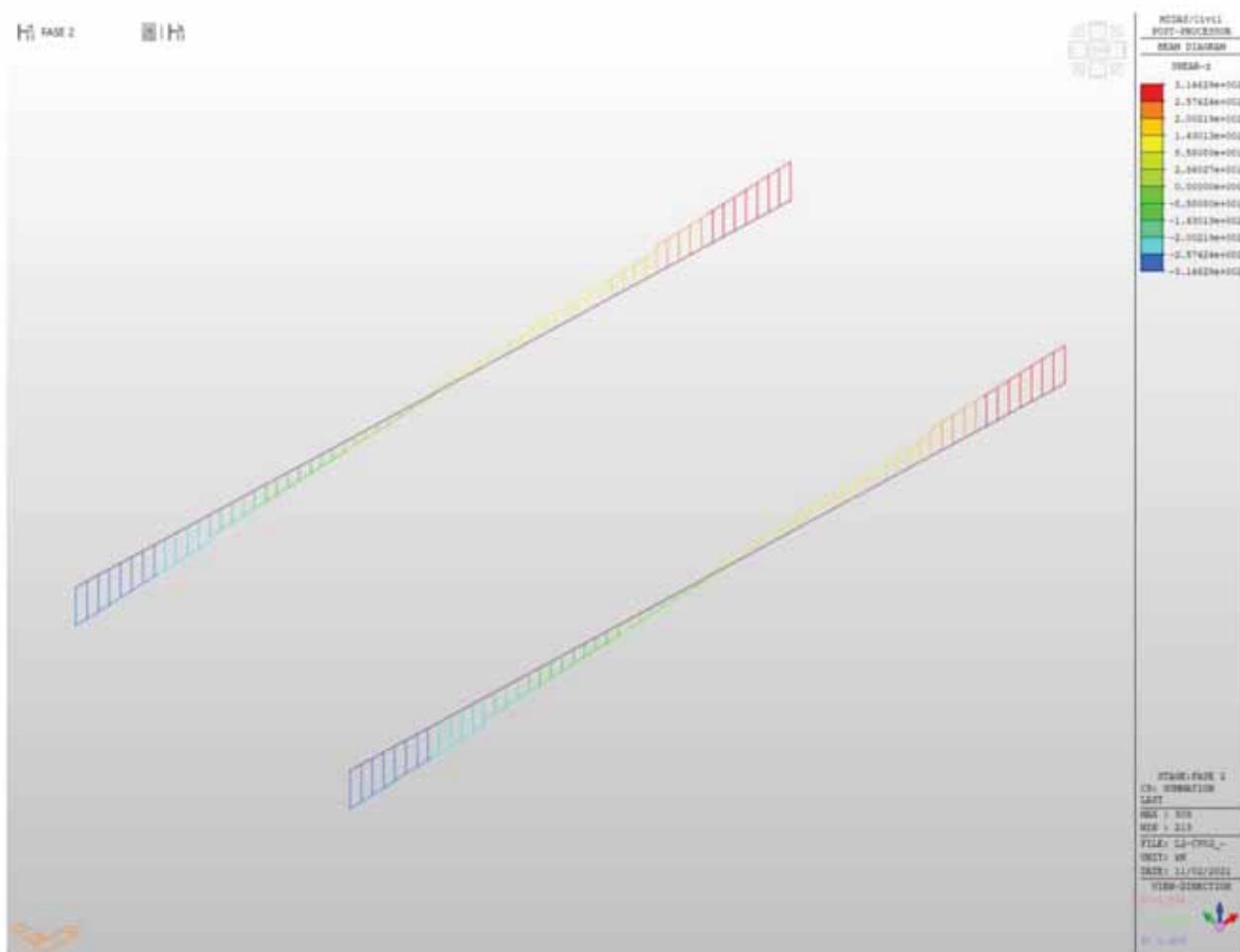


Figura 60 - Concio 3 - Diagramma Tz - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

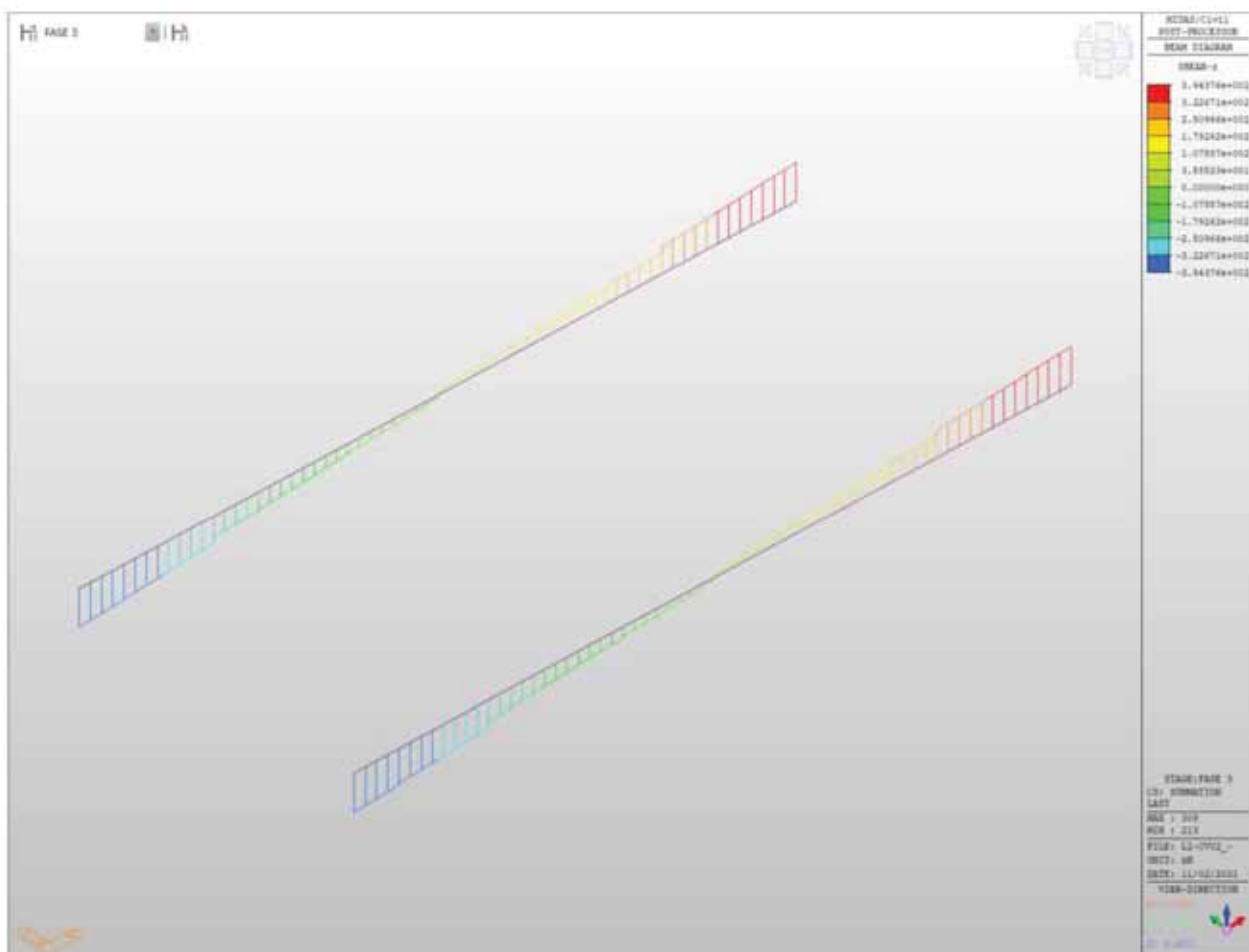


Figura 61 - Concio 3 - Diagramma Tz - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

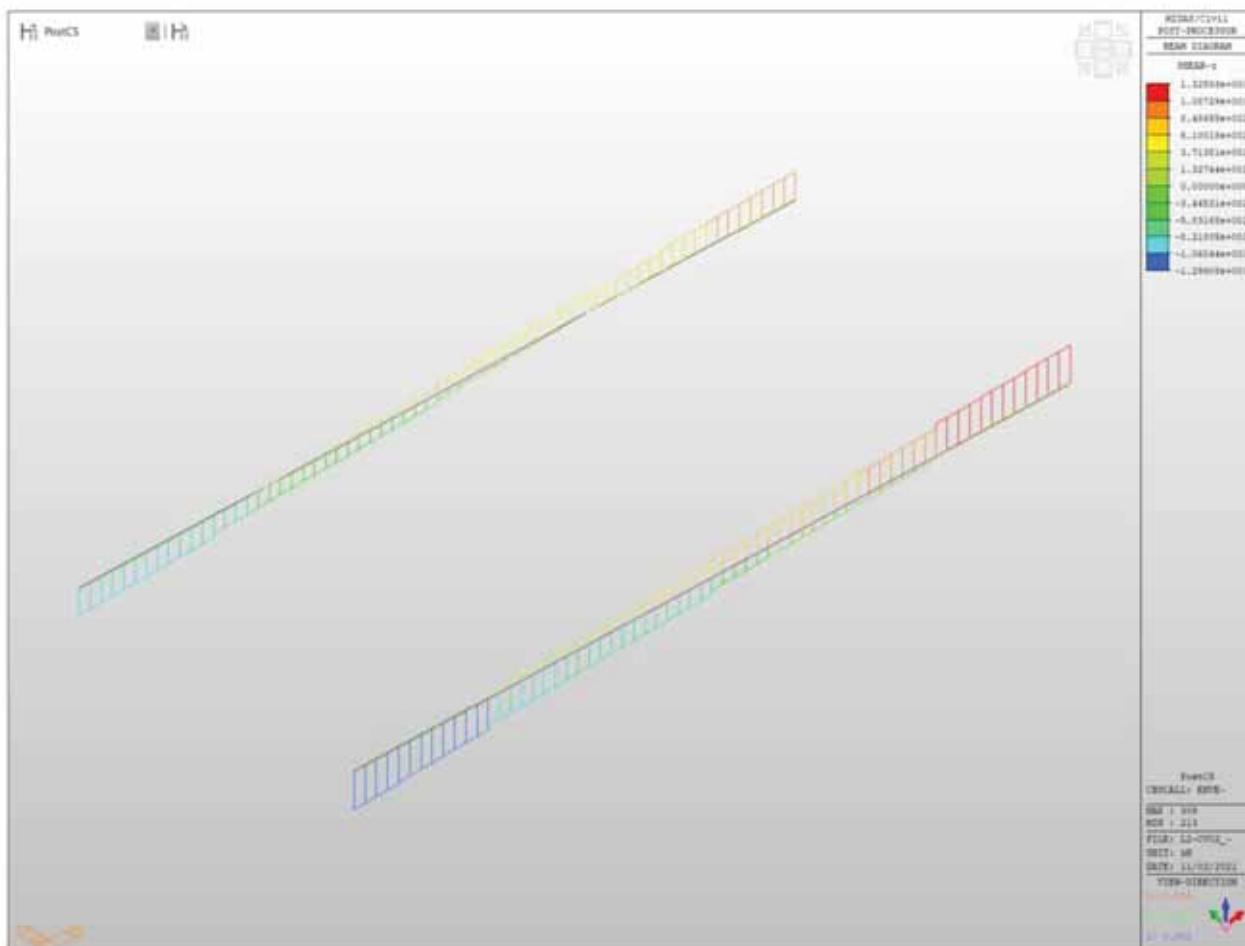


Figura 62 - Concio 3 - Diagramma Tz - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

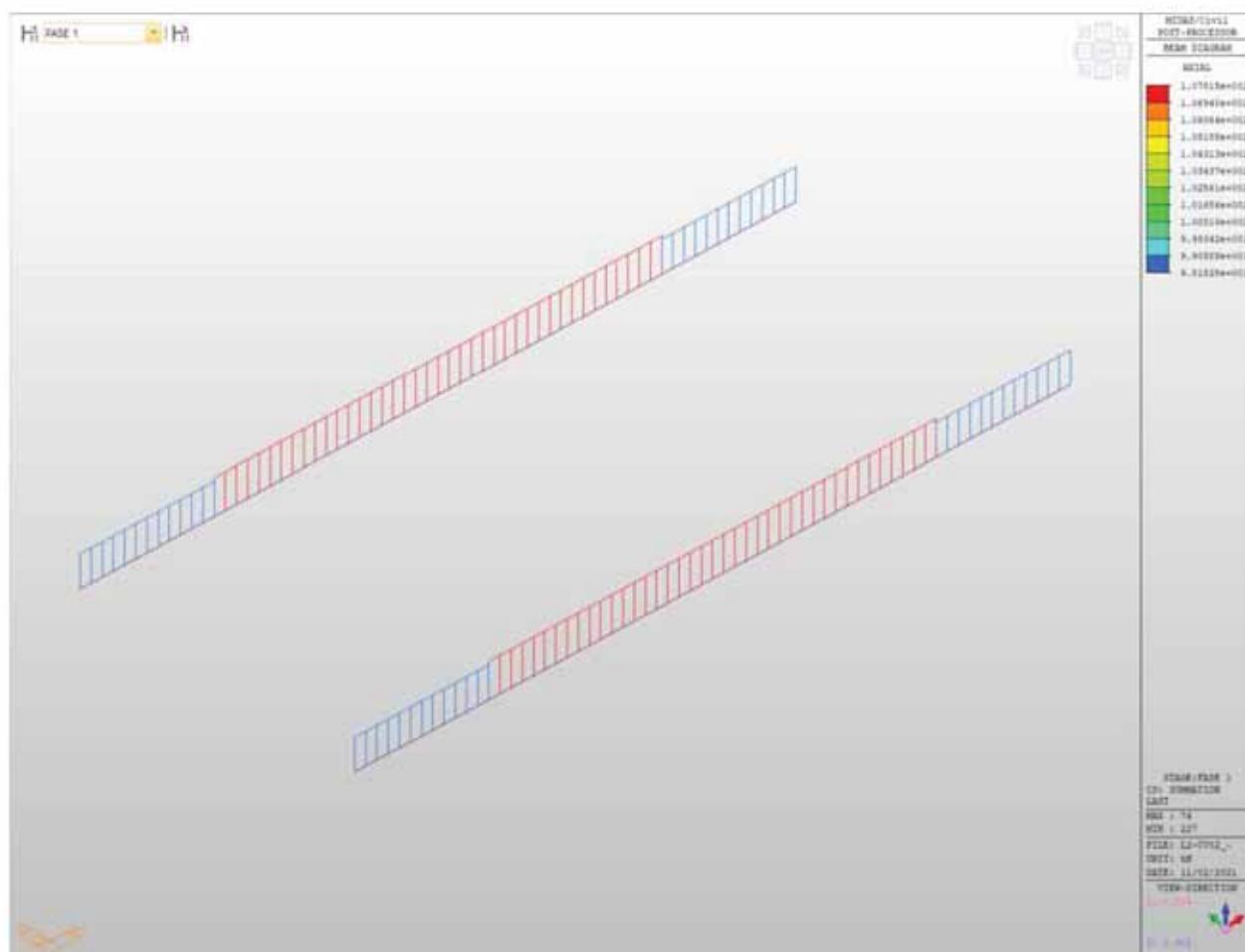


Figura 63 - Concio 3 - Diagramma N - Fase 1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

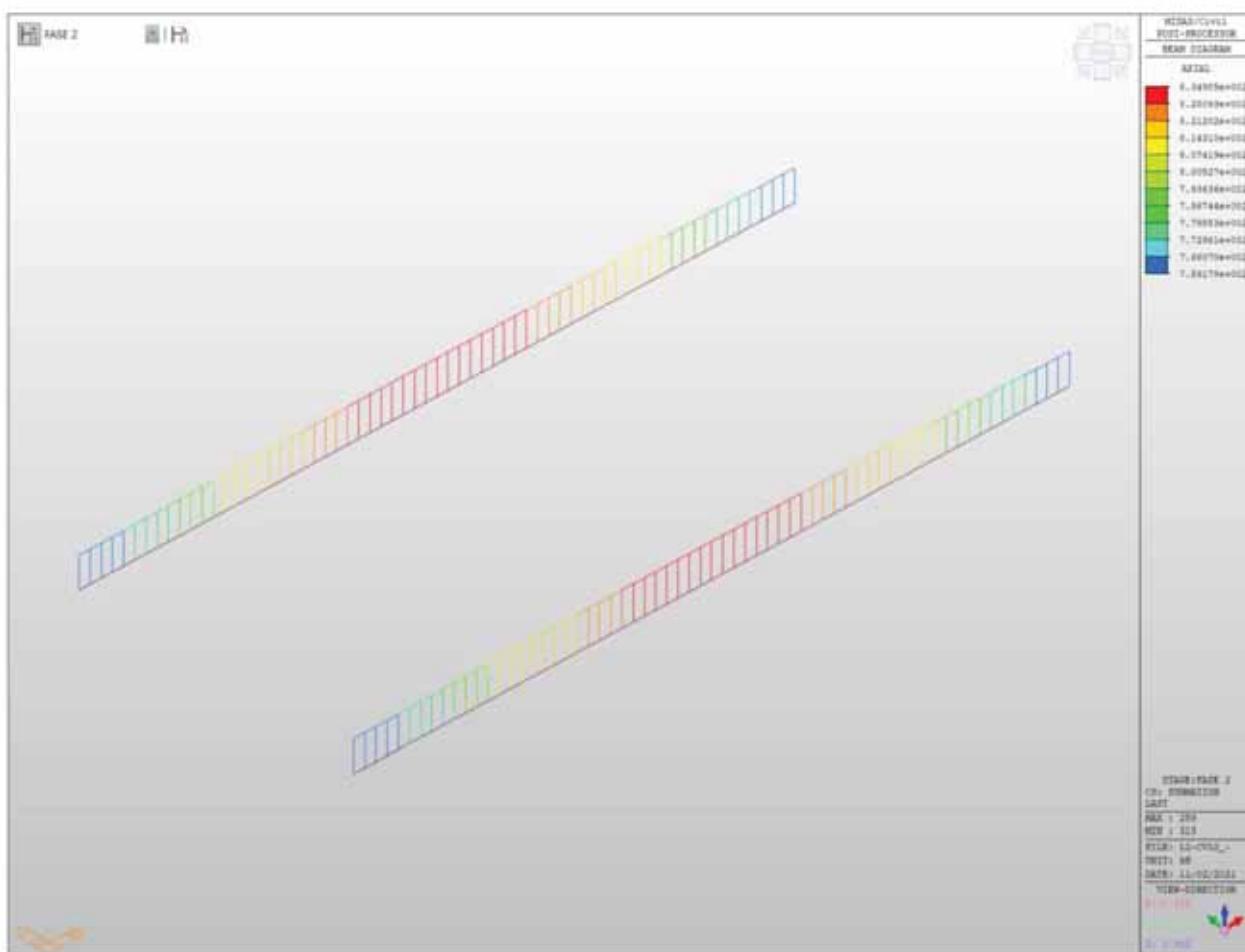


Figura 64 - Concio 3 - Diagramma N - Fase 2

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

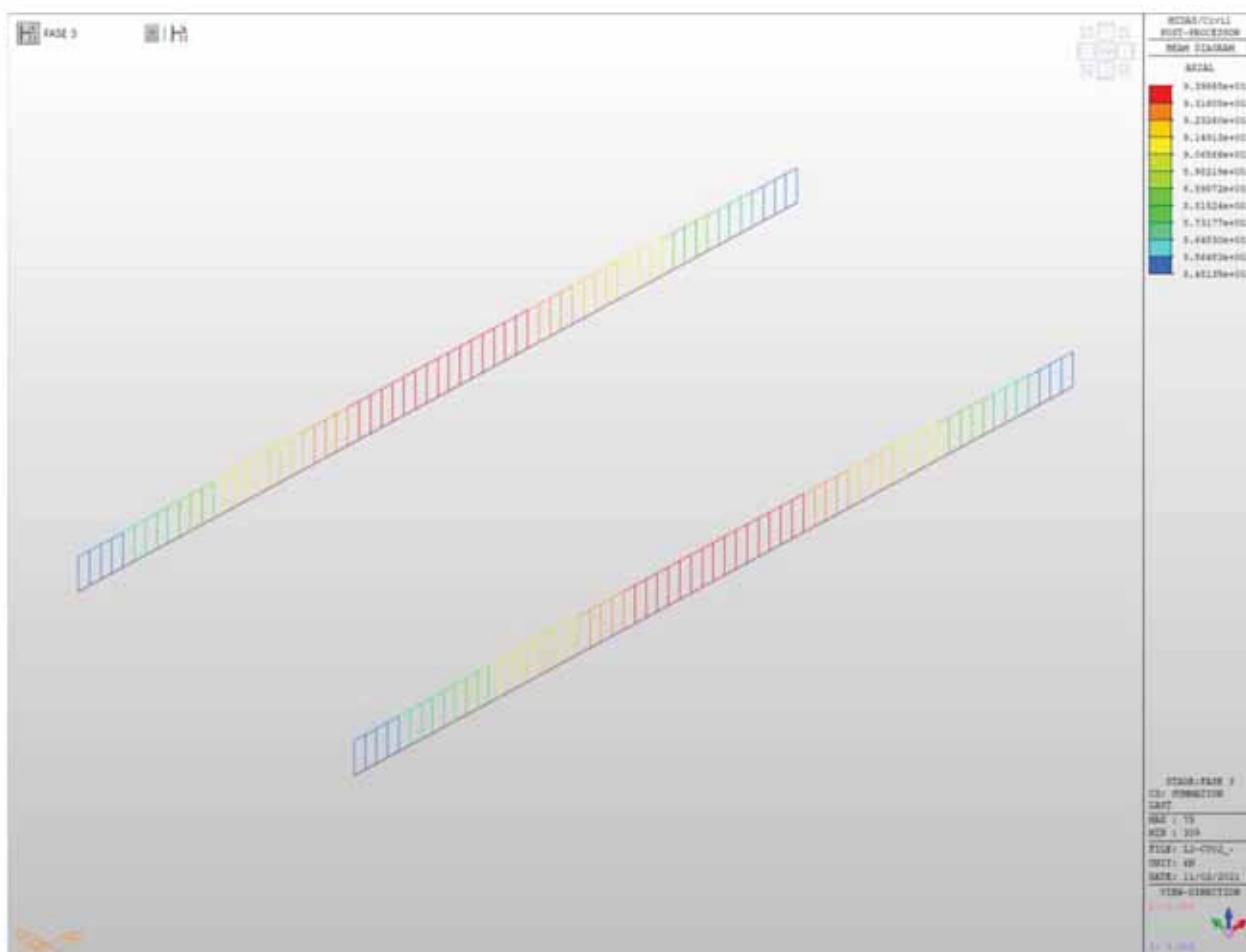


Figura 65 - Concio 3 - Diagramma N - Fase 3

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

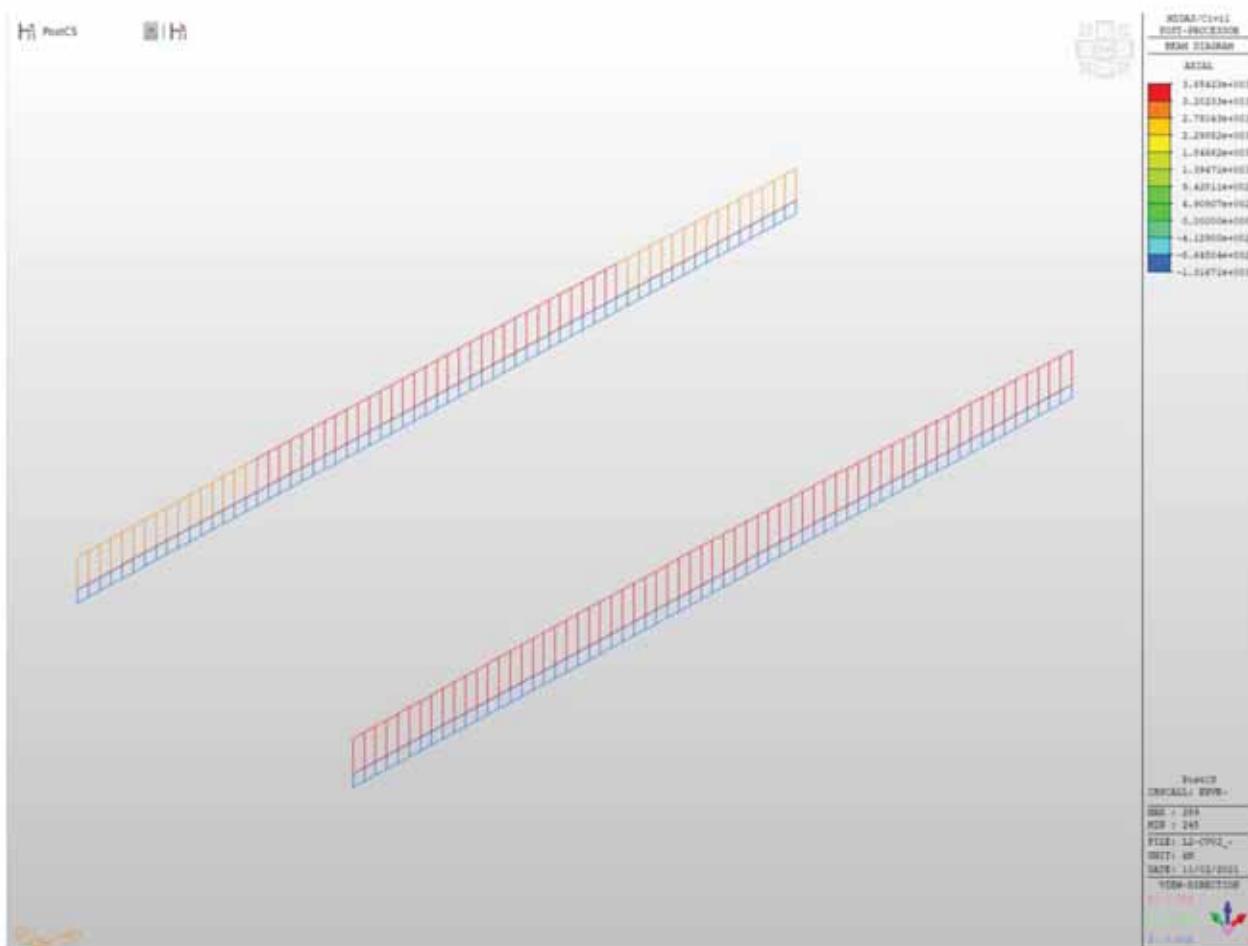


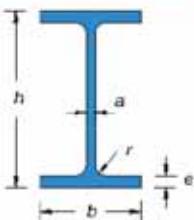
Figura 66 - Concio 3 - Diagramma N - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**VERIFICA SEZIONE IN ACCIAIO COMPOSTA**

**CARATTERISTICHE PROFILO:**

Tipo:	5355					
$B_{sup}$ =	750	mm				
$t_{sup}$ =	30.0	mm	$A_{t,sup}$ =	22500.00	$mm^2$	
$B_{inf}$ =	1200	mm	$A_{t,inf}$ =	66000.00	$mm^2$	
$t_{inf}$ =	55.0	mm				
H =	2100	mm				
$t_w = a$ =	20.0	mm	$A_w$ =	40300.00	$mm^2$	
L =	4400	mm	n. traversi:	1		
$A_{tot}$ =	128800.00	$mm^2$				
$y_{G,inf}$ =	710.76	mm	$y_{G,sup}$ =	1389.24	mm	
$J_{yy}$ =	9.19E+10	$mm^4$	$J_z$ =	78573333	$mm^4$	
$J_{zz}$ =	8976087500	$mm^4$	$J_w$ =	1.069E+09	$mm^4$	
$W_{el,yy,inf}$ =	129359105	$mm^3$	$W_{el,yy,inf}$ =	66182824	$mm^3$	
$W_{pl,yy}$ =	98791188	$mm^3$				
$W_{pl,zz}$ =	$2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$					
$\rho_{yy}$ =	844.90	mm	$\rho_{zz}$ =	$\sqrt{\frac{J_{zz}}{A}}$		
$\rho_{zz}$ =	263.99	mm				
Peso =	1011.08	kg/m				
E =	210000	$N/mm^2$	modulo elastico			
v =	0.3		coeff. poisson			
G =	80769	$N/mm^2$	$G = \frac{E}{2(1+v)}$	modulo elasticità trasversale		
$\alpha$ =	0.000012	$^{\circ}C^{-1}$	coeff. espansione termica lineare			
$f_yk$ =	355	$N/mm^2$	tensione snervamento caratteristica			
$f_{uk}$ =	510	$N/mm^2$	tensione rottura caratteristica			
$\gamma_{M0}$ =	1.05		per sezioni classe 1,2,3 e 4			
$\gamma_{M1}$ =	1.05		per instabilità membrature			
$\gamma_{M2}$ =	1.10		per instabilità membrature ponti stradali e ferroviari			
$\gamma_{M2}$ =	1.25		per sezioni tese indebolite dai fori nei riguardi della frattura			



**SOLLECITAZIONI:**

$N_{sd}$ =	0	N	caso per trave tesa
$N_{sd}$ =	835000	N	caso per trave compressa
$M_{sd}$ =	9525700000	Nmm	caso per trave inflessa in una direzione
$V_{sd}$ =	314630	N	taglio

Caso per trave a flessione deviata (Momenti flettenti agenti alle estremità  $|M_3| < |M_4|$ ):

$$M_3 = 7620560000 \quad Nmm \quad M_4 = 9525700000 \quad Nmm$$

Valori del momento massimo e minimo dell'asta:

$$M_{max,y,ud} = 0 \quad Nmm \quad M_{max,y,sd} = 0 \quad Nmm$$

$$M_{max,z,ud} = 0 \quad Nmm \quad M_{max,z,sd} = 0 \quad Nmm$$

**CLASSE APPARTENENZA PROFILO (NTC2008, 4,2,3,1):**

ALA SUPERIORE	ALA INFERIORE	ANIMA	
$C/t_{sup}$ = 12.17	$C/t_{inf}$ = 10.73	$C/t_w$ = 100.75	3
$\xi$ = 0.81	$\xi$ = 0.81	$\xi$ = 0.81	

Curva di instabilità "b" (Asse yy):

Fattore di imperfezione  $\alpha$ : 0.34

Curva di instabilità "c" (Asse zz):

Fattore di imperfezione  $a$ : 0.49

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{ed}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:  $A_v = 41150 \text{ mm}^2$

$$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}} \quad V_{c,Rd} = 8032454 \quad N \Rightarrow 0.04 \quad OK!(<1)$$

Verifica in presenza di torsione uniforme:

$$\tau_{t,ed} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,ed}}{\frac{f_{yk}}{\sqrt{3} \gamma_{M0}}} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 8032454 \quad N \Rightarrow 0.04 \quad OK!(<1)$$

Verifica in termini tensionali nel punto più sollecitato:

$$\frac{\tau_{ed}}{f_{yk}} \leq 1 \Rightarrow 0.00 \quad OK!(<1)$$

**SOLLECITAZIONE DELL'ASTA A COMPRESSIONE SEMPLICE (NTC2008, 4.2.4.1.2):**

$$\frac{N_{ed}}{N_{c,Rd}} \leq 1 \quad N_{cr} = \frac{\pi^2 E J}{(\eta l)^2} = 960949941 \quad N \quad N_{c,ed} < 0,04 N_{cr}$$

*Instabilità trascurabile se  $\lambda < 0.2$*

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$$\mu = 1.00 \quad l_0 = \mu l = 4400 \text{ mm}$$

$$\lambda = l_0 / \rho = 5$$

Calcolo resistenza per sezioni in classe 1,2 e 3:

$$N_{c,Rd} = \frac{A f_{yk}}{\gamma_{M0}} \quad N_{c,Rd} = 43546667 \quad N \Rightarrow 0.02 \quad OK!(<1)$$

**STABILITÀ DELLE MEMBRATURE (NTC2008, 4.2.4.1.3):**

Curva di instabilità "b" (Asse yy):

Fattore di imperfezione  $\alpha$ : 0.49

Verifica per sezioni classe 1,2 e 3:

$$\tilde{\lambda} = \sqrt{\frac{A f_{yk}}{N_{cr}}} = 0.22 \quad \tilde{\lambda} > 0.2 \quad \text{Serve verifica instabilità!}$$

$$\Phi = 0.5 [1 + \alpha(\tilde{\lambda} - 0.2) + \tilde{\lambda}^2] = 0.53$$

$$\chi = \frac{1}{\Phi + \sqrt{\Phi^2 - \tilde{\lambda}^2}} = 0.99$$

$$N_{b,Rd} = \frac{\chi A f_{yk}}{\gamma_{M1}} = 41183245 \quad N$$

$$\frac{N_{c,ed}}{N_{b,Rd}} \leq 1 \Rightarrow 0.02 \quad OK!(<1)$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):**

$$\frac{M_{sd}}{M_{c,Rd}} \leq 1$$

$$V_{sd} < 0,5 V_{c,Rd}$$

*Il taglio non influisce sulla verifica a flessione!*

$$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2 \quad 0.84946$$

$$f_{y,red} = (1 - \rho)f_{yk} \quad 53 \quad N/mm^2$$

$$f_{yk} = \quad 355 \quad N/mm^2$$

Calcolo resistenza elastica sezione linda, classe 3:

$$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}} \quad M_{c,Rd} = \quad 43735697332 \quad Nmm \quad \Rightarrow \quad 0.22 \quad **OK!(<1)**$$

$$\text{n. fori} = \quad \text{diametro} = \quad \text{mm}$$

$$\frac{0,9 A_f,net f_{ck}}{\gamma_{M2}} \geq \frac{A_f f_{yk}}{\gamma_{M0}} \quad **Non serve la verifica per la presenza dei fori!**$$

**TRAVI INFLESSE (NTC2008, 4.2.4.1.3.2):**

Calcolo snellezza membrana (< di 200 per le travature principali e < di 250 per le secondarie):

$$\mu = \quad 0.70 \quad L_{cr} = L / \mu = \quad 3080 \quad \text{mm}$$

$$W_{yy} = \quad 129359105 \quad \text{mm}^3 \quad W_{yy} = \quad 66182824 \quad \text{mm}^3$$

$$\psi = 1.75 - 1.05 \frac{M_B}{M_A} + 0.3 \left( \frac{M_B}{M_A} \right)^2 \quad 1.10$$

$$M_{cr} = \psi \frac{\pi}{L_{cr}} \sqrt{EJ_y G J_T} \sqrt{1 + \left( \frac{\pi}{L_{cr}} \right)^2 \frac{EJ_w}{G J_T}} \quad 3.937E+11 \quad Nmm \quad 0.024$$

$$M_{sd}/M_{cr} = \quad 0.024 \quad **Risulta < 0.16 Non serve la verifica instabilità**$$

$$\tilde{\lambda}_{LT} = \sqrt{\frac{W_y f_{yk}}{M_{cr}}} \quad 0.24 \quad **Risulta < 0.4 Non serve la verifica instabilità**$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Stabilità dei pannelli soggetti a taglio (NTC2008, 4.2.4.1.3.4.1):**

Se irrigiditori trasversali rigido o no,

$$I_{st} = 45000000 \text{ mm}^4 \quad 0.75hw^3 = 5740289 \text{ mm}^4 \quad \textcolor{red}{\text{Irrigiditore Trasversali rigidi}}$$

Coefficiente minimo di instabilità per taglio del panello (In assenza di irrigiditori longitudinali),

$$a/hw = 2.10 \quad k_t = 7.25$$

Coefficiente minimo di instabilità per taglio del panello (irrigiditori longitudinali più di due o  $a/hw > 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad k_{tl} = 0.00$$

$$a/hw = 2.10 \quad k_t = 7.25$$

Coefficiente minimo di instabilità per taglio del panello (irrigiditori longitudinali più di due o  $a/hw < 3$ ),

$$I_{sl} = 0 \text{ mm}^4 \quad a/hw = 2.10 \quad k_t = 5.54$$

$$k_t = 7.25$$

$$\eta = 1.2 \quad hw/t = 100.75 \quad \textcolor{red}{\text{Serve la verifica di instabilità}}$$

$$\sigma_E = 17.23356009 \text{ MPa} \quad \tau_{cr} = 124.93 \quad \lambda_w = 1.28$$

$$\text{Montanti di appoggio rigidi, } X_w = 0.69$$

$$\text{Gli altri casi, } X_w = 0.65$$

$$V_{bw,Rd} = 5311379 \text{ N}$$

Momento resistente sole piattabande,

$$y_{G,inf} = 550.59 \text{ mm} \quad y_{G,sup} = 1549.41 \text{ mm}$$

$$J_{yy} = 71051768856 \text{ mm}^4$$

$$W_{el,yy,inf} = 129045848 \text{ mm}^3 \quad W_{el,yy,sup} = 45857402 \text{ mm}^3$$

$$M_{t,red} = 15071503216 \text{ Nmm}$$

$$bf = 364.5 \text{ mm} \quad V_{bf,Rd} = 36239 \text{ N}$$

$$V_{b,Rd} = 5347618 \text{ N} \quad 0.059 \quad \textcolor{red}{OK!(<1)}$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

*Stabilità dei pannelli soggetti a Compressione (NTC2008, 4.2.4.1.3.4.1):*

Stress,

$\sigma_{\text{sup}} = 143.93 \text{ MPa}$

$\sigma_{\text{inf}} = -73.64 \text{ MPa}$

$\sigma_{\text{int.}} = 91.09 \text{ MPa}$

Irrigidati su entrambi lati (Anima),

$b = 2015.0 \text{ mm}$  (Web bw)  $a = 4400 \text{ mm}$

$\sigma_1 = 143.9 \text{ (Max)}$   $\sigma_2 = -73.64 \text{ (Min)}$

$\psi = -0.5$   $k\sigma = 13.59$

$\lambda_p = 1.2$   $p = 0.84$

$\sigma_{\text{cr},p} = 254.3 \text{ MPa}$   $\sigma_{\text{cr},c} = 3.921 \text{ MPa}$

$\zeta = 1.0$   $\lambda_c = 9.515$

$\alpha = 0.34$   $\Phi = 143.847$

$\chi_c = 0.003$

$p_c = 0.84$

$b_{\text{eff}} = 1121.94 \text{ mm}$   $b_{\text{e1}} = 448.78 \text{ mm}$   $b_{\text{e2}} = 673.16 \text{ mm}$

$b_t = 681.99 \text{ mm}$

Irrigidati a un lato (Piattebande),

Piattebanda Superiore,

$b = 365 \text{ mm}$  (Flange Sup)

$\sigma (\text{Inri.}) = 143.9$   $\sigma (\text{Lib.}) = 143.93$

$k\sigma = 0.43$

$\lambda_p = 1.2$   $p = 0.70$

$b_{\text{eff}} = 254.81 \text{ mm}$   $b_t = 0 \text{ mm}$

Effective Section Properties,

$A_{\text{c,eff}} (\text{Piattebanda Sup}) = 15288.8 \text{ mm}^2$

$A_{\text{c,eff}} (\text{Piattebanda Inf}) = 66000.0 \text{ mm}^2$

$A_{\text{c,eff}} (\text{Anima}) = 22438.8 \text{ mm}^2$

$A_{\text{c}} (\text{Anima tensione}) = 13639.9 \text{ mm}^2$

$A_{\text{c,eff}} (\text{Total}) = 117367.5 \text{ mm}^2$

$\gamma_{g,\text{inf}} = 597.4 \text{ mm}$   $\gamma_{g,\text{sup}} = 1502.6 \text{ mm}$

$I_{\text{eff}} = 7.90E+10 \text{ mm}^4$

$W_{\text{eff}} (\text{inf}) = 132172497 \text{ mm}^3$   $W_{\text{eff}} (\text{sup}) = 52546016 \text{ mm}^3$

$e = 113.39 \text{ mm}$

$\eta = 0.54$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Fase 2

**Soletta c.a.collaborante**

b <sub>eff</sub> =	2.84	m
Altezza soletta:	0.26	m
Area soletta:	0.74	m <sup>2</sup>
J <sub>yy</sub> =	0.00416	m <sup>4</sup>
Peso =	18.46	kN/m

**Trave acciaio**

Area trave acciaio:	0.1288	m <sup>2</sup>
H trave acciaio:	2.10	m
y <sub>c</sub> =	0.711	m
J <sub>yy</sub> =	0.091944	m <sup>4</sup>
Peso =	10.11	kN/m

**Barre armatura soletta**

<b>1° strato barre (superiore)</b>	<b>2° strato barre (inferiore)</b>
n. Barre:	14
Interasse:	0.20 m
Diametro:	0.024 m
Area:	0.006333 m <sup>2</sup>
y <sub>1</sub> =	0.05 m
	y <sub>2</sub> = 0.21 m

n = 18.0

Coeff. omogeneizzazione con viscosità in atto

**Materiali**

<b>ds</b>	<b>Barre armature</b>		<b>Acciaio</b>	
R <sub>cx</sub> = 40 MPa	f <sub>yk</sub> = 450 MPa	y <sub>M</sub> = 1.15	f <sub>yk</sub> = 355 MPa	y <sub>M0</sub> = 1.05
f <sub>ck</sub> = 33.20 MPa	f <sub>yd,L</sub> = 391.30 MPa		f <sub>yd,L</sub> = 338.10 MPa	
f <sub>cd</sub> = 18.81 MPa				
f <sub>cm</sub> = 41.2 MPa				
f <sub>ctm</sub> = 3.10 MPa				
E <sub>cm</sub> = 33643 MPa				

**Caratteristiche geometriche**

A <sub>id</sub> = 0.1825 m <sup>2</sup>	Area sezione omogeneizzata in area di acciaio
A <sub>v</sub> = 41150 mm <sup>2</sup>	Area taglio sezione di acciaio
y <sub>G,infl,id</sub> = 1.16 m	Asse neutro rispetto lembo inferiore:
y <sub>G,su,le</sub> = 1.20 m	Asse neutro rispetto lembo superiore
J <sub>xx,id</sub> = 0.179717 m <sup>4</sup>	Momento d'inerzia sezione omogeneizzata ad acciaio
W <sub>int,id</sub> = 0.155232 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio
W <sub>tau,id</sub> = 0.149481 m <sup>3</sup>	Modulo elastico sezione omogeneizzata ad acciaio

**Sollecitazioni**

M <sub>sd</sub> = 12183.00 kNm	(SLU)		
M <sub>rd</sub> = 50538.97 kNm	⇒	0.24	ok!
T <sub>sd</sub> = 394.37 kN	(SLU)		
T <sub>rd</sub> = 7649.96 kN	⇒	0.05	ok!

**Tensioni**

$\sigma_c$ = 4.53 MPa	⇒	0.24	ok!{<1}	Tensione al lembo superiore soletta
$\sigma_{s,su,0}$ = 78.11 MPa	⇒	0.20	ok!{<1}	Tensione barre superiori soletta
$\sigma_{s,in}$ = 67.27 MPa	⇒	0.17	ok!{<1}	Tensione barre inferiori soletta
$\sigma_{a,su,0}$ = 63.88 MPa	⇒	0.19	ok!{<1}	Tensione acciaio lembo superiore
$\sigma_{a,in}$ = 78.48 MPa	⇒	0.23	ok!{<1}	Tensione acciaio lembo inferiore

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Fase 3

**Soletta c.o.collaborante**

$b_{eff} =$	2.84	m
Altezza soletta:	0.26	m
Area soletta:	0.74	$m^2$
$J_{st} =$	0.00416	$m^4$

Peso = 18.46 kN/m

**Trave acciaio**

Area trave acciaio:	0.1288	$m^2$
H trave acciaio:	2.10	m
$y_c =$	0.711	m
$J_{st} =$	0.091944	$m^4$
Peso =	10.11	kN/m

**Barre armatura soletta**

<b>1° strato barre (superiore)</b>	<b>2° strato barre (inferiore)</b>
<b>n. Barre:</b> 14	<b>n. Barre:</b> 14
<b>Interasse:</b> 0.20 m	<b>Interasse:</b> 0.20 m
<b>Diametro:</b> 0.024 m	<b>Diametro:</b> 0.024 m
<b>Area:</b> 0.006333 $m^2$	<b>Area:</b> 0.006333 m
$y_1 =$ 0.05 m	$y_2 =$ 0.21 m

<b>1° strato barre (superiore)</b>	<b>2° strato barre (inferiore)</b>
<b>n. Barre:</b> 14	<b>n. Barre:</b> 14
<b>Interasse:</b> 0.20 m	<b>Interasse:</b> 0.20 m
<b>Diametro:</b> 0.024 m	<b>Diametro:</b> 0.024 m
<b>Area:</b> 0.006333 $m^2$	<b>Area:</b> 0.006333 m
$y_1 =$ 0.05 m	$y_2 =$ 0.21 m

$n =$  6.2 Coeff. omogeneizzazione a tempo infinito

**Materiali**

ds	Barre armature		Acciaio	
$R_{ck} =$ 40 MPa	$f_{yk} =$ 450 MPa	$f_{vk} =$ 355 MPa		
$f_{cx} =$ 33.20 MPa	$\gamma_M =$ 1.15	$\gamma_{M0} =$ 1.05		
$f_{cz} =$ 18.81 MPa	$f_{y0,3} =$ 391.30 MPa	$f_{y0,3} =$ 338.10 MPa		
$f_{cm} =$ 41.2 MPa				
$f_{ctm} =$ 3.10 MPa				
$E_{cm} =$ 33643 MPa				

**Caratteristiche geometriche**

$A_{el} =$ 0.2606 $m^2$	Area sezione omogeneizzata in area di acciaio
$A_y =$ 41150 $mm^2$	Area taglio sezione di acciaio
$y_{c,inf,id} =$ 1.48 m	Asse neutro rispetto lembo inferiore:
$y_{c,sup,id} =$ 0.88 m	Asse neutro rispetto lembo superiore
$J_{ex,id} =$ 0.243027 $m^4$	Momento d'inerzia sezione omogeneizzata ad acciaio
$W_{pl,id} =$ 0.164316 $m^3$	Modulo elastico sezione omogeneizzata ad acciaio
$W_{sl,id} =$ 0.27586 $m^3$	Modulo elastico sezione omogeneizzata ad acciaio

**Sollecitazioni**

$M_{sd} =$ 31913.80 kNm	$(SLU)$		
$M_{sd} =$ 55554.42 kNm	$\Rightarrow$	0.57	ok!
$T_{sd} =$ 1326.00 kN	$(SLU)$		
$T_{sd} =$ 7649.96 kN	$\Rightarrow$	0.17	ok!

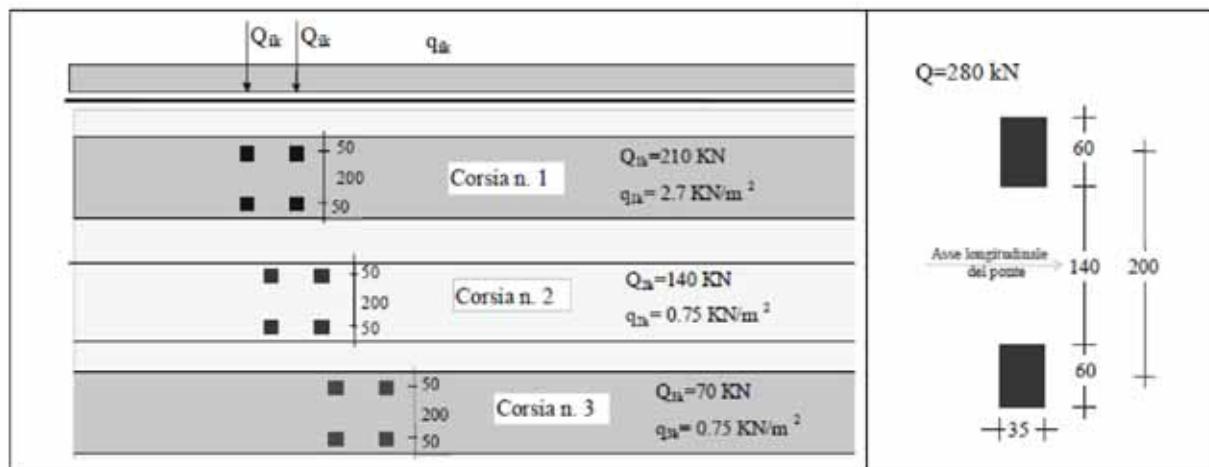
**Tensioni**

$\sigma_z =$ 18.66 MPa	$\Rightarrow$	0.99	<i>ok!(&lt;1)</i>	Tensione al lembo superiore soletta
$\sigma_{z,sup} =$ 109.12 MPa	$\Rightarrow$	0.28	<i>ok!(&lt;1)</i>	Tensione barre superiori soletta
$\sigma_{z,inf} =$ 88.11 MPa	$\Rightarrow$	0.23	<i>ok!(&lt;1)</i>	Tensione barre inferiori soletta
$\sigma_{x,up} =$ 81.55 MPa	$\Rightarrow$	0.24	<i>ok!(&lt;1)</i>	Tensione acciaio lembo superiore
$\sigma_{x,int} =$ 194.22 MPa	$\Rightarrow$	0.57	<i>ok!(&lt;1)</i>	Tensione acciaio lembo inferiore

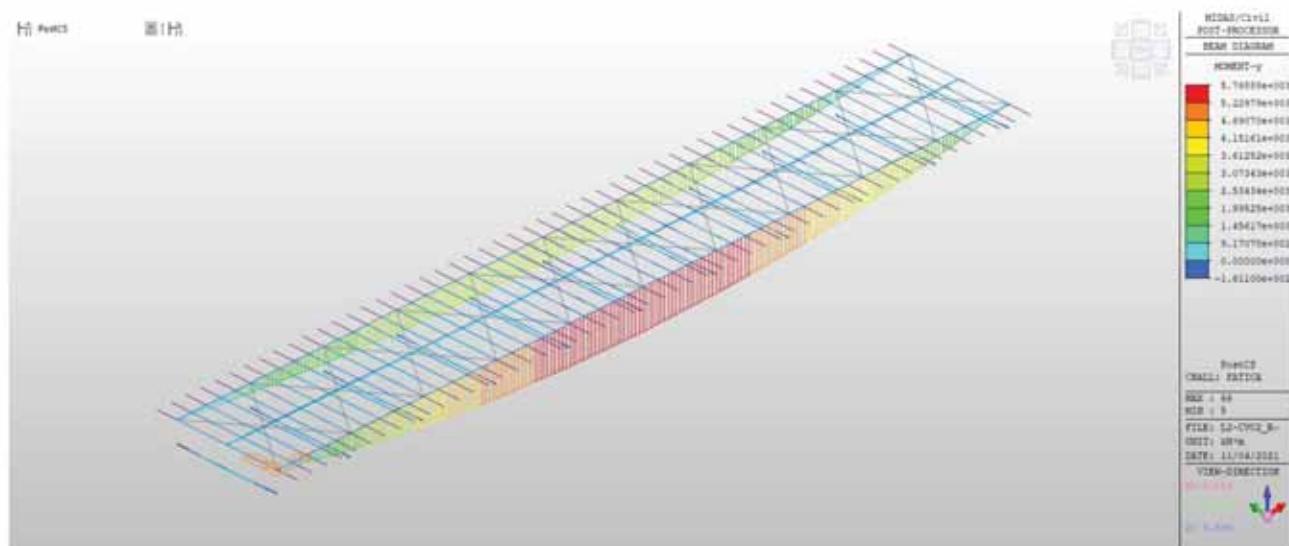
**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

## 6.2.2 VERIFICA A FATICA

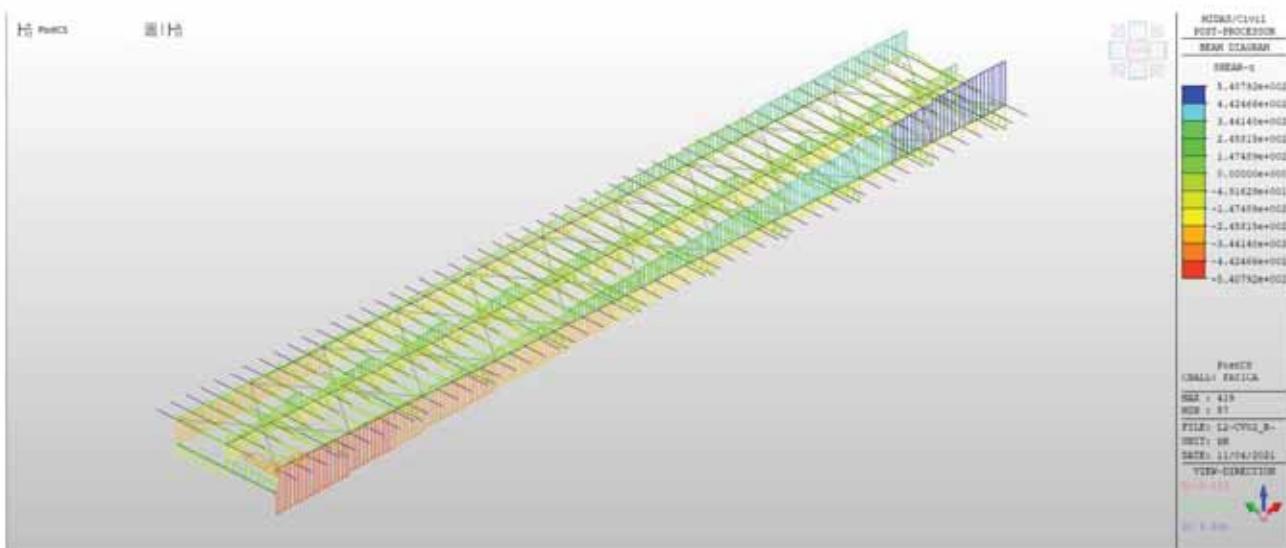
Le verifiche a fatica sono state effettuate considerando il modello di carico che è costituito dallo schema di carico 1 con valori dei carichi concentrati ridotti del 30% e valori dei carichi distribuiti ridotti del 70%.



Si riportano di seguito le sollecitazioni calcolate sulla trave oggetto di verifica soggetta ai carichi suddetti.



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 – RELAZIONE DI CALCOLO IMPALCATO**



Si procede con il calcolo delle variazioni di tensioni sulla trave in acciaio dovute al transito dei carichi suddetti.

**Sollecitazioni**

M <sub>sd</sub> =	5768.88	kNm	(SLU)		
M <sub>rd</sub> =	55554.42	kNm	=>	0.10	ok!
T <sub>sd</sub> =	540.79	kN	(SLU)		
T <sub>rd</sub> =	7649.96	kN	=>	0.07	ok!

**Tensioni**

$\sigma_{s,sup} =$	19.73	MPa	=>	0.05	ok!(<1)	Tensione barre superiori soletta
$\sigma_{s,inf} =$	15.93	MPa	=>	0.04	ok!(<1)	Tensione barre inferiori soletta
$\sigma_{a,sup} =$	14.74	MPa	=>	0.04	ok!(<1)	Tensione acciaio lembo superiore
$\sigma_{a,inf} =$	35.11	MPa	=>	0.10	ok!(<1)	Tensione acciaio lembo inferiore

**VERIFICHE PER VITA ILLIMITATA**

Le verifiche a fatica per vita illimitata possono essere condotte controllando che il massimo delta di tensione  $\Delta s_{max}$  indotto nel dettaglio stesso dallo spettro di carico risulti minore del limite di fatica del dettaglio stesso:

$$\Delta s_{max,d} = \gamma_{MF} \Delta s_{max} \leq \Delta s_A$$

oppure

$$\Delta t_{max,d} = \gamma_{MF} \Delta t_{max} < \Delta t_0 = \Delta t_L$$

$$\Delta s_0 = 0,737 \Delta s_c$$

$\gamma_{MF} = 1,35$  (strutture sensibili alla rottura per fatica - conseguenze significative)

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Travi in composizione saldata**

100	 	<p>5) Saldatura manuale a cordoni d'angolo o a piena penetrazione 6) Saldatura a piena penetrazione manuale o automatica eseguita da un sol lato, in particolare per travi a cassone</p> <p>5) e 6) Deve essere assicurato un corretto contatto tra anima e piattabande. Il bordo dell'anima deve essere preparato in modo da garantire una penetrazione regolare alla radice, senza interruzioni</p>
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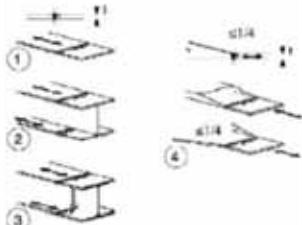
Delta massimo di tensione attacco anima-ala inf:

$$\Delta s_{\max} = 35,11 \text{ MPa}$$

$$\text{Limiti di fatica ad ampiezza costante: } \Delta s_D = 0,737 \times 100 = 73,70 \text{ MPa}$$

$$\text{Verifica a fatica: } \gamma_M \Delta s_{\max} < \Delta s_D \quad 1,35 \times 35,11 = 47,40 \text{ MPa} < 73,70 \text{ MPa} = \Delta s_D$$

**Saldature di testa trasversali**

112		<p>Saldature senza piatto di sostegno</p> <p>1) Giunti trasversali in piatti e lamiere 2) Giunti di anima e piattabande in travi composite eseguiti prima dell'assemblaggio 3) Giunti trasversali complessi di profili laminati, in assenza di lunette di scarico 4) Giunti trasversali di lamiere e piatti con rastremazioni in larghezza e spessore con pendenza non maggiore di 1:4. Nelle zone di transizione gli intagli nelle saldature devono essere eliminati</p> <p>Per spessori <math>t &gt; 25 \text{ mm}</math>, si deve adottare una classe ridotta del coefficiente  <math>k_s = (25/t)^{1/2}</math></p>	<p>Saldature effettuate da entrambi i lati, molate in direzione degli sforzi e sottoposte a controlli non distruttivi</p> <p>Le saldature devono essere iniziate e terminate su tacchi d'estremità, da rimuovere una volta completata la saldatura</p> <p>I bordi esterni delle saldature devono essere molati in direzione degli sforzi</p> <p>3) Vale solo per profili tagliati e risaldati</p>
-----	--	--	---

Delta massimo di tensione attacco anima-ala inf:

$$\Delta s_{\max} = 35,11 \text{ MPa}$$

$$\text{Limiti di fatica ad ampiezza costante: } \Delta s_D = 0,737 \times 112 = 82,54 \text{ MPa}$$

$$\text{Verifica a fatica: } \gamma_M \Delta s_{\max} < \Delta s_D \quad 1,35 \times 35,11 = 47,40 \text{ MPa} < 82,54 \text{ MPa} = \Delta s_D$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 – RELAZIONE DI CALCOLO IMPALCATO**

### 6.2.3 TRAVE DI SPINA

Si riporta di seguito la verifica della trave di spina. La trave presenta un valore di momento flettente di inviluppo positivo e negativo lungo tutto il suo sviluppo per effetto dell'inviluppo delle azioni statiche e sismiche. In particolare, le azioni sismiche verticali, producono proprio un effetto di inversione di momento. Per tale motivo a favore di sicurezza si verifica la trave di spina solo come elemento in acciaio trascurando l'effetto della soletta collaborante.

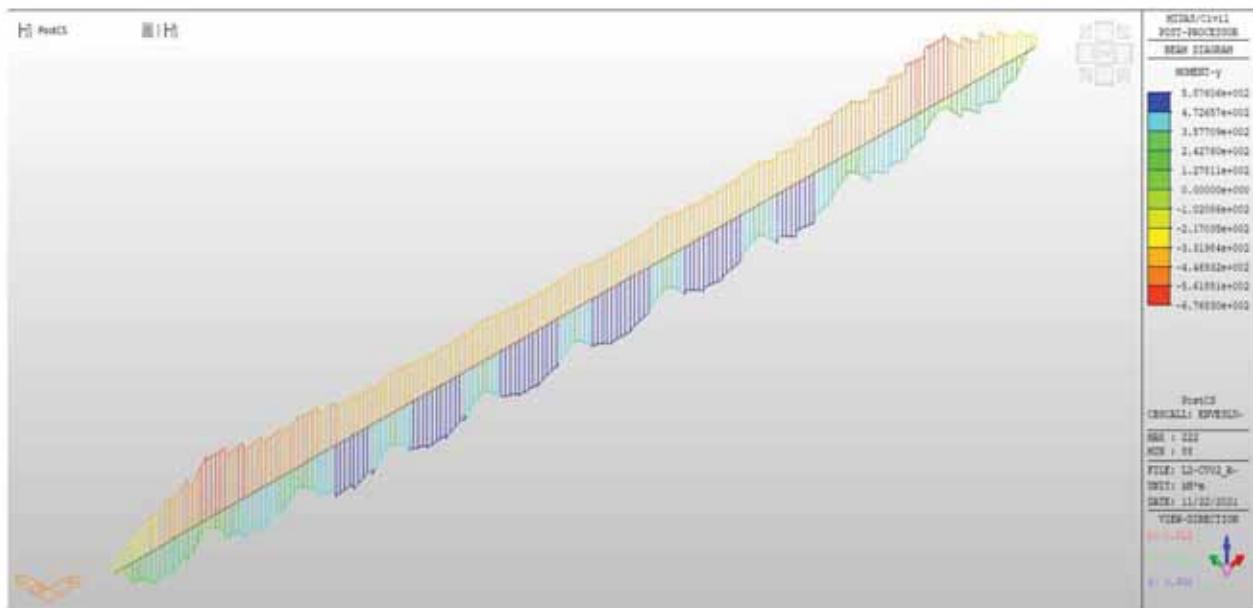


Figura 67- Trave di spina - Diagramma My - Inviluppo SLU-SLV

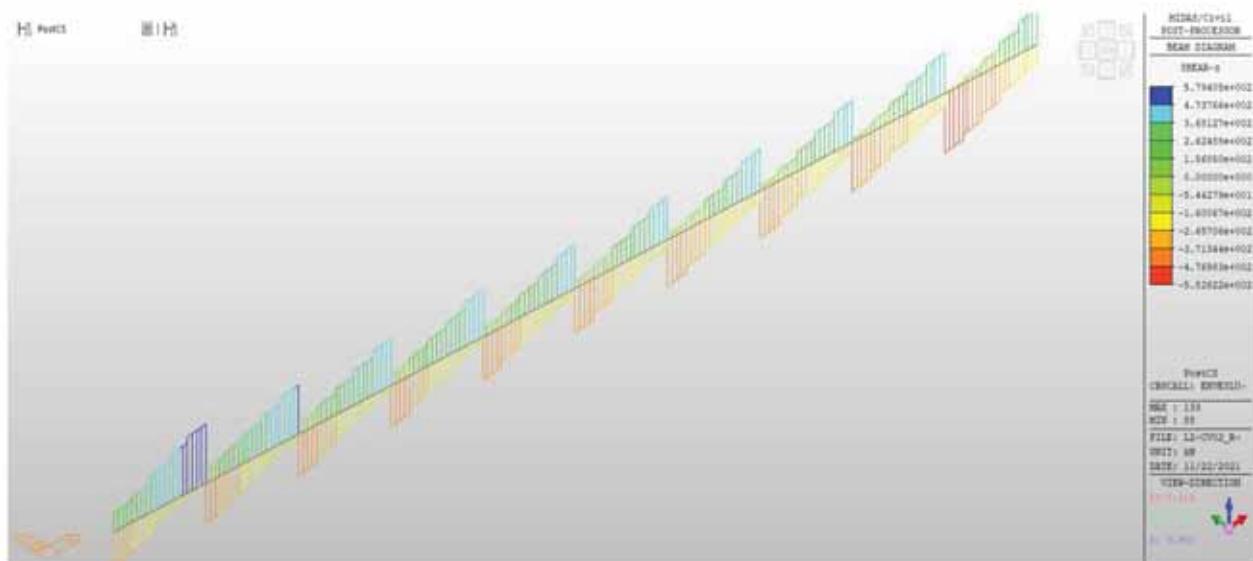
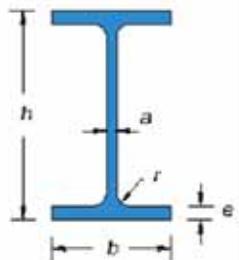


Figura 68 - Trave di spina - Diagramma Tz - Inviluppo SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**CARATTERISTICHE PROFILO:**

Tipo:	5355					
$B_{SUP}$ =	300	mm				
$t_{t,SUP}$ =	25.0	mm	$A_{t,sup}$ =	7500.00	mm <sup>2</sup>	
$B_{INF}$ =	300	mm				
$t_{t,INF}$ =	25.0	mm	$A_{t,INF}$ =	7500.00	mm <sup>2</sup>	
H =	590	mm				
$t_w = a$ =	13.0	mm	$A_w$ =	7020.00	mm <sup>2</sup>	
L =	4400	mm	n. traversi:	1		
$A_{TOT}$ =	22020.00	mm <sup>2</sup>				
$y_{G,inf}$ =	295.00	mm	$y_{G,sup}$ =	295.00	mm	
$J_{xx}$ =	1368461000	mm <sup>4</sup>	$J_t$ =	3520460	mm <sup>4</sup>	
$J_{yy}$ =	112608019	mm <sup>4</sup>	$J_o$ =	5.070E+11	mm <sup>6</sup>	
$W_{el,inf}$ =	4638851	mm <sup>3</sup>	$W_{el,sup}$ =	4638851	mm <sup>3</sup>	
$W_{pl}$ =	5185200	mm <sup>3</sup>				
$W_{pl,xx} = 2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$						
$\rho_{xx}$ =	249.29	mm	$\rho_{xx} = \sqrt{\frac{J_{xx}}{A}}$			
$\rho_{yy}$ =	71.51	mm				
Peso =	172.86	kg/m				
E =	210000	N/mm <sup>2</sup>	modulo elastico			
v =	0.3		coeff. poisson			
G =	80769	N/mm <sup>2</sup>	$G = \frac{E}{2(1+v)}$		modulo elasticità trasversale	
$\alpha$ =	0.000012	°C <sup>-1</sup>	coeff. espansione termica lineare			
$f_{yk}$ =	355	N/mm <sup>2</sup>	tensione snervamento caratteristica			
$f_{rk}$ =	510	N/mm <sup>2</sup>	tensione rottura caratteristica			
$\gamma_{M0}$ =	1.05	per sezioni classe 1,2,3 e 4				
$\gamma_{M1}$ =	1.05	per instabilità membrature				
$\gamma_{M1}$ =	1.10	per instabilità membrature ponti stradali e ferroviari				
$\gamma_{M2}$ =	1.25	per sezioni tese indebolite dai fori nei riguardi della frattura				



**SOLLECITAZIONI:**

$N_{t,sd}$ =	0	N	caso per trave tesa
$N_{c,sd}$ =	0	N	caso per trave compressa
$M_{sd}$ =	676830000	Nmm	caso per trave inflessa in una direzione
$V_{sd}$ =	582620	N	taglio

Caso per trave a flessione deviata:

$M_{y,sd}$ =	0	Nmm	$M_{z,sd}$ =	0	Nmm
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Valori del momento massimo e minimo dell'asta:

$M_{max,y,sd}$ =	0	Nmm	$M_{max,y,sd}$ =	0	Nmm
$M_{max,z,sd}$ =	0	Nmm	$M_{max,z,sd}$ =	0	Nmm

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1):							
ALA SUPERIORE		ALA INFERIORE		ANIMA			
$C/t_{f,sup} =$	5.74	$C/t_{f,inf} =$	5.74	$C/t_w =$	41.54		
$\varepsilon =$	0.81	$\varepsilon =$	0.81	$\varepsilon =$	0.81		
Curva di instabilità "b":							
Fattore di imperfezione $\alpha$ :			0.34	ANIMA			
<input checked="" type="radio"/> Parte soggetta a flessione <input type="radio"/> Parte soggetta a compressione <input type="radio"/> Parte soggetta a flessione ed a compressione							
<b>ANIMA</b> <i>Classe 1</i>							

SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):					
$\frac{V_{sd}}{V_{c,Rd}} \leq 1$					
Area taglio resistente:		$A_v =$	7345	$\text{mm}^2$	
$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3}\gamma_{M0}}$	$V_{c,Rd} =$	1433739	N	$\Rightarrow$	0.41 <b>OK!(&lt;1)</b>

SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):					
$\frac{M_{sd}}{M_{c,Rd}} \leq 1$					
$V_{sd} < 0,5 V_{c,Rd}$	<i>Il taglio non influisce sulla verifica a flessione!</i>				
$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2$	0.03507	$f_{y,red} = (1 - \rho)f_{yk}$	343	$\text{N/mm}^2$	
		$f_{yk} =$	355	$\text{N/mm}^2$	
Calcolo resistenza plastica sezione linda, classe 1 e 2:					
$M_{pl,Rd} = \frac{W_{pl} f_{yk}}{\gamma_{M0}}$	$M_{pl,Rd} =$	1753091429	Nmm	$\Rightarrow$	0.39 <b>OK!(&lt;1)</b>
Calcolo resistenza elastica sezione linda, classe 3:					
$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}}$	$M_{el,Rd} =$	1568373382	Nmm	$\Rightarrow$	0.43 <b>OK!(&lt;1)</b>

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

#### 6.2.4 TRAVERSO

Si riportano di seguito i diagrammi di sollecitazione e di verifica dei traversi.

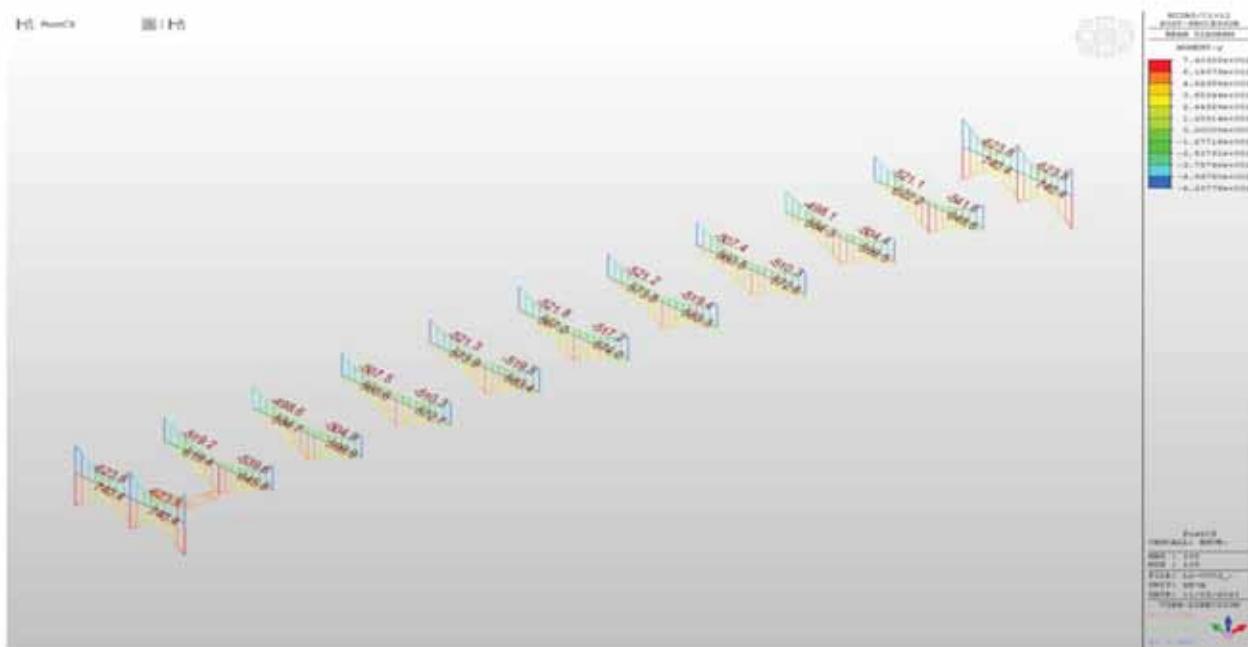


Figura 69 – Traverso - Momento SLU-SLV

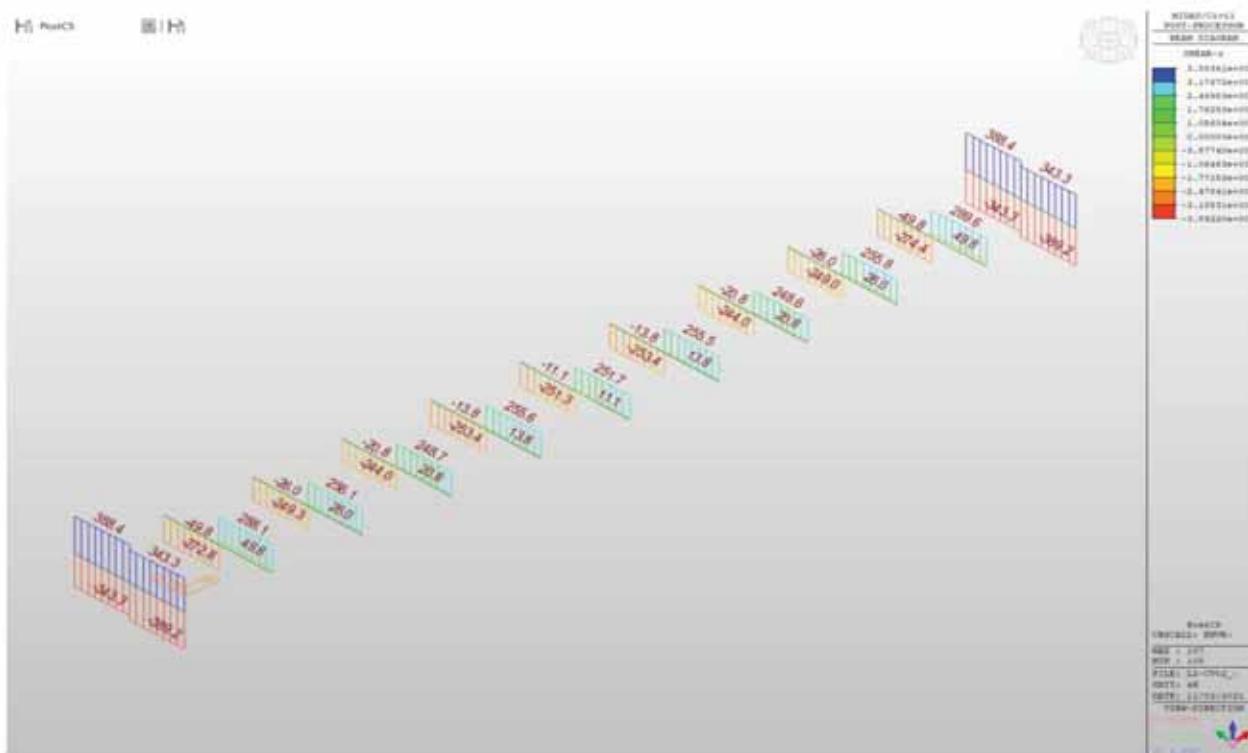


Figura 70 - Traverso - Taglio SLU-SLV

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**6.2.4.1 VERIFICA TRAVERSO DI CAMPATA**

CARATTERISTICHE PROFILO:		
Tipo:	S355	
$B_{SUF}$ =	350	mm
$t_{SUF}$ =	25,0	mm
$B_{inf}$ =	350	mm
$t_{inf}$ =	25,0	mm
H =	800	mm
$t_a = a$ =	20,0	mm
L =	6000	mm
$A_{tot}$ =	32500,00	mm <sup>2</sup>
$y_{G,inf}$ =	400,00	mm
$J_n$ =	3331770833	mm <sup>4</sup>
$J_n$ =	179179167	mm <sup>4</sup>
$W_{el,inf}$ =	8329427	mm <sup>3</sup>
$W_{pl}$ =	9593750	mm <sup>3</sup>
$W_{pl,xx} = 2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$		
$\rho_m$ =	320,18	mm
$\rho_m$ =	74,25	mm
Peso =	255,13	kg/m
E =	210000	N/mm <sup>2</sup>
v =	0,3	
G =	80769	N/mm <sup>2</sup>
$\alpha$ =	0,000012	°C <sup>-1</sup>
$f_u$ =	355	N/mm <sup>2</sup>
$f_u$ =	510	N/mm <sup>2</sup>
$\gamma_{M0}$ =	1,05	persezioni classe 1,2,3 e 4
$\gamma_{M1}$ =	1,05	per instabilità membrature
$\gamma_{M1}$ =	1,10	per instabilità membrature ponti stradali e ferroviari
$\gamma_{M2}$ =	1,25	persezioni tese indebolite dai fori nei riguardi della frattura
COLLEGAMENTI:		
$N_{L,inf}$ =	0	N caso per trave tesa
$N_{T,inf}$ =	0	N caso per trave compressa
$M_{L,inf}$ =	648600000	Nmm caso per trave inflessa in una direzione
$V_{L,inf}$ =	289000	N taglio
Caso per treve a flessione deviata:		
$M_{E,inf}$ =	0	Nmm
Valori del momento massimo e minimo dell'asta:		
$M_{max,inf}$ =	0	Nmm
$M_{min,inf}$ =	0	Nmm
$M_{max,inf}$ =	0	Nmm
$M_{min,inf}$ =	0	Nmm
CLASSE APPARTENENZA PROFILO (NTC2008-4-2-3-11)		
ALA SUPERIORE	ALA INFERIORE	ANIMA
$C/t_{SUF}$ =	6,60	$C/h_w$ = 37,50
$e$ =	0,81	$e$ = 0,81
Curva di instabilità "b":		
Fattore di imperfezione $\alpha_c$ :	0,34	<b>ANIMA</b> Classe 1
<input checked="" type="radio"/> Parte soggetta a flessione		
<input type="radio"/> Parte soggetta a compressione		
<input type="radio"/> Parte soggetta a flessione ed a compressione		

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{sd}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:  $A_v = 15500 \text{ mm}^2$

$$V_{c,Rd} = \frac{A_v f_{yk}}{\sqrt{3} \gamma_{M0}}$$

$V_{c,Rd} = 3025590 \text{ N}$   $\Rightarrow 0,10 \text{ OK!(<1)}$

Verifica in presenza di torsione uniforme:

$$\tau_{t,sd} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,sd}}{f_{yk}} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 3025590 \text{ N} \Rightarrow 0,10 \text{ OK!(<1)}$$

**SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):**

$$\frac{M_{sd}}{M_{c,Rd}} \leq 1$$

$V_{sd} < 0,5 V_{c,Rd}$

*Il taglio non influisce sulla verifica a flessione!*

$$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2 \quad 0,65442$$

$f_{y,red} = (1 - \rho)f_{yk}$	123	N/mm <sup>2</sup>
$f_{yk} =$	355	N/mm <sup>2</sup>

Calcolo resistenza plastica sezione linda, classe 1 e 2:

$$M_{pl,Rd} = \frac{W_{pl} f_{yk}}{\gamma_{M0}} \quad M_{pl,Rd} = 3243601190 \text{ Nmm} \Rightarrow 0,20 \text{ OK!(<1)}$$

Calcolo resistenza elastica sezione linda, classe 3:

$$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}} \quad M_{c,Rd} = 2816139633 \text{ Nmm} \Rightarrow 0,23 \text{ OK!(<1)}$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**6.2.4.2 VERIFICA TRAVERSO DI TESTATA**

CARATTERISTICHE PROFILO:				
Tipo:	S355			
B <sub>sup</sub> =	350 mm			
t <sub>sup</sub> =	25,0 mm			
B <sub>inf</sub> =	350 mm			
t <sub>inf</sub> =	25,0 mm			
H =	1350 mm			
t <sub>w</sub> = a =	20,0 mm			
L =	6000 mm			
A <sub>tot</sub> =	43500,00 mm <sup>2</sup>			
A <sub>v,up</sub> =	8750,00 mm <sup>2</sup>			
A <sub>v,inf</sub> =	8750,00 mm <sup>2</sup>			
n. traversi:	1			
Y <sub>0,inf</sub> =	675,00 mm			
J <sub>xx</sub> =	11343437500 mm <sup>4</sup>			
J <sub>yy</sub> =	179545833 mm <sup>4</sup>			
W <sub>el,inf</sub> =	16805093 mm <sup>3</sup>			
W <sub>pl</sub> =	20043750 mm <sup>3</sup>			
W <sub>pl,xx</sub> =	$2 \left[ \frac{t_w H^2}{8} + t_f (b - t_w) \frac{H - t_f}{2} + 2r^2 \left( \frac{H}{2} - t_f - \frac{r}{2} \right) - 2 \frac{\pi r^2}{4} \left( \frac{H}{2} - t_f + r + \frac{4r}{3\pi} \right) \right]$			
$\rho_{xx}$ =	510,66 mm			
$\rho_{yy}$ =	64,25 mm			
Peso =	341,48 kg/m			
E =	210000 N/mm <sup>2</sup>			
$\nu$ =	0,3			
G =	80769 N/mm <sup>2</sup>			
$\alpha$ =	0,000012 °C <sup>-1</sup>			
f <sub>a</sub> =	355 N/mm <sup>2</sup>			
f <sub>u</sub> =	510 N/mm <sup>2</sup>			
$\gamma_{M0}$ =	1,05 per sezioni classe 1,2,3 e 4			
$\gamma_{M1}$ =	1,05 per instabilità membrature			
$\gamma_{M2}$ =	1,10 per instabilità membrature ponti stradali e ferroviari			
$\gamma_{M3}$ =	1,25 per sezioni tese indebolite dai fori nei riguardi della frattura			
<b>COLLEGAMENTI:</b>				
N <sub>l,inf</sub> =	0 N caso per trave tesa			
N <sub>c,inf</sub> =	0 N caso per trave compressa			
M <sub>inf</sub> =	752430000 Nmm caso per trave inflessa in una direzione			
V <sub>ts</sub> =	397333 N taglio			
Caso per treve a flessione deviata:				
M <sub>ext</sub> =	0 Nmm			
M <sub>ext,inf</sub> =	0 Nmm			
Valori del momento massimo e minimo dell'asta:				
M <sub>max,x,inf</sub> =	0 Nmm			
M <sub>max,z,inf</sub> =	0 Nmm			
<b>CLASSE APPARTENENZA PROFILO (NTC2008, 4.2.3.1)</b>				
<b>ALA SUPERIORE</b>		<b>ALA INFERIORE</b>		<b>ANIMA</b>
C/t <sub>v,up</sub> =	6,60	C/t <sub>v,inf</sub> =	6,60	C/t <sub>w</sub> = 65,00
$\epsilon$ =	0,81	$\epsilon$ =	0,81	$\epsilon$ = 0,81
Curva di instabilità "b":				
Fattore di imperfezione $\alpha$ :		0,34		
<input checked="" type="radio"/> Parte soggetta a flessione <input type="radio"/> Parte soggetta a compressione <input type="radio"/> Parte soggetta a flessione ed a compressione		<b>ANIMA</b> <i>Classe 2</i>		

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**SOLLECITAZIONE DELL'ASTA A TAGLIO (NTC2008, 4.2.4.1.2):**

$$\frac{V_{sd}}{V_{c,Rd}} \leq 1$$

Area taglio resistente:  $A_y = 26500 \text{ mm}^2$

$$V_{c,Rd} = \frac{A_y f_{yk}}{\sqrt{3}\gamma_{M0}} \quad V_{c,Rd} = 5172783 \quad N \quad \Rightarrow \quad 0,08 \quad \text{OK!(<1)}$$

Verifica in presenza di torsione uniforme:

$$\tau_{t,sd} = 0 \text{ N/mm}^2$$

$$V_{c,Rd,red} = \left[ 1 - \frac{\tau_{t,sd}}{f_{yk}} \right] V_{c,Rd}$$

$$V_{c,Rd,red} = 5172783 \quad N \quad \Rightarrow \quad 0,08 \quad \text{OK!(<1)}$$

**SOLLECITAZIONE DELL'ASTA A FLESSIONE MONOASSIALE RETTA (NTC2008, 4.2.4.1.2):**

$$\frac{M_{sd}}{M_{c,Rd}} \leq 1$$

$V_{sd} < 0,5 V_{c,Rd}$  Il taglio non influenza sulla verifica a flessione!

$$\rho = \left[ \frac{2V_{sd}}{V_{c,Rd}} - 1 \right]^2 \quad 0,71635 \quad f_{y,red} = (1 - \rho)f_{yk} \quad 101 \quad \text{N/mm}^2$$

$$f_{yk} = 355 \quad \text{N/mm}^2$$

Calcolo resistenza plastica sezione linda, classe 1 e 2:

$$M_{pl,Rd} = \frac{W_{pl} f_{yk}}{\gamma_{M0}} \quad M_{pl,Rd} = 6776696429 \quad \text{Nmm} \quad \Rightarrow \quad 0,11 \quad \text{OK!(<1)}$$

Calcolo resistenza elastica sezione linda, classe 3:

$$M_{el,Rd} = \frac{W_{el,min} f_{yk}}{\gamma_{M0}} \quad M_{el,Rd} = 5681721781 \quad \text{Nmm} \quad \Rightarrow \quad 0,13 \quad \text{OK!(<1)}$$

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

### 6.2.5 VERIFICA PIOLI

Si riporta nel seguito la verifica del sistema di collegamento fra trave metallica e soletta gettata in opera.

Il sistema di connessione è realizzato mediante pioli di tipo Nelson resistenti a taglio, saldati alla piattabanda superiore della trave principale ed annegati nella soletta in calcestruzzo. Si ritiene che l'intera forza di scorrimento sia assorbita solo dalla piolatura predisposta sulle travi principali di impalcato, trascurando, a favore di sicurezza il contributo della piolatura posta sulla trave di spina. Il dimensionamento dei connettori è stato eseguito in riferimento alle prescrizioni indicate dalla normativa. Scegliendo di inserire i pioli a passo costante lungo tutto lo sviluppo della trave, si è scelto di determinare la forza di scorrimento applicata tramite l'integrazione delle tensioni indotte sulla soletta collaborante considerando le tensioni indotte ai lembi della soletta stessa.

La collaborazione tra la trave metallica e la soletta è assicurata mediante pioli elettrosaldati all'ala della trave di acciaio. Per le verifiche si fa riferimento al punto 4.3.4.3.1.2 delle NTC.

$$P_{rd,a} = 0,8 f_{tk} (\pi d^2 / 4) / \gamma_v \quad [4.3.9]$$

$$P_{rd,c} = 0,29 \alpha d^2 (f_{ck} E_{cm})^{0,5} / \gamma_v \quad [4.3.10]$$

In cui:

$\gamma_v$  è il fattore parziale definito al § 4.3.3;

$f_{tk}$  è la resistenza caratteristica a rottura dell'acciaio del piolo (comunque  $f_{tk} \leq 500$  MPa);

$f_{ck}$  è la resistenza cilindrica caratteristica del calcestruzzo della soletta;

$E_{cm}$  è il valore medio del modulo elastico secante del calcestruzzo della soletta definito al § 11.2.10.3;

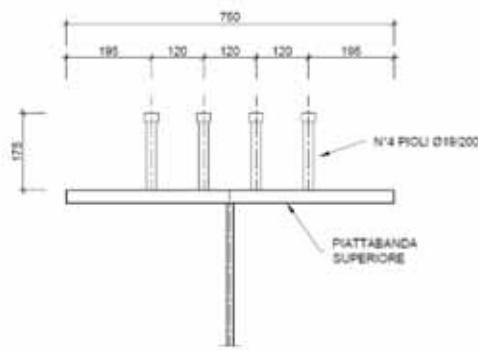
$d$  è il diametro del piolo, compreso tra 16 e 25 mm;

$h_{sc}$  è l'altezza del piolo dopo la saldatura;

$\alpha = 0,2 (h_{sc} / d + 1)$  per  $3 \leq h_{sc} / d \leq 4$ , [4.3.11 a]

$\alpha = 1,0$  per  $h_{sc} / d > 4$ . [4.3.11 b]

Per il caso in esame si ottiene:



<b>ftk =</b>	<b>450</b>	Mpa
<b>fck =</b>	<b>32</b>	Mpa
<b>Ecm =</b>	<b>33346</b>	Mpa
<b><math>\alpha</math> =</b>	<b>1</b>	
<b>d =</b>	<b>19</b>	mm
<b><math>\gamma_v</math> =</b>	<b>1,25</b>	
<b><math>P_{rd,a} =</math></b>	<b>81,656</b>	kN
<b><math>P_{rd,c} =</math></b>	<b>86,515</b>	kN

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Si procedere ora con il calcolo della forza di scorrimento e quindi con il valore di carico massimo determinato per ciascun piolo.

<b>L =</b>	44	m
<b>Bsol =</b>	2840	mm
<b>H tot =</b>	2360	mm
<b>H sol =</b>	260	mm
<b>Y<sub>G,sup,id</sub> =</b>	880,98	mm
<b>σ<sub>c1</sub> =</b>	18,66	Mpa
<b>σ<sub>c2</sub> =</b>	13,15	Mpa
<b>F<sub>V</sub> (l/2) =</b>	533,8624	kN
<b>Δx =</b>	200	mm
<b>N<sub>Pioli</sub> =</b>	4	
<b>F<sub>V_(PIOLO)</sub> =</b>	26,69	kN

F<sub>V</sub>\_Piolo < min (Prd,a ; Prd,c) - Ok Verificato

LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO

## 6.2.6 GIUNTI BULLONATI

Si riporta di seguito la verifica dei giunti bullonati previsti nei traversi di spalla che risultano essere maggiormente sollecitati.

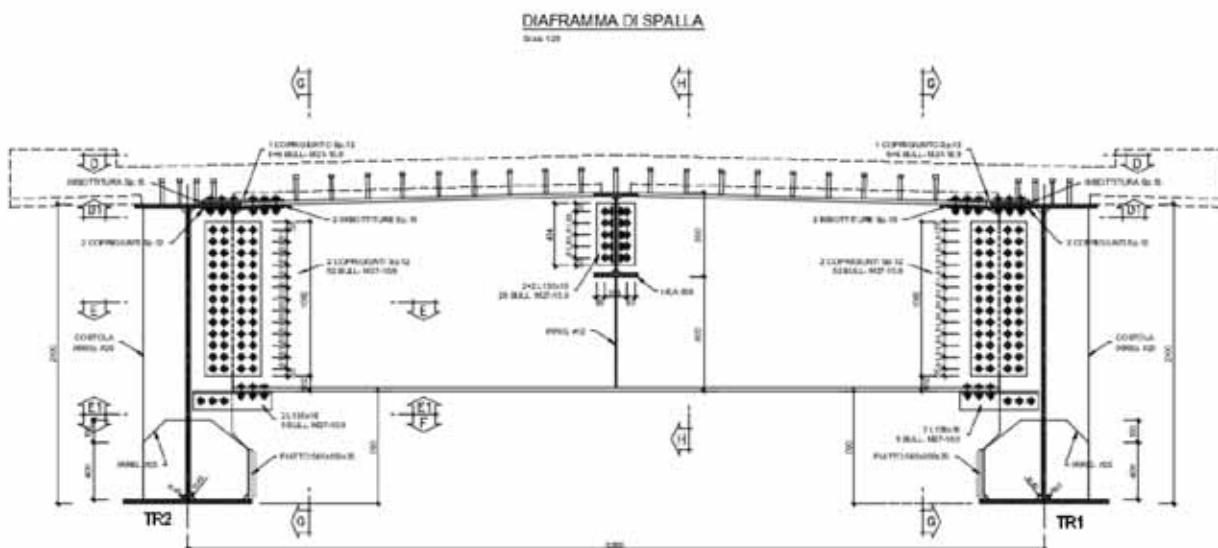


Figura 71 – Traverso di Testata

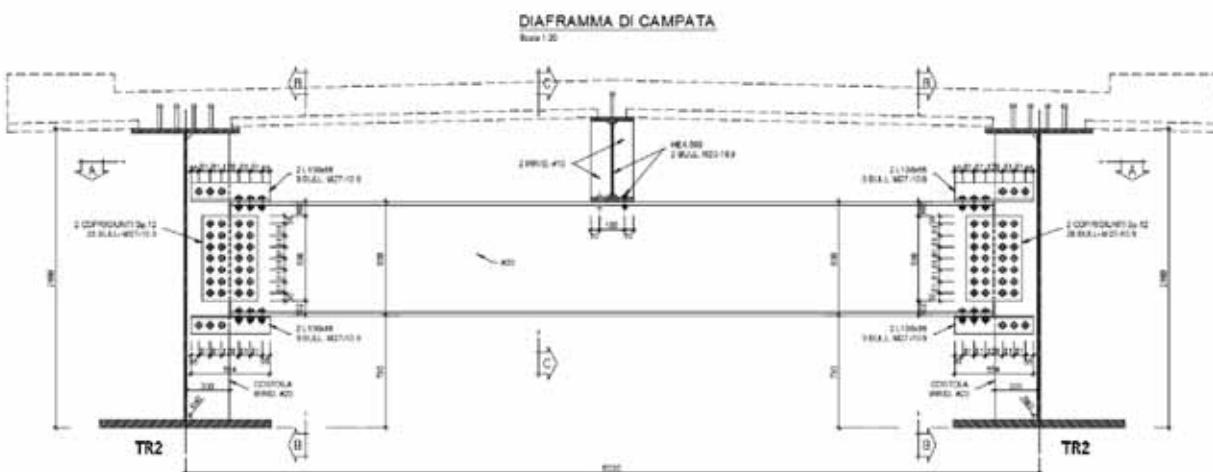


Figura 72 – Traverso di Campata

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**6.2.6.1 VERIFICA GIUNTO TRAVERSO DI CAMPATA**

$\gamma_{M_0} =$	1,05	per sezioni classe 1,2,3 e 4
$\gamma_{M_1} =$	1,05	per instabilità membrature
$\gamma_{M_2} =$	1,10	per instabilità membrature ponti stradali e ferroviari
$\gamma_{M_3} =$	1,25	per sezioni tese indebolite dai fori nei riguardi della frattura

**Resistenza a taglio bulloni 4,6, 5,6 e 8,8**

27	▼	10,9	▼
----	---	------	---

$$F_{V,Rd} = \frac{0,6 \cdot f_{tk} \cdot A_{res}}{\gamma_{M_2}} \quad 220,32 \quad \text{kN}$$

**Resistenza a taglio bulloni 6,8 e 10,9**

$$F_{V,Rd} = \frac{0,5 \cdot f_{tk} \cdot A_{res}}{\gamma_{M_2}} \quad 183,6 \quad \text{kN}$$

**Resistenza a rifollamento**

$$F_{h,Rd} = \frac{k \cdot \alpha \cdot f_{tk} \cdot d \cdot t}{\gamma_{M_2}} \quad 182,87 \quad \text{kN} \quad \text{di bordo}$$

$$F_{h,Rd} = \frac{k \cdot \alpha \cdot f_{tk} \cdot d \cdot t}{\gamma_{M_2}} \quad 182,87 \quad \text{kN} \quad \text{interni}$$

dove:

d è il diametro nominale del gambo del bullone,

t è lo spessore della piastra collegata,

$f_{tk}$  è la resistenza a rottura del materiale della piastra collegata,

$\alpha_{min}$   $\{e_1/(3 \cdot d_0) ; l_0/E_0 ; 1\}$  per bulloni di bordo nella direzione del carico applicato,

$\alpha_{min}$   $\{p_1/(3 \cdot d_0) - 0,25 ; f_{tk}/E_0 ; 1\}$  per bulloni interni nella direzione del carico applicato,

$k_{min}$   $\{2,8 \cdot e_2/d_0 - 1,7 ; 2,5\}$  per bulloni di bordo nella direzione perpendicolare al carico applicato,

$k_{min}$   $\{1,4 \cdot p_2 / d_0 - 1,7 ; 2,5\}$  per bulloni interni nella direzione perpendicolare al carico applicato,

essendo  $e_1$ ,  $e_2$ ,  $p_1$  e  $p_2$  indicati in Fig. 4.2.3 e  $d_0$  il diametro nominale del foro di alloggiamento del bullone.

S355	▼		
d =	27	mm	
$f_{tk} =$	510	mm	
$\alpha =$	0,50	mm	
$\alpha =$	0,50	mm	
k =	1,66	mm	
k =	1,66	mm	
$e_1 =$	36	mm	
$e_2 =$	36	mm	
$p_1 =$	66	mm	
$p_2 =$	72	mm	
$V_{s,d} =$	289,00	kN	taglio assorbito da anima
$M_{s,d} =$	541,60	kNm	
<b>Braccio =</b>	<b>0,800</b>	<b>m</b>	
$F_{s,d} =$	677,00	kN	forza alle flange

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

<b>Resistenza a taglio bulloni (flangia)</b>			
$t_{piastra} =$	20	mm	
$h_{piastra} =$	272	mm	
$n_{piastre} =$	1	piastre	
$n_f =$	1	file	
$n_{pt} =$	2	piani di taglio	
$n_b =$	3	bulloni	
$F_{V,Rd\_anima} = n \cdot n_b \cdot F_{V,Rd}$	1101,6	kN	<b>ok!</b>
<b>Resistenza a taglio delle piastre anima:</b>			
$n_f =$	2	file	
$n_{pt} =$	2	piani di taglio	
$n_b =$	7	bulloni	
$t_{piastra} =$	12	mm	
$h_{piastra} =$	596	mm	
$n_{piastre} =$	2	piastre	
$V_{V,Rd\_piastre} = n_{piastre} \cdot \frac{f_y \cdot A}{\gamma_{M_0} \cdot \sqrt{3}}$	5584	kN	<b>ok!</b>
<b>Resistenza a rifollamento flangia</b>			
$F_{V,Ed} =$	112,83	kN	Sollecitazione taglio bullone
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>
<b>Resistenza a rifollamento anima</b>			
$F_{V,Ed} =$	10,32	kN	Sollecitazione taglio bullone
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**6.2.6.2 VERIFICA GIUNTO TRAVERSO DI TESTATA**

$\gamma_{M0} =$	1,05	per sezioni classe 1,2,3 e 4
$\gamma_{M1} =$	1,05	per instabilità membrature
$\gamma_{M1} =$	1,10	per instabilità membrature ponti stradali e ferroviari
$\gamma_{M2} =$	1,25	per sezioni tese indebolite dai fori nei riguardi della frattura

**Resistenza a taglio bulloni 4,6, 5,6 e 8,8**

27	▼	10,9	▼
----	---	------	---

$$F_{V,Rd} = \frac{0,6 \cdot f_{tb} \cdot A_{res}}{\gamma_{M_2}} \quad 220,32 \quad \text{kN}$$

**Resistenza a taglio bulloni 6,8 e 10,9**

$$F_{V,Rd} = \frac{0,5 \cdot f_{tb} \cdot A_{res}}{\gamma_{M_2}} \quad 183,6 \quad \text{kN}$$

**Resistenza a rifollamento**

$$F_{b,Rd} = \frac{k \cdot \alpha \cdot f_{tk} \cdot d \cdot t}{\gamma_{M_2}} \quad 182,87 \quad \text{kN} \quad \begin{array}{l} \text{di bordo} \\ 182,87 \quad \text{kN} \quad \text{interni} \end{array}$$

dove:

d è il diametro nominale del gambo del bullone,

t è lo spessore della piastra collegata,

$f_{tk}$  è la resistenza a rottura del materiale della piastra collegata,

$\alpha = \min \{e_1/(3 d_0) ; f_{tb}/f_t ; 1\}$  per bulloni di bordo nella direzione del carico applicato,

$\alpha = \min \{p_1/(3 d_0) - 0,25 ; f_{tb}/f_t ; 1\}$  per bulloni interni nella direzione del carico applicato,

$k = \min \{2,8 e_2/d_0 - 1,7 ; 2,5\}$  per bulloni di bordo nella direzione perpendicolare al carico applicato,

$k = \min \{1,4 p_2 / d_0 - 1,7 ; 2,5\}$  per bulloni interni nella direzione perpendicolare al carico applicato,

essendo  $e_1$ ,  $e_2$ ,  $p_1$  e  $p_2$  indicati in Fig. 4.2.3 e  $d_0$  il diametro nominale del foro di alloggiamento del bullone,

S355

d =	27	mm
$f_{tk} =$	510	mm
$\alpha =$	0,50	mm
$\alpha =$	0,50	mm
k =	1,66	mm
k =	1,66	mm
$e_1 =$	36	mm
$e_2 =$	36	mm
$p_1 =$	66	mm
$p_2 =$	72	mm

$V_{s,d} =$	397,33	kN	taglio assorbito da anima
$M_{s,d} =$	752,43	kNm	
<b>Braccio =</b>	<b>1,260</b>	<b>m</b>	
$F_{s,d} =$	597,17	kN	forza alle flange

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

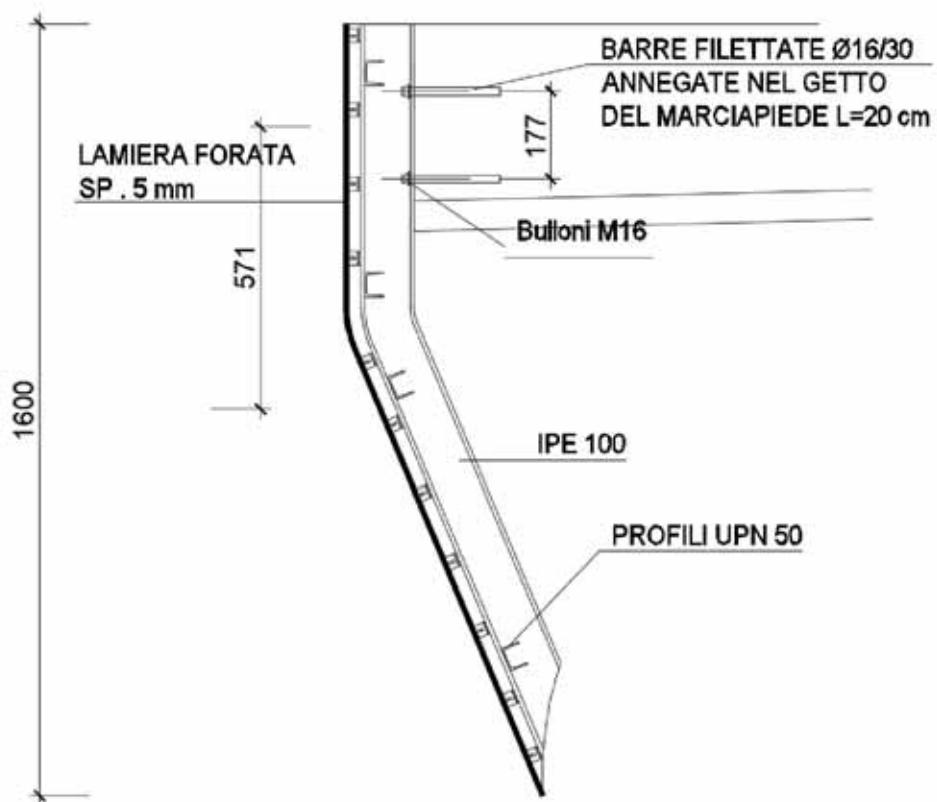
<b>Resistenza a taglio bulloni (flangia)</b>			
$t_{piastra} =$	20	mm	
$h_{piastra} =$	272	mm	
$n_{piastre} =$	1	piastre	
$n_f =$	1	file	
$n_{pt} =$	2	piani di taglio	
$n_b =$	6	bulloni	
$F_{V,Rd\_anima} = n \cdot n_b \cdot F_{V,Rd}$	2203,2	kN	<b>ok!</b>
<b>Resistenza a taglio delle piastre anima:</b>			
$n_f =$	2	file	
$n_{pt} =$	2	piani di taglio	
$n_b =$	13	bulloni	
$t_{piastra} =$	12	mm	
$h_{piastra} =$	1082	mm	
$n_{piastre} =$	2	piastre	
$V_{V,Rd\_piastre} = n_{piastre} \cdot \frac{f_y \cdot A}{\gamma_{M_0} \cdot \sqrt{3}}$	10138	kN	<b>ok!</b>
<b>Resistenza a rifollamento flangia</b>			
$F_{V,Ed} =$	49,76	kN	Sollecitazione taglio bullone
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>
<b>Resistenza a rifollamento anima</b>			
$F_{V,Ed} =$	7,64	kN	Sollecitazione taglio bullone
$F_{b,Rd} =$	182,87	kN	<b>ok!</b>

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**6.2.7 VERIFICA VELETTA METALLICA**

Si riporta di seguito la verifica degli elementi di supporto previsti a sostegno della veletta metallica laterale al cordolo dell'impalcato.

La veletta è composta da elementi di sostegno in acciaio sia trasversali IPE100 che trasversali UPN 50 ed una lamiera forata in alluminio di spessore pari a 5mm.



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

<b>Peso Proprio Veletta</b>	
Peso UPN 50 =	5,59 kg/ml
Peso Totale UPN 50 =	<b>22,36</b> kg/ml
Peso IPE 100 =	8,10 kg/ml
Lunghezza IPE 100 =	1,40 m
Peso Totale IPE 100 =	<b>11,34</b> kg/ml
Lunghezza Lamiera alluminio =	1,65 ml
Spessore Lamiera alluminio =	0,005 m
% Foratura Lamiera alluminio =	60%
Peso Lamiera alluminio =	8,91 kg/mq
<b>Peso Totale =</b>	<b>42,61</b> kg/mq
	<b>0,4261</b> kN/ml

<b>Forza Orizzontale Vento</b>	
ph =	1,88 kN/mq
H =	1,6 m
Fv =	3,008 kN
Braccio Fv =	0,571 m
Frazione =	6,469 kN
Taglio =	0,071 kN
Ft_d =	9,704 kN
T_d =	0,107 kN

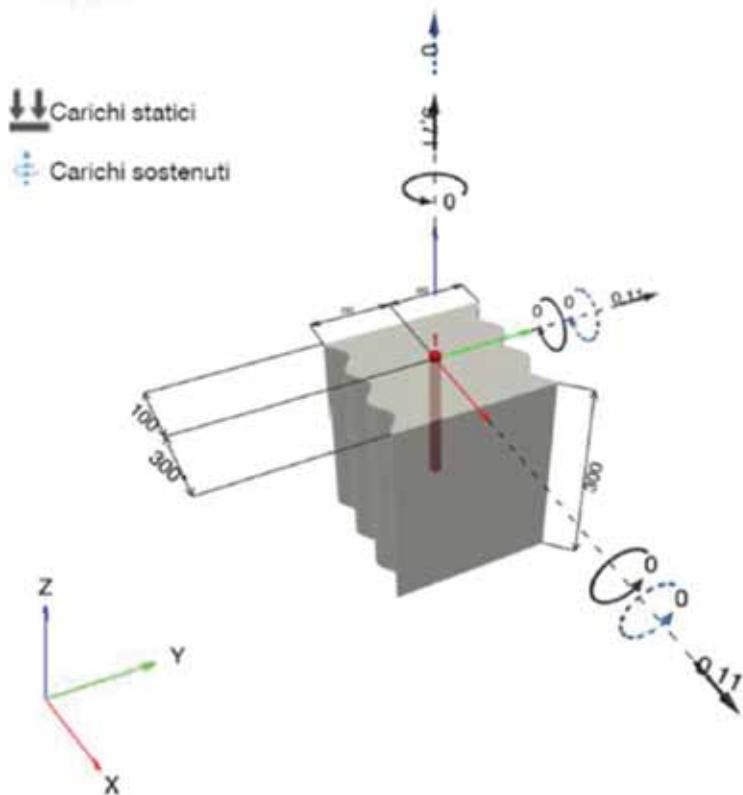
Si riporta di seguito la verifica di aderenza a trazione della barra filettata  $\phi 16$  prevista ammorsata per 20cm all'interno del cordolo laterale.

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**1 Dati da inserire**

Tipo e dimensione dell'ancorante:	HIT-RE 500 V4 + HAS-U 5.8 HDG M16
Periodo di ritorno (durata in anni):	50
Codice articolo:	2223897 HAS-U 5.8 HDG M16x260 (inserire) / 2287552 HIT-RE 500 V4 (composto indurente)
Profondità di posa effettiva:	$h_{ef,act} = 200.0 \text{ mm}$ ( $h_{ef,int} = - \text{ mm}$ )
Materiale:	5.8
Certificazione No.:	ETA 20/0541
Emesso I Valido:	04/09/2021   -
Prova:	metodo di calcolo EN 1992-4, chimica
Fissaggio distanziato:	
Profilo:	
Materiale base:	non fessurato calcestruzzo, C25/30, $f_{ck,cr} = 25.00 \text{ N/mm}^2$ , $h = 300.0 \text{ mm}$ , Temp. Breve/Lungo: 0/0 °C, Coefficiente parziale di sicurezza materiale definito dall'utente $\gamma_c = 1,500$
Installazione:	Foro eseguito con perforatore, Condizioni di installazione: asciutto
Armatura:	nessuna armatura o interasse tra le armature $\geq 150 \text{ mm}$ (qualsunque $\varnothing$ ) o $\geq 100 \text{ mm}$ ( $\varnothing \leq 10 \text{ mm}$ ) senza armatura di bordo longitudinale

**Geometria [mm] & Carichi [kN, kNm]**



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**1.1 Combinazione carichi**

Caso	Descrizione	Forze [kN] / Momenti [kNm]	Sismico	Fuoco	Util. max.	Tassello [%]
1	Combinazione 1	N = 9,710; V <sub>x</sub> = 0,110; V <sub>y</sub> = 0,110; M <sub>x</sub> = 0,000; M <sub>y</sub> = 0,000; M <sub>z</sub> = 0,000; N <sub>sus</sub> = 0,000; M <sub>x,sus</sub> = 0,000; M <sub>y,sus</sub> = 0,000;	no	no	20	

**2 Condizione di carico/Carichi risultanti sull'ancorante**

**Carichi sull'ancorante [kN]**

Trazione: (+ Trazione, - Compressione)

Ancorante	Trazione	Taglio	Taglio in dir. x	Taglio in dir. y
1	9,710	0,158	0,110	0,110

Compressione max. nel calcestruzzo: - [%]

Max. sforzo di compressione nel calcestruzzo: - [N/mm<sup>2</sup>]

risultante delle forze di trazione nel (x/y)=(0,0/0,0): 0,000 [kN]

risultante delle forze di compressione (x/y)=(0,0/0,0): 0,000 [kN]

**3 Carico di trazione (EN 1992-4, sezione 7.2.1)**

	Carico [kN]	Resistenza [kN]	Utilizzo $\beta_N$ [%]	Stato
Rottura dell'acciaio*	9,710	52,333	19	OK
Rottura combinata conica del calcestruzzo e per sfilamento**	9,710	67,984	15	OK
Rottura conica del calcestruzzo**	9,710	55,311	18	OK
Fessurazione**	9,710	49,893	20	OK

\*ancorante più sollecitato \*\*gruppo di ancoranti (ancoranti sollecitati)

**3.1 Rottura dell'acciaio**

$$N_{Ed} \leq N_{Rd,z} = \frac{N_{Rk,z}}{\gamma_{M,z}} \quad \text{EN 1992-4, Tabella 7.1}$$

N <sub>Rk,z</sub> [kN]	$\gamma_{M,z}$	N <sub>Rd,z</sub> [kN]	N <sub>Ed</sub> [kN]
78,500	1,500	52,333	9,710

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**3.2 Rottura combinata conica del calcestruzzo e per sfilamento**

$$N_{Ed} \leq N_{Rd,p} = \frac{N_{Rd,c}}{\gamma_{M,p}}$$

EN 1992-4, Tabella 7.1

$$N_{Rd,p} = N_{Rd,c}^0 \cdot \frac{A_{c,N}}{A_{c,N}^0} \cdot V_{s,N} \cdot V_{r,N} \cdot V_{ec1,N} \cdot V_{ec2,N}$$

EN 1992-4, Eq. (7.13)

$$\begin{aligned} N_{Rd,c}^0 &= V_{s,N} \cdot t_{Rd,c} \cdot \pi \cdot d \cdot h_e \\ V_{s,N} &= 1 \end{aligned}$$

EN 1992-4, Eq. (7.14)  
EN 1992-4, Eq. (7.14a)

$$s_{c,N} = 7,3 \cdot d \cdot \sqrt{V_{s,N} \cdot t_{Rd,c}} \leq 3 \cdot h_e$$

EN 1992-4, Eq. (7.15)

$$V_{s,N} = V_{s,N}^0 \cdot \left( \frac{s}{s_{c,N}} \right)^{0,5} \cdot (V_{s,N}^0 - 1) \geq 1,00$$

EN 1992-4, Eq. (7.17)

$$V_{s,N}^0 = \sqrt{n} \cdot (\sqrt{n} - 1) \cdot \left( \frac{t_{Rd,c}}{t_{Rd,c}} \right)^{1,5} \geq 1,00$$

EN 1992-4, Eq. (7.18)

$$t_{Rd,c} = \frac{k_1}{\pi \cdot d} \cdot \sqrt{h_e \cdot f_{ck}}$$

EN 1992-4, Eq. (7.19)

$$V_{r,N} = 0,7 + 0,3 \cdot \frac{c}{c_{c,N}} \leq 1,00$$

EN 1992-4, Eq. (7.20)

$$V_{ec1,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c1,N}}{s_{c,N}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.21)

$$V_{ec2,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c2,N}}{s_{c,N}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.21)

$A_{c,N} [\text{mm}^2]$	$A_{c,N}^0 [\text{mm}^2]$	$t_{Rd,c,20} [\text{N/mm}^2]$	$s_{c,N} [mm]$	$c_{c,N} [mm]$	$c_{s,N} [mm]$	$f_{ck} [\text{N/mm}^2]$
164.117	231.918	17,00	481,6	240,8	100,0	25,00
$V_{s,N}$	$t_{Rd,c,20} [\text{N/mm}^2]$	$k_1$	$t_{Rd,c} [\text{N/mm}^2]$	$V_{s,N}^0$	$V_{s,N}$	
1,023	17,38	11,000	15,47	1,000	1,000	
$e_{c1,N} [mm]$	$V_{ec1,N}$	$e_{c2,N} [mm]$	$V_{ec2,N}$	$V_{r,N}$	$V_{rs,N}$	
0,0	1,000	0,0	1,000	0,825	1,000	
$V_{s,N}^0$	$a_{s,N}$	$V_{s,N}$				
0,880	0,000	1,000				
$N_{Rd,c}^0 [\text{kN}]$	$N_{Rd,c} [\text{kN}]$	$\gamma_{M,p}$	$N_{Rd,c} [\text{kN}]$	$N_{Ed} [\text{kN}]$		
174,750	101,976	1,500	67,684	9,710		

ID gruppo ancoranti

1

**3.3 Rottura conica del calcestruzzo**

$$N_{Ed} \leq N_{Rd,c} = \frac{N_{Rd,c}}{\gamma_{M,c}}$$

EN 1992-4, Tabella 7.1

$$N_{Rd,c} = N_{Rd,c}^0 \cdot \frac{A_{c,N}}{A_{c,N}^0} \cdot V_{s,N} \cdot V_{r,N} \cdot V_{ec1,N} \cdot V_{ec2,N} \cdot V_{MN}$$

EN 1992-4, Eq. (7.1)

$$N_{Rd,c}^0 = k_1 \cdot \sqrt{f_{ck} \cdot h_e^{1,5}}$$

EN 1992-4, Eq. (7.2)

$$A_{c,N}^0 = s_{c,N} \cdot s_{c,N}$$

EN 1992-4, Eq. (7.3)

$$V_{s,N} = 0,7 + 0,3 \cdot \frac{c}{c_{c,N}} \leq 1,00$$

EN 1992-4, Eq. (7.4)

$$V_{ec1,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c1,N}}{s_{c,N}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.6)

$$V_{ec2,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{c2,N}}{s_{c,N}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.6)

$$V_{MN} = 1$$

EN 1992-4, Eq. (7.7)

$A_{c,N} [\text{mm}^2]$	$A_{c,N}^0 [\text{mm}^2]$	$c_{c,N} [\text{mm}]$	$s_{c,N} [\text{mm}]$	$f_{ck} [\text{N/mm}^2]$		
240.000	360.000	300,0	600,0	25,00		
$e_{c1,N} [\text{mm}]$	$V_{ec1,N}$	$e_{c2,N} [\text{mm}]$	$V_{ec2,N}$	$V_{s,N}$	$V_{r,N}$	
0,0	1,000	0,0	1,000	0,800	1,000	
$z [\text{mm}]$	$V_{MN}$	$k_1$	$N_{Rd,c}^0 [\text{MN}]$	$\gamma_{M,c}$	$N_{Rd,c} [\text{kN}]$	$N_{Ed} [\text{kN}]$
0,0	1,000	11,000	155,563	1,500	55,311	9,710

ID gruppo ancoranti

1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**3.4 Fessurazione**

$$N_{Ed} \leq N_{Rd,sp} = \frac{N_{Rd,sp}}{\gamma_{M0}}$$

EN 1992-4, Tabella 7.1

$$N_{Rd,sp} = N_{Rd,sp}^0 \cdot \frac{A_{c,N}}{A_{c,N}} \cdot \psi_{z,N} \cdot \psi_{rc,N} \cdot \psi_{ect,N} \cdot \psi_{et2,N} \cdot \psi_{h,sp}$$

EN 1992-4, Eq. (7.23)

$$N_{Rd,sp}^0 = \min(N_{Rd,sp}^0, N_{Rd,c}^0)$$

$$A_{c,N}^0 = s_{c,sp} \cdot s_{c,sp}$$

EN 1992-4, Eq. (7.3)

$$\psi_{z,N} = 0,7 + 0,3 \cdot \frac{c}{c_{c,sp}} \leq 1,00$$

EN 1992-4, Eq. (7.4)

$$\psi_{rc,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{N,1}}{s_{c,sp}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.6)

$$\psi_{et2,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{N,2}}{s_{c,sp}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.6)

$$\psi_{h,sp} = \left( \frac{h}{h_{mn}} \right)^{2,1} \leq \max \left\{ 1; \left( \frac{h_{ef} + 1,5 \cdot c_1}{h_{mn}} \right)^{2,1} \right\} \leq 2,00$$

EN 1992-4, Eq. (7.24)

$A_{c,N} [\text{mm}^2]$	$A_{c,N}^0 [\text{mm}^2]$	$c_{c,sp} [\text{mm}]$	$s_{c,sp} [\text{mm}]$	$\psi_{h,sp}$	$f_{c,Cf} [\text{N/mm}^2]$	
304.000	577.600	380,0	760,0	1,173	25,00	
$e_{c1,N} [\text{mm}]$	$\psi_{rc,N}$	$e_{c2,N} [\text{mm}]$	$\psi_{et2,N}$	$\psi_{z,N}$	$\psi_{et,N}$	$k_t$
0,0	1,000	0,0	1,000	0,779	1,000	11,000

$N_{Rd,sp}^0 [\text{kN}]$	$T_{M,sp}$	$N_{Rd,sp} [\text{kN}]$	$N_{Ed} [\text{kN}]$
155,563	1,500	49,893	9,710

ID gruppo ancoranti

1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**4 Carico di taglio (EN 1992-4, sezione 7.2.2)**

	Carico [kN]	Resistenza [kN]	Utilizzo $\beta_v$ [%]	Stato
Rottura dell'acciaio (senza braccio di leva)*	0,156	37,728	1	OK
Rottura dell'acciaio (con braccio di leva)*	N/A	N/A	N/A	N/A
Rottura per pryout**	0,156	110,623	1	OK
Rottura del bordo del calcestruzzo in direzione x-**	0,110	33,825	1	OK

\*ancorante più sollecitato \*\*gruppo di ancoranti (ancoranti specifici)

**4.1 Rottura dell'acciaio (senza braccio di leva)**

$$V_{Ed} \leq V_{Rd,3} = \frac{V_{Rd,3}}{\gamma_{M,3}}$$

EN 1992-4, Tabella 7.2

$$V_{Rd,3} = k_7 \cdot V_{Rd,3}^0$$

EN 1992-4, Eq. (7.35)

$V_{Rd,3}^0$ [kN]	$k_7$	$V_{Rd,3}$ [kN]	$\gamma_{M,3}$	$V_{Rd,3}$ [kN]	$V_{Ed}$ [kN]
47,160	1,000	47,160	1,250	37,728	0,156

**4.2 Rottura per pryout (cono del calcestruzzo)**

$$V_{Ed} \leq V_{Rd,CP} = \frac{V_{Rd,CP}}{\gamma_{M,C,P}}$$

EN 1992-4, Tabella 7.2

$$V_{Rd,CP} = k_8 \cdot \min\{N_{Rd,C}, N_{Rd,P}\}$$

EN 1992-4, Eq. (7.39c)

$$N_{Rd,C} = N_{Rd,C}^0 \cdot \frac{A_{C,N}}{A_{C,N}^0} \cdot \psi_{1,N} \cdot \psi_{n,N} \cdot \psi_{e1,N} \cdot \psi_{e2,N} \cdot \psi_{MN}$$

EN 1992-4, Eq. (7.1)

$$N_{Rd,C}^0 = k_1 \cdot \sqrt{f_{ck}} \cdot h_{ef}^{1,5}$$

EN 1992-4, Eq. (7.2)

$$A_{C,N}^0 = s_{ct,N} \cdot s_{ct,N}$$

EN 1992-4, Eq. (7.3)

$$\psi_{1,N} = 0,7 + 0,3 \cdot \frac{c}{c_{ct,N}} \leq 1,00$$

EN 1992-4, Eq. (7.4)

$$\psi_{e1,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{v,1}}{s_{ct,N}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.6)

$$\psi_{e2,N} = \frac{1}{1 + \left( \frac{2 \cdot e_{v,2}}{s_{ct,N}} \right)} \leq 1,00$$

EN 1992-4, Eq. (7.6)

$$\psi_{MN} = 1$$

EN 1992-4, Eq. (7.7)

$A_{C,N}$ [mm <sup>2</sup> ]	$A_{C,N}^0$ [mm <sup>2</sup> ]	$c_{ct,N}$ [mm]	$s_{ct,N}$ [mm]	$k_8$	$f_{ck}$ [N/mm <sup>2</sup> ]
240.000	300.000	300,0	600,0	2,000	25,00
$e_{v,1}$ [mm]	$\psi_{e1,N}$	$e_{v,2}$ [mm]	$\psi_{e2,N}$	$\psi_{MN}$	$\psi_{MN}$
0,0	1,000	0,0	1,000	0,800	1,000
$k_1$	$N_{Rd,C}^0$ [kN]	$\gamma_{M,C,P}$	$V_{Rd,CP}$ [kN]	$V_{Ed}$ [kN]	
11,000	155,563	1,500	110,623	0,156	

ID gruppo ancoranti

1

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**4.3 Rottura del bordo del calcestruzzo in direzione x-**

$$V_{Ed} \leq V_{Rd,c} = \frac{V_{Rd,c}}{\gamma_{Mz}}$$

EN 1992-4, Tabella 7.2

$$V_{Rd,c} = k_T \cdot V_{Rd,c}^0 \cdot \frac{A_{cv}}{A_{cv}^0} \cdot \psi_{z,V} \cdot \psi_{h,V} \cdot \psi_{c,V} \cdot \psi_{ec,V} \cdot \psi_{m,V}$$

EN 1992-4, Eq. (7.40)

$$V_{Rd,c}^0 = k_3 \cdot d_{nom}^x \cdot \frac{\rho}{c_1} \cdot \sqrt{f_{ck}} \cdot c_1^{1.5}$$

EN 1992-4, Eq. (7.41)

$$\alpha = 0.1 \cdot \left( \frac{h}{c_1} \right)^{0.5}$$

EN 1992-4, Eq. (7.42)

$$\beta = 0.1 \cdot \left( \frac{d_{nom}}{c_1} \right)^{0.5}$$

EN 1992-4, Eq. (7.43)

$$A_{cv}^0 = 4.5 \cdot c_1^2$$

EN 1992-4, Eq. (7.44)

$$\psi_{z,V} = 0.7 + 0.3 \cdot \frac{c_2}{1.5 \cdot c_1} \leq 1.00$$

EN 1992-4, Eq. (7.45)

$$\psi_{h,V} = \left( \frac{1.5 \cdot c_1}{h} \right)^{0.5} \geq 1.00$$

EN 1992-4, Eq. (7.46)

$$\psi_{c,V} = \frac{1}{1 + \left( \frac{2 \cdot e_V}{3 \cdot c_1} \right)} \leq 1.00$$

EN 1992-4, Eq. (7.47)

$$\psi_{ec,V} = \sqrt{\frac{1}{(\cos \alpha_V)^2 + (0.5 \cdot \sin \alpha_V)^2}} \geq 1.00$$

EN 1992-4, Eq. (7.48)

$l$ [mm]	$d_{nom}$ [mm]	$k_T$	$\alpha$	$\beta$	$f_{ck,p}$ [N/mm $^2$ ]
192.0	16.00	2.400	0.139	0.060	25.00
$c_1$ [mm]	$A_{cv}$ [mm $^2$ ]	$A_{cv}^0$ [mm $^2$ ]			
100.0	45.000	45.000			
$\psi_{z,V}$	$\psi_{h,V}$	$\psi_{c,V}$	$e_V$ [mm]	$\psi_{ec,V}$	$\psi_{m,V}$
1.000	1.000	2.000	0.0	1.000	1.000
$V_{Rd,c}^0$ [kN]	$k_T$	$\gamma_{Mz}$	$V_{Rd,c}$ [kN]	$V_{Ed}$ [kN]	
25,369	1,0	1.500	33.825	0.110	

**5 Carichi combinati di trazione e di taglio (EN 1992-4, sezione 7.2.3)**

Rottura dell'acciaio

$\beta_N$	$\beta_V$	$\alpha$	Utilizzo $\beta_{NV}$ [%]	Stato
0,186	0,004	2,000	4	OK

$$\beta_N^0 + \beta_V^0 \leq 1,0$$

Rottura del calcestruzzo

$\beta_N$	$\beta_V$	$\alpha$	Utilizzo $\beta_{NV}$ [%]	Stato
0,195	0,003	1,500	9	OK

$$\beta_N^0 + \beta_V^0 \leq 1,0$$

**6 Spostamenti (ancorante più sollecitato)**

Carichi a breve termine:

$$N_{sk} = 7.193 \text{ [kN]} \quad \delta_N = 0.0429 \text{ [mm]}$$

$$V_{sk} = 0.115 \text{ [kN]} \quad \delta_V = 0.0046 \text{ [mm]}$$

$$V_{nv} = 0.0432 \text{ [mm]}$$

Carichi a lungo termine:

$$N_{sk} = 7.193 \text{ [kN]} \quad \delta_N = 0.0930 \text{ [mm]}$$

$$V_{sk} = 0.115 \text{ [kN]} \quad \delta_V = 0.0069 \text{ [mm]}$$

$$V_{nv} = 0.0033 \text{ [mm]}$$

## 7 VERIFICA SOLETTA

Si riportano di seguito le verifiche effettuate per il dimensionamento delle solette di impalcato.

### 7.1 VERIFICA SOLETTA FASE TRANSITORIA

In questa fase di calcolo viene verificata la lastra di predalles con relativi tralicci al fine di sostenere i carichi della soletta, del peso degli operatori e di eventuali barriere di protezione laterali provvisorie per la sicurezza degli operatori.

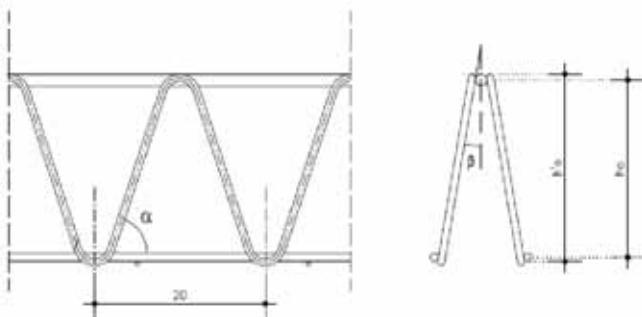
Si riportano di seguito i valori dei carichi utilizzati ed il calcolo delle sollecitazioni in campata ed in appoggio al fine di dimensionare il traliccio da prevedere nel predalles.

#### 7.1.1 ANALISI DEI CARICHI

Si considera il peso proprio della lastra di predalles, il peso del getto della soletta, il peso di un parapetto di protezione e di un mezzo operativo per le varie lavorazioni.

		Sp [m]	$\gamma$ [kN/m³]		
Gk1	Peso Proprio	0.05	25	1.25	kN/m²
Gk2	Peso Soletta	0.21	25	5.25	kN/m²
Gk2	Peso Parapetto Sic.			0.4	kN/m
Qk	Peso Mezzo Operativo			1	kN/m²

Le verifiche vengono eseguite facendo riferimento allo Stato Limite Ultimo della sezione. Si verifica che le tensioni sugli elementi metallici che costituiscono il traliccio siano inferiori alla tensione di calcolo e che la stabilità degli elementi compressi risulti soddisfatta. Di seguito si riporta una rappresentazione schematica del traliccio utilizzato.



Altezza totale del traliccio:  $h'_0 = 165 \text{ mm}$   
 Diametro correnti superiori:  $f_{cs} = 16 \text{ mm}$   
 Diametro correnti inferiori:  $f_{ci} = 14 \text{ mm}$   
 Diametro staffa:  $f_{ci} = 10 \text{ mm}$

#### 7.1.2 VERIFICA A MOMENTO POSITIVO - CAMPATA

Analizzando gli elaborati grafici allegati al presente progetto esecutivo, si evince che la distanza tra le anime delle travi di appoggio è pari a 2.55m e considerando 5 cm di appoggio su entrambe le strutture,

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

si considera una luce di calcolo pari a 2.65m. Si riporta di seguito il calcolo delle sollecitazioni di progetto.

SEZIONE IN CAMPATA				
	LC	2.65		
	MOMENTO FLETTENTE	TAGLIO	Med	Ted
<b>Gk1</b>	$1.25 \times 1.20 \times 2.65^2 / 8$	$1.25 \times 1.20 \times 2.65/2$	1.32	1.99
<b>Gk2</b>	$5.25 \times 1.20 \times 2.65^2 / 8$	$5.25 \times 1.20 \times 2.65/2$	5.53	8.35
<b>Qk</b>	$1 \times 1.20 \times 2.65^2 / 8$	$1 \times 1.20 \times 2.65/2$	1.05	1.59
	Med		<b>13.85</b>	<b>15.50</b>
	Ted			
	<b>Ned = (Med / h₀) / 3</b>		<b>30.77</b>	

Altezza totale traliccio	ht=	16.50	cm
Larghezza totale traliccio	B=	9.00	cm
Interasse tralicci	i=	40.00	cm
Numero di tralicci /m	n=	3	
Passo traliccio	p=	20.00	cm
Quota corrente inferiore	s=	2.00	cm
Corrente superiore	1 Øs=	1.60	cm
Ferro integr sup (I)	0 Øs'=	0.00	cm
Ferro integr sup (II)	0 Øs"=	0.00	cm
Corrente inferiore	2 Øi=	1.40	cm
Diagonale	Ød=	1.00	cm
Altezza utile traliccio	d=	13.0	cm
Interasse ferri inferiori	pi=	7.6	cm
Interasse trasv. diagonale	bd=	5.2	cm
Inclinazione longitudinale	α=	1.065	rad
Inclinazione trasversale	β=	0.192	rad
Area corrente superiore	As=	2.01	cm <sup>2</sup>
Area ferro integr sup (I)	As'=	0.00	cm <sup>2</sup>
Area ferro integr sup (II)	As"=	0.00	cm <sup>2</sup>
Area corrente inferiore	Ai=	3.08	cm <sup>2</sup>
Area diagonale	Ad=	0.79	cm <sup>2</sup>
Inerzia corrente superiore	Js=	0.3217	cm <sup>4</sup>
Inerzia ferro integr sup (I)	Js'=	0.0000	cm <sup>4</sup>
Inerzia ferro integr sup (II)	Js"=	0.0000	cm <sup>4</sup>
Inerzia corrente inferiore	Ji=	0.3771	cm <sup>4</sup>
Inerzia diagonale	Jd=	0.0491	cm <sup>4</sup>

61.0	°
11.0	°

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**CARATTERISTICHE DI RESISTENZA TRALICCIO**

	Area cm <sup>2</sup>	Quota cm	J cm <sup>4</sup>	Jtrasp cm <sup>4</sup>	J+Jtrasp cm <sup>4</sup>
Corr sup + ferro (I)	2.01	15.7	0.322	124.3	124.7
Ferro (II)	0.00	14.9	0.000	0.0	0.0
Corrente inf	3.08	2.7	0.377	81.2	81.6

Quota baricentro	YG=	7.84	cm
Area totale	Atot=	5.1	cm <sup>2</sup>
Inerzia totale	Jtot=	206.3	cm <sup>4</sup>
Modulo di resistenza c. sup.	Ws=	26.23	cm <sup>3</sup>
Modulo di resistenza ferro (II)	Ws'=	29.2	cm <sup>3</sup>
Modulo di resistenza c. inf.	Wi=	40.2	cm <sup>3</sup>

**CARATTERISTICHE DI RESISTENZA ALL'INSTABILITÀ TRALICCIO**

**Corrente superiore**

Lunghezza asta	L=	20.0	cm
Coefficiente riduttivo	$\beta$ =	0.7	
Lunghezza libera inflessione	L0=	14.0	cm
Carico critico Euleriano	Ncr=	340183	N
Snellezza adimensionale	$\lambda$ =	0.52	
Fattore di imperfezione	$\alpha$ =	0.49	
Coefficiente	$\Phi$ =	0.71	
Fattore di riduzione ( $\leq 1$ )	$\chi$ =	0.83	
Resistenza asta compressione	NbRd=	-65.6	kN

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Corrente inferiore**

Lunghezza asta	L=	20.0	cm
Coefficiente riduttivo	$\beta$ =	0.7	
Lunghezza libera inflessione	L <sub>0</sub> =	14.0	cm
Carico critico Euleriano	N <sub>cr</sub> =	199409	N
Snellezza adimensionale	$\lambda$ =	0.59	
Fattore di imperfezione	$\alpha$ =	0.49	
Coefficiente	$\Phi$ =	0.77	
Fattore di riduzione ( $\leq 1$ )	$\chi$ =	0.79	
Resistenza asta compressione	N <sub>bRd</sub> =	-47.7	kN

**Diagonale**

Lunghezza asta	L=	16.6	
Coefficiente riduttivo	$\beta$ =	0.7	
Lunghezza libera inflessione	L <sub>0</sub> =	11.6	cm
Carico critico Euleriano	N <sub>cr</sub> =	75294	N
Snellezza adimensionale	$\lambda$ =	0.69	
Fattore di imperfezione	$\alpha$ =	0.49	
Coefficiente	$\Phi$ =	0.85	
Fattore di riduzione ( $\leq 1$ )	$\chi$ =	0.73	
Resistenza asta compressione	N <sub>bRd</sub> =	-22.6	kN

**VERIFICA TRALICCIO**

Momento sollecitante	M <sub>sd</sub> =	13.85	kNm	campata
Taglio sollecitante	V <sub>sd</sub> =	15.5	kN	appoggio
Momento soll. /traliccio	m <sub>sd</sub> =	4.62	kNm	campata
Taglio sollec. /traliccio	v <sub>sd</sub> =	5.17	kN	appoggio

**Corrente superiore compresso**

**Verifica di resistenza**

Tensione limite di calcolo	f <sub>yD</sub> =	-391	N/mm <sup>2</sup>
Tensione corrente superiore	$\sigma_s$ =	-176	N/mm <sup>2</sup>
Coefficiente di sicurezza	c.s.=	2.22	VERIFICATO

**Verifica di instabilità**

Resistenza asta compressa	N <sub>bRd</sub> =	-65.6	kN
Compressione	N <sub>sd</sub> =	-35.4	kN
Coefficiente di sicurezza	c.s.=	1.85	VERIFICATO

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**Corrente inferiore teso**

**Verifica di resistenza**

Tensione limite di calcolo	$f_yd =$	391	N/mm <sup>2</sup>
Tensione corrente inferiore	$\sigma_i =$	115	N/mm <sup>2</sup>
Coefficiente di sicurezza	c.s.=	3.40	VERIFICATO

**Diagonale**

All'appoggio, per effetto del taglio si ha:

Compressione sul diagonale -3.0 kN

**Verifica di resistenza**

Tensione limite	$f_yd =$	-391.3	N/mm <sup>2</sup>
Tensione diagonale	$\sigma =$	-38.3	N/mm <sup>2</sup>
Coefficiente di sicurezza	c.s.=	10.21	VERIFICATO

**Verifica di instabilità**

Resistenza asta compressa	$N_b R_d =$	-22.6	kN
Compressione	$N_s d =$	-3.0	kN
Coefficiente di sicurezza	c.s.=	7.50	VERIFICATO

Si esegue ora la verifica della struttura allo SLU.

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Verifica C.A. S.L.U. - File: TRALICCIO M+

File Materiali Opzioni Visualizza Progetto Sez. Rett. Sismica Normativa: NTC 2008 ?

**Titolo :** [ ]

N° figure elementari [2] Zoom N° strati barre [2] Zoom

N°	b [cm]	h [cm]
1	0.1	14
2	120	5

N°	A <sub>s</sub> [cm <sup>2</sup> ]	d [cm]
1	6.03	0.8
2	9.24	16.5

**Tipo Sezione**  
 Rettangolare  Trapezio  
 a T  Circolare  
 Rettangoli  Coord.

**Sollecitazioni**  
 S.L.U. → Metodo n  
 N<sub>Ed</sub> [0] kN M<sub>xEd</sub> [0] kNm M<sub>yEd</sub> [0] kNm

Punto applicazione N  
 Centro  Baricentro cls  
 Coord.[cm] xN [0] yN [0]

**Lato calcestruzzo - Acciaio elastico**

**Materiali**

B450C	C30/37
ε <sub>su</sub> 67.5 %	ε <sub>c2</sub> 2 %
f <sub>yd</sub> 391.3 N/mm <sup>2</sup>	ε <sub>cu</sub> 3.5
E <sub>s</sub> 200.000 N/mm <sup>2</sup>	f <sub>cd</sub> 17
E <sub>s</sub> /E <sub>c</sub> 15	f <sub>cc</sub> /f <sub>cd</sub> 0.8
ε <sub>syd</sub> 1.957 %	σ <sub>c,adm</sub> 11.5
σ <sub>s,adm</sub> 255 N/mm <sup>2</sup>	T <sub>00</sub> 0.6933
	T <sub>01</sub> 2.029

M<sub>xRd</sub> 37.24 kNm

**Metodo di calcolo**  
 S.L.U.+  S.L.U.-  
 Metodo n

**Tipo flessione**  
 Retta  Deviata

N° rett. 100 Calcola MRd Dominio M-N

L<sub>0</sub> 0 cm Col. modello

Precompresso

ε<sub>c</sub> -17 N/mm<sup>2</sup>  
 ε<sub>s</sub> 257.2 N/mm<sup>2</sup>  
 ε<sub>c</sub> 3.5 %  
 ε<sub>s</sub> 1.286 %  
 d 16.5 cm  
 x 12.07 x/d 0.7313  
 ε 1

Med < Mrd Verifica soddisfatta

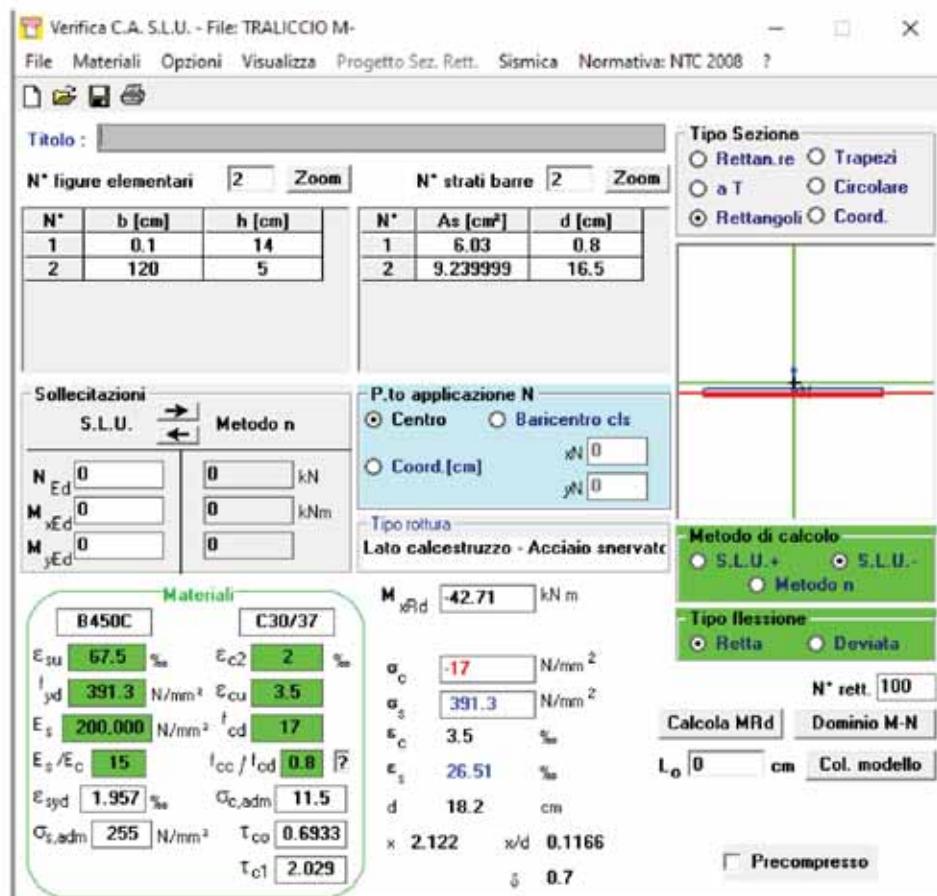
**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**7.1.3 VERIFICA A MOMENTO NEGATIVO APPOGGIO**

Analizzando gli elaborati grafici allegati al presente progetto esecutivo, si evince che la distanza dello sbalzo rispetto all'asse dell'anima della trave di appoggio è pari a 1.25m. Si riporta di seguito il calcolo delle sollecitazioni e la verifica strutturale allo SLU.

SEZIONE IN APPOGGIO		
	LC	1.25
<b>MOMENTO FLETTENTE</b>		
<b>Gk1</b>	$1.25 \times 1.20 \times 1.25^2 / 2$	1.17
<b>Gk2</b>	$5.25 \times 1.20 \times 1.25^2 / 2$	4.92
<b>Gk2</b>	$0.4 \times 1.20 \times 1.25$	0.60
<b>Qk</b>	$1 \times 1.20 \times 1.25^2 / 2$	0.94
	<b>Med</b>	<b>11.27</b>

Si esegue ora la verifica della struttura allo SLU.



Med < Mrd Verifica soddisfatta

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

## 7.2 VERIFICA SOLETTA FASE DEFINITIVA

In questa fase di calcolo viene effettuato il calcolo della soletta considerando i seguenti carichi permanenti compatibilmente a quanto utilizzato per il dimensionamento dell'impalcato metallico.

### PESO SOLETTA

Hsoletta =	<b>0.26</b>	m
$\gamma$ =	25	kN/mc
peso =	6.5	kN/mq

### CORDOLI

Hcordoli =	<b>0.15</b>	m
$\gamma$ =	25	kN/mc
peso =	3.75	kN/mq

### PERMANENTE PORTATO

Hsoletta =	<b>0.11</b>	cm
$\gamma$ =	24	kN/mc
peso =	<b>2.64</b>	kN/mq

### BARRIERE+VELETTE E TUBAZIONI

Barriere =	1.5	kN/ml
Tubazioni =	0.1	kN/ml
Veletta =	0.4	kN/ml
<b>Totale</b>	<b>2.000</b>	kN/ml

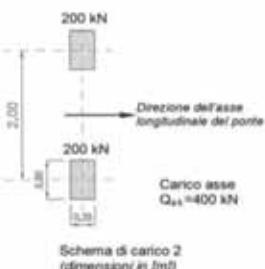
I carichi accidentali considerati sono quelli prescritti dalla normativa ed in particolare lo schema di carico 1, costituito da carichi concentrati su due assi in tandem, applicati su impronte di pneumatico di forma quadrata e lato pari a 0,40 m (Qik), oltre che da un carico distribuito a metro quadrato (qik) applicato alla corsia di carico.

I valori adottati per i carichi variano in base alla corsia di carico, secondo lo schema di normativa riportato nel seguente.



Oltre a questo, è stato adottato anche lo schema di carico 2, costituito da un singolo asse applicato su specifiche impronte di pneumatico di forma rettangolare, con larghezza 0,60 m ed altezza 0,35 m. Nel seguente l'immagine estratta dalla normativa.

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



In aggiunta a quanto sopra, occorre tenere in conto anche l'urto di veicoli in svio ( $q_s$ ). La normativa, a tal proposito, indica che le forze causate da collisioni accidentali sugli elementi di sicurezza, possono essere considerate attraverso una forza orizzontale equivalente di collisione pari a 100 kN. Tale forza è da intendersi applicata trasversalmente ed orizzontalmente 100 mm sotto la sommità dell'elemento su cui insiste, oppure, 1,0 m sopra il piano di marcia, a seconda di quale sia il minore per una striscia di 0.5m. Ai fini dell'applicazione di tale forza alla soletta di impalcato, è stata considerata una diffusione a terra ottenendo una larghezza di carico pari a  $1.00 + 0.50 + 1.00$  ottenendo un effetto torcente sul bordo della soletta pari a  $M_t = 100 \text{ kN}/2.5\text{m} = 40 \text{ kNm/m}$ . I carichi concentrati previsti nei casi di carico, sono stati applicati tenendo in considerazione dell'effettiva geometria della mesh di calcolo utilizzata e considerando (come previsto dalla norma) una diffusione di carico a 45° fino al baricentro della soletta in cls.

Si riportano di seguito alcune immagini relative al modello e ai carichi applicati.

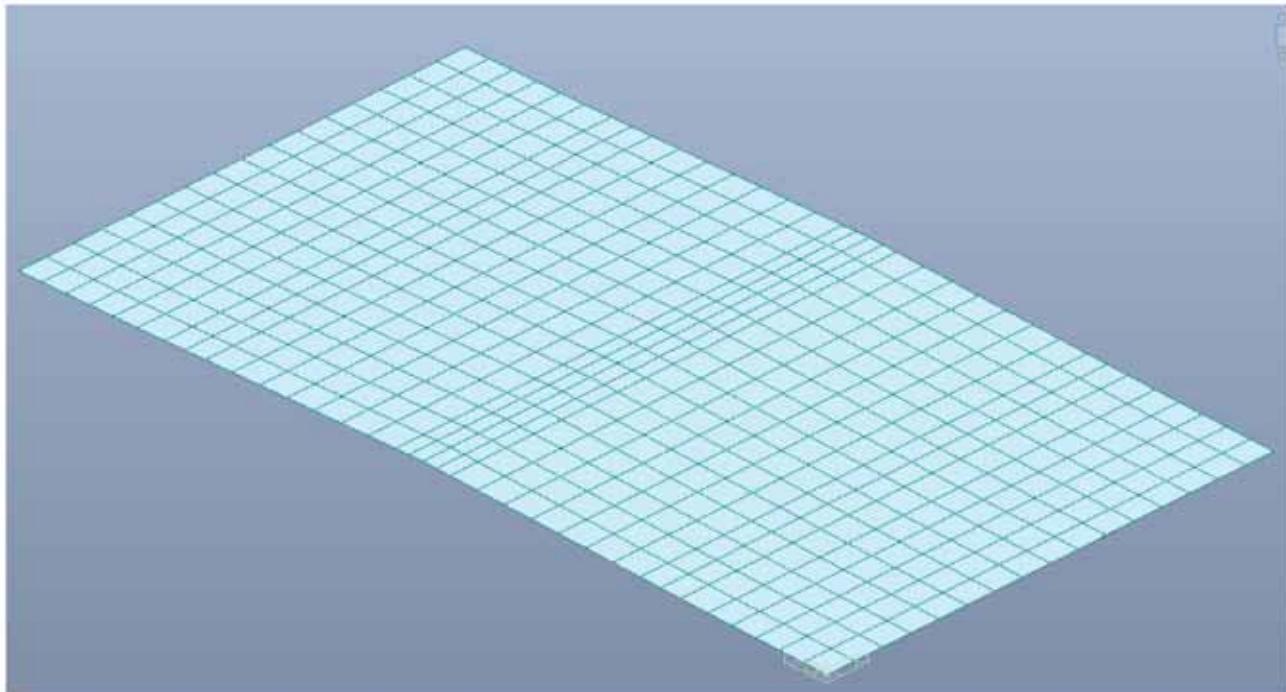


Figura 73 - Modello di calcolo

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

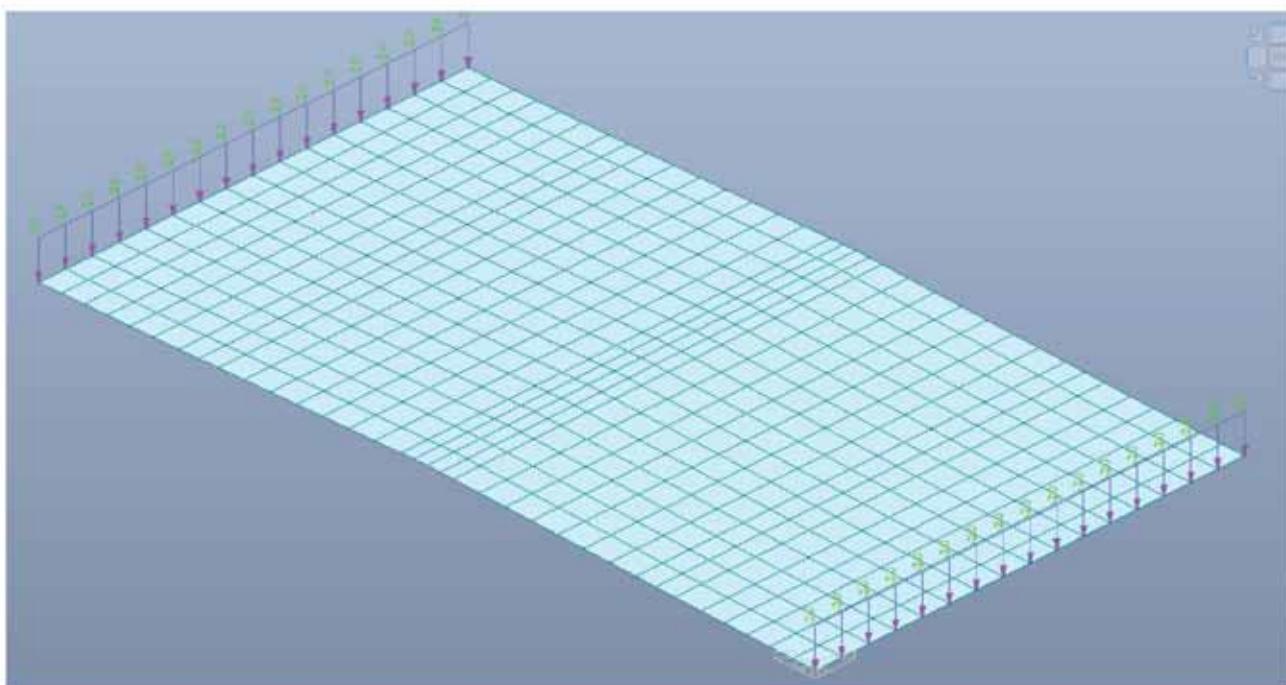


Figura 74 – Peso Barriera + Veletta + Tubazioni

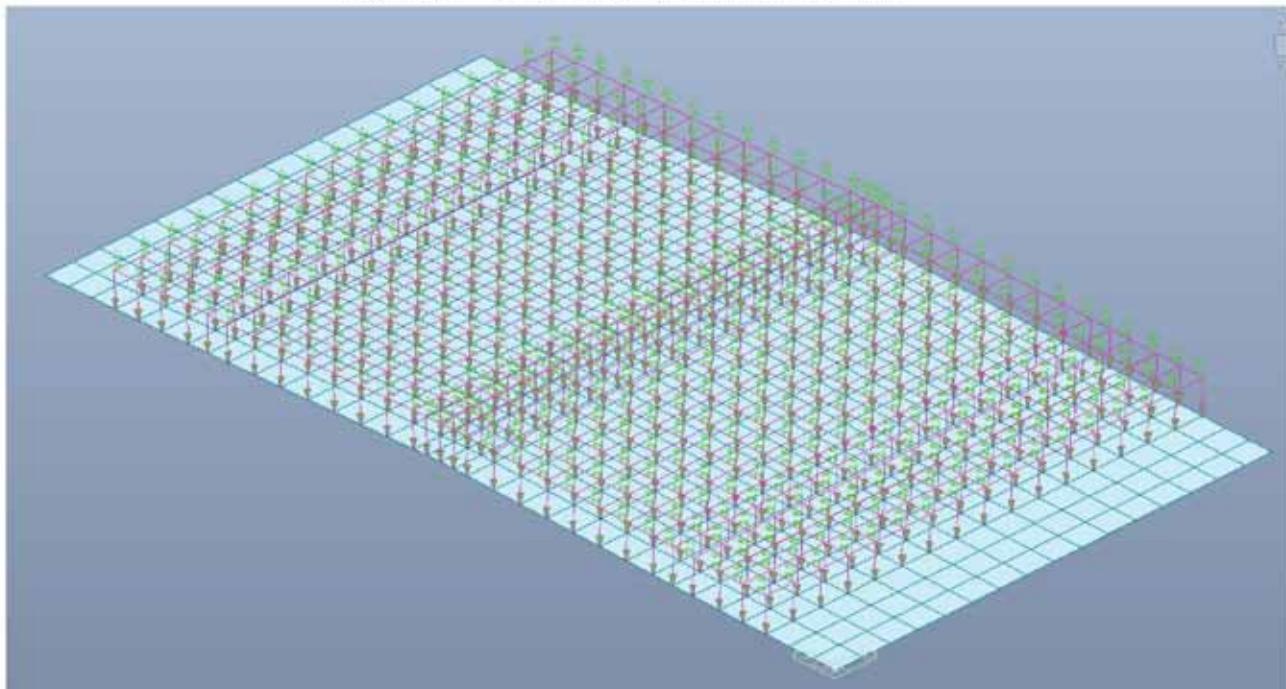
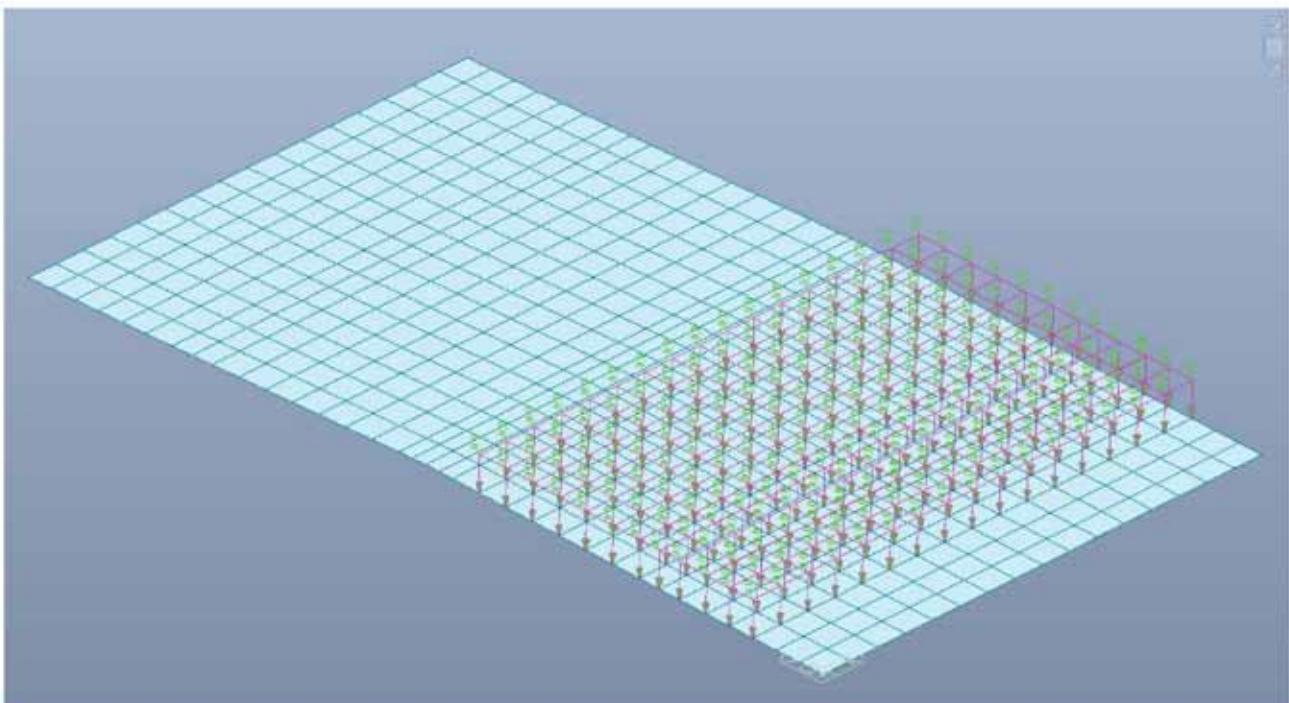
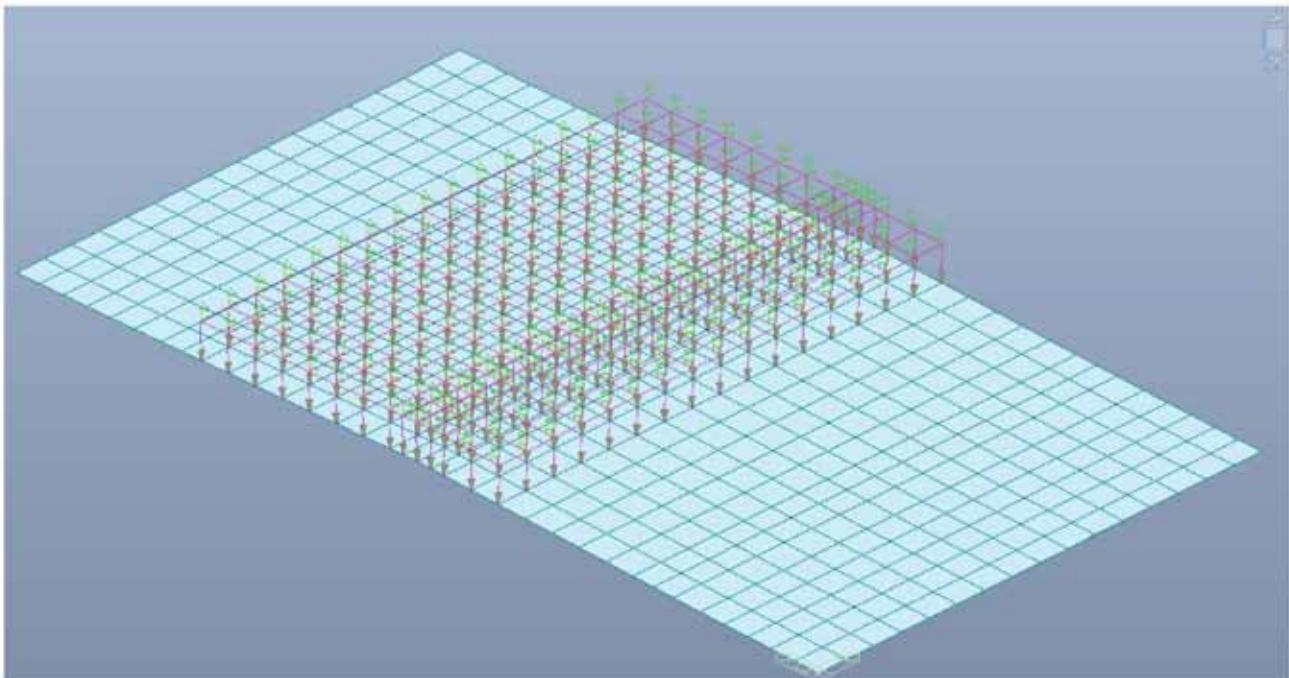


Figura 75 - Peso permanente portato

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

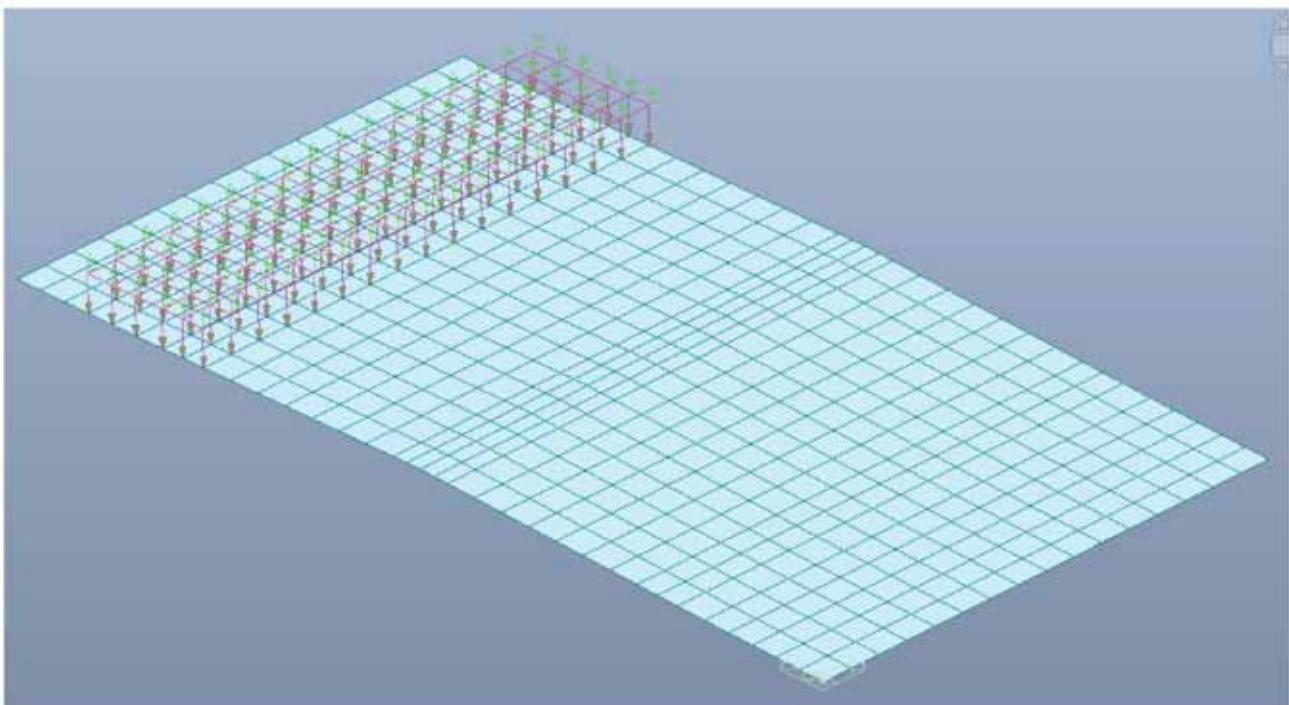


*Figura 76 - Carico Distribuito prima colonna di carico*

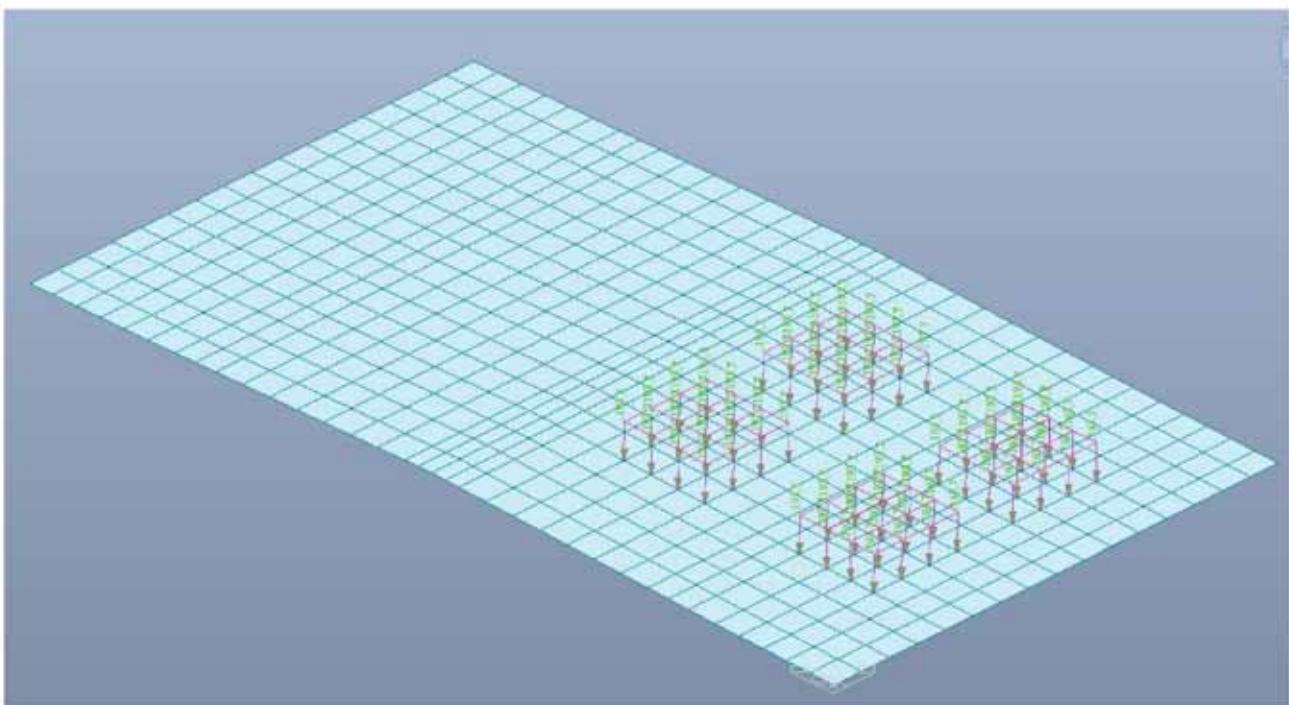


*Figura 77 - Carico Distribuito seconda colonna di carico*

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 – RELAZIONE DI CALCOLO IMPALCATO**

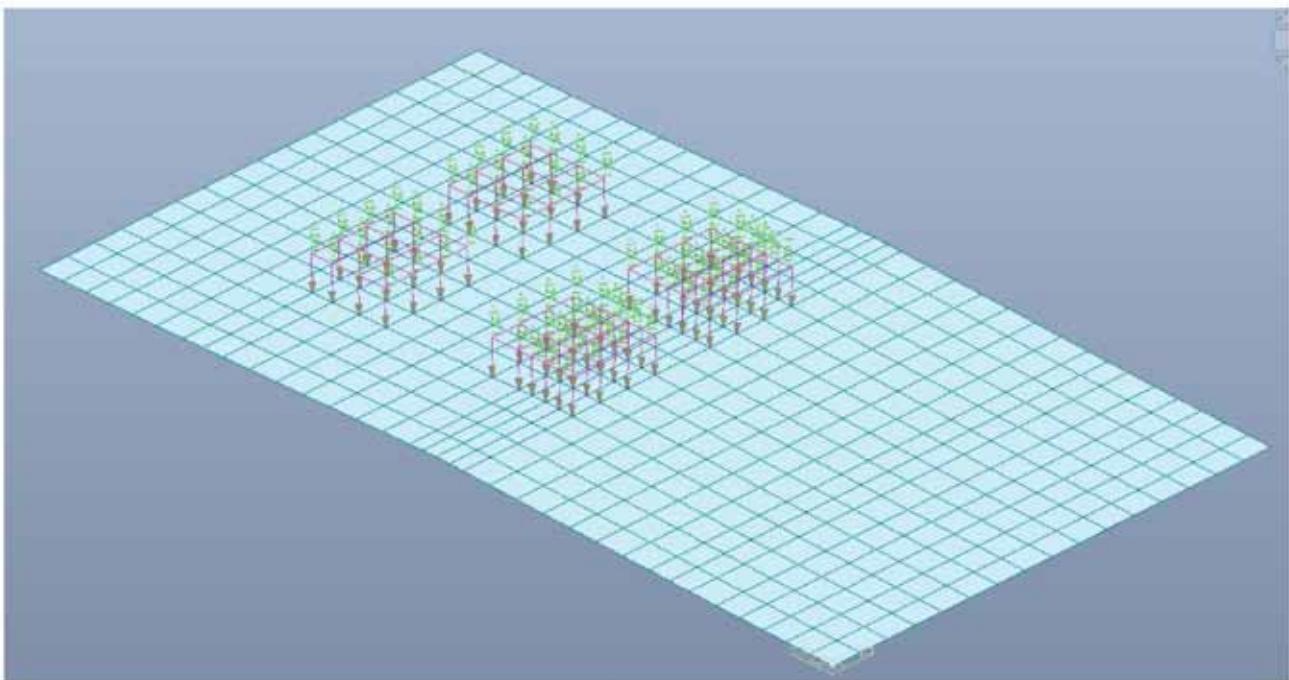


*Figura 78 - Carico Distribuito terza colonna di carico*

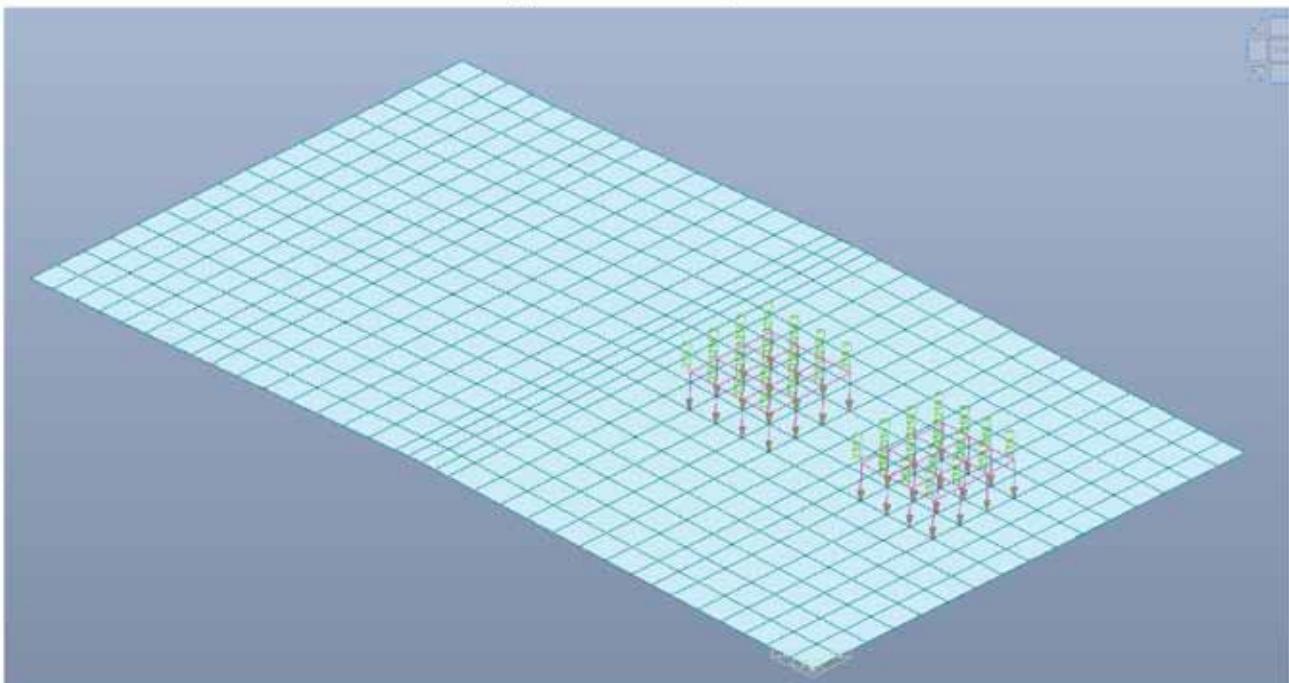


*Figura 79 – Caico Q1a*

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

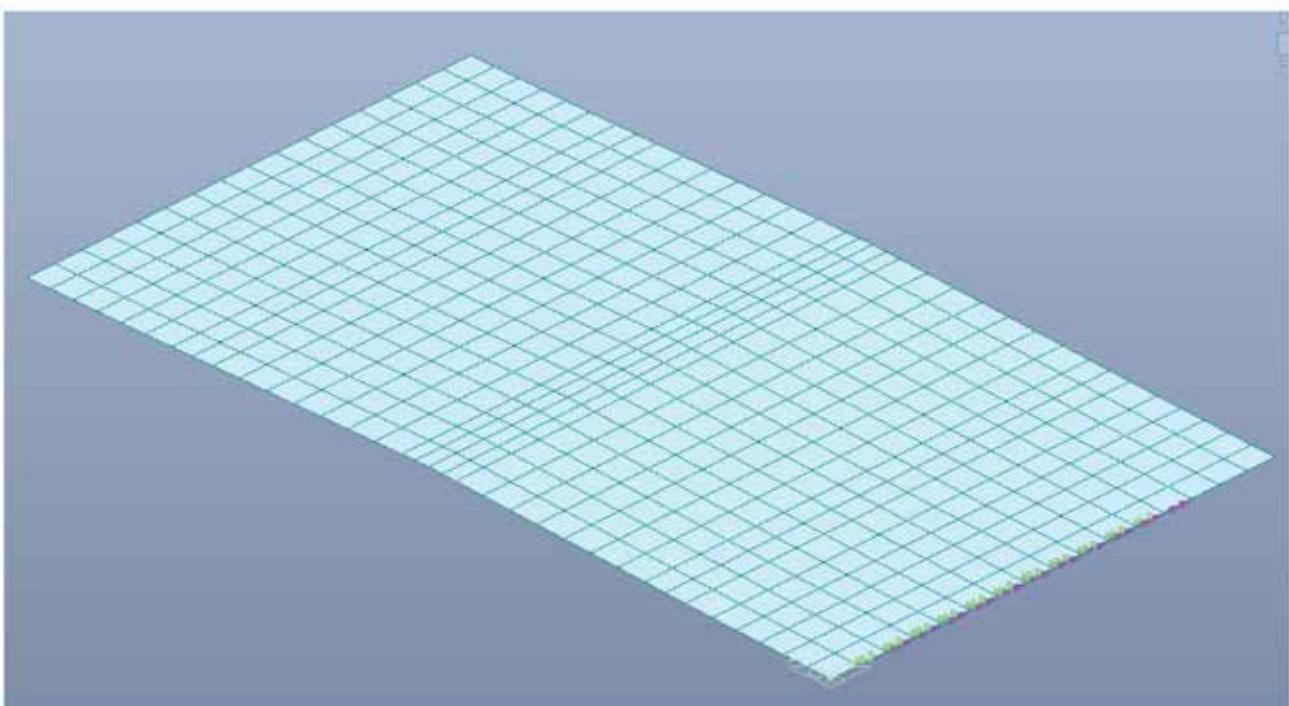


*Figura 80 – Carico Q1b*



*Figura 81 - Carico Schema 2*

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



*Figura 82 - Momento torcente che simula l'effetto dell'urto*

Di riporta di seguito i diagrammi delle sollecitazioni ottenute sia agli Stati limiti Ultimi che agli Stati limiti di esercizio con le relative verifiche strutturali.

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**7.2.1 Verifica STRUTTURALE soletta**

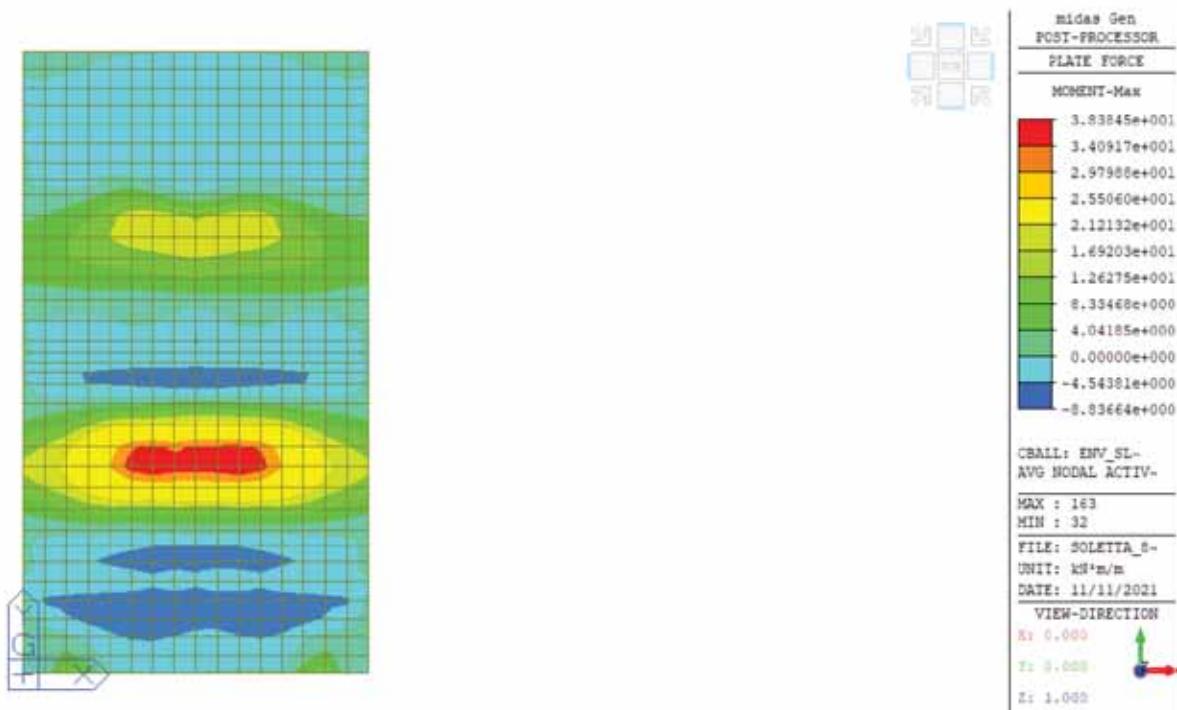


Figura 83 - Max Momento SLU

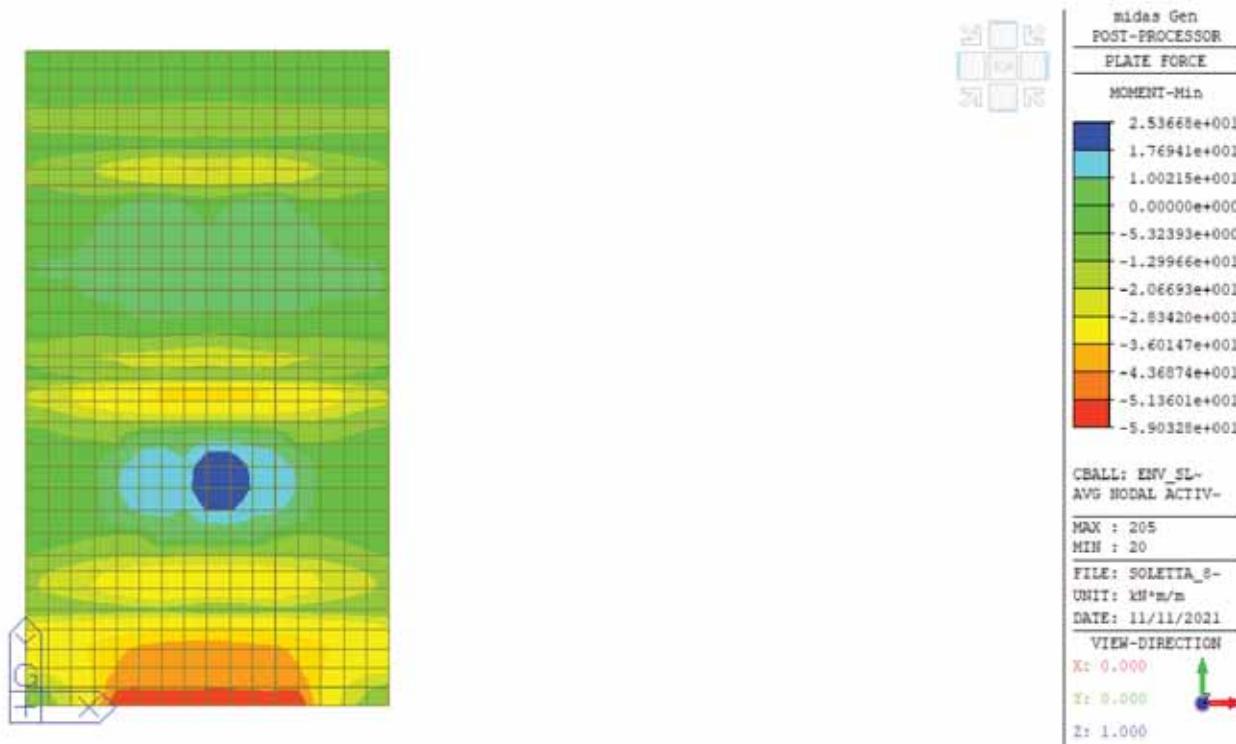


Figura 84 - Min Momento SLU

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

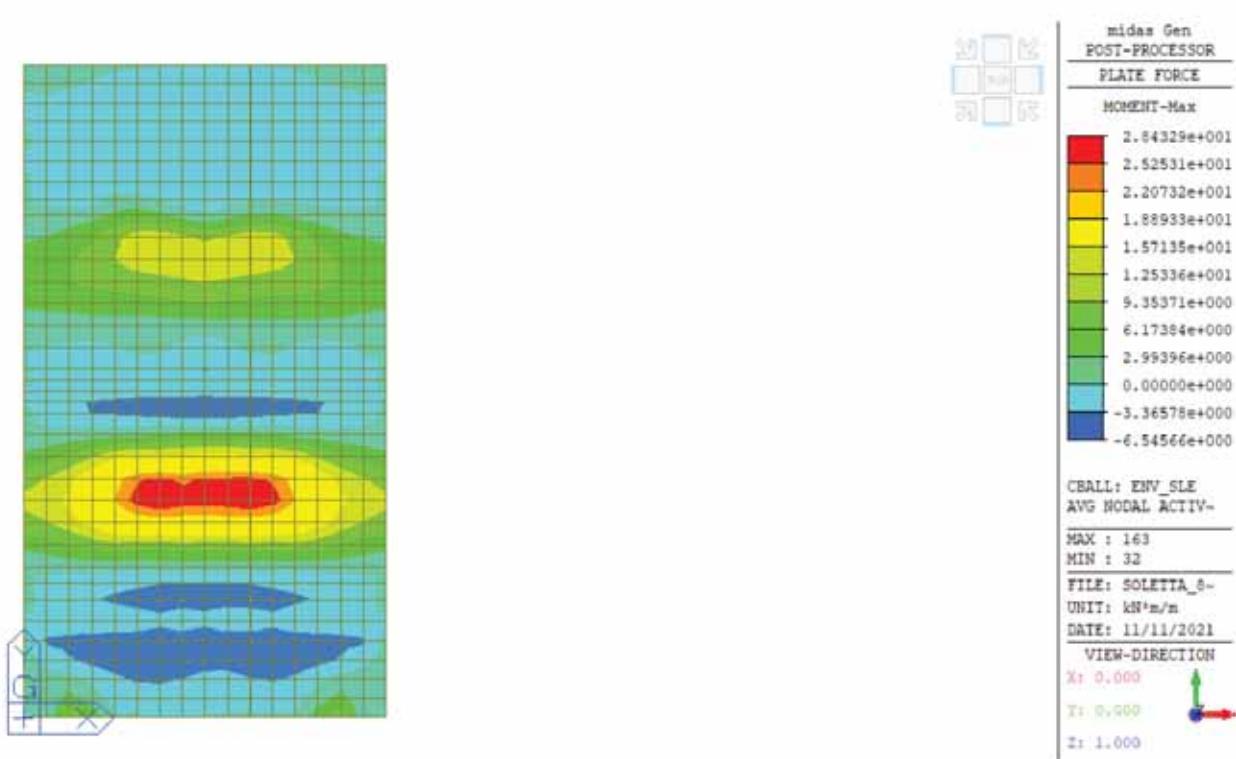


Figura 85 - Max Momento SLE

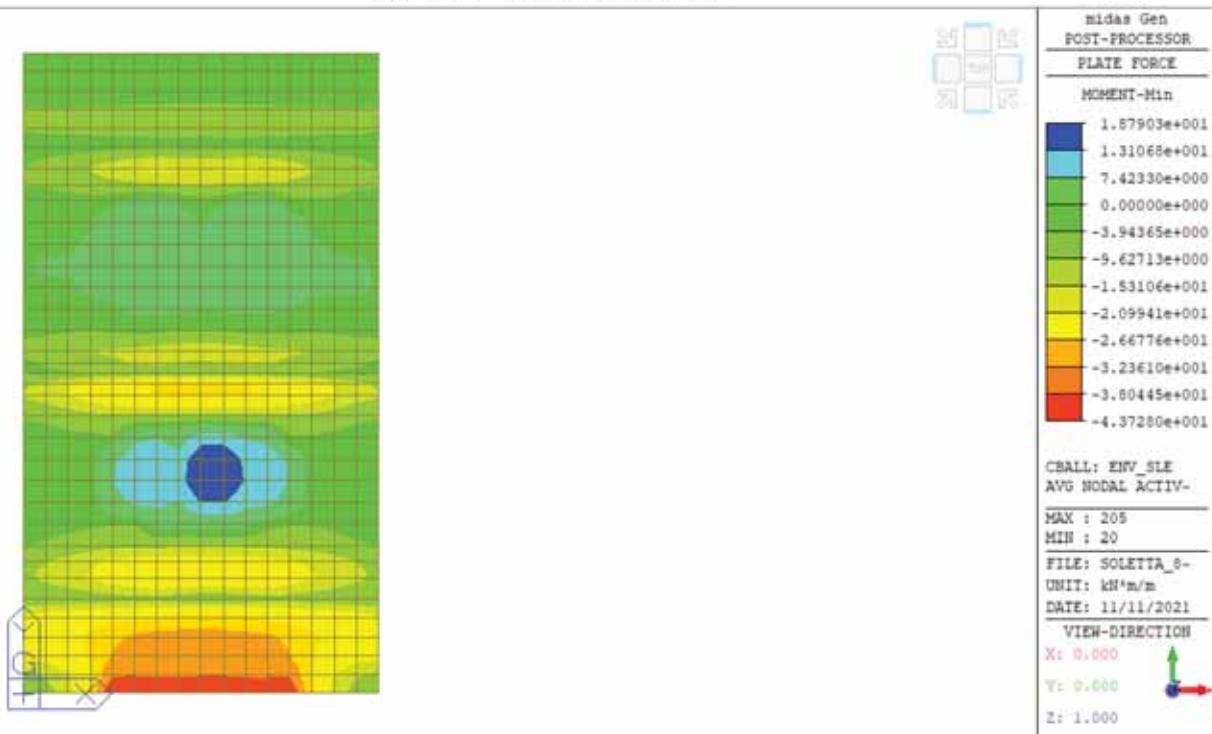
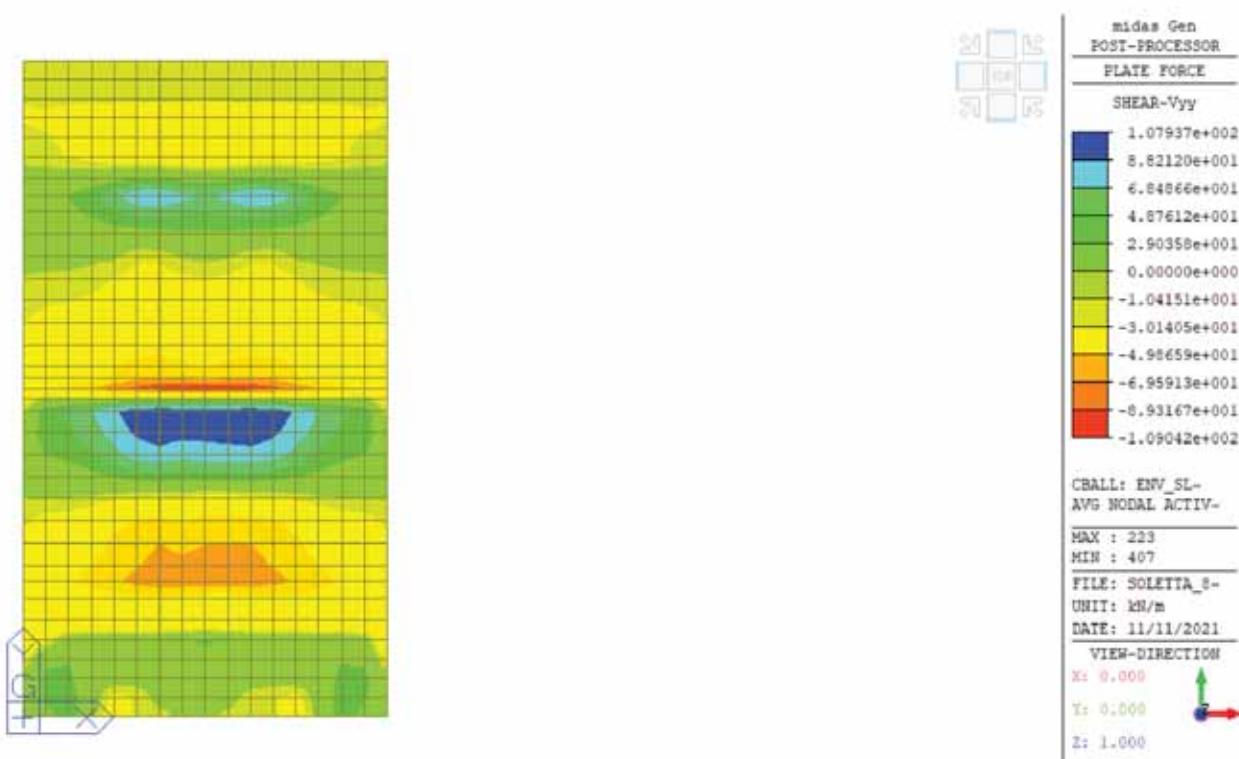


Figura 86 - Min Momento SLE

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**



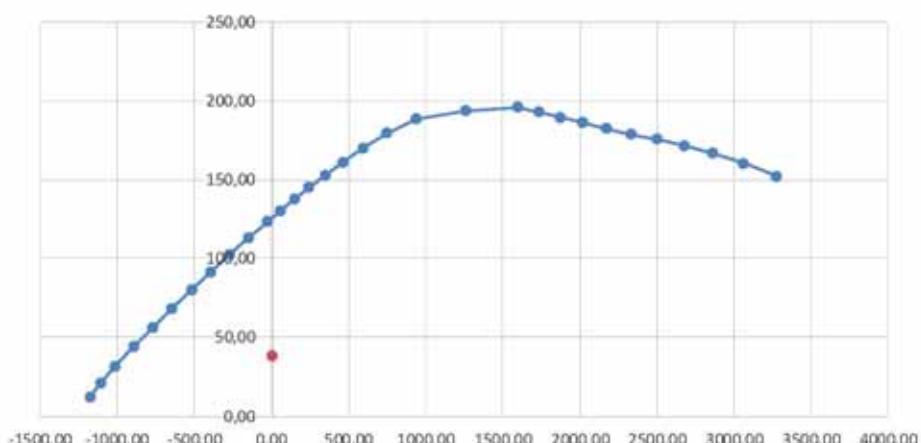
*Figura 87 - Taglio SLU*

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**VERIFICA FLESSIONALE MOMENTO POSITIVO**

RIEPILOGO SOLLECITAZIONI			RIEPILOGO ARMATURE			RIEPILOGO GEOMETRIA			RIEPILOGO MATERIALI		
	M	N	n°	φ (mm)	y	B =	100	cm	fck	33,2	N/mm <sup>2</sup>
	kNm	kN		(mm)	(cm)	H =	26	cm	Rck	40	N/mm <sup>2</sup>
SLU/SLV	38,38	0,0							fcd =	18,81	
RARA	28,43	0,0	5	16	5				c	5	cm
FREQUENTE	28,43	0,0	1	0	0				fmax	16	mm
QUASI PERMANENTE	28,43	0,0	5	16	21				i	10	cm
			5	16	21				i'	0	cm
									fyk	450	N/mm <sup>2</sup>

DIAGRAMMA M-N



VERIFICHE TENSIONALI	RARA	σc	σs	σcmax	σsmax
		N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>
	QUASI PERMANENTE	3,37	78,65	19,92	360,00
		3,37		14,94	

VERIFICHE A FESSURAZIONE	QUASI PERMANENTE	Wk	Mf	M	Wklm
		mm	kNm	kNm	
	FREQUENTE	0,0275	43,28	36,36	0,2 Ms < M1f
		0,0275	43,28	36,36	0,3 Ms < M1f

**VERIFICA TENSIONALE COMBINAZIONE QUASI PERMANENTE :**

M = 28,43 kNm

N = 0 kN

H/6 = 4,33 cm

B = 100 cm

H = 26 cm

Verifica:

n = 15

x = 8,21 cm

s-c = 3,37 N/mm<sup>2</sup>

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

s-a = -19,76 N/mm<sup>2</sup> (y = 5 cm: 5 f 16, Aa = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a' = 78,65 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a'' = 78,65 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa'' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

**VERIFICA TENSIONALE COMBINAZIONE RARA :**

M = 28,43 kNm

N = 0 kN

H/6 = 4,33 cm

B = 100 cm

H = 26 cm

Verifica:

n = 15

x = 8,21 cm

s-c = 3,37 N/mm<sup>2</sup>

s-a = -19,76 N/mm<sup>2</sup> (y = 5 cm: 5 f 16, Aa = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a' = 78,65 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a'' = 78,65 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa'' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

**VERIFICA A FESSURAZIONE COMBINAZIONE QUASI PERMANENTE :**

M = 28,43 kNm

N = 0 kNm

Rck = 40 N/mm<sup>2</sup>

fctm = 3,16 N/mm<sup>2</sup> (0,27xRck<sup>2/3</sup>)

B = 100 cm (larghezza sezione)

H = 26 cm (altezza sezione)

y = 5 cm (posizione di armatura)

Aa = 10,05 cm<sup>2</sup> (n 5 f 16 passo 20 cm)

y' = 21 cm (posizione di armatura)

Aa' = 10,05 cm<sup>2</sup> (n 5 f 16 passo 10 cm)

y'' = 21 cm (posizione di armatura)

Aa'' = 10,05 cm<sup>2</sup> (n 5 f 16 passo 20 cm)

STADIO NON FESSURATO

n = 15 (coeff. omogeneizzazione)

Jid(I) = 173'070 cm<sup>4</sup>

Aid = 3'022 cm<sup>2</sup>

x(I) = 13,373 cm (asse neutro stato I, dal lembo compresso)

M = 36,36 kNm (Momento di formazione fessura fcfk = 0,7x1,2xfctm)

Mf = 43,28 kNm (Momento di fessurazione fctm)

**Momento d'esercizio inferiore al momento di fessurazione quindi non serve calcolare l'ampiezza delle fessure**

**VERIFICA A FESSURAZIONE COMBINAZIONE FREQUENTE :**

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

M = 28,43 kNm

N = 0 kNm

Rck = 40 N/mm<sup>2</sup>

fctm = 3,16 N/mm<sup>2</sup> (0,27xRck<sup>2/3</sup>)

B = 100 cm (larghezza sezione)

H = 26 cm (altezza sezione)

y = 5 cm (posizione di armatura)

Aa = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 20 cm)

y' = 21 cm (posizione di armatura)

Aa' = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 10 cm)

y'' = 21 cm (posizione di armatura)

Aa'' = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 20 cm)

STADIO NON FESSURATO

n = 15 (coeff. omogeneizzazione)

Jid(l) = 173'070 cm<sup>4</sup>

Aid = 3'022 cm<sup>2</sup>

x(l) = 13,373 cm (asse neutro stato l, dal lembo compresso)

M = 36,36 kNm (Momento di formazione fessura fcfk = 0,7x1,2xfctm)

Mf = 43,28 kNm (Momento di fessurazione fctm)

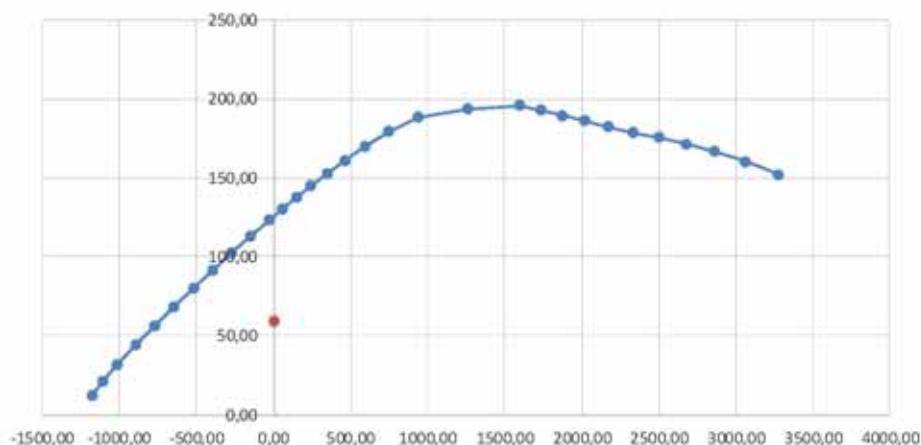
**Momento d'esercizio inferiore al momento di fessurazione quindi non serve calcolare l'ampiezza delle fessure**

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**VERIFICA FLESSIONALE MOMENTO NEGATIVO**

RIEPILOGO SOLLECITAZIONI			RIEPILOGO ARMATURE			RIEPILOGO GEOMETRIA			RIEPILOGO MATERIALI		
	M	N	n°	φ (mm)	y	B =	100	cm	fck	33,2	N/mm <sup>2</sup>
	kNm	kN		(mm)	(cm)	H =	26	cm	Rck	40	N/mm <sup>2</sup>
SLU/SLV	59,03	0,0							fcd =	18,81	
RARA	43,73	0,0	5	16	5				c	5	cm
FREQUENTE	43,73	0,0	1	0	0				fmax	16	mm
QUASI PERMANENTE	43,73	0,0	5	16	21				i	10	cm
			5	16	21				i'	0	cm
									fyk	450	N/mm <sup>2</sup>

DIAGRAMMA M-N



VERIFICHE TENSIONALI	RARA	σC	σS	σCmax	σSmax
		N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>	N/mm <sup>2</sup>
	QUASI PERMANENTE	5,18	120,97	19,92	360,00
		5,18		14,94	

VERIFICHE A FESSURAZIONE	QUASI PERMANENTE	Wk	Mf	M	Wklm
		mm	kNm	kNm	
	FREQUENTE	0,0539	43,28	36,36	0,2 Controllo Wk
		0,0539	43,28	36,36	0,3 Controllo Wk

**VERIFICA TENSIONALE COMBINAZIONE QUASI PERMANENTE :**

M = 43,73 kNm

N = 0 kN

H/6 = 4,33 cm

B = 100 cm

H = 26 cm

Verifica:

n = 15

x = 8,21 cm

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

s-c = 5,18 N/mm<sup>2</sup>

s-a = -30,39 N/mm<sup>2</sup> (y = 5 cm: 5 f 16, Aa = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a' = 120,97 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a'' = 120,97 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa'' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

**VERIFICA TENSIONALE COMBINAZIONE RARA :**

M = 43,73 kNm

N = 0 kN

H/6 = 4,33 cm

B = 100 cm

H = 26 cm

Verifica:

n = 15

x = 8,21 cm

s-c = 5,18 N/mm<sup>2</sup>

s-a = -30,39 N/mm<sup>2</sup> (y = 5 cm: 5 f 16, Aa = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a' = 120,97 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

s-a'' = 120,97 N/mm<sup>2</sup> (y = 21 cm: 5 f 16, Aa'' = 10,05 cm<sup>2</sup>, passo 20,0 cm)

**VERIFICA A FESSURAZIONE COMBINAZIONE QUASI PERMANENTE :**

M = 43,73 kNm

N = 0 kNm

Rck = 40 N/mm<sup>2</sup>

fctm = 3,16 N/mm<sup>2</sup> (0,27xRck<sup>2/3</sup>)

B = 100 cm (larghezza sezione)

H = 26 cm (altezza sezione)

y = 5 cm (posizione di armatura)

Aa = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 20 cm)

y' = 21 cm (posizione di armatura)

Aa' = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 10 cm)

y'' = 21 cm (posizione di armatura)

Aa'' = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 20 cm)

STADIO NON FESSURATO

n = 15 (coeff. omogeneizzazione)

Jid(l) = 173'070 cm<sup>4</sup>

Aid = 3'022 cm<sup>2</sup>

x(l) = 13,373 cm (asse neutro stato l, dal lembo compresso)

M = 36,36 kNm (Momento di formazione fessura fcfk = 0,7x1,2xfctm)

Mf = 43,28 kNm (Momento di fessurazione fctm)

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**STADIO FESSURATO (II)**

$n = 15$  (coeff. omogeneizzazione)

$x(II) = 8,2 \text{ cm}$  (asse neutro stato II)

$Jid(II) = 69'336 \text{ cm}^2$

$s_s = 120,97 \text{ N/mm}^2$  (M,N)

$e_s = 0,000576$

Geometria tirante ideale

$b = 100 \text{ cm}$

$h' = 17,8 \text{ cm}$  ( $c + f/2 + i' + 7,5xf$ )

$A_s = 20,11 \text{ cm}^2$

$A_c = 1780,00 \text{ cm}^2$

$A_s/A_c = 0,0113$

Distanza media fessure:

$c = 5 \text{ cm}$  (copriferro, da estradosso cls ad estradosso barra)

$i = 10,0 \text{ cm}$  (interasse ferri in orizzontali)

$i' = 0 \text{ cm}$  (interasse ferri verticale, nel caso di due strati di barre)

$f_{\max} = 16 \text{ mm}$  diametro massimo barre

$r = 0,0113$  ( $A_s/A_c$  tirante ideale)

$k_1 = 0,4$  (barre a.m.)

$k_2 = 0,074$  ( $0,25x[1-h'/(2x(H-x_1))]$ )

$s_m = 16,18 \text{ cm}$  ( $2x(c+i/10)+k_1xk_2xf/r$ )

Deformazione media:

$b_1 = 1$  (barre a.m.)

$b_2 = 0,5$  (carichi di lunga durata o numerosi cicli di carico)

$s_{sr} = 119,74 \text{ N/mm}^2$  (stato II, Mf)

$s_{ss} = 120,97 \text{ N/mm}^2$  (stato II, Magente)

$e_{sm} = 0,000294$  ( $s_s/E_s x (1-b_1 x b_2 x (s_r/s_s)^2)$ )

Aampiezza teorica di fessura:

$w_k = 0,081 \text{ mm}$  ( $1,7 x e_{sm} x s_m$ )

**VERIFICA A FESSURAZIONE COMBINAZIONE FREQUENTE :**

$M = 43,73 \text{ kNm}$

$N = 0 \text{ kNm}$

$R_{ck} = 40 \text{ N/mm}^2$

$f_{ctm} = 3,16 \text{ N/mm}^2$  ( $0,27 x R_{ck}^{2/3}$ )

$B = 100 \text{ cm}$  (larghezza sezione)

$H = 26 \text{ cm}$  (altezza sezione)

$y = 5 \text{ cm}$  (posizione di armatura)

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

Aa = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 20 cm)

y' = 21 cm (posizione di armatura)

Aa' = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 10 cm)

y'' = 21 cm (posizione di armatura)

Aa'' = 10,05 cm<sup>2</sup> (n<sub>i</sub> 5 f 16 passo 20 cm)

STADIO NON FESSURATO

n = 15 (coeff. omogeneizzazione)

Jid(I) = 173'070 cm<sup>4</sup>

Aid = 3'022 cm<sup>2</sup>

x(I) = 13,373 cm (asse neutro stato I, dal lembo compresso)

M = 36,36 kNm (Momento di formazione fessura fcfk = 0,7x1,2xfctm)

Mf = 43,28 kNm (Momento di fessurazione fctm)

STADIO FESSURATO (II)

n = 15 (coeff. omogeneizzazione)

x(II) = 8,2 cm (asse neutro stato II)

Jid(II) = 69'336 cm<sup>2</sup>

s s = 120,97 N/mm<sup>2</sup> (M,N)

e s = 0,000576

Geometria tirante ideale

b = 100 cm

h' = 17,8 cm (c + f/2 + i' + 7,5xf)

As = 20,11 cm<sup>2</sup>

Ac = 1780,00 cm<sup>2</sup>

As/Ac = 0,0113

Distanza media fessure:

c = 5 cm (copriferro, da estradosso cls ad estradosso barra)

i = 10,0 cm (interasse ferri in orizzontali)

i' = 0 cm (interasse ferri verticale, nel caso di due strati di barre)

f max = 16 mm diametro massimo barre

r = 0,0113 (As/Ac tirante ideale)

k1 = 0,4 (barre a.m.)

k2 = 0,074 (0,25x[1-h'/(2x(H-x1))])

sm = 16,18 cm (2x(c+i/10)+k1xk2xf/r)

Deformazione media:

b1 = 1 (barre a.m.)

b2 = 0,5 (carichi di lunga durata o numerosi cicli di carico)

s sr = 119,74 N/mm<sup>2</sup> (stato II, Mf)

s ss = 120,97 N/mm<sup>2</sup> (stato II, Magente)

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

e sm = 0,000294 (ss/Esx(1-b1xb2x(ssr/ss)^2))

Aampiezza teorica di fessura:

wk = 0,081 mm (1,7xesmxsm)

**VERIFICA A TAGLIO**

**VERIFICA A TAGLIO DELLA SEZIONE IN C.A. SECONDO NTC 14/01/2008**

• Caratteristiche della sezione

b <sub>w</sub> = 1000	mm larghezza	f <sub>yk</sub> = 450	MPa	resist. caratteristica
h = 260	mm altezza	g <sub>s</sub> = 1,15		coeff. sicurezza
c = 50	mm copriferro	f <sub>yd</sub> = 391,3	MPa	resist. di calcolo
f <sub>ck</sub> = 33,2	MPa resist. caratteristica	Armatura longitudinale tesa:		
g <sub>c</sub> = 1,50	coeff. sicurezza	A <sub>sl,1</sub> = 5	Ø 16	= 10,05 cm <sup>2</sup>
a <sub>cc</sub> = 0,85	coeff. riduttivo	A <sub>sl,2</sub> = 5	Ø 16	= 10,05 cm <sup>2</sup>
d = 210	mm altezza utile	A <sub>sl,3</sub> = 0	Ø 0	= 0,00 cm <sup>2</sup>
f <sub>cd</sub> = 18,81	MPa resist. di calcolo			20,11 cm <sup>2</sup>

• Sollecitazioni (compressione<0, trazione>0, taglio in valore assoluto)

$$N_{ed} = 0,0 \text{ kN} \quad V_{ed} = 109,40 \text{ kN}$$

• Elementi senza armature trasversali resistenti a taglio

$$k = 1 + (200/d)^{1/2} < 2 \quad k = 1,976 \quad < 2$$

$$n_{min} = 0,035 k^{3/2} f_{ck}^{1/2} \quad n_{min} = 0,560$$

$$r_1 = A_{sl}/(b_w \times d) < 0,02 \quad r_1 = 0,010 \quad < 0,02$$

$$\sigma_{cp} = N_{Ed}/A_c < 0,2 f_{cd} \quad \sigma_{cp} = 0,00 \text{ MPa} < 0,2 f_{cd}$$

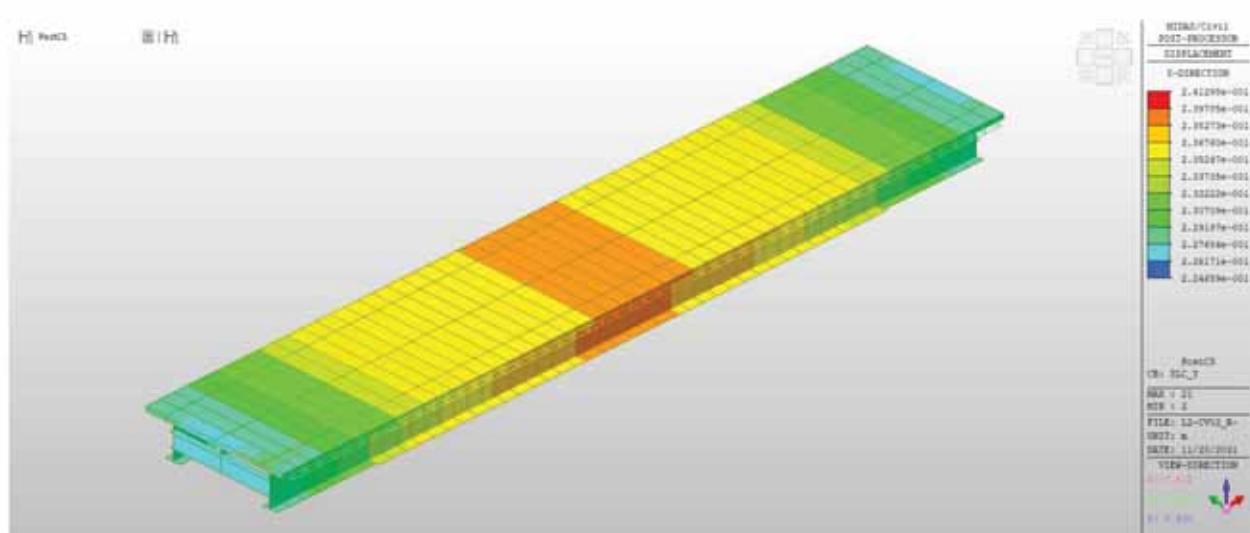
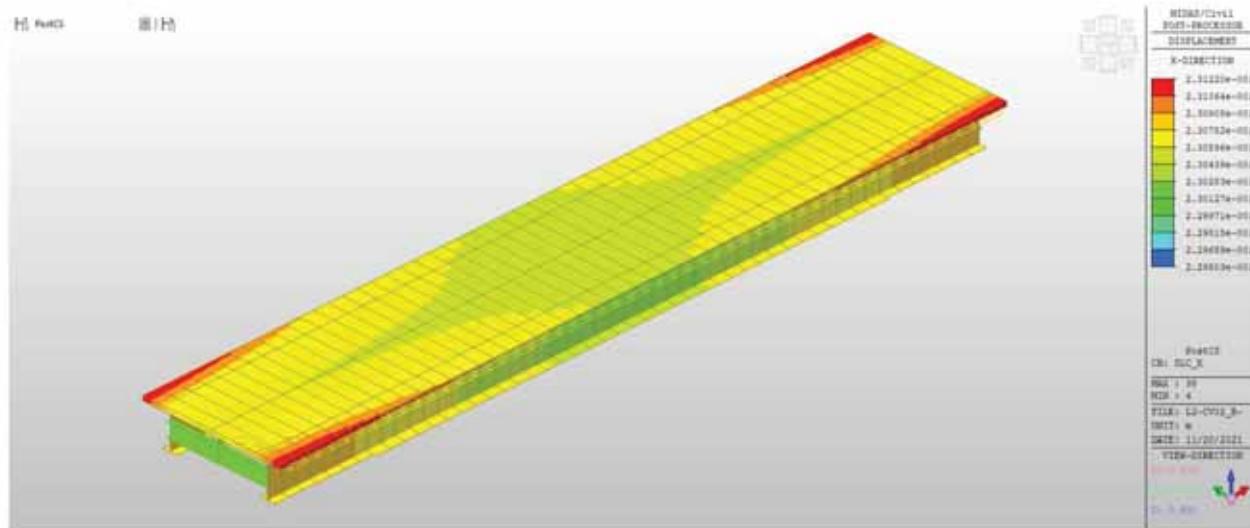
$$(0,18 \times k \times (100 \times r_1 \times f_{ck})^{1/3} / g_c + 0,15 \times s_{cp} \times b_w \times d = 157,7 \text{ kN}$$

$$(n_{min} + 0,15 \times s_{cp}) \times b_w \times d = 117,6 \text{ kN} \quad V_{Rd} = 157,7 \text{ kN}$$

la sezione è verificata in assenza di armature per il taglio

## 8 VERIFICA ELEMENTI DI APPOGGIO

Si riporta di seguito l'andamento degli spostamenti orizzontali necessari per il dimensionamento dei varchi, nonché gli scarichi necessari per il dimensionamento dei baggioli e dei ritegni sismici.



**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

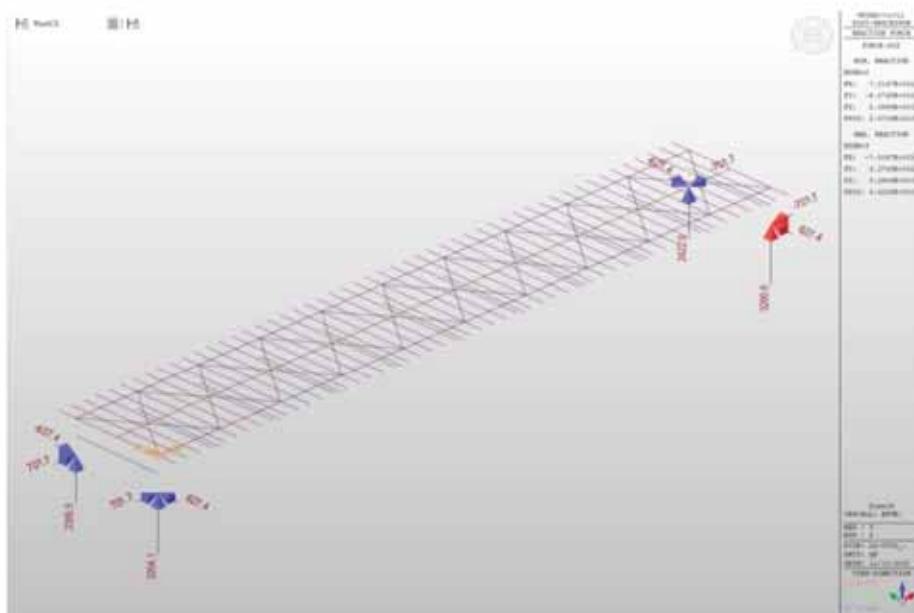


Figura 90 – Reazioni agli appoggi

Node	Load	FX (kN)	FY (kN)	FZ (kN)
1	FASE 3	47.905	0.739	1197.249
2	FASE 3	47.905	-0.739	1197.249
1	VENTO PONTE SCARICO	0.210	49.571	44.578
2	VENTO PONTE SCARICO	-0.210	49.583	-44.578
1	VENTO PONTE CARICO	-0.174	111.548	70.184
2	VENTO PONTE CARICO	0.174	111.576	-70.184
1	TERMICA UNIFORME +	36.436	9.192	0.000
2	TERMICA UNIFORME +	36.436	-9.192	0.000
1	TERMICA UNIFORME -	-34.085	-8.599	0.000
2	TERMICA UNIFORME -	-34.085	8.599	0.000
1	TERMICA GRADIENTE +	-12.462	-0.612	0.000
2	TERMICA GRADIENTE +	-12.462	0.612	0.000
1	TERMICA GRADIENTE -	14.955	0.735	0.000
2	TERMICA GRADIENTE -	14.955	-0.735	0.000
1	FRENATURA	-116.721	0.002	-9.826
2	FRENATURA	-116.721	-0.002	-9.826
1	EX(RS)	635.293	0.016	61.196
2	EX(RS)	635.293	0.016	61.196
1	EY(RS)	0.838	622.108	440.281
2	EY(RS)	0.838	622.108	440.281
1	EZ(RS)	19.908	0.161	624.923
2	EZ(RS)	19.908	0.161	624.923
1	DX(RS)	109.230	0.003	10.544
2	DX(RS)	109.230	0.003	10.544
1	CX(RS)	1030.089	0.026	99.192
2	CX(RS)	1030.089	0.026	99.192
1	LM01(all)	20.998	0.772	988.013
2	LM01(all)	6.439	-0.734	197.805
1	LM02(all)	4.421	0.742	186.469
2	LM02(all)	7.652	-0.752	373.076
1	LM03(all)	0.333	0.013	9.171
2	LM03(all)	1.120	-0.015	45.829

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

**8.1 VERIFICA BAGGIOLI**

DATI DI INPUT		
Forza orizzontale	$V_{Sd}$ [kN] =	701.700
Forza verticale (+ se di trazione)	$N_{Sd} / 4$ [kN] =	-822.650
Altezza di applicazione forza orizzontale	a [m] =	0.16
Base baggiolo	h [m] =	1.05
Copriferro	c [m] =	0.05
Altezza utile	d [m] =	0.875
Profondità di incastro assunta	h' [m] =	0.00
Luce di calcolo mensola	L [m] =	0.16
Profondità baggiolo	b [m] =	1.00
Angolo inclinazione traliccio [°]	$\alpha$ [°] =	11.48
Angolo inclinazione traliccio [rad]	$\alpha$ [rad] =	0.20
Angolo inclinazione traliccio [°]	$\psi$ [°] =	78.52
Angolo inclinazione traliccio [rad]	$\psi$ [rad] =	1.37
VERIFICA CORRENTE TESO		
Trazione sulle barre d'armatura corrente tesio	$Z_{Sd}$ [kN] =	-680.08
Classe acciaio		B450C
Resistenza a snervamento armatura	$f_{yd}$ [MPa] =	391.3
Area armatura necessaria	$A_{nec}$ [cm <sup>2</sup> ] =	17.4
Diametro armatura 1	$\varnothing 1$ [mm] =	16
Diametro armatura 2	$\varnothing 2$ [mm] =	0
Numero ferri $\varnothing 1$	$n_{\varnothing 1}$ [-] =	9
Numero ferri $\varnothing 2$	$n_{\varnothing 2}$ [-] =	0
Area armatura 1 (singola barra)	$A_{s1}$ [cm <sup>2</sup> ] =	2.01
Area armatura 2 (singola barra)	$A_{s2}$ [cm <sup>2</sup> ] =	0.00
Area armatura totale	$A_{STOT}$ [cm <sup>2</sup> ] =	18.1
Resistenza corrente tesio	$Z_{Rd}$ [kN] =	708.09
Coefficiente di sicurezza	$Z_{Rd} / Z_{Sd} = c.s.$ =	1.04
VERIFICA PUNTONE CLS COMPRESSO		
Resistenza cubica caratteristica	$R_{ck}$ [MPa] =	40
Resistenza cilindrica caratteristica	$f_{ck}$ [MPa] =	32
Coefficiente parziale di sicurezza	$\gamma_c$ [-] =	1.50
Resistenza di calcolo a compressione	$f_{cd}$ [MPa] =	18.13
Presenza di staffe		Si
Coefficiente presenza staffe (1 senza staffe; 1.5 con staffe)	$c$ [-] =	1.50
Pendenza del puntone di calcestruzzo	$\cotg\psi = \tg\alpha = \lambda$ =	0.20
Altezza puntone	$0.4*c*d*\operatorname{sen}\psi = h_p$ [m] =	0.514
Compressione nel puntone	$C_{Sd}$ [kN] =	3415.67
Resistenza del puntone di calcestruzzo	$0.4*b*d*f_{cd}*c / (1+\lambda^2) = C_{Rd}$ [kN] =	9143
Coefficiente di sicurezza	$C_{Rd} / C_{Sd} = c.s.$ =	2.68

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

## 8.2 VERIFICA ISOLATORI

Si riportano di seguito le caratteristiche degli isolatori individuati.

V	Fzd	Ke	ξe	Kv	Dg	te	h	H	Z
kN	kN	kN/mm	%	kN/mm	mm	mm	mm	mm	mm
23710	37010	4,49	21	4330	1100	190	322	402	1150

<b>V</b>	Carico verticale massimo agente in presenza del sisma allo SLC
<b>Fzd</b>	Carico verticale massimo in assenza di SISMA e spostamento 10mm
<b>Ke</b>	Rigidezza orizzontale equivalente
<b>ξe</b>	Coefficiente di smorzamento viscoso equivalente
<b>Kv</b>	Rigidezza verticale
<b>Dg</b>	Diametro elastomero
<b>te</b>	Spessore totale gomma
<b>h</b>	Altezza escluse piastre di ancoraggio
<b>H</b>	Altezza totale incluse piastre di ancoraggio
<b>Z</b>	Lato piastre di ancoraggio

Il valore di Fz max = 3290.6 kN inferiore ai limiti previsti per l'isolatore introdotto (Fzd = 37010 kN). Nel caso specifico, sono stati scelti isolatori elastomerici a nucleo di piombo e nell'ambito della rigidezza è stata effettuata una simulazione (che ha fornito sostanzialmente i medesimi risultati in termini di spostamento e reazioni agli appoggi) modificando la rigidezza dell'isolatore di +/-20% così come previsto dalle NTC2008 al punto 11.9.7.

## 8.3 VERIFICA GIUNTI

Per la determinazione dei varchi e quindi l'individuazione dei corretti giunti da utilizzare, sono stati considerati gli spostamenti sia dovuti alle azioni statiche che sismiche. Nel dettaglio si nota che lo spostamento massimo, dovuto all'azione sismica in condizione SLC è pari a dmax = 23.122 cm. Si introduce quindi un giunto che permetta tale quantità di spostamento.

## 8.4 VERIFICA RITEGNI SISMICI.

Il presente cavalcavia presenta come appoggi 4 isolatori di appoggio che permettono uno spostamento massimo orizzontale pari a 300mm. Si è scelto di introdurre comunque dei ritegni sismici per bloccare lo spostamento trasversale dell'impalcato in conseguenza dei valori di spostamento generati dal sisma. Si è scelto di permettere uno spostamento trasversale maggiore o comunque prossimo al valore riferito al sisma SLV e di dimensionare il ritegno con la forza che deriva dalla differenza di spostamento tra lo SLC ed il varco previsto.

## LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO

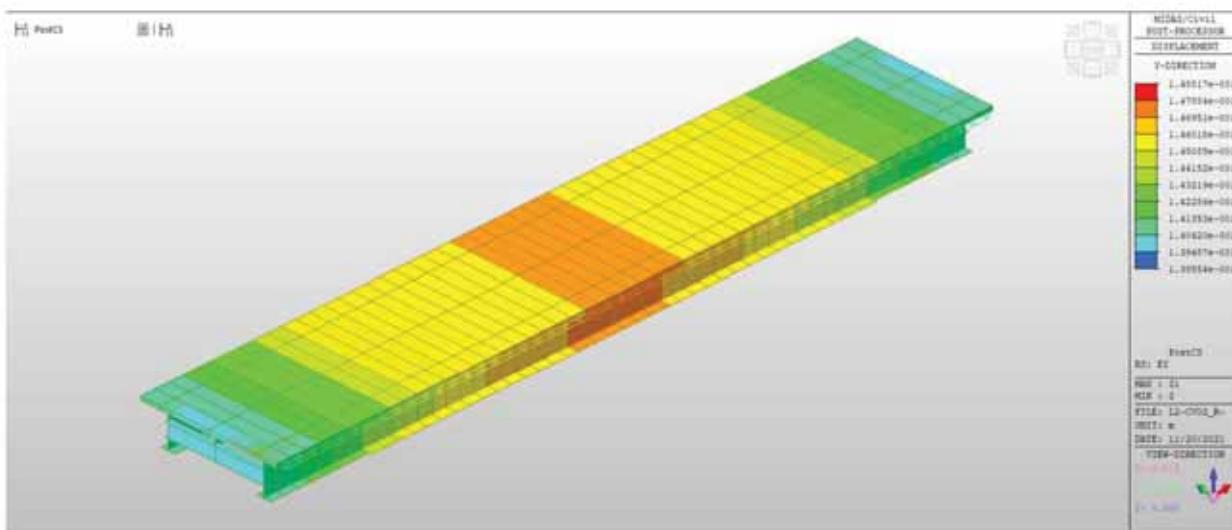
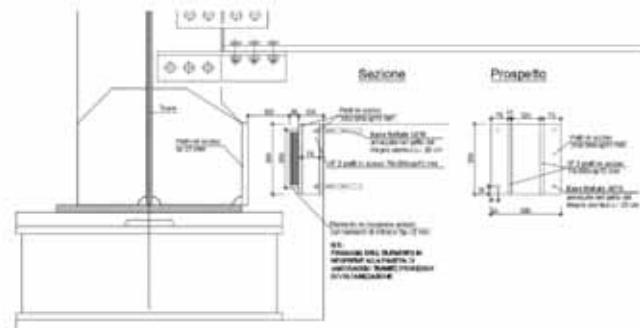


Figura 91 - Spostamento Y SLY (SISMA Y + 0.3 SISMA X)



$$\text{Spostamento residuo} = 24.13 - 18.00 = 6.13\text{cm}$$

$$K_{isolatore} = 4.49 \text{ kN/mm}$$

$$F_{\text{residua}} = 275.24 \text{ kN}$$

Con tale forza vengono dimensionate le armature dei ritegni sismici

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

DATI DI INPUT		
Forza orizzontale	$V_{sd}$ [kN] =	550.480
Forza verticale (+ se di trazione)	$N_{sd} / 4$ [kN] =	0
Altezza di applicazione forza orizzontale	a [m] =	0.85
Base baggiolo	h [m] =	1.00
Copriferro	c [m] =	0.05
Altezza utile	d [m] =	0.825
Profondità di incastro assunta	h' [m] =	0.00
Luce di calcolo mensola	L [m] =	0.85
Profondità baggiolo	b [m] =	1.00
Angolo inclinazione traliccio [°]	$\alpha$ [°] =	48.86
Angolo inclinazione traliccio [rad]	$\alpha$ [rad] =	0.85
Angolo inclinazione traliccio [°]	$\psi$ [°] =	41.14
Angolo inclinazione traliccio [rad]	$\psi$ [rad] =	0.72
VERIFICA CORRENTE TESO		
Trazione sulle barre d'armatura corrente tesio	$Z_{sd}$ [kN] =	630.18
Classe acciaio		B450C
Resistenza a snervamento armatura	$f_{yd}$ [MPa] =	391.3
Area armatura necessaria	$A_{nec}$ [ $\text{cm}^2$ ] =	16.1
Diametro armatura 1	$\varnothing 1$ [mm] =	16
Diametro armatura 2	$\varnothing 2$ [mm] =	0
Numero ferri $\varnothing 1$	$n_{\varnothing 1}$ [-] =	8
Numero ferri $\varnothing 2$	$n_{\varnothing 2}$ [-] =	0
Area armatura 1 (singola barra)	$A_{s1}$ [ $\text{cm}^2$ ] =	2.01
Area armatura 2 (singola barra)	$A_{s2}$ [ $\text{cm}^2$ ] =	0.00
Area armatura totale	$A_{sTOT}$ [ $\text{cm}^2$ ] =	16.1
Resistenza corrente tesio	$Z_{rd}$ [kN] =	629.41
<b>Coefficiente di sicurezza</b>	$Z_{rd} / Z_{sd} = c.s.$ =	<b>1.01</b>
VERIFICA PUNTONE CLS COMPRESO		
Resistenza cubica caratteristica	$R_{ck}$ [MPa] =	40
Resistenza cilindrica caratteristica	$f_{ck}$ [MPa] =	32
Coefficiente parziale di sicurezza	$\gamma_c$ [-] =	1.50
Resistenza di calcolo a compressione	$f_{cd}$ [MPa] =	18.13
Presenza di staffe		Si
Coefficiente presenza staffe (1 senza staffe; 1.5 con staffe)	$c$ [-] =	1.50
Pendenza del puntone di calcestruzzo	$\cotg\psi = \tg\alpha = \lambda$ =	1.14
Altezza puntone	$0.4*c*d*\sin\psi = h_p$ [m] =	0.326
Compressione nel puntone	$C_{sd}$ [kN] =	836.75
Resistenza del puntone di calcestruzzo	$0.4*b*d*f_{cd}*c / (1+\lambda^2) = C_{rd}$ [kN] =	3885
<b>Coefficiente di sicurezza</b>	$C_{rd} / C_{sd} = c.s.$ =	<b>4.64</b>

**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

### 8.5 VERIFICA DEFORMAZIONI

Si riportano di seguito le deformazioni dell'impalcato dovuto al peso proprio, soletta e permanente, nonché la freccia in condizioni di esercizio.

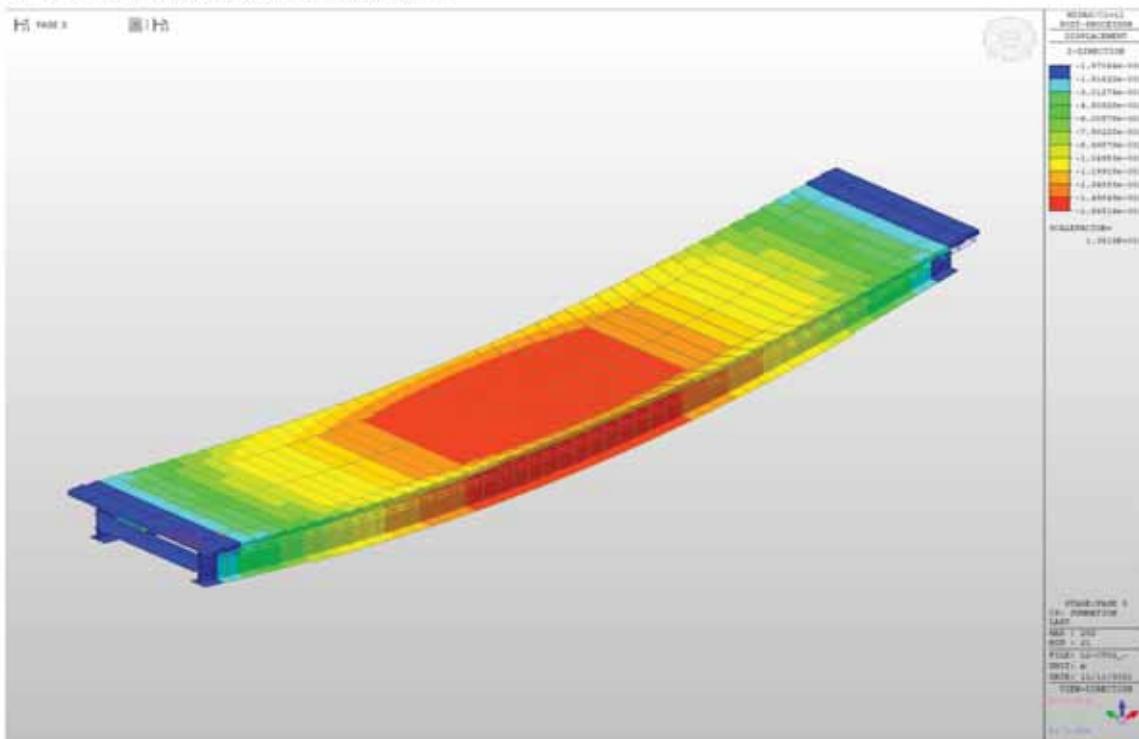


Figura 92 – Deformate - Fase 3

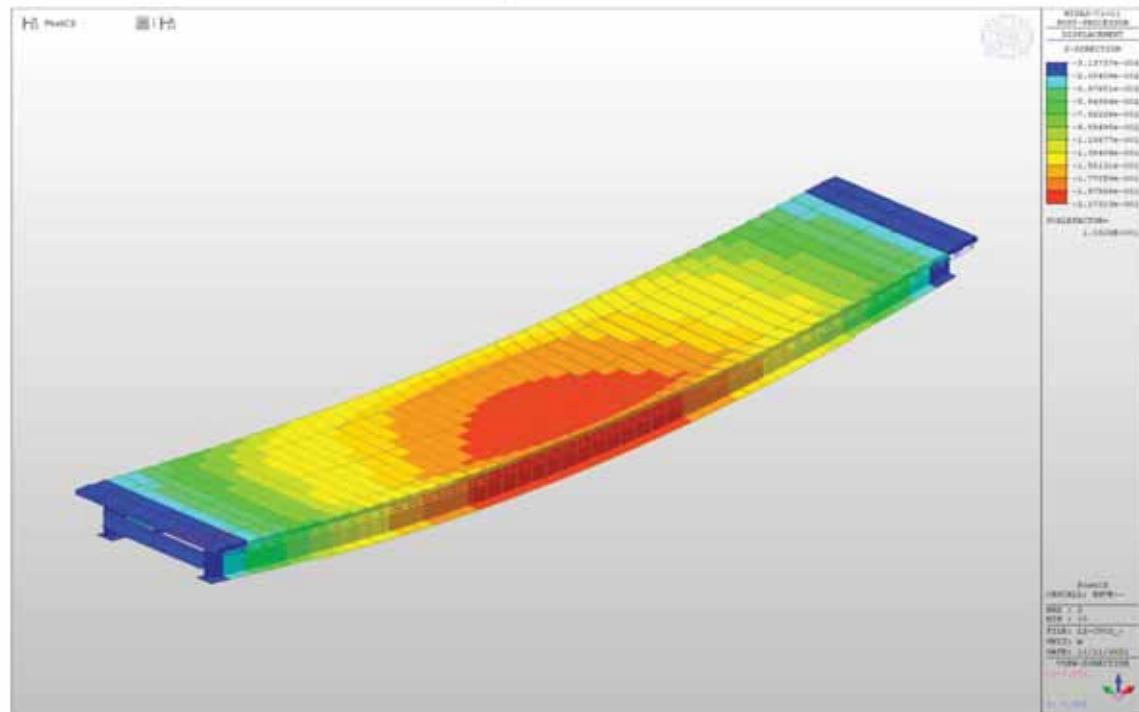


Figura 93 – Deformate - Inviluppo SLE

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**LOTTO 1 – CAVALCAVIA AL KM. 15+818 - RELAZIONE DI CALCOLO IMPALCATO**

---

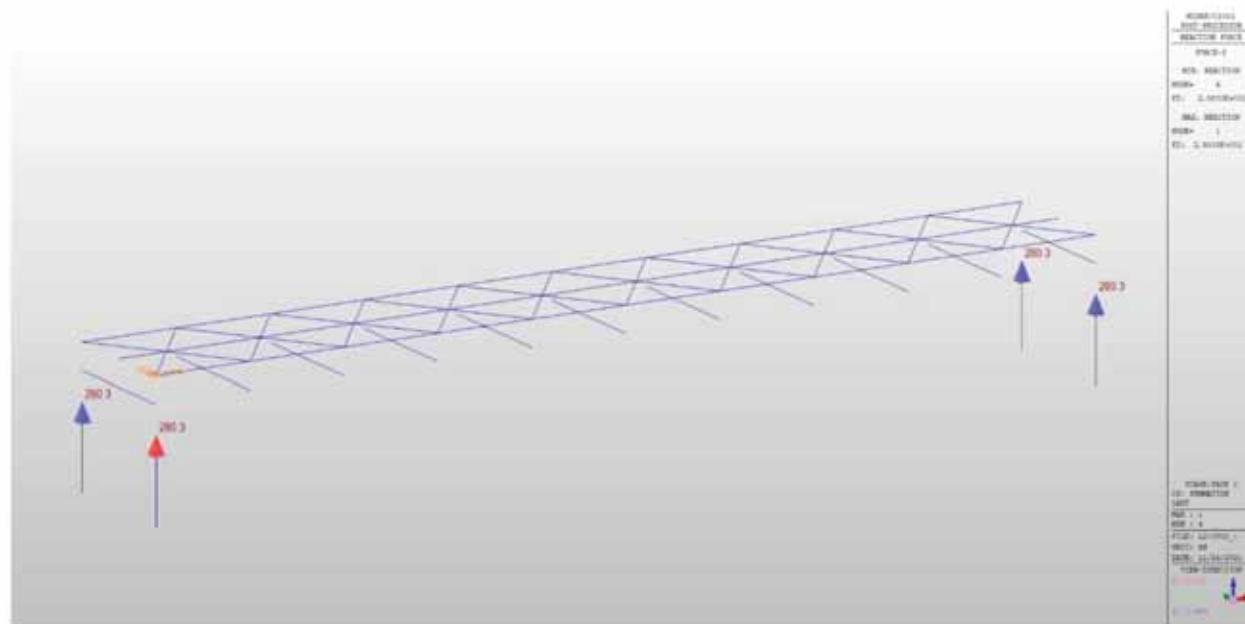
Per effetto del peso proprio dell'impalcato, della soletta e dei permanenti portati, si ottiene uno spostamento in mezzeria pari a circa 16.48 cm (monta da applicare in fase di costruzione). Considerando quindi tale valore come monta iniziale dai imporre all'impalcato, in condizione di esercizio si ottiene il seguente valore di freccia.

$$f = 21.73\text{cm} - 16.48\text{cm} = 5.25 \text{ cm}$$

## 9 VALIDAZIONE CODICE DI CALCOLO

Per effettuare la validazione del codice di calcolo, è stato determinato il peso della struttura ed è stato confrontato con gli scarichi derivanti dal modello dovuto ai soli pesi propri.

In base al punto 10.2 delle norme per le costruzioni NTC2008, qualora per la determinazione delle sollecitazioni di progetto si ricorra all'utilizzo di codici di calcolo, dovrà essere verificata l'attendibilità dei risultati tramite una validazione del modello di carico. Si riporta pertanto di seguito la validazione del modello andando a confrontare le reazioni vincolari per effetto del peso proprio della struttura ottenuti dal modello di calcolo e manualmente calcolando i singoli Pesi strutturali.



Calcolando i pesi in modo manuale e considerando l'incremento del peso proprio dovuto alla presenza delle piastre varie si ottiene:

LUNGHEZZA CONCIO	NUMERO	PIATTABANDA SUP		PIATTABANDA INF		ANIMA			AREA	VOLUME	PESO
		mm	Sp	mm	Sp	Htrave	B	Sp			
C1	6750	1	750	25	900	30	2100	2045	20	0.08665	0.584888 4591.367
C2	10000	1	750	28	1200	40	2100	2032	20	0.10964	1.0964 8606.74
C3	12000	1	750	30	1200	55	2100	2015	20	0.1288	1.5458 12132.96
C4	10000	1	750	28	1200	40	2100	2032	20	0.10964	1.0964 8606.74
C5	6750	1	750	25	900	30	2100	2045	20	0.08665	0.584888 4591.367
TRAVERSI	45500	1	300	25	300	25	590	2032	13	0.10964	1.0964 8606.74
TRAVERSI	5400	11	350	25	350	25	800	750	20	0.3575	1.9305 15154.43
Peso tot											
Reazione											
12% <b>282.2946</b> kN											

La reazione vincolare ottenuta risulta essere compatibile con quella calcolata manualmente.

## 10 ALLEGATO 1 - TABULATO MIDAS

# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client
	Author		
			L2-CV02_REVISIONE_2

```
;-----  
; MIDAS/Civil Text(MCT) File.  
; Date : 2021/11/22  
;  
*VERSION  
8.7.5  
  
*UNIT ; Unit System  
; FORCE, LENGTH, HEAT, TEMPER  
KN , M, BTU, C  
  
*PROJINFO ; Project Information  
USER=.  
ADDRESS=.  
  
*STRUCTYPE ; Structure Type  
; iSTYP, iMASS, iSMAS, bMASSOFFSET, bSELFWEIGHT, GRAV, TEMPER, bALIGNBEAM, bALIGNSLAB,  
bROTRIGID  
0, 1, 1, NO, YES, 9.806, 18, NO, NO, NO  
  
*REBAR-MATL-CODE ; Rebar Material Code  
; CONC_CODE, CONC_MDB, SRC_CODE, SRC_MDB  
NTC08(RC), B450C, ASTM(RC), Grade 60  
  
*NODE ; Nodes  
; iNO, X, Y, Z  
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4, 44, 6, -2.46  
5, 0, 0, -1.2  
6, 4.4, 0, -1.2  
7, 8.8, 0, -1.2  
8, 13.2, 0, -1.2  
9, 17.6, 0, -1.2  
10, 22, 0, -1.2  
11, 26.4, 0, -1.2  
12, 30.8, 0, -1.2  
13, 35.2, 0, -1.2  
14, 39.6, 0, -1.2  
15, 44, 0, -1.2  
16, 0, 3, -1.2  
17, 4.4, 3, -1.2  
18, 8.8, 3, -1.2  
19, 13.2, 3, -1.2  
20, 17.6, 3, -1.2  
21, 22, 3, -1.2  
22, 26.4, 3, -1.2  
23, 30.8, 3, -1.2  
24, 35.2, 3, -1.2  
25, 39.6, 3, -1.2  
26, 44, 3, -1.2  
27, 0, 6, -1.2  
28, 4.4, 6, -1.2  
29, 8.8, 6, -1.2  
30, 13.2, 6, -1.2  
31, 17.6, 6, -1.2  
32, 22, 6, -1.2  
33, 26.4, 6, -1.2  
34, 30.8, 6, -1.2  
35, 35.2, 6, -1.2
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# MIDAS

## PROJECT TITLE :

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	Author			

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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118, 25.52, 0, 0  
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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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281, 34.32, 7.25, 0  
282, 35.2, 7.25, 0  
283, 36.08, 7.25, 0

# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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286, 38.72, 7.25, 0
287, 39.6, 7.25, 0
288, 40.48, 7.25, 0
289, 41.36, 7.25, 0
290, 42.24, 7.25, 0
291, 43.12, 7.25, 0
292, 44, 7.25, 0

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```

*ELEMENT ; Elements
; iEL, TYPE, iMAT, iPRO, iN1, iN2, ANGLE, iSUB, EXVAL, iOPT(EXVAL2) ; Frame Element
; iEL, TYPE, iMAT, iPRO, iN1, iN2, ANGLE, iSUB, EXVAL, EXVAL2, bLMT ; Comp/Tens Truss
; iEL, TYPE, iMAT, iPRO, iN1, iN2, iN3, iN4, iSUB, iWID, LCAXIS ; Planar Element
; iEL, TYPE, iMAT, iPRO, iN1, iN2, iN3, iN4, iN5, iN6, iN7, iN8 ; Solid Element
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   55, BEAM , 3, 6, 140, 191, 0, 0
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   97, BEAM , 1, 4, 32, 21, 0, 0
   98, BEAM , 1, 4, 21, 10, 0, 0
   99, BEAM , 1, 4, 33, 22, 0, 0

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# MIDAS

PROJECT TITLE :

MIDAS	Company						Client	File Name	L2-CV02_REVISIONE_2
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103, BEAM ,	1,	4,	35,	24,	0,	0			
104, BEAM ,	1,	4,	24,	13,	0,	0			
105, BEAM ,	1,	4,	36,	25,	0,	0			
106, BEAM ,	1,	4,	25,	14,	0,	0			
107, BEAM ,	1,	4,	37,	26,	0,	0			
108, BEAM ,	1,	4,	26,	15,	0,	0			
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110, BEAM ,	1,	7,	90,	91,	0,	0			
111, BEAM ,	3,	6,	90,	141,	0,	0			
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114, BEAM ,	1,	7,	192,	193,	0,	0			
115, BEAM ,	3,	6,	192,	243,	0,	0			
116, BEAM ,	3,	6,	40,	91,	0,	0			
117, BEAM ,	1,	7,	91,	92,	0,	0			
118, BEAM ,	3,	6,	91,	142,	0,	0			
119, BEAM ,	1,	3,	142,	143,	0,	0			
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122, BEAM ,	3,	6,	193,	244,	0,	0			
123, BEAM ,	3,	6,	41,	92,	0,	0			
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139, BEAM ,	3,	6,	145,	196,	0,	0			
140, BEAM ,	3,	6,	196,	247,	0,	0			
141, BEAM ,	3,	6,	44,	95,	0,	0			
142, BEAM ,	1,	7,	95,	96,	0,	0			
143, BEAM ,	3,	6,	95,	146,	0,	0			
144, BEAM ,	1,	3,	146,	147,	0,	0			
145, BEAM ,	3,	6,	146,	197,	0,	0			
146, BEAM ,	1,	7,	197,	198,	0,	0			
147, BEAM ,	3,	6,	197,	248,	0,	0			
148, BEAM ,	3,	6,	45,	96,	0,	0			
149, BEAM ,	1,	8,	96,	97,	0,	0			
150, BEAM ,	3,	6,	96,	147,	0,	0			
151, BEAM ,	1,	3,	147,	148,	0,	0			
152, BEAM ,	3,	6,	147,	198,	0,	0			
153, BEAM ,	1,	8,	198,	199,	0,	0			
154, BEAM ,	3,	6,	198,	249,	0,	0			
155, BEAM ,	3,	6,	46,	97,	0,	0			
156, BEAM ,	1,	8,	97,	98,	0,	0			
157, BEAM ,	3,	6,	97,	148,	0,	0			
158, BEAM ,	1,	3,	148,	149,	0,	0			
159, BEAM ,	3,	6,	148,	199,	0,	0			
160, BEAM ,	1,	8,	199,	200,	0,	0			
161, BEAM ,	3,	6,	199,	250,	0,	0			

# MIDAS

PROJECT TITLE :

MIDAS	Company						Client	File Name	L2-CV02_REVISIONE_2
	Author								
162, BEAM ,	3,	6,	47,	98,	0,	0			
163, BEAM ,	1,	8,	98,	99,	0,	0			
164, BEAM ,	3,	6,	98,	149,	0,	0			
165, BEAM ,	1,	3,	149,	150,	0,	0			
166, BEAM ,	3,	6,	149,	200,	0,	0			
167, BEAM ,	1,	8,	200,	201,	0,	0			
168, BEAM ,	3,	6,	200,	251,	0,	0			
169, BEAM ,	3,	6,	48,	99,	0,	0			
170, BEAM ,	3,	6,	99,	150,	0,	0			
171, BEAM ,	3,	6,	150,	201,	0,	0			
172, BEAM ,	3,	6,	201,	252,	0,	0			
173, BEAM ,	3,	6,	49,	100,	0,	0			
174, BEAM ,	1,	8,	100,	101,	0,	0			
175, BEAM ,	3,	6,	100,	151,	0,	0			
176, BEAM ,	1,	3,	151,	152,	0,	0			
177, BEAM ,	3,	6,	151,	202,	0,	0			
178, BEAM ,	1,	8,	202,	203,	0,	0			
179, BEAM ,	3,	6,	202,	253,	0,	0			
180, BEAM ,	3,	6,	50,	101,	0,	0			
181, BEAM ,	1,	8,	101,	102,	0,	0			
182, BEAM ,	3,	6,	101,	152,	0,	0			
183, BEAM ,	1,	3,	152,	153,	0,	0			
184, BEAM ,	3,	6,	152,	203,	0,	0			
185, BEAM ,	1,	8,	203,	204,	0,	0			
186, BEAM ,	3,	6,	203,	254,	0,	0			
187, BEAM ,	3,	6,	51,	102,	0,	0			
188, BEAM ,	1,	8,	102,	103,	0,	0			
189, BEAM ,	3,	6,	102,	153,	0,	0			
190, BEAM ,	1,	3,	153,	154,	0,	0			
191, BEAM ,	3,	6,	153,	204,	0,	0			
192, BEAM ,	1,	8,	204,	205,	0,	0			
193, BEAM ,	3,	6,	204,	255,	0,	0			
194, BEAM ,	3,	6,	52,	103,	0,	0			
195, BEAM ,	1,	8,	103,	104,	0,	0			
196, BEAM ,	3,	6,	103,	154,	0,	0			
197, BEAM ,	1,	3,	154,	155,	0,	0			
198, BEAM ,	3,	6,	154,	205,	0,	0			
199, BEAM ,	1,	8,	205,	206,	0,	0			
200, BEAM ,	3,	6,	205,	256,	0,	0			
201, BEAM ,	3,	6,	53,	104,	0,	0			
202, BEAM ,	3,	6,	104,	155,	0,	0			
203, BEAM ,	3,	6,	155,	206,	0,	0			
204, BEAM ,	3,	6,	206,	257,	0,	0			
205, BEAM ,	3,	6,	54,	105,	0,	0			
206, BEAM ,	1,	8,	105,	106,	0,	0			
207, BEAM ,	3,	6,	105,	156,	0,	0			
208, BEAM ,	1,	3,	156,	157,	0,	0			
209, BEAM ,	3,	6,	156,	207,	0,	0			
210, BEAM ,	1,	8,	207,	208,	0,	0			
211, BEAM ,	3,	6,	207,	258,	0,	0			
212, BEAM ,	3,	6,	55,	106,	0,	0			
213, BEAM ,	1,	9,	106,	107,	0,	0			
214, BEAM ,	3,	6,	106,	157,	0,	0			
215, BEAM ,	1,	3,	157,	158,	0,	0			
216, BEAM ,	3,	6,	157,	208,	0,	0			
217, BEAM ,	1,	9,	208,	209,	0,	0			
218, BEAM ,	3,	6,	208,	259,	0,	0			
219, BEAM ,	3,	6,	56,	107,	0,	0			
220, BEAM ,	1,	9,	107,	108,	0,	0			
221, BEAM ,	3,	6,	107,	158,	0,	0			
222, BEAM ,	1,	3,	158,	159,	0,	0			
223, BEAM ,	3,	6,	158,	209,	0,	0			

# MIDAS

PROJECT TITLE :

MIDAS	Company						Client	File Name	L2-CV02_REVISIONE_2
	Author								
224, BEAM ,	1,	9,	209,	210,	0,	0			
225, BEAM ,	3,	6,	209,	260,	0,	0			
226, BEAM ,	3,	6,	57,	108,	0,	0			
227, BEAM ,	1,	9,	108,	109,	0,	0			
228, BEAM ,	3,	6,	108,	159,	0,	0			
229, BEAM ,	1,	3,	159,	160,	0,	0			
230, BEAM ,	3,	6,	159,	210,	0,	0			
231, BEAM ,	1,	9,	210,	211,	0,	0			
232, BEAM ,	3,	6,	210,	261,	0,	0			
233, BEAM ,	3,	6,	58,	109,	0,	0			
234, BEAM ,	3,	6,	109,	160,	0,	0			
235, BEAM ,	3,	6,	160,	211,	0,	0			
236, BEAM ,	3,	6,	211,	262,	0,	0			
237, BEAM ,	3,	6,	59,	110,	0,	0			
238, BEAM ,	1,	9,	110,	111,	0,	0			
239, BEAM ,	3,	6,	110,	161,	0,	0			
240, BEAM ,	1,	3,	161,	162,	0,	0			
241, BEAM ,	3,	6,	161,	212,	0,	0			
242, BEAM ,	1,	9,	212,	213,	0,	0			
243, BEAM ,	3,	6,	212,	263,	0,	0			
244, BEAM ,	3,	6,	60,	111,	0,	0			
245, BEAM ,	1,	9,	111,	112,	0,	0			
246, BEAM ,	3,	6,	111,	162,	0,	0			
247, BEAM ,	1,	3,	162,	163,	0,	0			
248, BEAM ,	3,	6,	162,	213,	0,	0			
249, BEAM ,	1,	9,	213,	214,	0,	0			
250, BEAM ,	3,	6,	213,	264,	0,	0			
251, BEAM ,	3,	6,	61,	112,	0,	0			
252, BEAM ,	1,	9,	112,	113,	0,	0			
253, BEAM ,	3,	6,	112,	163,	0,	0			
254, BEAM ,	1,	3,	163,	164,	0,	0			
255, BEAM ,	3,	6,	163,	214,	0,	0			
256, BEAM ,	1,	9,	214,	215,	0,	0			
257, BEAM ,	3,	6,	214,	265,	0,	0			
258, BEAM ,	3,	6,	62,	113,	0,	0			
259, BEAM ,	1,	9,	113,	114,	0,	0			
260, BEAM ,	3,	6,	113,	164,	0,	0			
261, BEAM ,	1,	3,	164,	165,	0,	0			
262, BEAM ,	3,	6,	164,	215,	0,	0			
263, BEAM ,	1,	9,	215,	216,	0,	0			
264, BEAM ,	3,	6,	215,	266,	0,	0			
265, BEAM ,	3,	6,	63,	114,	0,	0			
266, BEAM ,	3,	6,	114,	165,	0,	0			
267, BEAM ,	3,	6,	165,	216,	0,	0			
268, BEAM ,	3,	6,	216,	267,	0,	0			
269, BEAM ,	3,	6,	64,	115,	0,	0			
270, BEAM ,	1,	9,	115,	116,	0,	0			
271, BEAM ,	3,	6,	115,	166,	0,	0			
272, BEAM ,	1,	3,	166,	167,	0,	0			
273, BEAM ,	3,	6,	166,	217,	0,	0			
274, BEAM ,	1,	9,	217,	218,	0,	0			
275, BEAM ,	3,	6,	217,	268,	0,	0			
276, BEAM ,	3,	6,	65,	116,	0,	0			
277, BEAM ,	1,	9,	116,	117,	0,	0			
278, BEAM ,	3,	6,	116,	167,	0,	0			
279, BEAM ,	1,	3,	167,	168,	0,	0			
280, BEAM ,	3,	6,	167,	218,	0,	0			
281, BEAM ,	1,	9,	218,	219,	0,	0			
282, BEAM ,	3,	6,	218,	269,	0,	0			
283, BEAM ,	3,	6,	66,	117,	0,	0			
284, BEAM ,	1,	9,	117,	118,	0,	0			
285, BEAM ,	3,	6,	117,	168,	0,	0			

# MIDAS

PROJECT TITLE :

MIDAS	Company						Client	File Name	L2-CV02_REVISIONE_2
	Author								
286, BEAM ,	1,	3,	168,	169,	0,	0			
287, BEAM ,	3,	6,	168,	219,	0,	0			
288, BEAM ,	1,	9,	219,	220,	0,	0			
289, BEAM ,	3,	6,	219,	270,	0,	0			
290, BEAM ,	3,	6,	67,	118,	0,	0			
291, BEAM ,	1,	9,	118,	119,	0,	0			
292, BEAM ,	3,	6,	118,	169,	0,	0			
293, BEAM ,	1,	3,	169,	170,	0,	0			
294, BEAM ,	3,	6,	169,	220,	0,	0			
295, BEAM ,	1,	9,	220,	221,	0,	0			
296, BEAM ,	3,	6,	220,	271,	0,	0			
297, BEAM ,	3,	6,	68,	119,	0,	0			
298, BEAM ,	3,	6,	119,	170,	0,	0			
299, BEAM ,	3,	6,	170,	221,	0,	0			
300, BEAM ,	3,	6,	221,	272,	0,	0			
301, BEAM ,	3,	6,	69,	120,	0,	0			
302, BEAM ,	1,	9,	120,	121,	0,	0			
303, BEAM ,	3,	6,	120,	171,	0,	0			
304, BEAM ,	1,	3,	171,	172,	0,	0			
305, BEAM ,	3,	6,	171,	222,	0,	0			
306, BEAM ,	1,	9,	222,	223,	0,	0			
307, BEAM ,	3,	6,	222,	273,	0,	0			
308, BEAM ,	3,	6,	70,	121,	0,	0			
309, BEAM ,	1,	9,	121,	122,	0,	0			
310, BEAM ,	3,	6,	121,	172,	0,	0			
311, BEAM ,	1,	3,	172,	173,	0,	0			
312, BEAM ,	3,	6,	172,	223,	0,	0			
313, BEAM ,	1,	9,	223,	224,	0,	0			
314, BEAM ,	3,	6,	223,	274,	0,	0			
315, BEAM ,	3,	6,	71,	122,	0,	0			
316, BEAM ,	1,	8,	122,	123,	0,	0			
317, BEAM ,	3,	6,	122,	173,	0,	0			
318, BEAM ,	1,	3,	173,	174,	0,	0			
319, BEAM ,	3,	6,	173,	224,	0,	0			
320, BEAM ,	1,	8,	224,	225,	0,	0			
321, BEAM ,	3,	6,	224,	275,	0,	0			
322, BEAM ,	3,	6,	72,	123,	0,	0			
323, BEAM ,	1,	8,	123,	124,	0,	0			
324, BEAM ,	3,	6,	123,	174,	0,	0			
325, BEAM ,	1,	3,	174,	175,	0,	0			
326, BEAM ,	3,	6,	174,	225,	0,	0			
327, BEAM ,	1,	8,	225,	226,	0,	0			
328, BEAM ,	3,	6,	225,	276,	0,	0			
329, BEAM ,	3,	6,	73,	124,	0,	0			
330, BEAM ,	3,	6,	124,	175,	0,	0			
331, BEAM ,	3,	6,	175,	226,	0,	0			
332, BEAM ,	3,	6,	226,	277,	0,	0			
333, BEAM ,	3,	6,	74,	125,	0,	0			
334, BEAM ,	1,	8,	125,	126,	0,	0			
335, BEAM ,	3,	6,	125,	176,	0,	0			
336, BEAM ,	1,	3,	176,	177,	0,	0			
337, BEAM ,	3,	6,	176,	227,	0,	0			
338, BEAM ,	1,	8,	227,	228,	0,	0			
339, BEAM ,	3,	6,	227,	278,	0,	0			
340, BEAM ,	3,	6,	75,	126,	0,	0			
341, BEAM ,	1,	8,	126,	127,	0,	0			
342, BEAM ,	3,	6,	126,	177,	0,	0			
343, BEAM ,	1,	3,	177,	178,	0,	0			
344, BEAM ,	3,	6,	177,	228,	0,	0			
345, BEAM ,	1,	8,	228,	229,	0,	0			
346, BEAM ,	3,	6,	228,	279,	0,	0			
347, BEAM ,	3,	6,	76,	127,	0,	0			

# MIDAS

PROJECT TITLE :

MIDAS	Company						Client	File Name	L2-CV02_REVISIONE_2
	Author								
348, BEAM ,	1,	8,	127,	128,	0,	0			
349, BEAM ,	3,	6,	127,	178,	0,	0			
350, BEAM ,	1,	3,	178,	179,	0,	0			
351, BEAM ,	3,	6,	178,	229,	0,	0			
352, BEAM ,	1,	8,	229,	230,	0,	0			
353, BEAM ,	3,	6,	229,	280,	0,	0			
354, BEAM ,	3,	6,	77,	128,	0,	0			
355, BEAM ,	1,	8,	128,	129,	0,	0			
356, BEAM ,	3,	6,	128,	179,	0,	0			
357, BEAM ,	1,	3,	179,	180,	0,	0			
358, BEAM ,	3,	6,	179,	230,	0,	0			
359, BEAM ,	1,	8,	230,	231,	0,	0			
360, BEAM ,	3,	6,	230,	281,	0,	0			
361, BEAM ,	3,	6,	78,	129,	0,	0			
362, BEAM ,	3,	6,	129,	180,	0,	0			
363, BEAM ,	3,	6,	180,	231,	0,	0			
364, BEAM ,	3,	6,	231,	282,	0,	0			
365, BEAM ,	3,	6,	79,	130,	0,	0			
366, BEAM ,	1,	8,	130,	131,	0,	0			
367, BEAM ,	3,	6,	130,	181,	0,	0			
368, BEAM ,	1,	3,	181,	182,	0,	0			
369, BEAM ,	3,	6,	181,	232,	0,	0			
370, BEAM ,	1,	8,	232,	233,	0,	0			
371, BEAM ,	3,	6,	232,	283,	0,	0			
372, BEAM ,	3,	6,	80,	131,	0,	0			
373, BEAM ,	1,	8,	131,	132,	0,	0			
374, BEAM ,	3,	6,	131,	182,	0,	0			
375, BEAM ,	1,	3,	182,	183,	0,	0			
376, BEAM ,	3,	6,	182,	233,	0,	0			
377, BEAM ,	1,	8,	233,	234,	0,	0			
378, BEAM ,	3,	6,	233,	284,	0,	0			
379, BEAM ,	3,	6,	81,	132,	0,	0			
380, BEAM ,	1,	7,	132,	133,	0,	0			
381, BEAM ,	3,	6,	132,	183,	0,	0			
382, BEAM ,	1,	3,	183,	184,	0,	0			
383, BEAM ,	3,	6,	183,	234,	0,	0			
384, BEAM ,	1,	7,	234,	235,	0,	0			
385, BEAM ,	3,	6,	234,	285,	0,	0			
386, BEAM ,	3,	6,	82,	133,	0,	0			
387, BEAM ,	1,	7,	133,	134,	0,	0			
388, BEAM ,	3,	6,	133,	184,	0,	0			
389, BEAM ,	1,	3,	184,	185,	0,	0			
390, BEAM ,	3,	6,	184,	235,	0,	0			
391, BEAM ,	1,	7,	235,	236,	0,	0			
392, BEAM ,	3,	6,	235,	286,	0,	0			
393, BEAM ,	3,	6,	83,	134,	0,	0			
394, BEAM ,	3,	6,	134,	185,	0,	0			
395, BEAM ,	3,	6,	185,	236,	0,	0			
396, BEAM ,	3,	6,	236,	287,	0,	0			
397, BEAM ,	3,	6,	84,	135,	0,	0			
398, BEAM ,	1,	7,	135,	136,	0,	0			
399, BEAM ,	3,	6,	135,	186,	0,	0			
400, BEAM ,	1,	3,	186,	187,	0,	0			
401, BEAM ,	3,	6,	186,	237,	0,	0			
402, BEAM ,	1,	7,	237,	238,	0,	0			
403, BEAM ,	3,	6,	237,	288,	0,	0			
404, BEAM ,	3,	6,	85,	136,	0,	0			
405, BEAM ,	1,	7,	136,	137,	0,	0			
406, BEAM ,	3,	6,	136,	187,	0,	0			
407, BEAM ,	1,	3,	187,	188,	0,	0			
408, BEAM ,	3,	6,	187,	238,	0,	0			
409, BEAM ,	1,	7,	238,	239,	0,	0			

# MIDAS

## PROJECT TITLE :

	Company						Client	File Name
	Author							

410, BEAM , 3, 6, 238, 289, 0, 0
411, BEAM , 3, 6, 86, 137, 0, 0
412, BEAM , 1, 7, 137, 138, 0, 0
413, BEAM , 3, 6, 137, 188, 0, 0
414, BEAM , 1, 3, 188, 189, 0, 0
415, BEAM , 3, 6, 188, 239, 0, 0
416, BEAM , 1, 7, 239, 240, 0, 0
417, BEAM , 3, 6, 239, 290, 0, 0
418, BEAM , 3, 6, 87, 138, 0, 0
419, BEAM , 1, 7, 138, 139, 0, 0
420, BEAM , 3, 6, 138, 189, 0, 0
421, BEAM , 1, 3, 189, 190, 0, 0
422, BEAM , 3, 6, 189, 240, 0, 0
423, BEAM , 1, 7, 240, 241, 0, 0
424, BEAM , 3, 6, 240, 291, 0, 0
425, BEAM , 3, 6, 88, 139, 0, 0
426, BEAM , 3, 6, 139, 190, 0, 0
427, BEAM , 3, 6, 190, 241, 0, 0
428, BEAM , 3, 6, 241, 292, 0, 0
429, TRUSS , 1, 5, 191, 94, 0, 0
430, TRUSS , 1, 5, 89, 196, 0, 0
431, TRUSS , 1, 5, 196, 99, 0, 0
432, TRUSS , 1, 5, 94, 201, 0, 0
433, TRUSS , 1, 5, 201, 104, 0, 0
434, TRUSS , 1, 5, 99, 206, 0, 0
435, TRUSS , 1, 5, 206, 109, 0, 0
436, TRUSS , 1, 5, 104, 211, 0, 0
437, TRUSS , 1, 5, 211, 114, 0, 0
438, TRUSS , 1, 5, 109, 216, 0, 0
439, TRUSS , 1, 5, 216, 119, 0, 0
440, TRUSS , 1, 5, 114, 221, 0, 0
441, TRUSS , 1, 5, 221, 124, 0, 0
442, TRUSS , 1, 5, 119, 226, 0, 0
443, TRUSS , 1, 5, 226, 129, 0, 0
444, TRUSS , 1, 5, 124, 231, 0, 0
445, TRUSS , 1, 5, 231, 134, 0, 0
446, TRUSS , 1, 5, 129, 236, 0, 0
447, TRUSS , 1, 5, 236, 139, 0, 0
448, TRUSS , 1, 5, 134, 241, 0, 0

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*GROUP ; Group
; NAME, NODE LIST, ELEM LIST, PLANE TYPE
FASE 1 , 1to37 89to241, 4to334by110 5to320by105 57to59 62to108 \
112to133by7 117to242by25 121to163by14 124to412by32 128to176by16 \
131to281by50 146to402by64 151to391by48 153to373by55 158to302by16 \
165to421by32 178 183 185to377by64 195to419by32 213to341by32 240to304by16 \
279to423by48 306 311 336to416by16 345 350to414by16 359 370 405 407 409 \
429to448, 0
FASE 2 , 38to292, 53to56 109to115by2 116to404by32 118to406by32 \
120to408by32 122to410by32 123to411by32 125to413by32 127to415by32 \
129to417by32 130to418by32 132to420by32 134to422by32 136to424by32 \
137to393by32 138to394by32 139to395by32 140to396by32 141to397by32 \
143to399by32 145to401by32 147to403by32 425to428, 0
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*BNDR-GROUP ; Boundary Group
; NAME, AUTOTYPE
FASE 1, 0
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*LOAD-GROUP ; Load Group
; NAME
FASE 1
FASE 2
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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

FASE 3

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*MATERIAL      ; Material
; iMAT, TYPE, MNAME, SPHEAT, HEATCO, PLAST, TUNIT, bMASS, DAMPRATIO, [DATA1]
; STEEL, CONC, USER
; iMAT, TYPE, MNAME, SPHEAT, HEATCO, PLAST, TUNIT, bMASS, DAMPRATIO, [DATA2], [DATA2]
; SRC
; [DATA1] : 1, DB, NAME, CODE, USEELAST, ELAST
; [DATA1] : 2, ELAST, POISN, THERMAL, DEN, MASS
; [DATA1] : 3, Ex, Ey, Ez, Tx, Ty, Tz, Sxy, Sxz, Syz, Pxy, Pxz, Pyz, DEN, MASS
; Orthotropic
; [DATA2] : 1, DB, NAME, CODE, USEELAST, ELAST or 2, ELAST, POISN, THERMAL, DEN, MASS
;           1, STEEL, S355          , 0, 0, , C, NO, 0.02, 1, EN05-PS(S) , , S3
55           , NO, 2.1e+008      , 0, 0, , C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e
-005,        0,        0      , 0, 0, , C, YES, 0.05, 2, 3.3345e+007, 0.2, 1.0000e
-005,        0,        0

*MATL-COLOR
; iMAT, W_R, W_G, W_B, HF_R, HF_G, HF_B, HE_R, HE_G, HE_B, bBLEND, FACT
;   1, 115,    0,    0, 255,   87,   87, 115,    0,    0, NO, 0.5
;   2,    0,   66,   45,     0, 192, 128,     0,   66,   45, NO, 0.5
;   3,   38,   57,    0, 128, 192,     0,   38,   57,    0, NO, 0.5

*TDM-TYPE      ; Time Dependent Material
; NAME=NAME, CODE, STR, HU, VOL, AGE, CFACTA, CFACTB, TYPE, [ACI1 or ACI2]      ; CODE=
ACI
; NAME=NAME, CODE, STR, HU, MSIZE, CTYPE, AGE                                ; CODE=
CEB1990,KS,CEB1978,KSC 2010,KCI-USD12
; NAME=NAME, CODE, N1, PHI1, N2, PHI2                                         ; CODE=
MEM
; NAME=NAME, CODE, STR, HU, USS, UCS, VOL, RR, MOD                           ; CODE=
PCA
; NAME=NAME, CODE, STR, HU, VOL, UCS, VSR1, LAF, US, VSR, PST, bRCE, RR, MOD ; CODE=
COMBINED
; NAME=NAME, CODE, STR, HTYPE, HU, MSIZE, CTYPE, AGE, CM                      ; CODE=
JAPAN
; NAME=NAME, CODE, ELAST, HU, VOL, CC, WC, AGE                               ; CODE=
JSCE
; NAME=NAME, CODE, STR, HTYPE, HU, MSIZE, AGE                                ; CODE=
CHINA
; NAME=NAME, CODE, STR, HU, MSIZE, BSC, AGE                                ; CODE=
JTG
; NAME=NAME, CODE, STR, HU, VSR, AGE, bEXPOSE                             ; CODE=
AASHTO
; NAME=NAME, CODE, STR, HU, MSIZE, AGE                                     ; CODE=
INDIA(IRC:18-2000)
; NAME=NAME, CODE, STR, HU, MSIZE, CTYPE, AGE                            ; CODE=
INDIA(IRC:112-2011)
; NAME=NAME, CODE, STR, HU, MSIZE, CTYPE, AGE, TCode, bSILICA             ; CODE=
European
; NAME=NAME, CODE, STR, EE(Not Use), FS, HT, DSE, DSC, AGE                 ; CODE=
NZ Bridge(SP/M/022)
; NAME=NAME, CODE, STR, HU, AGE, M, CMETH, CTYPE, CREEP, CONCT, W, MAXS, A, PZ ; CODE=
Russian
; NAME=NAME, CODE, STR, HU, MSIZE, BSC, AGE, FLYASH                         ; CODE=
China(JTG D62-2016)
; NAME=NAME, CODE, STR, EE, HT, DSC, AGE                                    ; CODE=
Australia
; NAME=NAME, CODE, bSSF, SSFNAME                                         ; CODE=
USER(line1)

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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; CREEPFUNC1, AGE1, CREEPFUNC2, AGE2, ... ; USER(
from line 2)
; [ACI1] : CURE, SLUMP, FAP, AIR, CC
; [ACI2] : UCC, USS
NAME=C+S, European, 35000, 70, 0.25, Class N, 3, 1, NO

*TDM-ELAST ; Time Dependent Material (Comp. Strength)
; NAME=NAME, TYPE, CODE, STRENGTH, A, B ; TYPE=CODE (Korean Standard, ACI)
; NAME=NAME, TYPE, CODE, STRENGTH, iCTYPE ; TYPE=CODE (CEB-FIP, Ohzagi)
; NAME=NAME, TYPE, CODE, STRENGTH ; TYPE=CODE (Chinese Standard)
; NAME=NAME, TYPE, CODE, STRENGTH, bUSE, [DATA] ; TYPE=CODE (Japan (Hydratation))
; NAME=NAME, TYPE, CODE, STRENGTH, iTYPE ; TYPE=CODE (Japan (Elastic))
; NAME=NAME, TYPE, CODE, STRENGTH ; TYPE=CODE (INDIA (IRC :18-2000))
; NAME=NAME, TYPE, CODE, STRENGTH, iCTYPE ; TYPE=CODE (INDIA (IRC :112-2011))
; NAME=NAME, TYPE, CODE, STRENGTH, iCTYPE ; TYPE=CODE (European)
; NAME=NAME, TYPE, CODE, STRENGTH, ; TYPE=CODE (CEB-FIP (1978))
; NAME=NAME, TYPE, CODE, STRENGTH, TYPE, CMETH, CTYPE, MAXS, PZ ; TYPE=CODE (Russian))
; NAME=NAME, TYPE, CODE, STRENGTH, ; CODE=Australia
; NAME=NAME, TYPE, SCALE ; TYPE=USER (line 1)
; DAY1, COMP1, TENS1, ELAST1, DAY2, COMP2, ... ; USER (from line 2)
; [DATA] : A, B, D, TSF or iCTYPE, TSF
NAME=MATURAZIONE, CODE, EUROPEAN, 43000, 2

*TDM-LINK ; Time Dependent Material Link
; iMAT, TDM-TYPE1(CREEP/SHRINKAGE), TDM-TYPE2(ELASTICITY)
2, C+S, MATURAZIONE

*SECTION ; Section
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, [DATA1], [DATA2] ; 1st line - DB/USER
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, BLT, D1, ..., D8, iCEL ; 1st line - VALUE
; AREA, ASy, ASz, Ixx, Iyy, Izz ; 2nd line
; CyP, CyM, CzP, CzM, QyB, QzB, PERI_OUT, PERI_IN, Cy, Cz ; 3rd line
; Y1, Y2, Y3, Y4, Z1, Z2, Z3, Z4, Zyy, Zzz ; 4th line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, ELAST, DEN, POIS, POIC, SF, THERMAL ; 1st line - SRC
; D1, D2, [SRC] ; 2nd line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 1, DB, NAME1, NAME2, D1, D2 ; 1st line - COMBINED
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 2, D11, D12, D13, D14, D15, D21, D22, D23, D24
; iSEC, TYPE, SNAME, [OFFSET2], bSD, bWE, SHAPE, iyVAR, izVAR, STYPE ; 1st line - TAPERED
; DB, NAME1, NAME2 ; 2nd line (STYPE=DB)

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

```

;      [DIM1], [DIM2]
2nd line(STYPE=USER) ;  

;      D11, D12, D13, D14, D15, D16, D17, D18  

2nd line(STYPE=VALUE)  

;      AREA1, ASy1, ASz1, Ixx1, Iyy1, Izz1  

3rd line(STYPE=VALUE)  

;      CyP1, CyM1, CzP1, CzM1, QyB1, QzB1, PERI_OUT1, PERI_IN1, Cy1, Cz1  

4th line(STYPE=VALUE)  

;      Y11, Y12, Y13, Y14, Z11, Z12, Z13, Z14, Zyy1, Zyy2  

5th line(STYPE=VALUE)  

;      D21, D22, D23, D24, D25, D26, D27, D28  

6th line(STYPE=VALUE)  

;      AREA2, ASy2, ASz2, Ixx2, Iyy2, Izz2  

7th line(STYPE=VALUE)  

;      CyP2, CyM2, CzP2, CzM2, QyB2, QzB2, PERI_OUT2, PERI_IN2, Cy2, Cz2  

8th line(STYPE=VALUE)  

;      Y21, Y22, Y23, Y24, Z21, Z22, Z23, Z24, Zyy2, Zzz2  

9th line(STYPE=VALUE)  

;      OPT1, OPT2, [JOINT] ; 2nd l  

ine(STYPE=PSC)  

;      ELAST, DEN, POIS, POIC, THERMAL ; 2nd l  

ine(STYPE=PSC-CMPW)  

;      bSEARCHK, [SCHK-I], [SCHK-J], [WT-I], [WT-J], WI, WJ, bSYM, bSIDEHOLE ; 3rd l  

ine(STYPE=PSC)  

;      bSEARCHK, bSYM, bHUNCH, [CMPWEB-I], [CMPWEB-J] ; 3rd l  

ine(STYPE=PSC-CMPW)  

;      bUSERDEFMESHSIZE, MESHSIZE, bUSERINPSTIFF, [STIFF-I], [STIFF-J] ; 4th l  

ine(STYPE=PSC)  

;      [SIZE-A]-i ; 5th l  

ine(STYPE=PSC)  

;      [SIZE-B]-i ; 6th l  

ine(STYPE=PSC)  

;      [SIZE-C]-i ; 7th l  

ine(STYPE=PSC)  

;      [SIZE-D]-i ; 8th l  

ine(STYPE=PSC)  

;      [SIZE-A]-j ; 9th l  

ine(STYPE=PSC)  

;      [SIZE-B]-j ; 10th  

line(STYPE=PSC)  

;      [SIZE-C]-j ; 11th  

line(STYPE=PSC)  

;      [SIZE-D]-j ; 12th  

line(STYPE=PSC)  

;      GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMULTI, EsEc-L, EsEc-S ; 2nd l  

ine(STYPE=CMP-B/I)  

;      SW_i, Hw_i, tw_i, B_i, Bf1_i, tf1_i, B2_i, Bf2_i, tf2_i ; 3rd l  

ine(STYPE=CMP-B/I)  

;      SW_j, Hw_j, tw_j, B_j, Bf1_j, tf1_j, B2_j, Bf2_j, tf2_j ; 4th l  

ine(STYPE=CMP-B/I)  

;      N1, N2, Hr, Hr2, tr1, tr2 ; 5th l  

ine(STYPE=CMP-B)  

;      GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb, bSYM, SW_i, SW_j ; 2nd l  

ine(STYPE=CMP-CI/CT)  

;      OPT1, OPT2, [JOINT] ; 3rd l  

ine(STYPE=CMP-CI/CT)  

;      [SIZE-A]-i ; 4th l  

ine(STYPE=CMP-CI/CT)  

;      [SIZE-B]-i ; 5th l  

ine(STYPE=CMP-CI/CT)  

;      [SIZE-C]-i ; 6th l  

ine(STYPE=CMP-CI/CT)

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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;      [SIZE-D]-i ; 7th l
ine(STYPE=CMP-CI/CT)
;      [SIZE-A]-j ; 8th l
ine(STYPE=CMP-CI/CT)
;      [SIZE-B]-j ; 9th l
ine(STYPE=CMP-CI/CT)
;      [SIZE-C]-j ; 10th
line(STYPE=CMP-CI/CT)
;      [SIZE-D]-j ; 11th
line(STYPE=CMP-CI/CT)
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, STYPE1, STYPE2 ; 1st l
ine - CONSTRUCT
;      SHAPE, ... (same with other type data from shape) ; Before
e (STYPE1)
;      SHAPE, ... (same with other type data from shape) ; After
(STYPE2)
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st l
ine - COMPOSITE-SB
;      Hw, tw, B, Bf1, tf1, B2, Bf2, tf2 ; 2nd l
ine
;      N1, N2, Hr, Hr2, tr1, tr2 ; 3rd l
ine
;      SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 4th l
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st l
ine - COMPOSITE-SI
;      Hw, tw, B, tf1, B2, tf2 ; 2nd l
ine
;      SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 3rd l
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st l
ine - COMPOSITE-CI/CT
;      OPT1, OPT2, [JOINT] ; 2nd l
ine
;      [SIZE-A] ; 3rd l
ine
;      [SIZE-B] ; 4th l
ine
;      [SIZE-C] ; 5th l
ine
;      [SIZE-D] ; 6th l
ine
;      SW, GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb ; 7th l
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st l
ine - PSC
;      OPT1, OPT2, [JOINT] ; 2nd l
ine
;      bSHEARCHK, [SCHK], [WT], WIDTH, bSYM, bSIDEHOLE ; 3rd l
ine
;      bUSERDEFMESHSIZE, MESHSIZE, bUSERINPSTIFF, [STIFF] ; 4th l
ine
;      bWE, [WARPING POINT]-i, [WARPING POINT]-j ; 5th l
ine
;      [SIZE-A] ; 6th l
ine
;      [SIZE-B] ; 7th l
ine
;      [SIZE-C] ; 8th l
ine
;      [SIZE-D] ; 9th l
ine

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# MIDAS

PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			

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; [DATA1] : 1, DB, NAME or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10
; [DATA2] : CCSHAPE or iCEL or iN1, iN2
; [SRC] : 1, DB, NAME1, NAME2 or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10, iN1, iN2

; [DIM1], [DIM2] : D1, D2, D3, D4, D5, D6, D7, D8
; [OFFSET] : OFFSET, iCENT, iREF, iHORZ, HUSER, iVERT, VUSER
; [OFFSET2] : OFFSET, iCENT, iREF, iHORZ, HUSERI, HUSERJ, iVERT, VUSERI, VUSERJ
; [JOINT] : 8(1CELL, 2CELL), 13(3CELL), 9(PSCM), 8(PSCH), 9(PSCT), 2(PSCB), 0(nCE
LL), 2(nCEL2)
; [SIZE-A] : 6(1CELL, 2CELL), 10(3CELL), 10(PSCM), 6(PSCH), 8(PSCT), 10(PSCB), 5(nCE
LL), 11(nCEL2)
; [SIZE-B] : 6(1CELL, 2CELL), 12(3CELL), 6(PSCM), 6(PSCH), 8(PSCT), 6(PSCB), 8(nCE
LL), 18(nCEL2)
; [SIZE-C] : 10(1CELL, 2CELL), 13(3CELL), 9(PSCM), 10(PSCH), 7(PSCT), 8(PSCB), 0(nCE
LL), 11(nCEL2)
; [SIZE-D] : 8(1CELL, 2CELL), 13(3CELL), 6(PSCM), 7(PSCH), 8(PSCT), 5(PSCB), 0(nCE
LL), 18(nCEL2)
; [STIFF] : AREA, ASy, ASz, Ixx, Iyy, Izz
; [SCHK] : bAUTO_Z1, Z1, bAUTO_Z3, Z3
; [WT] : bAUTO_TOR, TOR, bAUTO_SHR1, SHR1, bAUTO_SHR2, SHR2, bAUTO_SHR3, SHR3
; [CMPWEB] : EFD, LRF, A, B, H, T
; [WARPING POINT] : nWarpingCheck, X1,X2,X3,X4,X5,X6, Y1,Y2,Y3,Y4,Y5,Y6
    1, COMPOSITE , TRAVE PRINCIPALE CAMPATA, CT, 0, 0, 0, 0, 0, 0, YES, NO, I
        2.13, 0.022, 0.7, 0.03, 1.2, 0.04
        0, 0, 0, 0, 0
        0
        0
        0
        2.85, 1, 2.85, 2.85, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    2, COMPOSITE , TRAVE PRINCIPALE APPOGGIO, CT, 0, 0, 0, 0, 0, 0, YES, NO, I
        2.13, 0.022, 0.7, 0.03, 0.9, 0.04
        0, 0, 0, 0, 0
        0
        0
        0
        2.85, 1, 2.85, 2.85, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    3, COMPOSITE , TRAVE SPINA , CT, 0, 0, 0, 0, 0, 0, YES, NO, I
        0.54, 0.013, 0.3, 0.025, 0.3, 0.025
        0, 0, 0, 0, 0
        0
        0
        0
        2.8, 1, 2.8, 2.8, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    4, DBUSER , TRAVERSO , CC, 0, 0, 0, 0, 0, 0, YES, NO, H , 2, 0.8, 0.3
    5, 0.02, 0.028, 0.35, 0.028, 0, 0, 0, 0
    5, DBUSER , CONTROVENTI , CC, 0, 0, 0, 0, 0, 0, YES, NO, L , 1, UNI, L70
x7
    6, DBUSER , SOLETTA , CC, 0, 0, 0, 0, 0, 0, YES, NO, SB , 2, 0.25, 0.
88, 0, 0, 0, 0, 0, 0, 0, 0
    7, COMPOSITE , CONCIO C1 , CT, 0, 0, 0, 0, 0, 0, YES, NO, I
        2.045, 0.02, 0.75, 0.025, 0.9, 0.03
        0, 0, 0, 0, 0
        0
        0
        0
        2.84, 1, 2.84, 2.84, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    8, COMPOSITE , CONCIO C2 , CT, 0, 0, 0, 0, 0, 0, YES, NO, I
        2.032, 0.02, 0.75, 0.028, 1.2, 0.04
        0, 0, 0, 0, 0
        0
        0
        0

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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2.84, 1, 2.84, 2.84, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
9, COMPOSITE, CONCIO C3, CT, 0, 0, 0, 0, 0, YES, NO, I
2.015, 0.02, 0.75, 0.03, 1.2, 0.055
0, 0, 0, 0, 0
0
0
0
2.84, 1, 2.84, 2.84, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
*SECT-COLOR
; iSEC, W_R, W_G, W_B, HF_R, HF_G, HF_B, HE_R, HE_G, HE_B, bBLEND, FACT
1, 0, 33, 66, 0, 128, 255, 0, 33, 66, NO, 0.5
2, 31, 0, 81, 148, 87, 255, 31, 0, 81, NO, 0.5
3, 108, 54, 0, 255, 128, 0, 108, 54, 0, NO, 0.5
4, 0, 66, 45, 0, 192, 128, 0, 66, 45, NO, 0.5
5, 65, 65, 65, 255, 255, 255, 65, 65, 65, NO, 0.5
6, 100, 0, 100, 192, 0, 192, 100, 0, 100, NO, 0.5
7, 85, 0, 57, 192, 0, 128, 85, 0, 57, NO, 0.5
8, 0, 45, 66, 0, 128, 192, 0, 45, 66, NO, 0.5
9, 96, 0, 96, 255, 160, 255, 96, 0, 96, NO, 0.5

*COMP-GEN-SECT-PSC-DESIGN ; Composite Section for PSC Design
; SECT, bCompPSC, (Z1, Z2, Z3, t1, t2, t3, TotT)-I, (Z1, Z2, Z3, t1, t2, t3, TotT)-J
1, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
2, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
3, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
7, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
8, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
9, NO, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0

*DGN-SECT
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, [DATA1], [DATA2] ;
1st line - DB/USER
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, BLT, D1, ..., D8, iCEL ;
1st line - VALUE
; AREA, ASy, ASz, Ixx, Iyy, Izz ;
2nd line
; CyP, CyM, CzP, CzM, QyB, QzB, PERI_OUT, PERI_IN, Cy, Cz ;
3rd line
; Y1, Y2, Y3, Y4, Z1, Z2, Z3, Z4, Zyy, Zzz ;
4th line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, ELAST, DEN, POIS, POIC, SF, THERMAL ;
1st line - SRC
; D1, D2, [SRC] ;
2nd line
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 1, DB, NAME1, NAME2, D1, D2 ;
1st line - COMBINED
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE, 2, D11, D12, D13, D14, D15, D21, D22, D23, D24
; iSEC, TYPE, SNAME, [OFFSET2], bSD, bWE, SHAPE, iyVAR, izVAR, STYPE ;
1st line - TAPERED
; DB, NAME1, NAME2 ;
2nd line(STYPE=DB)
; [DIM1], [DIM2] ;
2nd line(STYPE=USER)
; D11, D12, D13, D14, D15, D16, D17, D18 ;
2nd line(STYPE=VALUE)
; AREA1, ASy1, ASz1, Ixx1, Iyy1, Izz1 ;
3rd line(STYPE=VALUE)
; CyP1, CyM1, CzP1, CzM1, QyB1, QzB1, PERI_OUT1, PERI_IN1, Cy1, Cz1 ;
4th line(STYPE=VALUE)
; Y11, Y12, Y13, Y14, Z11, Z12, Z13, Z14, Zyy1, Zyy2 ;

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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5th line(STYPE=VALUE)
;          D21, D22, D23, D24, D25, D26, D27, D28 ; 
6th line(STYPE=VALUE)
;          AREA2, ASy2, ASz2, Ixx2, Iyy2, Izz2 ; 
7th line(STYPE=VALUE)
;          CyP2, CyM2, CzP2, CzM2, QyB2, QzB2, PERI_OUT2, PERI_IN2, Cy2, Cz2 ; 
8th line(STYPE=VALUE)
;          Y21, Y22, Y23, Y24, Z21, Z22, Z23, Z24, Zyy2, Zzz2 ; 
9th line(STYPE=VALUE)
;          OPT1, OPT2, [JOINT] ; 2nd l
ine(STYPE=PSC)
;          ELAST, DEN, POIS, POIC, THERMAL ; 2nd l
ine(STYPE=PSC-CMPW)
;          bSEARCHK, [SCHK-I], [SCHK-J], [WT-I], [WT-J], WI, WJ, bSYM, bSIDEHOLE ; 3rd l
ine(STYPE=PSC)
;          bSEARCHK, bSYM, bHUNCH, [CMPWEB-I], [CMPWEB-J] ; 3rd l
ine(STYPE=PSC-CMPW)
;          bUSERDEFMESHSIZE, MESHSIZE, bUSERINPSTIFF, [STIFF-I], [STIFF-J] ; 4th l
ine(STYPE=PSC)
;          [SIZE-A]-i ; 5th l
ine(STYPE=PSC)
;          [SIZE-B]-i ; 6th l
ine(STYPE=PSC)
;          [SIZE-C]-i ; 7th l
ine(STYPE=PSC)
;          [SIZE-D]-i ; 8th l
ine(STYPE=PSC)
;          [SIZE-A]-j ; 9th l
ine(STYPE=PSC)
;          [SIZE-B]-j ; 10th
line(STYPE=PSC)
;          [SIZE-C]-j ; 11th
line(STYPE=PSC)
;          [SIZE-D]-j ; 12th
line(STYPE=PSC)
;          GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMULTI, EsEc-L, EsEc-S ; 2nd l
ine(STYPE=CMP-B/I)
;          SW_i, Hw_i, tw_i, B_i, Bf1_i, tf1_i, B2_i, Bf2_i, tf2_i ; 3rd l
ine(STYPE=CMP-B/I)
;          SW_j, Hw_j, tw_j, B_j, Bf1_j, tf1_j, B2_j, Bf2_j, tf2_j ; 4th l
ine(STYPE=CMP-B/I)
;          N1, N2, Hr, Hr2, tr1, tr2 ; 5th l
ine(STYPE=CMP-B)
;          GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb, bSYM, SW_i, SW_j ; 2nd l
ine(STYPE=CMP-CI/CT)
;          OPT1, OPT2, [JOINT] ; 3rd l
ine(STYPE=CMP-CI/CT)
;          [SIZE-A]-i ; 4th l
ine(STYPE=CMP-CI/CT)
;          [SIZE-B]-i ; 5th l
ine(STYPE=CMP-CI/CT)
;          [SIZE-C]-i ; 6th l
ine(STYPE=CMP-CI/CT)
;          [SIZE-D]-i ; 7th l
ine(STYPE=CMP-CI/CT)
;          [SIZE-A]-j ; 8th l
ine(STYPE=CMP-CI/CT)
;          [SIZE-B]-j ; 9th l
ine(STYPE=CMP-CI/CT)
;          [SIZE-C]-j ; 10th
line(STYPE=CMP-CI/CT)
;          [SIZE-D]-j ; 11th

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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line(STYPE=CMP-CI/CT)
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, STYPE1, STYPE2 ; 1st 1
ine - CONSTRUCT
;      SHAPE, ... (same with other type data from shape) ; Before
e (STYPE1)
;      SHAPE, ... (same with other type data from shape) ; After
(STYPE2)
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st 1
ine - COMPOSITE-SB
;      Hw, tw, B, Bf1, tf1, B2, Bf2, tf2 ; 2nd 1
ine
;      N1, N2, Hr, Hr2, tr1, tr2 ; 3rd 1
ine
;      SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 4th 1
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st 1
ine - COMPOSITE-SI
;      Hw, tw, B, tf1, B2, tf2 ; 2nd 1
ine
;      SW, GN, CTC, Bc, Tc, Hh, EsEc, DsDc, Ps, Pc, bMulti, Elong, Esh ; 3rd 1
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st 1
ine - COMPOSITE-CI/CT
;      OPT1, OPT2, [JOINT] ; 2nd 1
ine
;      [SIZE-A] ; 3rd 1
ine
;      [SIZE-B] ; 4th 1
ine
;      [SIZE-C] ; 5th 1
ine
;      [SIZE-D] ; 6th 1
ine
;      SW, GN, CTC, Bc, Tc, Hh, EgdEsb, DgdDsb, Pgd, Psb ; 7th 1
ine
; iSEC, TYPE, SNAME, [OFFSET], bSD, bWE, SHAPE ; 1st 1
ine - PSC
;      OPT1, OPT2, [JOINT] ; 2nd 1
ine
;      bSHEARCHK, [SCHK], [WT], WIDTH, bSYM, bSIDEHOLE ; 3rd 1
ine
;      bUSERDEFMESHSIZE, MESHSIZE, bUSERINPSTIFF, [STIFF] ; 4th 1
ine
;      bWE, [WARPING POINT]-i, [WARPING POINT]-j ; 5th 1
ine
;      [SIZE-A] ; 6th 1
ine
;      [SIZE-B] ; 7th 1
ine
;      [SIZE-C] ; 8th 1
ine
;      [SIZE-D] ; 9th 1
ine
; [DATA1] : 1, DB, NAME or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10
; [DATA2] : CCSHAPE or iCEL or iN1, iN2
; [SRC]   : 1, DB, NAME1, NAME2 or 2, D1, D2, D3, D4, D5, D6, D7, D8, D9, D10, iN1, iN2

; [DIM1], [DIM2] : D1, D2, D3, D4, D5, D6, D7, D8
; [OFFSET] : OFFSET, iCENT, iREF, iHORZ, HUSER, iVERT, VUSER
; [OFFSET2] : OFFSET, iCENT, iREF, iHORZ, HUSERI, HUSERJ, iVERT, VUSERI, VUSERJ
; [JOINT]   : 8(1CELL, 2CELL), 13(3CELL), 9(PSCM), 8(PSCH), 9(PSCT), 2(PSCB), 0(nCELL), 2(nCEL2)

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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; [SIZE-A] : 6(1CELL, 2CELL), 10(3CELL), 10(PSCM), 6(PSCH), 8(PSCT), 10(PSCB), 5(nCE
LL), 11(nCEL2)
; [SIZE-B] : 6(1CELL, 2CELL), 12(3CELL), 6(PSCM), 6(PSCH), 8(PSCT), 6(PSCB), 8(nCE
LL), 18(nCEL2)
; [SIZE-C] : 10(1CELL, 2CELL), 13(3CELL), 9(PSCM), 10(PSCH), 7(PSCT), 8(PSCB), 0(nCE
LL), 11(nCEL2)
; [SIZE-D] : 8(1CELL, 2CELL), 13(3CELL), 6(PSCM), 7(PSCH), 8(PSCT), 5(PSCB), 0(nCE
LL), 18(nCEL2)
; [STIFF] : AREA, ASy, ASz, Ixx, Iyy, Iz
; [SCHK] : bAUTO_Z1, Z1, bAUTO_Z3, Z3
; [WT] : bAUTO_TOR, TOR, bAUTO_SHR1, SHR1, bAUTO_SHR2, SHR2, bAUTO_SHR3, SHR3
; [CMPWEB] : EFD, LRF, A, B, H, T
; [WARPING POINT] : nWarpingCheck, X1,X2,X3,X4,X5,X6, Y1,Y2,Y3,Y4,Y5,Y6
    1, COMPOSITE , TRAVE PRINCIPALE CAMPATA, CT, 0, 0, 0, 0, 0, YES, NO, I
        2.13, 0.022, 0.7, 0.03, 1.2, 0.04
        0, 0, 0, 0, 0
        0
        0
        0
        2.85, 1, 2.85, 2.85, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    2, COMPOSITE , TRAVE PRINCIPALE APPOGGIO, CT, 0, 0, 0, 0, 0, YES, NO, I
        2.13, 0.022, 0.7, 0.03, 0.9, 0.04
        0, 0, 0, 0, 0
        0
        0
        0
        2.85, 1, 2.85, 2.85, 0.25, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    3, COMPOSITE , TRAVE SPINA , CT, 0, 0, 0, 0, 0, YES, NO, I
        0.54, 0.013, 0.3, 0.025, 0.3, 0.025
        0, 0, 0, 0, 0
        0
        0
        0
        2.8, 1, 2.8, 2.8, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    4, DBUSER , TRAVERSO , CC, 0, 0, 0, 0, 0, YES, NO, H , 2, 0.8, 0.3
    5, 0.02, 0.028, 0.35, 0.028, 0, 0, 0, 0
        5, DBUSER , CONTROVENTI , CC, 0, 0, 0, 0, 0, YES, NO, L , 1, UNI, L70
    x7
        6, DBUSER , SOLETTA , CC, 0, 0, 0, 0, 0, YES, NO, SB , 2, 0.25, 0.
    88, 0, 0, 0, 0, 0, 0, 0
        7, COMPOSITE , CONCIO C1 , CT, 0, 0, 0, 0, 0, YES, NO, I
            2.045, 0.02, 0.75, 0.025, 0.9, 0.03
            0, 0, 0, 0, 0
            0
            0
            0
            2.84, 1, 2.84, 2.84, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    8, COMPOSITE , CONCIO C2 , CT, 0, 0, 0, 0, 0, YES, NO, I
        2.032, 0.02, 0.75, 0.028, 1.2, 0.04
        0, 0, 0, 0, 0
        0
        0
        0
        2.84, 1, 2.84, 2.84, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,
    9, COMPOSITE , CONCIO C3 , CT, 0, 0, 0, 0, 0, YES, NO, I
        2.015, 0.02, 0.75, 0.03, 1.2, 0.055
        0, 0, 0, 0, 0
        0
        0
        0
        2.84, 1, 2.84, 2.84, 0.26, 0, 6.16251, 3.0792, 0.3, 0.2, 1.2, NO, ,

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

```
*STLDCASE ; Static Load Cases
; LCNAME, LCTYPE, DESC
PESO PROPRIO ACCIAIO, CS, PESO PROPRIO ACCIAIO
PESO PROPRIO SOLETTA, CS, PESO PROPRIO SOLETTA
PERMANENTE PORTATO, CS, PERMANENTE PORTATO
VENTO A PONTE SCARICO, USER, VENTO A PONTE CARICO
VENTO A PONTE CARICO, USER, VENTO A PONTE CARICO
TERMICA UNIFORME +, T , TERMICA UNIFORME +
TERMICA UNIFORME -, T , TERMICA UNIFORME -
TERMICA GRADIENTE +, TPG, TERMICA GRADIENTE +
TERMICA GRADIENTE -, TPG, TERMICA GRADIENTE -
FRENATURA, USER, FRENATURA

*SPRING ; Point Spring Supports
; NODE_LIST, Type, SDx, SDy, SDz, SRx, SRY, SRz, GROUP, FROMTYPE, EFFAREA, Kx, Ky, Kz
; LINEAR
; NODE_LIST, Type, Direction, Vx, Vy, Vz, Stiffness, GROUP, FROMTYPE, EFFAREA
; COMP, TENS
; NODE_LIST, Type, Multi-Linear Type, Direction, Vx, Vy, Vz, ax, ay, bx, by, cx, cy, dx,
; dy, ex, ey, fx, fy, GROUP, FROMTYPE, EFFAREA ; MULTI
    1to4, LINEAR, 4490, 4490, 4.33e+006, 0, 0, 0, NO, 0, 0, 0, 0, 0, 0, 0, FASE 1, 0, 0, 0
, 0, 0

*RIGIDLINK ; Rigid Link
; KEY, M-NODE, DOF, S-NODE LIST, GROUP
    2,      1, 111111, 5 89, FASE 1
    1,      2, 111111, 27 191, FASE 1
    3,      3, 111111, 15 139, FASE 1
    4,      4, 111111, 37 241, FASE 1
    8,      6, 111111, 94, FASE 1
   11,      7, 111111, 99, FASE 1
   14,      8, 111111, 104, FASE 1
   17,      9, 111111, 109, FASE 1
   20,     10, 111111, 114, FASE 1
   23,     11, 111111, 119, FASE 1
   26,     12, 111111, 124, FASE 1
   29,     13, 111111, 129, FASE 1
   32,     14, 111111, 134, FASE 1
   33,     16, 111111, 140, FASE 1
    7,     17, 111111, 145, FASE 1
   10,     18, 111111, 150, FASE 1
   13,     19, 111111, 155, FASE 1
   16,     20, 111111, 160, FASE 1
   19,     21, 111111, 165, FASE 1
   22,     22, 111111, 170, FASE 1
   25,     23, 111111, 175, FASE 1
   28,     24, 111111, 180, FASE 1
   31,     25, 111111, 185, FASE 1
    5,     26, 111111, 190, FASE 1
    6,     28, 111111, 196, FASE 1
    9,     29, 111111, 201, FASE 1
   12,     30, 111111, 206, FASE 1
   15,     31, 111111, 211, FASE 1
   18,     32, 111111, 216, FASE 1
   21,     33, 111111, 221, FASE 1
   24,     34, 111111, 226, FASE 1
   27,     35, 111111, 231, FASE 1
   30,     36, 111111, 236, FASE 1

; *LOADTOMASS, DIR, bNODAL, bBEAM, bFLOOR, bPRES, GRAV
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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; LCNAME1, FACTOR1, LCNAME2, FACTOR2, ... ; from line 1
*LOADTOMASS, XYZ, YES, YES, YES, YES, 9.806
    PESO PROPRIO SOLETTA, 1, PERMANENTE PORTATO, 1

*USE-STLD, PESO PROPRIO ACCIAIO

; *SELFWEIGHT, X, Y, Z, GROUP
*SELFWEIGHT, 0, 0, -1.12, FASE 1

; End of data for load case [PESO PROPRIO ACCIAIO] ----

*USE-STLD, PESO PROPRIO SOLETTA

*BEAMLOAD ; Element Beam Loads
; ELEM_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM_LIST, CMD, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE] : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
    57, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    58, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    59, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -16.85, 1, -16.85, 0, 0, 0,
, FASE 2, NO, 0, 0, NO,
    62, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    63, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    64, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    65, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    66, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    67, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    68, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    69, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    70, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    71, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    72, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    73, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    74, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    75, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    76, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    77, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    78, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    79, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
    80, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , 0, -16.85, 1, -16.85, 0, 0, 0

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

```
, FASE 2, NO, 0, 0, NO,
  81, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  82, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  83, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  84, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  85, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  86, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  87, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  88, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  110, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  112, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  114, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  117, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  119, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  121, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  124, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  126, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  128, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  131, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  133, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  135, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  142, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  144, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  146, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  149, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  151, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  153, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  156, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  158, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
, FASE 2, NO, 0, 0, NO,
  160, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  163, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0, 0
0, FASE 2, NO, 0, 0, NO,
  165, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -16.85, 1, -16.85, 0, 0, 0, 0
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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, FASE 2, NO, 0, 0, NO,
167, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
174, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -22.013, 1, -22.013, 0, 0, 0,
0, FASE 2, NO, 0, 0, NO,
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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# MIDAS

PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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; End of data for load case [PESO PROPRIO SOLETTA] -----

\*USE-STLD, PERMANENTE PORTATO

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*BEAMLOAD ; Element Beam Loads
; ELEM LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM LIST, CMD, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE] : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
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MIDAS

**PROJECT TITLE :**

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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183, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -8.566, 1, -8.566, 0, 0, 0, 0
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185, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -6.957, 1, -6.957, 0, 0, 0, 0
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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249, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -6.957, 1, -6.957, 0, 0, 0, 0
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327, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -6.957, 1, -6.957, 0, 0, 0, 0
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# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2
,	334, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -6.957, 1, -6.957, 0, 0, 0, 0			
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,	338, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -6.957, 1, -6.957, 0, 0, 0, 0			
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,	370, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -6.957, 1, -6.957, 0, 0, 0, 0			
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,	412, BEAM , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -6.957, 1, -6.957, 0, 0, 0, 0			
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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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        414, BEAM      , UNILOAD, GZ, NO , NO, aDir[1], , , , 0, -8.566, 1, -8.566, 0, 0, 0, 0
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; End of data for load case [PERMANENTE PORTATO] -----

\*USE-STLD, VENTO A PONTE SCARICO

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*BEAMLOAD    ; Element Beam Loads
; ELEM_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM_LIST, CMD, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE]      : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN]      : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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    163, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.789, 1, -0.789, 0, 0, 0, 0
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# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			

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167, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.789, 1, 0.789, 0, 0, 0, 0,
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167, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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174, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.789, 1, -0.789, 0, 0, 0, 0
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178, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.789, 1, 0.789, 0, 0, 0, 0
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178, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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181, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.789, 1, -0.789, 0, 0, 0, 0
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185, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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185, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.789, 1, 0.789, 0, 0, 0, 0
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224, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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242, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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242, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.789, 1, 0.789, 0, 0, 0, 0
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249, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.789, 1, 0.789, 0, 0, 0, 0,
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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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249, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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256, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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327, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , 0, 0.789, 1, 0.789, 0, 0, 0, 0,
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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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 338, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.789, 1, 0.789, 0, 0, 0, 0
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 377, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -4.507, 1, -4.507, 0, 0, 0, 0
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 412, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, -0.789, 1, -0.789, 0, 0, 0, 0
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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; End of data for load case [VENTO A PONTE SCARICO] -----

\*USE-STLD, VENTO A PONTE CARICO

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*BEAMLOAD ; Element Beam Loads
; ELEM_LIST, CMD, TYPE, DIR, bPROJ, [ECCEN], [VALUE], GROUP
; ELEM_LIST, CMD, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE] : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
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    58, BEAM , UNILOAD, LZ, NO , NO, aDir[1], , , , 0, 0.406, 1, 0.406, 0, 0, 0, 0
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			

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423, BEAM , UNILOAD, LY, NO , NO, aDir[1], , , , 0, -10.142, 1, -10.142, 0, 0, 0,  
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; End of data for load case [VENTO A PONTE CARICO] -----

\*USE-STLD, TERMICA UNIFORME +

\*ELTEMPER ; Element Temperatures

; ELEM\_LIST, TEMPER, GROUP

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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412, 49,

# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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423, 49,

; End of data for load case [TERMICA UNIFORME +] -----

\*USE-STLD, TERMICA UNIFORME -

\*ELTEMPER ; Element Temperatures  
; ELEM\_LIST, TEMPER, GROUP

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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423, -11,
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; End of data for load case [TERMICA UNIFORME -] -----

# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

\*USE-STLD, TERMICA GRADIENTE +

```
*THERGRAD ; Temperature Gradient
; ELEM_LIST, iETYP, TZ, bUSEHZ, HZ, TY, bUSEHY, HY, GROUP
; ELEM_LIST, iETYP, TZ, bUSEHZ, HZ, GROUP
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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

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# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			

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; End of data for load case [TERMICA GRADIENTE +] -----

\*USE-STLD, TERMICA GRADIENTE -

\*THERGRAD ; Temperature Gradient  
; ELEM\_LIST, iETYP, TZ, bUSEHZ, HZ, TY, bUSEHY, HY, GROUP  
; ELEM\_LIST, iETYP, TZ, bUSEHZ, HZ, GROUP

# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			

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# MIDAS

PROJECT TITLE :

	Company		Client	File Name
	Author			

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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; End of data for load case [TERMICA GRADIENTE -] -----

\*USE-STLD, FRENATURA

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; ELEM_LIST, CMD, TYPE, DIR, VX, VY, VZ, bPROJ, [ECCEN], [VALUE], GROUP
; [VALUE] : D1, P1, D2, P2, D3, P3, D4, P4
; [ECCEN] : bECCEN, ECCDIR, I-END, J-END, bJ-END
; [ADDITIONAL] : bADDITIONAL, ADDITIONAL_I-END, ADDITIONAL_J-END, bADDITIONAL_J-END
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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270, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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272, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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274, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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277, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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279, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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, NO, 0, 0, NO,
281, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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284, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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286, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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288, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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291, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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293, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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295, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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302, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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304, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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306, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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309, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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311, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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313, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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316, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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318, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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320, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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323, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
325, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
327, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
334, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
336, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
338, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
341, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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343, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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345, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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350, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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352, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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355, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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357, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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359, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
```

# MIDAS

PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			
				L2-CV02_REVISIONE_2

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, NO, 0, 0, NO,
 366, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 368, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 370, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 373, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 375, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 382, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 384, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 387, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
 389, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
 391, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 400, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 407, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 409, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 412, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 414, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 421, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
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 423, BEAM , UNILOAD, GX, NO , NO, aDir[1], , , , 0, 3.537, 1, 3.537, 0, 0, 0, 0,
, NO, 0, 0, NO,
; End of data for load case [FRENATURA] -----
```

```
*STAGE ; Define Construction Stage
; NAME=NAME, DURATION, bSAVESTAGE, bSAVESTEP, bINCRESTEP, INCRESTEP ; line 1
; STEP=DAY1, DAY2, ... ; line 2
; AELEM=GROUP1, AGE1, GROUP2, AGE2, ... ; line 3
; DELEM=GROUP1, REDIST1, GROUP2, REDIST2, ... ; line 4
; ABNDR=BGROUP1, POS1, BGROUP2, POS2, ... ; line 5
; DBNDR=BGROUP1, BGROUP2, ... ; line 6
; ALOAD=LGROUP1, DAY1, LGROUP2, DAY2, ... ; line 7
; DLOAD=LGROUP1, DAY1, LGROUP2, DAY2, ... ; line 8
NAME=FASE 1, 2, YES, NO, NO, 5
```

# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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AELEM=FASE 1, 0
ABNDR=FASE 1, DEFORMED
ALOAD=FASE 1, FIRST
NAME=FASE 2, 28, YES, NO, NO, 5
AELEM=FASE 2, 0
ALOAD=FASE 2, FIRST
NAME=FASE 3, 10000, YES, NO, NO, 5
ALOAD=FASE 3, FIRST

*SFUNCTION ; Spectrum Function
; FUNC=NAME, iTYPE, iMETHOD, SCALE/MAX, GRAV, DRATIO, DESC, RMF ; line 1
; SPEC_CODE, [CODE_DATA] ; line 2
; PERIOD1, VALUE1, PERIOD2, VALUE2, ... ; from line 3
;[CODE_DATA] : NSC, SFI, SC, EQ, TG, DP, MaxEQ
; CH2001
;[CODE DATA] : NSC, SFI, SC, EQ, TG, DP, MaxEQ, nLForce ; CH2010
;[CODE DATA] : SFI, SC, EQ, TG, DP, MaxEQ ; CHSH2003
;[CODE DATA] : DIV, SC, SFI, EQ, TG, G ; GB50111_2006
;[CODE DATA] : BT, ZM, ST, SI, SC, TG, CI, CS, CD, EPA, SMAX, PERIOD ; JTGT B02-01-20
08
;[CODE DATA] : iSPE, SParam, TB, TC, TD, AG, Q, IF, FPX, FPY ; P100-1(2013)
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USER
  0.000000, 0.483785998, 0.010000, 0.581731143
  0.010641, 0.588011386, 0.011324, 0.594694183
  0.012049, 0.601805, 0.012822, 0.609372242
  0.013644, 0.617424313, 0.014519, 0.625992554
  0.015450, 0.635111247, 0.016441, 0.644813693
  0.017495, 0.655138091, 0.018617, 0.666125577
  0.019810, 0.67781631, 0.021080, 0.690257302
  0.022432, 0.703495568, 0.023870, 0.717583018
  0.025401, 0.732573523, 0.027029, 0.748524869
  0.028762, 0.765499742, 0.030607, 0.783562786
  0.032569, 0.802783541, 0.034657, 0.823236446
  0.036879, 0.845001817, 0.039244, 0.868161926
  0.041760, 0.892807863, 0.044438, 0.919032675
  0.047287, 0.946940185, 0.050319, 0.976636174
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  0.060632, 1.077646023, 0.064519, 1.115723178
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  0.099682, 1.460127692, 0.106074, 1.522729332
  0.112875, 1.589341825, 0.120112, 1.613050779
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  0.185573, 1.613050779, 0.197472, 1.613050779
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  0.237944, 1.613050779, 0.253200, 1.613050779
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  0.305094, 1.613050779, 0.324656, 1.613050779
  0.345472, 1.613050779, 0.367623, 1.517677475
  0.391194, 1.426231349, 0.416277, 1.340292993
  0.442967, 1.259536594, 0.471369, 1.183644123
  0.501593, 1.112322434, 0.533754, 1.045300169
  0.567977, 0.982316443, 0.604394, 0.923128202
  0.643146, 0.867506206, 0.684384, 0.815234059
  0.728265, 0.766112811, 0.774959, 0.719951825
  0.824648, 0.676571272, 0.877523, 0.635804584
  0.933788, 0.597494449, 0.993660, 0.561493012
  1.057370, 0.527661222, 1.125170, 0.495865644

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# MIDAS

PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

1.197310,	0.46598888,	1.274080,	0.437910607
1.355770,	0.411524924,	1.442700,	0.386728458
1.535200,	0.36342701,	1.633640,	0.341527599
1.738380,	0.320950049,	1.849840,	0.30161157
1.968450,	0.283437805,	2.094660,	0.266359765
2.228970,	0.25030985,	2.371880,	0.235228235
2.523960,	0.221054671,	2.685800,	0.207734435
2.858000,	0.195218036,	3.041250,	0.183455206
3.236250,	0.172401127,	3.443750,	0.162013255
3.664560,	0.146874228,	3.899520,	0.129708061
4.149550,	0.114547936,	4.415620,	0.101159317
4.698740,	0.089335987,	5.000000,	0.07889496
5.320590,	0.06967383,	5.661740,	0.061530356
6.024750,	0.054338946,	6.411040,	0.047987961
6.822120,	0.042378986,	7.259530,	0.037425901
7.724990,	0.033051671,	8.220300,	0.029188645
8.747380,	0.025777054,	9.308210,	0.022764438
9.905010,	0.020103859,	10.000000,	0.01972374

FUNC=SLD-H, 1, 0, 1, 9.806, 0.05, , 1.000000  
USER

0.000000,	0.12184271,	0.010000,	0.158842143
0.010641,	0.161214546,	0.011324,	0.163739018
0.012049,	0.166425177,	0.012822,	0.169283753
0.013644,	0.172325476,	0.014519,	0.175562186
0.015450,	0.179006834,	0.016441,	0.182671997
0.017495,	0.186572108,	0.018617,	0.190722704
0.019810,	0.195138956,	0.021080,	0.199838624
0.022432,	0.204839467,	0.023870,	0.210161096
0.025401,	0.215823859,	0.027029,	0.221849587
0.028762,	0.228261958,	0.030607,	0.235085394
0.032569,	0.242346162,	0.034657,	0.250072384
0.036879,	0.258294398,	0.039244,	0.267043283
0.041760,	0.276353451,	0.044438,	0.286260049
0.047287,	0.296802297,	0.050319,	0.308020155
0.053545,	0.319957652,	0.056979,	0.332660297
0.060632,	0.3461773,	0.064519,	0.360561199
0.068656,	0.375867124,	0.073058,	0.392154645
0.077743,	0.409486659,	0.082727,	0.410237489
0.088032,	0.410237489,	0.093676,	0.410237489
0.099682,	0.410237489,	0.106074,	0.410237489
0.112875,	0.410237489,	0.120112,	0.410237489
0.127814,	0.410237489,	0.136009,	0.410237489
0.144730,	0.410237489,	0.154009,	0.410237489
0.163884,	0.410237489,	0.174392,	0.410237489
0.185573,	0.410237489,	0.197472,	0.410237489
0.210134,	0.410237489,	0.223607,	0.410237489
0.237944,	0.403156969,	0.253200,	0.378865647
0.269435,	0.356036825,	0.286711,	0.334583542
0.305094,	0.314423692,	0.324656,	0.295478235
0.345472,	0.277674549,	0.367623,	0.260943363
0.391194,	0.245220484,	0.416277,	0.230444588
0.442967,	0.216559658,	0.471369,	0.203511011
0.501593,	0.191248247,	0.533754,	0.179724708
0.567977,	0.168895539,	0.604394,	0.158718951
0.643146,	0.149155529,	0.684384,	0.140168066
0.728265,	0.131722356,	0.774959,	0.123785622
0.824648,	0.116326944,	0.877523,	0.109317684
0.933788,	0.102730793,	0.993660,	0.096540851
1.057370,	0.090723949,	1.125170,	0.085257145
1.197310,	0.080120254,	1.274080,	0.075292589
1.355770,	0.070755941,	1.442700,	0.066492536
1.535200,	0.062486179,	1.633640,	0.058720882

# MIDAS

PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

1.738380,	0.055182861,	1.849840,	0.051857881
1.968450,	0.048733156,	2.094660,	0.04563746
2.228970,	0.040303252,	2.371880,	0.035592879
2.523960,	0.031432839,	2.685800,	0.027758833
2.858000,	0.024514559,	3.041250,	0.021649322
3.236250,	0.0191118967,	3.443750,	0.016884387
3.664560,	0.014910935,	3.899520,	0.013168195
4.149550,	0.011629112,	4.415620,	0.010269875
4.698740,	0.009069549,	5.000000,	0.008009558
5.320590,	0.007073412,	5.661740,	0.006246672
6.024750,	0.005516587,	6.411040,	0.004871824
6.822120,	0.004302391,	7.259530,	0.003799544
7.724990,	0.003355465,	8.220300,	0.002963284
8.747380,	0.002616933,	9.308210,	0.002311086
9.905010,	0.00204098,	10.000000,	0.002002389
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USER			
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0.010641,	0.816289951,	0.011324,	0.825428986
0.012049,	0.835153358,	0.012822,	0.845501912
0.013644,	0.856513491,	0.014519,	0.868230958
0.015450,	0.880701192,	0.016441,	0.893969737
0.017495,	0.908088829,	0.018617,	0.923114725
0.019810,	0.939102343,	0.021080,	0.956115976
0.022432,	0.974219917,	0.023870,	0.993485158
0.025401,	1.013985367,	0.027029,	1.035799571
0.028762,	1.059013496,	0.030607,	1.083715545
0.032569,	1.110000818,	0.034657,	1.137971113
0.036879,	1.167736264,	0.039244,	1.199408785
0.041760,	1.233113245,	0.044438,	1.268976891
0.047287,	1.307141703,	0.050319,	1.347752342
0.053545,	1.390968202,	0.056979,	1.436954035
0.060632,	1.485887987,	0.064519,	1.537960259
0.068656,	1.593370429,	0.073058,	1.652334146
0.077743,	1.715079117,	0.082727,	1.781846443
0.088032,	1.852894635,	0.093676,	1.92849828
0.099682,	2.008949378,	0.106074,	2.094560018
0.112875,	2.185655687,	0.120112,	2.282591337
0.127814,	2.374673468,	0.136009,	2.374673468
0.144730,	2.374673468,	0.154009,	2.374673468
0.163884,	2.374673468,	0.174392,	2.374673468
0.185573,	2.374673468,	0.197472,	2.374673468
0.210134,	2.374673468,	0.223607,	2.374673468
0.237944,	2.374673468,	0.253200,	2.374673468
0.269435,	2.374673468,	0.286711,	2.374673468
0.305094,	2.374673468,	0.324656,	2.374673468
0.345472,	2.374673468,	0.367623,	2.374673468
0.391194,	2.312549438,	0.416277,	2.173205498
0.442967,	2.042263791,	0.471369,	1.919208656
0.501593,	1.803564772,	0.533754,	1.69489215
0.567977,	1.592767779,	0.604394,	1.496797561
0.643146,	1.406609798,	0.684384,	1.321853616
0.728265,	1.242206429,	0.774959,	1.167359131
0.824648,	1.097020141,	0.877523,	1.030919377
0.933788,	0.968801768,	0.993660,	0.910427576
1.057370,	0.855571337,	1.125170,	0.804016695
1.197310,	0.755573298,	1.274080,	0.710046045
1.355770,	0.667263227,	1.442700,	0.627057229
1.535200,	0.589275316,	1.633640,	0.55376672
1.738380,	0.520401446,	1.849840,	0.48904525
1.968450,	0.459577569,	2.094660,	0.431886542
2.228970,	0.405862558,	2.371880,	0.381408615

# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

2.523960, 0.358427021, 2.685800, 0.336829051  
 2.858000, 0.316534452, 3.041250, 0.297461723  
 3.236250, 0.279538189, 3.443750, 0.262694872  
 3.664560, 0.246866053, 3.899520, 0.231991493  
 4.149550, 0.218012909, 4.415620, 0.199280952  
 4.698740, 0.175989333, 5.000000, 0.155420809  
 5.320590, 0.137255446, 5.661740, 0.121213036  
 6.024750, 0.107046165, 6.411040, 0.094534907  
 6.822120, 0.083485387, 7.259530, 0.073727951  
 7.724990, 0.065110843, 8.220300, 0.057500794  
 8.747380, 0.050780057, 9.308210, 0.044845289  
 9.905010, 0.039604026, 10.000000, 0.038855202  
 FUNC=SLV-V, 1, 0, 1, 9.806, 0.05, , 1.000000  
 USER  
 0.000000, 0.241892999, 0.010000, 0.290865571  
 0.010641, 0.294005693, 0.011324, 0.297347091  
 0.012049, 0.3009025, 0.012822, 0.304686121  
 0.013644, 0.308712156, 0.014519, 0.312996277  
 0.015450, 0.317555624, 0.016441, 0.322406847  
 0.017495, 0.327569045, 0.018617, 0.333062789  
 0.019810, 0.338908155, 0.021080, 0.345128651  
 0.022432, 0.351747784, 0.023870, 0.358791509  
 0.025401, 0.366286761, 0.027029, 0.374262435  
 0.028762, 0.382749871, 0.030607, 0.391781393  
 0.032569, 0.401391771, 0.034657, 0.411618223  
 0.036879, 0.422500908, 0.039244, 0.434080963  
 0.041760, 0.446403931, 0.044438, 0.459516338  
 0.047287, 0.473470093, 0.050319, 0.488318087  
 0.053545, 0.504118598, 0.056979, 0.520931862  
 0.060632, 0.538823011, 0.064519, 0.557861589  
 0.068656, 0.578120563, 0.073058, 0.599678779  
 0.077743, 0.622619491, 0.082727, 0.647030849  
 0.088032, 0.673007371, 0.093676, 0.70064945  
 0.099682, 0.730063846, 0.106074, 0.761364666  
 0.112875, 0.794670912, 0.120112, 0.80652539  
 0.127814, 0.80652539, 0.136009, 0.80652539  
 0.144730, 0.80652539, 0.154009, 0.80652539  
 0.163884, 0.80652539, 0.174392, 0.80652539  
 0.185573, 0.80652539, 0.197472, 0.80652539  
 0.210134, 0.80652539, 0.223607, 0.80652539  
 0.237944, 0.80652539, 0.253200, 0.80652539  
 0.269435, 0.80652539, 0.286711, 0.80652539  
 0.305094, 0.80652539, 0.324656, 0.80652539  
 0.345472, 0.80652539, 0.367623, 0.758838738  
 0.391194, 0.713115675, 0.416277, 0.670146497  
 0.442967, 0.629768297, 0.471369, 0.591822061  
 0.501593, 0.556161217, 0.533754, 0.522650084  
 0.567977, 0.491158222, 0.604394, 0.461564101  
 0.643146, 0.433753103, 0.684384, 0.40761703  
 0.728265, 0.383056406, 0.774959, 0.359975913  
 0.824648, 0.338285636, 0.877523, 0.317902292  
 0.933788, 0.298747224, 0.993660, 0.280746506  
 1.057370, 0.263830611, 1.125170, 0.247932822  
 1.197310, 0.23299444, 1.274080, 0.218955304  
 1.355770, 0.205762462, 1.442700, 0.193364229  
 1.535200, 0.181713505, 1.633640, 0.170763799  
 1.738380, 0.160475025, 1.849840, 0.150805785  
 1.968450, 0.141718902, 2.094660, 0.133179883  
 2.228970, 0.125154925, 2.371880, 0.117614118  
 2.523960, 0.110527335, 2.685800, 0.103867218  
 2.858000, 0.097609018, 3.041250, 0.091727603  
 3.236250, 0.086200563, 3.443750, 0.081006627

# MIDAS

PROJECT TITLE :

MIDAS	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

3.664560,	0.073437114,	3.899520,	0.06485403
4.149550,	0.057273968,	4.415620,	0.050579658
4.698740,	0.044667994,	5.000000,	0.03944748
5.320590,	0.034836915,	5.661740,	0.030765178
6.024750,	0.027169473,	6.411040,	0.023993981
6.822120,	0.021189493,	7.259530,	0.01871295
7.724990,	0.016525835,	8.220300,	0.014594323
8.747380,	0.012888527,	9.308210,	0.011382219
9.905010,	0.01005193,	10.000000,	0.00986187

```

*SPLDCASE      ; Spectrum Load Cases
; TYPE, bADDSSIGN, iSIGNTYPE
; NAME=NAME, DIR, ANGLE, SCALE, PMFT, bDAMP, bECC, INTERP, DESC,          ; line 1
;      COMTYPE, bADDSSIGN, iSIGNTYPE, bMODE                         ; line 2
;      FUNC1, FUNC2, FUNC3, ...                                     ; line 3
;      bUSE1, dFACTOR1, bUSE2, dFACTOR2, ..., bUSEn, dFACTORn        ; line 4 (bMODE
=YES)
;      bCDR, [DR-DC]                                              ; line 5 (bDAMP
=YES)
; [DR-DC] : iMDTYPE, DALL, iMODE1, DAMPING1, iMODE2, DAMPING2, ...       ; iMDTYPE=1
;           : iMDTYPE, iCOEF, bMASSP, MASSC, bSTIFFP, STIFFC            ; iMDTYPE=2, iC
OEF=1
;           : iMDTYPE, iCOEF, iCALC, bMASSP, FP1, DR1, bSTIFFP, FP2, DR2 ; iMDTYPE=2, iC
OEF=2
      NAME=EX, XY, 0, 1, 1, NO, NO, LOG, EX
      CQC, NO, 0, YES
      SLV-H
      YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1
      NAME=EY, XY, 90, 1, 1, NO, NO, LOG, EX
      CQC, NO, 0, YES
      SLV-H
      YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1
      NAME=EZ, Z, 0, 1, 1, NO, NO, LOG, EX
      CQC, NO, 0, YES
      SLV-V
      YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1
      NAME=DX, XY, 0, 1, 1, NO, NO, LOG, DX
      CQC, NO, 0, YES
      SLD-H
      YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1
      NAME=CX, XY, 0, 1, 1, NO, NO, LOG, CX
      CQC, NO, 0, YES
      SLC-H
      YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1
      NAME=CY, XY, 90, 1, 1, NO, NO, LOG, CY
      CQC, NO, 0, YES
      SLC-H
      YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1,YES, 1

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*MVLDCODE      ; Moving Load Code
; CODE=CODE
;     CODE=EUROCODE

*LINELANE      ; Traffic Line Lanes
; NAME=NAME, LDIST, GROUP, SKEWS, SKEWE, MOVING, LW, WS, bLANEOPT, ALLOWWIDTH ; line
1
;     iELEM1, ECC1, FACT1, bSPAN1, ECCV1...                                ; from
line 2
      NAME=LC01, CROSS, FASE 2, 0, 0, BOTH, 3, 2, NO, 3
      57, -1, 0, NO, 0,    110, -1, 0, NO, 0,    117, -1, 0, NO, 0
      124, -1, 0, NO, 0,    131, -1, 0, NO, 0,    62, -1, 0, NO, 0
      142, -1, 0, NO, 0,    149, -1, 0, NO, 0,    156, -1, 0, NO, 0

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# MIDAS

PROJECT TITLE :

MIDAS	Company		Client	File Name	L2-CV02_REVISIONE_2
	Author				

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163, -1, 0, NO, 0,      63, -1, 0, NO, 0,      174, -1, 0, NO, 0
181, -1, 0, NO, 0,      188, -1, 0, NO, 0,      195, -1, 0, NO, 0
64, -1, 0, NO, 0,      206, -1, 0, NO, 0,      213, -1, 0, NO, 0
220, -1, 0, NO, 0,      227, -1, 0, NO, 0,      65, -1, 0, NO, 0
238, -1, 0, NO, 0,      245, -1, 0, NO, 0,      252, -1, 0, NO, 0
259, -1, 0, NO, 0,      66, -1, 0, NO, 0,      270, -1, 0, NO, 0
277, -1, 0, NO, 0,      284, -1, 0, NO, 0,      291, -1, 0, NO, 0
67, -1, 0, NO, 0,      302, -1, 0, NO, 0,      309, -1, 0, NO, 0
316, -1, 0, NO, 0,      323, -1, 0, NO, 0,      68, -1, 0, NO, 0
334, -1, 0, NO, 0,      341, -1, 0, NO, 0,      348, -1, 0, NO, 0
355, -1, 0, NO, 0,      69, -1, 0, NO, 0,      366, -1, 0, NO, 0
373, -1, 0, NO, 0,      380, -1, 0, NO, 0,      387, -1, 0, NO, 0
70, -1, 0, NO, 0,      398, -1, 0, NO, 0,      405, -1, 0, NO, 0
412, -1, 0, NO, 0,      419, -1, 0, NO, 0

NAME=LC02, CROSS, FASE 2, 0, 0, BOTH, 3, 2, NO, 3
57, -4, 0, NO, 0,      110, -4, 0, NO, 0,      117, -4, 0, NO, 0
124, -4, 0, NO, 0,      131, -4, 0, NO, 0,      62, -4, 0, NO, 0
142, -4, 0, NO, 0,      149, -4, 0, NO, 0,      156, -4, 0, NO, 0
163, -4, 0, NO, 0,      63, -4, 0, NO, 0,      174, -4, 0, NO, 0
181, -4, 0, NO, 0,      188, -4, 0, NO, 0,      195, -4, 0, NO, 0
64, -4, 0, NO, 0,      206, -4, 0, NO, 0,      213, -4, 0, NO, 0
220, -4, 0, NO, 0,      227, -4, 0, NO, 0,      65, -4, 0, NO, 0
238, -4, 0, NO, 0,      245, -4, 0, NO, 0,      252, -4, 0, NO, 0
259, -4, 0, NO, 0,      66, -4, 0, NO, 0,      270, -4, 0, NO, 0
277, -4, 0, NO, 0,      284, -4, 0, NO, 0,      291, -4, 0, NO, 0
67, -4, 0, NO, 0,      302, -4, 0, NO, 0,      309, -4, 0, NO, 0
316, -4, 0, NO, 0,      323, -4, 0, NO, 0,      68, -4, 0, NO, 0
334, -4, 0, NO, 0,      341, -4, 0, NO, 0,      348, -4, 0, NO, 0
355, -4, 0, NO, 0,      69, -4, 0, NO, 0,      366, -4, 0, NO, 0
373, -4, 0, NO, 0,      380, -4, 0, NO, 0,      387, -4, 0, NO, 0
70, -4, 0, NO, 0,      398, -4, 0, NO, 0,      405, -4, 0, NO, 0
412, -4, 0, NO, 0,      419, -4, 0, NO, 0

NAME=LC03, CROSS, FASE 2, 0, 0, BOTH, 1, 1, NO, 3
57, -5, 0, NO, 0,      110, -5, 0, NO, 0,      117, -5, 0, NO, 0
124, -5, 0, NO, 0,      131, -5, 0, NO, 0,      62, -5, 0, NO, 0
142, -5, 0, NO, 0,      149, -5, 0, NO, 0,      156, -5, 0, NO, 0
163, -5, 0, NO, 0,      63, -5, 0, NO, 0,      174, -5, 0, NO, 0
181, -5, 0, NO, 0,      188, -5, 0, NO, 0,      195, -5, 0, NO, 0
64, -5, 0, NO, 0,      206, -5, 0, NO, 0,      213, -5, 0, NO, 0
220, -5, 0, NO, 0,      227, -5, 0, NO, 0,      65, -5, 0, NO, 0
238, -5, 0, NO, 0,      245, -5, 0, NO, 0,      252, -5, 0, NO, 0
259, -5, 0, NO, 0,      66, -5, 0, NO, 0,      270, -5, 0, NO, 0
277, -5, 0, NO, 0,      284, -5, 0, NO, 0,      291, -5, 0, NO, 0
67, -5, 0, NO, 0,      302, -5, 0, NO, 0,      309, -5, 0, NO, 0
316, -5, 0, NO, 0,      323, -5, 0, NO, 0,      68, -5, 0, NO, 0
334, -5, 0, NO, 0,      341, -5, 0, NO, 0,      348, -5, 0, NO, 0
355, -5, 0, NO, 0,      69, -5, 0, NO, 0,      366, -5, 0, NO, 0
373, -5, 0, NO, 0,      380, -5, 0, NO, 0,      387, -5, 0, NO, 0
70, -5, 0, NO, 0,      398, -5, 0, NO, 0,      405, -5, 0, NO, 0
412, -5, 0, NO, 0,      419, -5, 0, NO, 0

```

```

*VEHICLE ; Vehicles
; if Moving Load Code is China
; NAME=NAME, 1, TYPE-NAME, CODE
; standard
; NAME=NAME, 2, LTYPE, [TRUCK/LANE] or [TRAIN/SUBWAY] or [CROWD]
; user: line 1
; LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2
; [TRUCK/LANE] : 1, P, Qm, Qq
; truck(JTG)
; [TRUCK/LANE] : 2, P, Qm, Qq

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# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			

```
; lane load1
; [TRUCK/LANE] : 3, Qk, Pk1, L1, Pk2, L2
; lane load2
; [TRUCK/LANE] : 4, dW, dD
; crawler type
; [TRUCK/LANE] : 5
; GC type load
; [TRAIN/SUBWAY] : iTYPE, W1, D1, W2, D2
; train-type1,3
; [TRAIN/SUBWAY] : iTYPE, DD, FD, BD, MAINCOUNT
; train-type2
; [TRAIN/SUBWAY] : 4, P1, D1, P2, D2, P3, D3, P4, dD, Po, n, IFR
; subway
; [CROWD] : 1, dW1
; crowd-type1
; [CROWD] : 2, dW1, dL1, dw2, dL2, WIDTH
; crowd-type2
; if Moving Load Type is India
; NAME=NAME, 1, TYPE-NAME, CODE
; standard
; NAME=NAME, 2, bWTB, P, D, Pb, Db, dD1, dD2, NDIST
; user: line 1
; NAME=NAME, 2, bWTB, dD1, dD2, NDIST
; user: line 1
; LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2
; if Moving Load Code is CANADA
; NAME=NAME, 1, TYPE-NAME, DLA, CODE, [DYNA]
; standard
; NAME=NAME, 2, bTRAIN, W(W1), PL(D1), PLM(W2), PLV(D2), NDIST, [DYNA]
; user: line 1
; LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2
; [DYNA] : nDYNA, FACT1AXLE, FACT2AXLE, FACT3AXLE
; Dynamic Load Allowance
; if Moving Load Code is BS
; NAME=NAME, 1, TYPE-NAME, CODE, UNITNUM
; standard
; NAME=nLane, FACTOR1, FACTOR2, FACTOR3, FACTOR4, ADDDATA, AL, CA, LL
; HA, HA & HB, HA & HB(Auto)
; NAME=NAME, 2, iSTYPE, W1, W2, W3, L, Pa, Pb, D1, D2, d, UNITNUM
; user(BS 5400)
; NAME=NAME, 2, iSTYPE, [BD37/01-HA], [BS-DATA-LF]
; user(HA)
; NAME=NAME, 2, iSTYPE, [BS-DATA-HB]
; user(HB)
; NAME=NAME, 2, iSTYPE, [BD37/01-HA2], [BS-DATA-HB2], [BS-DATA-LF]
; user(HA&HB)
; NAME=NAME, 2, iSTYPE, [BD37/01-HA], [BD37/01-HB], [BS-DATA-LF]
; user(HA&HB(AUTO))
; NAME=NAME, 2, iSTYPE, W, L
; user(Pedestrian)
; NAME=NAME, 2, iSTYPE, V, AN, MINS, MAXS, P1, D1, P2, D2, ...
; user(Special Vehicle)
; [BS-DATA-HA] : W1, W2, W3, EXP, EXP2, L1, L2, Pa
; [BS-DATA-HA2] : W1, W2, W3, EXP, EXP2, L1, L2
; [BS-DATA-HB] : Pb, D1, D2, d, UNITNUM
; [BS-DATA-HB2] : Pb, D1, D2, d, dd, UNITNUM
; [BS-DATA-LF] : nLT, LF1, LF2, LF3, LF4
; if Moving Load Code is EUROCODE
; NAME=NAME, 1, iTYPE, TYPE-NAME, PSY1, PSY2, PHI, [AF7]
; standard (LM1, FLM1)
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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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; NAME=NAME, 1, iTYPE, TYPE-NAME, bDF, bU, PHI, PSY, ADJ, IN
; standard (others)
; NAME=NAME, 2, 1, [AF7]
; user(Type 1)
;     [LOAD7], D, PHI, TPSY, UPSY
; user(Type 1): line 2
; NAME=NAME, 2, 2, ALP, TPSY, W, BET, LPSY, P1, D1, P2, D2, ...
; user(Type 2)
; NAME=NAME, 2, 3
; user(Type 3)
;     [LOADCASE1]
; user(Type 3): line 2
;     [LOADCASE2]
; user(Type 3): line 3
;     [LOADCASE3]
; user(Type 3): line 4
; NAME=NAME, 2, 4, WS, V, AN, MINS, MAXS, DYF, UI, F, P1, D1, P2, D2, ...
; user(Type 4)
; NAME=NAME, 2, 5, INT, bPHI1, PHI1, bPHI2, PHI2
; user(Type 5)
;     [VEHICLE1]
; user(Type 5): line 2
;     [VEHICLE2]
; user(Type 5): line 3
;     [VEHICLE3]
; user(Type 5): line 4
; [AF7] : TF1, TF2, TF3, UF1, UF2, UF3, UF4
; adjustment factor
; [LOAD7] : TL1, TL2, TL3, UL1, UL2, UL3, UL4
; tandem/udl loads
; [LOADCASE] : bUSE, N, bDF, bUI, PHI, P1, L1, P2, L2, ...
; load case
; [VEHICLE] : bUSE, N, P1, L1, P2, L2, ...
; vehicle
; if Moving Load Code is RUSSIA
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; standard (SK)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; standard (SK FATIGUE)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, dDYNAFAC_UDL
;
;             bFATI, nLOADFAC, dLOADFAC, dLOADFAC_UDL, s1[3], s1_UDL[3]
; standard (AK)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC,
;
;             bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; standard (N14)
; NAME=NAME, 1, iTYPE, K, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC,
;
;             bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; standard (N11)
; NAME=NAME, 1, iTYPE, nDYNAFAC, dDYNAFAC, dEMPTYCAR
;
;             bFATI, nLOADFAC, dLOADFAC,
; standard (SUBWAY TRAINS)
; NAME=NAME, 1, iTYPE, VARIABLE, nDYNAFAC, dDYNAFAC, dEMPTYCAR
;
;             bFATI, nLOADFAC, dLOADFAC,
; standard (TRAMCARS)
; NAME=NAME, 1, iTYPE, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; standard (NK-80)
; NAME=NAME, 1, iTYPE, W, D, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC

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# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	File Name
	Author			

```
; standard (NG-60)
; NAME=NAME, 1, iTYPE, BRIDGETYPE, W, bFATI, nLOADFAC, dLOADFAC
; standard (UNIFORM LOAD)
; NAME=NAME, 1, iTYPE, BRIDGETYPE, W, bFATI, nLOADFAC, dLOADFAC
; standard (UNIFORM LOAD(W/O OTHER LOADS))
; NAME=NAME, 1, iTYPE, BRIDGETYPE, P
; standard (CONCENTRATED LOAD (W/O OTHER LOADS))
; NAME=NAME, 2, iTYPE, W, nDYNAFAC, dDYNAFAC, dDYNAFAC_UDL

; bFATI, nLOADFAC, dLOADFAC, dLOADFAC_UDL, s1[3], s1_UDL[3]
; user (Type 1)
; NAME=NAME, 2, iTYPE, nDYNAFAC, dDYNAFAC, dEMPTYCAR

; bFATI, nLOADFAC, dLOADFAC,
; user (Type 2)
; NAME=NAME, 2, iTYPE, Variable, nDYNAFAC, dDYNAFAC, dEMPTYCAR

; bFATI, nLOADFAC, dLOADFAC,
; user (Type 3)
; NAME=NAME, 2, iTYPE, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC

; bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; user (Type 4)
; NAME=NAME, 2, iTYPE, nDYNAFAC, dDYNAFAC, nLOADFAC, dLOADFAC

; bTWOVEHI, TWOVEHI_FACT, b2NDREDUC, 2NDREDUC_FACT
; user (Type 5)
; NAME=NAME, 2, iTYPE, W, D, nDYNAFAC, dDYNAFAC, bFATI, nLOADFAC, dLOADFAC
; user (Type 6)
; NAME=NAME, 2, iTYPE, P, W, bFATI, nLOADFAC, dLOADFAC
; user (Type 7)
; if Moving Load Code is KSCE-LSD15
; NAME=NAME, 1, TYPE-NAME, nLANETYPE, dDYNAFAC, CODE, nLANELOAD, L, CONVERTDIST
; standard
; NAME=NAME, 2, 8, L1, W1, W2, EXP, dDYNAFAC, nLANELOAD, L, CONVERTDIST
; user: line 1 (Type 1)
; NAME=NAME, 2, 1, W1, W2, D1, D2, 0, 0, 0, NO
; user: line 1 (Type 2)
; NAME=NAME, 2, 6, LOADNUM, DIST, W, L, 0, 0, 0, NO
; user: line 1 (Type 3)
; LOAD1, DIST1, (DIST2_1), LOAD2, DIST2, (DIST2_2), ...
; user: from line 2
; if Moving Load Code is South Africa
; NAME=NAME, 1, TYPE-NAME, CODE, bINCREL, dINCREL
; standard NA
; NAME=NAME, 1, TYPE-NAME, CODE, UNITNUM
; standard NB
; NAME=NAME, 1, TYPE-NAME, CODE, OPPOSITE
; standard NC
; NAME=NAME, 2, TYPE, W1, L, W2, W3, PA, bINCREL, dINCREL
; user NA
; NAME=NAME, 2, TYPE, PB, UNITNUM, DELTA, D1, D2
; user NB
; NAME=NAME, 2, TYPE, P, OPPOSITE, NUM1, NUM2, NUM3 [DIST1], [DIST2], [DIST3]
; user NC
; if Load Type is Permit Truck
; NAME=NAME, 3, AXLE-TYPE-NUM, IMP-FACTOR
; user(Permit Truck)
; AXLE-TYPE-NAME1, bEDWL1, bSV1, P1, D1, P2, D2, ..., Pn, Dn
; user(from line 2)
; ...
; AXLE-TYPE-NAMEn, bEDWLn, bSVn, P1, D1, P2, D2, ..., Pn, Dn
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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

```
; AXLE-TYPE1, SPACING1, bVS1, AXLE-TYPE2, SPACING2, bVS2 ...
; line 2+AXLE-TYPE-NUM
; if Moving Load Code is not one of those specified above.
; NAME=NAME, 1, TYPE-NAME, DLA, CODE
; standard
; NAME=NAME, 2, bTRAIN, W(W1), PL(D1), PLM(W2), PLV(D2), NDIST
; user: line 1
; LOAD1, DIST1, LOAD2, DIST2, ...
; user: from line 2
NAME=Q1A_q1a, 2, 1, 1, 1, 0, 1, 0, 0, 0
  300, 0, 0, 9, 0, 0, 0, 1.2, 1, 1, 1
NAME=Q2A_q2a, 2, 1, 1, 0, 0, 1, 0, 0, 0
  200, 0, 0, 2.5, 0, 0, 0, 1.2, 1, 1, 1
NAME=q3a, 2, 1, 0, 0, 0, 1, 0, 0, 0
  0, 0, 0, 2.5, 0, 0, 0, 1.2, 1, 1, 1

*MVLDCASE(EURO) ; Moving Load Cases
; NAME=NAME, bOPTIM, iTYPE, DESC, VHL1, VHL2, bLEAD, SERIAL
;           ; type 1, 3
; nType, SLN1, SLN2, SLN3, ... ; line 2 (nType=1)
; nType, SRA1, SRA2, SRA3, ... ; line 2/3 (nType=2)
; nType, FLN1, FLN2, FLN3, ... ; line 4 (nType=3)
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, SERIAL
;           ; type 2
; [SUB1] ; line 2
; ...
; [SUBn] ; ...
; [SUB] : VCLA, SCA, MIN, MAX, SLN1, SLN2, ... ; sub-loadcase
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, SERIAL
;           ; type 4
; 0, SLN1, SLN2, SLN3, ... ; line 2
; 1, SRA1, SRA2, SRA3, ... ; line 3
; 2, STradd1, STradd2, STradd3, ... ; line 4
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, SF1, SF2, SF3, bPSI1, MULF1, MULF2, MULF3, SERIAL
;           ; type 5
; [SUB1] ; line 2
; ...
; [SUBn] ; ...
; ///////////////////////////////////////////////////////////////////
; ///////////////////////////////////////////////////////////////////
; ///////////////////////////////////////////////////////////////////
; ///////////////////////////////////////////////////////////////////
; // Moving Load Optimization
;           ;
; NAME=NAME, bOPTIM, iTYPE, DESC, VHL1, VHL2, bLEAD, MINVEHL, LANEOP, LOADEDLANE, SERIAL
;           ; type 1, 3
; nType, SRA1, SRA2, SRA3, ... ; line 2 (nType=2)
; nType, FLN1, FLN2, FLN3, ... ; line 3 (nType=3)
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, MINVEHL, LANEOP, MIN, MAX, SERIAL
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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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;           ; type 2
; [ASSGNVEHL1]          ; line 2
; ...
;           ; ...
; [ASSGNVEHLn]          ; line n+1
; [ASSGNVEHL] : VC, VCLA, SCA ...          ; Assignment-Vehicle
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, bLEAD, MINVEHL, LANEOP, LOADEDLANE, SERIAL
;           ; type 4
; 1, SRA1, SRA2, SRA3, ...          ; line 3
; NAME=NAME, bOPTIM, iTYPE, DESC, iOPT, SF1, SF2, SF3, bPSI1, MULF1, MULF2, MULF3, MIN
VEHL, LANEOP, MIN, MAX, SERIAL ; type 5
; [ASSGNVEHL1]          ; line 2
; ...
;           ; ...
; [ASSGNVEHLn]          ; line n+1
NAME=LM01, NO, 1, , Q1A_q1a, , YES, 1
1, LC01
NAME=LM02, NO, 1, , Q2A_q2a, , YES, 2
1, LC02
NAME=LM03, NO, 1, , q3a, , YES, 3
1, LC03

*CPOSECT4CS      ; Composite Section for Construction Stage
; SEC=SEC, ASTAGE, TYPE, bTAP          // 
line 1
; [PART-INFO]-1          // 
from line 2
; ...
; [PART-INFO]-n          //
; [PART-INFO] : [COMMON], [SCALE], H, VS          // 
TYPE=A,B,NORMAL
; [COMMON], [SCALE], CY, CZ, [STIFF], H, VS          // 
TYPE=USER
; [COMMON], [SCALE], CYI, CZI, CYJ, CZJ, [STIFF]-I, [STIFF]-J, H, VS // 
TYPE=USER, bTAP=YES
; [COMMON]: PART, MTYPE, MAT, CSTAGE, AGE
; [SCALE] : AREA, ASY, ASZ, IXX, IYY, IZZ, WAREA
; [STIFF] : AREA, ASY, ASZ, IXX, IYY, CYP, CYM, CZP, CZM, QYB, QZB, \
X1, X2, X3, X4, Y1, Y2, Y3, Y4
SEC=7, FASE 1, NORMAL, NO
1, ELEM, , 0, 1, 1, 1, 1, 1, 1, 1, 0.0233196, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 0.229839, 0
SEC=9, FASE 1, NORMAL, NO
1, ELEM, , 0, 1, 1, 1, 1, 1, 1, 0.0321717, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 0.229839, 0
SEC=8, FASE 1, NORMAL, NO
1, ELEM, , 0, 1, 1, 1, 1, 1, 1, 0.027365, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 0.229839, 0
SEC=3, FASE 1, NORMAL, NO
1, ELEM, , 0, 1, 1, 1, 1, 1, 1, 0.0187086, 0
2, MATL, 2, FASE 3, 0, 1, 1, 1, 1, 1, 1, 0.229508, 0

*STAGE-COLOR      ; Diagram Color for Construction Stage
; STAGENAME, iR(COLOR), iG(COLOR), iB(COLOR)
FASE 1, 210, 210, 210
FASE 2, 0, 128, 128

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

FASE 3, 0, 192, 192

```
*LOADCOMB      ; Combinations
; NAME=NAME, KIND, ACTIVE, bES, iTYPE, DESC, iSERV-TYPE, nLCOMTYPE      ; line 1
;       ANAL1, LCNAME1, FACT1, ...
NAME=SLU1, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU2, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU3, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU4, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU5, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU6, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU7, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU8, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU9, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU10, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU11, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU12, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU13, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU14, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU15, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU16, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU17, GEN, ACTIVE, 0, 0, , 0, 0
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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU18, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU19, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU20, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU21, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU22, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU23, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU24, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLV1, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV2, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV3, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV4, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV5, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV6, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV7, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV8, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV9, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV10, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV11, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV12, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV13, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV14, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV15, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV16, GEN, ACTIVE, 0, 0, , 0, 0

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV17, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV18, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV19, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV20, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, -0.3
CS, Summation, 1
NAME=SLV21, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV22, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV23, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV24, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -1
CS, Summation, 1
NAME=SLV25, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV26, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, 0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV27, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV28, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, -0.3, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV29, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV30, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV31, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, 1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV32, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, -1, MV, LM03, 0.75
CS, Summation, 1
NAME=SLV33, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV34, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV35, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV36, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV37, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV38, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV39, GEN, ACTIVE, 0, 0, , 0, 0

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV40, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV41, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV42, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV43, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV44, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV45, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV46, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV47, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV48, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV49, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV50, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV51, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV52, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV53, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV54, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV55, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV56, GEN, ACTIVE, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=RARA_1, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 1, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_2, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 1, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_3, GEN, ACTIVE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 1, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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NAME=RARA_4, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_5, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA UNIFORME +, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_6, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA UNIFORME -, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_7, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA GRADIENTE +, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_8, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_9, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_10, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_11, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_12, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_13, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_14, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_15, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_16, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=FREQ_1, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_2, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_3, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_4, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_5, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    CS, Summation, 1
NAME=FREQ_6, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    CS, Summation, 1
NAME=FREQ_7, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    CS, Summation, 1
NAME=FREQ_8, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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CS, Summation, 1
NAME=FREQ_9, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_10, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_11, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_12, GEN, ACTIVE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_13, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_14, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_15, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_16, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=QP_1, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, CS, Summation, 1
NAME=QP_2, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, CS, Summation, 1
NAME=QP_3, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, CS, Summation, 1
NAME=QP_4, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, CS, Summation, 1
NAME=SLD1, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD2, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=SLD3, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD4, GEN, ACTIVE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=ENVESLU-SLV, GEN, ACTIVE, 0, 1, , 0, 0
    CB, SLU1, 1, CB, SLU2, 1, CB, SLU3, 1, CB, SLU4, 1, CB, SLU5, 1
    CB, SLU6, 1, CB, SLU7, 1, CB, SLU8, 1, CB, SLU9, 1, CB, SLU10, 1
    CB, SLU11, 1, CB, SLU12, 1, CB, SLU13, 1, CB, SLU14, 1, CB, SLU15, 1
    CB, SLU16, 1, CB, SLU17, 1, CB, SLU18, 1, CB, SLU19, 1, CB, SLU20, 1
    CB, SLU21, 1, CB, SLU22, 1, CB, SLU23, 1, CB, SLU24, 1, CB, SLV1, 1
    CB, SLV2, 1, CB, SLV3, 1, CB, SLV4, 1, CB, SLV5, 1, CB, SLV6, 1
    CB, SLV7, 1, CB, SLV8, 1, CB, SLV9, 1, CB, SLV10, 1, CB, SLV11, 1
    CB, SLV12, 1, CB, SLV13, 1, CB, SLV14, 1, CB, SLV15, 1, CB, SLV16, 1
    CB, SLV17, 1, CB, SLV18, 1, CB, SLV19, 1, CB, SLV20, 1, CB, SLV21, 1
    CB, SLV22, 1, CB, SLV23, 1, CB, SLV24, 1, CB, SLV25, 1, CB, SLV26, 1
    CB, SLV27, 1, CB, SLV28, 1, CB, SLV29, 1, CB, SLV30, 1, CB, SLV31, 1
    CB, SLV32, 1, CB, SLV33, 1, CB, SLV34, 1, CB, SLV35, 1, CB, SLV36, 1
    CB, SLV37, 1, CB, SLV38, 1, CB, SLV39, 1, CB, SLV40, 1, CB, SLV41, 1
    CB, SLV42, 1, CB, SLV43, 1, CB, SLV44, 1, CB, SLV45, 1, CB, SLV46, 1
    CB, SLV47, 1, CB, SLV48, 1, CB, SLV49, 1, CB, SLV50, 1, CB, SLV51, 1
    CB, SLV52, 1, CB, SLV53, 1, CB, SLV54, 1, CB, SLV55, 1, CB, SLV56, 1
NAME=ENVE-RARA, GEN, ACTIVE, 0, 1, , 0, 0
    CB, RARA_1, 1, CB, RARA_2, 1, CB, RARA_3, 1, CB, RARA_4, 1
    CB, RARA_5, 1, CB, RARA_6, 1, CB, RARA_7, 1, CB, RARA_8, 1
    CB, RARA_9, 1, CB, RARA_10, 1, CB, RARA_11, 1, CB, RARA_12, 1

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# MIDAS

## PROJECT TITLE :

MIDAS	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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CB, RARA_13, 1, CB, RARA_14, 1, CB, RARA_15, 1, CB, RARA_16, 1
NAME=ENVE-FREQ, GEN, ACTIVE, 0, 1, , 0, 0
    CB, FREQ_1, 1, CB, FREQ_2, 1, CB, FREQ_3, 1, CB, FREQ_4, 1
    CB, FREQ_5, 1, CB, FREQ_6, 1, CB, FREQ_7, 1, CB, FREQ_8, 1
    CB, FREQ_9, 1, CB, FREQ_10, 1, CB, FREQ_11, 1, CB, FREQ_12, 1
    CB, FREQ_13, 1, CB, FREQ_14, 1, CB, FREQ_15, 1, CB, FREQ_16, 1
NAME=ENVE-QP, GEN, ACTIVE, 0, 1, , 0, 0
    CB, QP_1, 1, CB, QP_2, 1, CB, QP_3, 1, CB, QP_4, 1
NAME=ENVE-SLD, GEN, ACTIVE, 0, 1, , 0, 0
    CB, SLD1, 1, CB, SLD2, 1, CB, SLD3, 1, CB, SLD4, 1
NAME=SLC_X, GEN, ACTIVE, 0, 0, , 0, 0
    RS, CX, 1, RS, CY, 0.3
NAME=SLC_Y, GEN, ACTIVE, 0, 0, , 0, 0
    RS, CY, 1, RS, CX, 0.3
NAME=SLU1, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU2, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU3, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU4, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU5, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU6, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU7, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU8, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU9, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU10, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU11, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU12, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU13, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU14, STEEL, STRENGTH, 0, 0, , 0, 0

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU15, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU16, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU17, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU18, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU19, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU20, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU21, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU22, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU23, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLU24, STEEL, STRENGTH, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
CS, Summation, 1.35
NAME=SLV1, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV2, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV3, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV4, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV5, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV6, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV7, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV8, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV9, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV10, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV11, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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NAME=SLV12, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV13, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV14, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV15, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV16, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV17, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV18, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV19, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV20, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, -0.3
    CS, Summation, 1
NAME=SLV21, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV22, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV23, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV24, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -1
    CS, Summation, 1
NAME=SLV25, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, 0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV26, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, 0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV27, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, -0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV28, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, -0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV29, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, 1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV30, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, -1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV31, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, 1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV32, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, -1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV33, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
    CS, Summation, 1
NAME=SLV34, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
    CS, Summation, 1
NAME=SLV35, STEEL, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
    CS, Summation, 1
NAME=SLV36, STEEL, STRENGTH, 0, 0, , 0, 0

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV37, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV38, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV39, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV40, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV41, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV42, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV43, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV44, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV45, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV46, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV47, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV48, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV49, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV50, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV51, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV52, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV53, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV54, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV55, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV56, STEEL, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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NAME=RARA_1, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1, ST, TERMICA UNIFORME +, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_2, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1, ST, TERMICA UNIFORME -, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_3, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1, ST, TERMICA GRADIENTE +, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_4, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_5, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA UNIFORME +, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_6, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA UNIFORME -, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_7, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA GRADIENTE +, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_8, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_9, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_10, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_11, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_12, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_13, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_14, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_15, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_16, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
    ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=FREQ_1, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_2, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_3, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_4, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_5, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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CS, Summation, 1
NAME=FREQ_6, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_7, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_8, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
CS, Summation, 1
NAME=FREQ_9, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_10, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_11, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_12, STEEL, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_13, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_14, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_15, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_16, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=QP_1, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, CS, Summation, 1
NAME=QP_2, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, CS, Summation, 1
NAME=QP_3, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, CS, Summation, 1
NAME=QP_4, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, CS, Summation, 1
NAME=SLD1, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD2, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=SLD3, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD4, STEEL, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=ENVESLU-SLV, STEEL, STRENGTH, 0, 1, , 0, 0
    CBS, SLU1, 1, CBS, SLU2, 1, CBS, SLU3, 1, CBS, SLU4, 1, CBS, SLU5, 1
    CBS, SLU6, 1, CBS, SLU7, 1, CBS, SLU8, 1, CBS, SLU9, 1, CBS, SLU10, 1
    CBS, SLU11, 1, CBS, SLU12, 1, CBS, SLU13, 1, CBS, SLU14, 1
    CBS, SLU15, 1, CBS, SLU16, 1, CBS, SLU17, 1, CBS, SLU18, 1
    CBS, SLU19, 1, CBS, SLU20, 1, CBS, SLU21, 1, CBS, SLU22, 1
    CBS, SLU23, 1, CBS, SLU24, 1, CBS, SLV1, 1, CBS, SLV2, 1, CBS, SLV3, 1
    CBS, SLV4, 1, CBS, SLV5, 1, CBS, SLV6, 1, CBS, SLV7, 1, CBS, SLV8, 1
    CBS, SLV9, 1, CBS, SLV10, 1, CBS, SLV11, 1, CBS, SLV12, 1
    CBS, SLV13, 1, CBS, SLV14, 1, CBS, SLV15, 1, CBS, SLV16, 1
    CBS, SLV17, 1, CBS, SLV18, 1, CBS, SLV19, 1, CBS, SLV20, 1
    CBS, SLV21, 1, CBS, SLV22, 1, CBS, SLV23, 1, CBS, SLV24, 1

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# MIDAS

PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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CBS, SLV25, 1, CBS, SLV26, 1, CBS, SLV27, 1, CBS, SLV28, 1
CBS, SLV29, 1, CBS, SLV30, 1, CBS, SLV31, 1, CBS, SLV32, 1
CBS, SLV33, 1, CBS, SLV34, 1, CBS, SLV35, 1, CBS, SLV36, 1
CBS, SLV37, 1, CBS, SLV38, 1, CBS, SLV39, 1, CBS, SLV40, 1
CBS, SLV41, 1, CBS, SLV42, 1, CBS, SLV43, 1, CBS, SLV44, 1
CBS, SLV45, 1, CBS, SLV46, 1, CBS, SLV47, 1, CBS, SLV48, 1
CBS, SLV49, 1, CBS, SLV50, 1, CBS, SLV51, 1, CBS, SLV52, 1
CBS, SLV53, 1, CBS, SLV54, 1, CBS, SLV55, 1, CBS, SLV56, 1
NAME=ENVE-RARA, STEEL, SERVICE, 0, 1, , 0, 0
    CBS, RARA_1, 1, CBS, RARA_2, 1, CBS, RARA_3, 1, CBS, RARA_4, 1
    CBS, RARA_5, 1, CBS, RARA_6, 1, CBS, RARA_7, 1, CBS, RARA_8, 1
    CBS, RARA_9, 1, CBS, RARA_10, 1, CBS, RARA_11, 1, CBS, RARA_12, 1
    CBS, RARA_13, 1, CBS, RARA_14, 1, CBS, RARA_15, 1, CBS, RARA_16, 1
NAME=ENVE-FREQ, STEEL, SERVICE, 0, 1, , 0, 0
    CBS, FREQ_1, 1, CBS, FREQ_2, 1, CBS, FREQ_3, 1, CBS, FREQ_4, 1
    CBS, FREQ_5, 1, CBS, FREQ_6, 1, CBS, FREQ_7, 1, CBS, FREQ_8, 1
    CBS, FREQ_9, 1, CBS, FREQ_10, 1, CBS, FREQ_11, 1, CBS, FREQ_12, 1
    CBS, FREQ_13, 1, CBS, FREQ_14, 1, CBS, FREQ_15, 1, CBS, FREQ_16, 1
NAME=ENVE-QP, STEEL, SERVICE, 0, 1, , 0, 0
    CBS, QP_1, 1, CBS, QP_2, 1, CBS, QP_3, 1, CBS, QP_4, 1
NAME=ENVE-SLD, STEEL, SERVICE, 0, 1, , 0, 0
    CBS, SLD1, 1, CBS, SLD2, 1, CBS, SLD3, 1, CBS, SLD4, 1
NAME=SLU1, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU2, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU3, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU4, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU5, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU6, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU7, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU8, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 1.5, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.0125, MV, LM01, 1.0125, MV, LM02, 1.0125
    MV, LM03, 1.0125, CS, Summation, 1.35
NAME=SLU9, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU10, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU11, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9

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# MIDAS

PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU12, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU13, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU14, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU15, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU16, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, CS, Summation, 1.35
NAME=SLU17, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLU18, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLU19, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLU20, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLU21, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLU22, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA UNIFORME -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLU23, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE +, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLU24, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, VENTO A PONTE CARICO, 0.9, ST, TERMICA GRADIENTE -, 0.9
    ST, FRENATURA, 1.35, MV, LM01, 1.35, MV, LM02, 1.35, MV, LM03, 1.35
    CS, Summation, 1.35
NAME=SLV1, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV2, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV3, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV4, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV5, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV6, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV7, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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NAME=SLV8, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV9, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV10, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV11, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV12, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV13, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV14, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV15, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV16, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV17, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV18, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, 0.3, CS, Summation, 1
NAME=SLV19, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 1, RS, EY, -0.3, CS, Summation, 1
NAME=SLV20, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -1, RS, EY, -0.3
    CS, Summation, 1
NAME=SLV21, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV22, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -1, CS, Summation, 1
NAME=SLV23, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 1, CS, Summation, 1
NAME=SLV24, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -1
    CS, Summation, 1
NAME=SLV25, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, 0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV26, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, 0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV27, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 1, RS, EY, -0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV28, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -1, RS, EY, -0.3, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV29, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, 1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV30, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, 0.3, RS, EY, -1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV31, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, 1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV32, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, RS, EX, -0.3, RS, EY, -1, MV, LM03, 0.75
    CS, Summation, 1
NAME=SLV33, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1

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# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

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CS, Summation, 1
NAME=SLV34, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV35, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV36, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV37, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV38, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV39, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV40, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV41, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV42, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV43, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV44, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, 1
CS, Summation, 1
NAME=SLV45, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV46, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV47, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV48, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV49, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV50, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV51, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV52, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV53, STLCOMP, STRENGTH, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV54, STLCOMP, STRENGTH, 0, 0, , 0, 0

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# MIDAS

## PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, 0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV55, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, 0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=SLV56, STLCOMP, STRENGTH, 0, 0, , 0, 0
ST, TERMICA GRADIENTE +, 0.5, RS, EX, -0.3, RS, EY, -0.3, RS, EZ, -1
CS, Summation, 1
NAME=RARA_1, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 1, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_2, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 1, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_3, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 1, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_4, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 1, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_5, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 1, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_6, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 1, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_7, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 1, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_8, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 1, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=RARA_9, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_10, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_11, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_12, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_13, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA UNIFORME +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_14, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA UNIFORME -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_15, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA GRADIENTE +, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=RARA_16, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE CARICO, 0.6, ST, TERMICA GRADIENTE -, 0.6
ST, FRENATURA, 1, MV, LM01, 1, CS, Summation, 1
NAME=FREQ_1, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_2, STLCOMP, SERVICE, 0, 0, , 0, 0
ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1

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# MIDAS

PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	
	<b>Author</b>		<b>File Name</b>	L2-CV02_REVISIONE_2

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NAME=FREQ_3, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_4, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, CS, Summation, 1
NAME=FREQ_5, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    CS, Summation, 1
NAME=FREQ_6, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    CS, Summation, 1
NAME=FREQ_7, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    CS, Summation, 1
NAME=FREQ_8, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    CS, Summation, 1
NAME=FREQ_9, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_10, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA UNIFORME -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_11, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE +, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_12, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, VENTO A PONTE SCARICO, 0.2, ST, TERMICA GRADIENTE -, 0.5
    ST, FRENATURA, 0.75, MV, LM01, 0.75, MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_13, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_14, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_15, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=FREQ_16, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, ST, FRENATURA, 0.75, MV, LM01, 0.75
    MV, LM02, 0.75, CS, Summation, 1
NAME=QP_1, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, CS, Summation, 1
NAME=QP_2, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME -, 0.5, CS, Summation, 1
NAME=QP_3, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE +, 0.5, CS, Summation, 1
NAME=QP_4, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA GRADIENTE -, 0.5, CS, Summation, 1
NAME=SLD1, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD2, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=SLD3, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, 1, CS, Summation, 1
NAME=SLD4, STLCOMP, SERVICE, 0, 0, , 0, 0
    ST, TERMICA UNIFORME +, 0.5, RS, DX, -1, CS, Summation, 1
NAME=ENVESLU-SLV, STLCOMP, STRENGTH, 0, 1, , 0, 0
    CBSC, SLU1, 1, CBSC, SLU2, 1, CBSC, SLU3, 1, CBSC, SLU4, 1
    CBSC, SLU5, 1, CBSC, SLU6, 1, CBSC, SLU7, 1, CBSC, SLU8, 1
    CBSC, SLU9, 1, CBSC, SLU10, 1, CBSC, SLU11, 1, CBSC, SLU12, 1

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# MIDAS

PROJECT TITLE :

	<b>Company</b>		<b>Client</b>	L2-CV02_REVISIONE_2
	<b>Author</b>		<b>File Name</b>	

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CBSC, SLU13, 1, CBSC, SLU14, 1, CBSC, SLU15, 1, CBSC, SLU16, 1
CBSC, SLU17, 1, CBSC, SLU18, 1, CBSC, SLU19, 1, CBSC, SLU20, 1
CBSC, SLU21, 1, CBSC, SLU22, 1, CBSC, SLU23, 1, CBSC, SLU24, 1
CBSC, SLV1, 1, CBSC, SLV2, 1, CBSC, SLV3, 1, CBSC, SLV4, 1
CBSC, SLV5, 1, CBSC, SLV6, 1, CBSC, SLV7, 1, CBSC, SLV8, 1
CBSC, SLV9, 1, CBSC, SLV10, 1, CBSC, SLV11, 1, CBSC, SLV12, 1
CBSC, SLV13, 1, CBSC, SLV14, 1, CBSC, SLV15, 1, CBSC, SLV16, 1
CBSC, SLV17, 1, CBSC, SLV18, 1, CBSC, SLV19, 1, CBSC, SLV20, 1
CBSC, SLV21, 1, CBSC, SLV22, 1, CBSC, SLV23, 1, CBSC, SLV24, 1
CBSC, SLV25, 1, CBSC, SLV26, 1, CBSC, SLV27, 1, CBSC, SLV28, 1
CBSC, SLV29, 1, CBSC, SLV30, 1, CBSC, SLV31, 1, CBSC, SLV32, 1
CBSC, SLV33, 1, CBSC, SLV34, 1, CBSC, SLV35, 1, CBSC, SLV36, 1
CBSC, SLV37, 1, CBSC, SLV38, 1, CBSC, SLV39, 1, CBSC, SLV40, 1
CBSC, SLV41, 1, CBSC, SLV42, 1, CBSC, SLV43, 1, CBSC, SLV44, 1
CBSC, SLV45, 1, CBSC, SLV46, 1, CBSC, SLV47, 1, CBSC, SLV48, 1
CBSC, SLV49, 1, CBSC, SLV50, 1, CBSC, SLV51, 1, CBSC, SLV52, 1
CBSC, SLV53, 1, CBSC, SLV54, 1, CBSC, SLV55, 1, CBSC, SLV56, 1
NAME=ENVE-RARA, STLCOMP, SERVICE, 0, 1, , 0, 0
    CBSC, RARA_1, 1, CBSC, RARA_2, 1, CBSC, RARA_3, 1, CBSC, RARA_4, 1
    CBSC, RARA_5, 1, CBSC, RARA_6, 1, CBSC, RARA_7, 1, CBSC, RARA_8, 1
    CBSC, RARA_9, 1, CBSC, RARA_10, 1, CBSC, RARA_11, 1, CBSC, RARA_12, 1
    CBSC, RARA_13, 1, CBSC, RARA_14, 1, CBSC, RARA_15, 1, CBSC, RARA_16, 1
NAME=ENVE-FREQ, STLCOMP, SERVICE, 0, 1, , 0, 0
    CBSC, FREQ_1, 1, CBSC, FREQ_2, 1, CBSC, FREQ_3, 1, CBSC, FREQ_4, 1
    CBSC, FREQ_5, 1, CBSC, FREQ_6, 1, CBSC, FREQ_7, 1, CBSC, FREQ_8, 1
    CBSC, FREQ_9, 1, CBSC, FREQ_10, 1, CBSC, FREQ_11, 1, CBSC, FREQ_12, 1
    CBSC, FREQ_13, 1, CBSC, FREQ_14, 1, CBSC, FREQ_15, 1, CBSC, FREQ_16, 1
NAME=ENVE-QP, STLCOMP, SERVICE, 0, 1, , 0, 0
    CBSC, QP_1, 1, CBSC, QP_2, 1, CBSC, QP_3, 1, CBSC, QP_4, 1
NAME=ENVE-SLD, STLCOMP, SERVICE, 0, 1, , 0, 0
    CBSC, SLD1, 1, CBSC, SLD2, 1, CBSC, SLD3, 1, CBSC, SLD4, 1

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*LC-COLOR ; Diagram Color for Load Case
; ANAL, LCNAME, iR1(ALL), iG1(ALL), iB1(ALL), iR2(MIN), iG2(MIN), iB2(MIN), iR3(MAX),
iG2(MAX), iB2(MAX)
CS, Dead Load, 255, 87, 255, 0, 192, 0, 128, 192
CS, Erection Load 1, 0, 128, 57, 255, 87, 87, 160, 192, 255
CS, Erection Load 2, 0, 157, 192, 212, 160, 255, 192, 72, 0
CS, Erection Load 3, 212, 160, 255, 128, 192, 0, 192, 128, 0
CS, Tendon Primary, 192, 0, 192, 192, 72, 0, 128, 192, 0
CS, Tendon Secondary, 163, 160, 255, 0, 128, 192, 255, 255, 255
CS, Creep Primary, 192, 72, 0, 212, 160, 255, 85, 0, 192
CS, Creep Secondary, 0, 192, 192, 192, 128, 0, 0, 128, 57
CS, Shrinkage Primary, 128, 192, 0, 192, 192, 192, 210, 210, 210
CS, Shrinkage Secondary, 0, 192, 192, 255, 0, 192, 255, 255, 87
CS, Summation, 192, 128, 0, 85, 192, 0, 0, 192, 128
ST, PESO PROPRIO ACCIAIO, 128, 192, 0, 160, 192, 255, 212, 160, 255
ST, PESO PROPRIO SOLETTA, 0, 128, 192, 212, 160, 255, 255, 192, 87
ST, PERMANENTE PORTATO, 255, 128, 0, 146, 0, 255, 192, 192, 192
ST, VENTO A PONTE SCARICO, 255, 87, 87, 255, 160, 255, 78, 0, 255
ST, VENTO A PONTE CARICO, 255, 87, 87, 0, 128, 255, 128, 192, 0
ST, TERMICA UNIFORME +, 255, 128, 0, 255, 255, 255, 146, 0, 255
ST, TERMICA UNIFORME -, 255, 192, 160, 85, 0, 192, 255, 0, 192
ST, TERMICA GRADIENTE +, 255, 192, 160, 255, 255, 87, 128, 192, 0
ST, TERMICA GRADIENTE -, 0, 128, 255, 255, 160, 255, 255, 87, 87
ST, FRENATURA, 0, 192, 128, 255, 87, 128, 163, 255, 160
MV, LM01, 160, 255, 85, 0, 192, 255, 192, 87
MV, LM02, 255, 192, 160, 85, 192, 0, 0, 157, 192
MV, LM03, 192, 0, 192, 192, 192, 0, 255, 87, 128
RS, EX, 85, 192, 0, 128, 192, 0, 255, 160, 255
RS, EY, 255, 192, 87, 255, 87, 128, 255, 87, 128
RS, EZ, 255, 255, 87, 0, 192, 128, 0, 192, 192

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# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

RS, DX, 0, 128, 128, 192, 72, 0, 160, 255, 255  
RS, CX, 0, 192, 128, 192, 192, 0, 255, 192, 87  
CB, SLU1, 146, 0, 255, 78, 0, 255, 192, 192, 192  
CB, SLU2, 255, 192, 87, 93, 255, 87, 146, 0, 255  
CB, SLU3, 255, 192, 87, 212, 160, 255, 0, 128, 192  
CB, SLU4, 0, 128, 128, 210, 210, 210, 0, 192, 128  
CB, SLU5, 255, 128, 0, 163, 160, 255, 0, 128, 192  
CB, SLU6, 0, 157, 192, 0, 128, 128, 255, 255, 255  
CB, SLU7, 0, 128, 255, 148, 87, 255, 160, 255, 255  
CB, SLU8, 192, 0, 128, 212, 160, 255, 0, 192, 192  
CB, SLU9, 0, 128, 128, 192, 192, 192, 0, 128, 255  
CB, SLU10, 255, 255, 255, 160, 192, 255, 160, 192, 255  
CB, SLU11, 192, 0, 192, 0, 128, 255, 255, 0, 192  
CB, SLU12, 93, 255, 87, 255, 192, 160, 255, 128, 0  
CB, SLU13, 146, 0, 255, 255, 0, 192, 255, 87, 87  
CB, SLU14, 0, 157, 192, 163, 160, 255, 0, 192, 192  
CB, SLU15, 128, 192, 0, 255, 255, 255, 85, 0, 192  
CB, SLU16, 255, 192, 160, 0, 157, 192, 255, 255, 255  
CB, SLU17, 78, 0, 255, 160, 192, 255, 255, 0, 128  
CB, SLU18, 128, 192, 0, 160, 255, 255, 163, 160, 255  
CB, SLU19, 212, 160, 255, 163, 160, 255, 0, 128, 255  
CB, SLU20, 255, 0, 192, 192, 128, 0, 85, 192, 0  
CB, SLU21, 255, 128, 0, 210, 210, 210, 255, 255, 255  
CB, SLU22, 0, 157, 192, 255, 255, 255, 146, 0, 255  
CB, SLU23, 255, 87, 87, 192, 0, 128, 0, 128, 192  
CB, SLU24, 85, 192, 0, 212, 160, 255, 192, 192, 192  
CB, SLV1, 255, 255, 255, 128, 192, 0, 93, 255, 87  
CB, SLV2, 212, 160, 255, 192, 128, 0, 255, 87, 87  
CB, SLV3, 255, 0, 192, 0, 128, 57, 210, 210, 210  
CB, SLV4, 0, 128, 128, 0, 128, 57, 255, 128, 0  
CB, SLV5, 192, 128, 0, 255, 255, 87, 192, 128, 0  
CB, SLV6, 192, 192, 0, 192, 0, 192, 255, 160, 255  
CB, SLV7, 255, 0, 192, 192, 0, 128, 128, 192, 0  
CB, SLV8, 192, 128, 0, 163, 255, 160, 192, 72, 0  
CB, SLV9, 0, 192, 192, 146, 0, 255, 85, 0, 192  
CB, SLV10, 255, 255, 255, 163, 255, 160, 160, 255, 255  
CB, SLV11, 210, 210, 210, 128, 192, 0, 128, 192, 0  
CB, SLV12, 85, 192, 0, 255, 192, 87, 85, 0, 192  
CB, SLV13, 93, 255, 87, 255, 87, 87, 0, 128, 128  
CB, SLV14, 210, 210, 210, 0, 157, 192, 192, 0, 128  
CB, SLV15, 0, 128, 57, 0, 128, 255, 255, 0, 192  
CB, SLV16, 210, 210, 210, 0, 157, 192, 192, 128, 0  
CB, SLV17, 128, 192, 0, 160, 255, 255, 212, 160, 255  
CB, SLV18, 255, 160, 255, 160, 192, 255, 255, 160, 255  
CB, SLV19, 160, 255, 255, 0, 192, 128, 85, 0, 192  
CB, SLV20, 192, 192, 0, 192, 0, 192, 0, 192, 192  
CB, SLV21, 146, 0, 255, 192, 128, 0, 255, 0, 192  
CB, SLV22, 146, 0, 255, 85, 0, 192, 160, 255, 255  
CB, SLV23, 255, 255, 87, 192, 0, 128, 212, 160, 255  
CB, SLV24, 255, 87, 128, 210, 210, 210, 160, 192, 255  
CB, SLV25, 0, 192, 128, 160, 255, 255, 160, 192, 255  
CB, SLV26, 255, 255, 87, 93, 255, 87, 192, 0, 192  
CB, SLV27, 0, 157, 192, 85, 0, 192, 255, 160, 255  
CB, SLV28, 255, 255, 255, 128, 192, 0, 192, 192, 0  
CB, SLV29, 192, 192, 192, 255, 255, 255, 85, 0, 192  
CB, SLV30, 192, 192, 0, 255, 192, 87, 0, 192, 192  
CB, SLV31, 255, 128, 0, 85, 192, 0, 192, 72, 0  
CB, SLV32, 192, 192, 192, 0, 128, 57, 0, 192, 192  
CB, SLV33, 255, 128, 0, 160, 192, 255, 192, 192, 0  
CB, SLV34, 192, 0, 192, 85, 192, 0, 148, 87, 255  
CB, SLV35, 255, 0, 192, 255, 255, 255, 255, 255, 255  
CB, SLV36, 78, 0, 255, 160, 192, 255, 255, 255, 87

# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

CB, SLV37, 128, 192, 0, 0, 157, 192, 93, 255, 87  
 CB, SLV38, 160, 255, 255, 255, 192, 87, 148, 87, 255  
 CB, SLV39, 148, 87, 255, 192, 0, 128, 192, 192, 192  
 CB, SLV40, 0, 128, 57, 0, 157, 192, 163, 255, 160  
 CB, SLV41, 255, 255, 255, 192, 128, 0, 210, 210, 210  
 CB, SLV42, 192, 0, 192, 128, 192, 0, 192, 72, 0  
 CB, SLV43, 0, 128, 128, 85, 192, 0, 192, 0, 128  
 CB, SLV44, 163, 255, 160, 192, 0, 192, 78, 0, 255  
 CB, SLV45, 0, 128, 128, 0, 192, 192, 192, 192, 0  
 CB, SLV46, 0, 128, 57, 255, 255, 87, 255, 0, 128  
 CB, SLV47, 85, 0, 192, 163, 160, 255, 255, 192, 160  
 CB, SLV48, 255, 0, 128, 0, 128, 128, 85, 0, 192  
 CB, SLV49, 192, 72, 0, 192, 0, 192, 255, 87, 87  
 CB, SLV50, 192, 192, 0, 255, 160, 255, 212, 160, 255  
 CB, SLV51, 0, 128, 192, 128, 192, 0, 192, 192, 0  
 CB, SLV52, 163, 255, 160, 210, 210, 210, 0, 128, 192  
 CB, SLV53, 0, 157, 192, 255, 0, 128, 212, 160, 255  
 CB, SLV54, 255, 160, 255, 255, 87, 128, 192, 192, 192  
 CB, SLV55, 0, 128, 128, 160, 255, 255, 93, 255, 87  
 CB, SLV56, 255, 192, 160, 160, 255, 255, 160, 255, 255  
 CB, RARA\_1, 210, 210, 210, 0, 192, 128, 192, 192, 0  
 CB, RARA\_2, 192, 72, 0, 0, 192, 192, 0, 128, 128  
 CB, RARA\_3, 163, 160, 255, 255, 255, 87, 78, 0, 255  
 CB, RARA\_4, 255, 0, 192, 93, 255, 87, 160, 192, 255  
 CB, RARA\_5, 192, 0, 128, 255, 255, 87, 0, 128, 57  
 CB, RARA\_6, 255, 192, 160, 192, 128, 0, 192, 72, 0  
 CB, RARA\_7, 192, 128, 0, 255, 192, 160, 148, 87, 255  
 CB, RARA\_8, 192, 192, 192, 163, 255, 160, 148, 87, 255  
 CB, RARA\_9, 93, 255, 87, 85, 192, 0, 192, 72, 0  
 CB, RARA\_10, 163, 255, 160, 255, 0, 192, 163, 255, 160  
 CB, RARA\_11, 93, 255, 87, 255, 160, 255, 160, 255, 255  
 CB, RARA\_12, 255, 87, 128, 255, 87, 87, 78, 0, 255  
 CB, RARA\_13, 255, 255, 255, 192, 128, 0, 0, 192, 128  
 CB, RARA\_14, 160, 192, 255, 160, 192, 255, 255, 255, 255  
 CB, RARA\_15, 128, 192, 0, 255, 87, 87, 255, 128, 0  
 CB, RARA\_16, 210, 210, 210, 0, 128, 255, 255, 255, 87  
 CB, FREQ\_1, 212, 160, 255, 163, 255, 160, 192, 128, 0  
 CB, FREQ\_2, 0, 128, 128, 146, 0, 255, 255, 192, 160  
 CB, FREQ\_3, 128, 192, 0, 192, 192, 0, 192, 0, 192  
 CB, FREQ\_4, 255, 0, 128, 192, 128, 0, 255, 255, 255  
 CB, FREQ\_5, 255, 0, 192, 192, 72, 0, 255, 0, 192  
 CB, FREQ\_6, 0, 192, 192, 148, 87, 255, 192, 192, 0  
 CB, FREQ\_7, 255, 0, 192, 192, 72, 0, 255, 87, 87  
 CB, FREQ\_8, 192, 72, 0, 210, 210, 210, 128, 192, 0  
 CB, FREQ\_9, 85, 0, 192, 128, 192, 0, 255, 192, 160  
 CB, FREQ\_10, 93, 255, 87, 255, 255, 255, 93, 255, 87  
 CB, FREQ\_11, 0, 128, 128, 0, 157, 192, 255, 87, 87  
 CB, FREQ\_12, 192, 128, 0, 255, 87, 128, 128, 192, 0  
 CB, FREQ\_13, 0, 128, 128, 78, 0, 255, 255, 87, 128  
 CB, FREQ\_14, 0, 128, 192, 255, 0, 128, 255, 0, 128  
 CB, FREQ\_15, 192, 72, 0, 128, 192, 0, 0, 192, 128  
 CB, FREQ\_16, 255, 192, 160, 163, 160, 255, 146, 0, 255  
 CB, QP\_1, 85, 192, 0, 93, 255, 87, 255, 255, 255  
 CB, QP\_2, 163, 255, 160, 255, 192, 87, 0, 128, 192  
 CB, QP\_3, 0, 128, 255, 0, 128, 57, 0, 192, 128  
 CB, QP\_4, 255, 192, 87, 93, 255, 87, 192, 192, 192  
 CB, SLD1, 148, 87, 255, 148, 87, 255, 255, 192, 87  
 CB, SLD2, 192, 0, 128, 192, 72, 0, 160, 192, 255  
 CB, SLD3, 0, 192, 128, 163, 160, 255, 192, 192, 0  
 CB, SLD4, 146, 0, 255, 255, 87, 87, 255, 160, 255  
 CBSC, SLU1, 210, 210, 210, 255, 0, 192, 192, 192, 192  
 CBSC, SLU2, 160, 255, 255, 255, 255, 0, 192

# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

CBSC, SLU3, 0, 128, 255, 0, 128, 57, 160, 255, 255  
CBSC, SLU4, 0, 157, 192, 255, 0, 128, 78, 0, 255  
CBSC, SLU5, 85, 0, 192, 192, 72, 0, 255, 255, 255  
CBSC, SLU6, 93, 255, 87, 0, 128, 255, 255, 87, 128  
CBSC, SLU7, 93, 255, 87, 255, 0, 128, 0, 128, 57  
CBSC, SLU8, 0, 157, 192, 85, 192, 0, 128, 192, 0  
CBSC, SLU9, 128, 192, 0, 148, 87, 255, 192, 128, 0  
CBSC, SLU10, 0, 128, 128, 210, 210, 210, 255, 0, 192  
CBSC, SLU11, 255, 255, 255, 78, 0, 255, 0, 128, 57  
CBSC, SLU12, 212, 160, 255, 192, 128, 0, 255, 128, 0  
CBSC, SLU13, 255, 0, 192, 210, 210, 210, 148, 87, 255  
CBSC, SLU14, 85, 0, 192, 255, 192, 87, 93, 255, 87  
CBSC, SLU15, 0, 192, 192, 0, 128, 128, 0, 157, 192  
CBSC, SLU16, 160, 192, 255, 255, 0, 128, 160, 192, 255  
CBSC, SLU17, 128, 192, 0, 85, 0, 192, 163, 160, 255  
CBSC, SLU18, 0, 128, 57, 255, 255, 87, 146, 0, 255  
CBSC, SLU19, 255, 192, 87, 160, 255, 255, 0, 128, 192  
CBSC, SLU20, 146, 0, 255, 160, 192, 255, 160, 192, 255  
CBSC, SLU21, 255, 128, 0, 163, 255, 160, 0, 128, 192  
CBSC, SLU22, 148, 87, 255, 255, 0, 192, 163, 160, 255  
CBSC, SLU23, 0, 192, 128, 85, 192, 0, 0, 128, 128  
CBSC, SLU24, 192, 192, 0, 160, 255, 255, 255, 0, 192  
CBSC, SLV1, 0, 157, 192, 192, 0, 128, 192, 0, 128  
CBSC, SLV2, 160, 192, 255, 255, 87, 128, 0, 157, 192  
CBSC, SLV3, 0, 128, 192, 192, 128, 0, 192, 0, 128  
CBSC, SLV4, 255, 192, 160, 255, 160, 255, 0, 128, 128  
CBSC, SLV5, 160, 192, 255, 192, 192, 0, 255, 160, 255  
CBSC, SLV6, 160, 255, 255, 0, 128, 192, 0, 128, 57  
CBSC, SLV7, 85, 192, 0, 212, 160, 255, 255, 255, 87  
CBSC, SLV8, 255, 0, 192, 255, 128, 0, 0, 128, 255  
CBSC, SLV9, 93, 255, 87, 85, 192, 0, 0, 157, 192  
CBSC, SLV10, 93, 255, 87, 146, 0, 255, 0, 192, 192  
CBSC, SLV11, 0, 192, 192, 0, 128, 255, 210, 210, 210  
CBSC, SLV12, 160, 192, 255, 85, 0, 192, 0, 192, 192  
CBSC, SLV13, 163, 255, 160, 0, 128, 255, 163, 255, 160  
CBSC, SLV14, 255, 0, 128, 85, 0, 192, 148, 87, 255  
CBSC, SLV15, 128, 192, 0, 0, 157, 192, 192, 192, 192  
CBSC, SLV16, 0, 128, 57, 192, 192, 192, 255, 128, 0  
CBSC, SLV17, 255, 255, 255, 160, 255, 255, 0, 128, 57  
CBSC, SLV18, 146, 0, 255, 192, 0, 192, 163, 160, 255  
CBSC, SLV19, 85, 0, 192, 0, 157, 192, 192, 0, 128  
CBSC, SLV20, 255, 0, 192, 163, 160, 255, 78, 0, 255  
CBSC, SLV21, 163, 160, 255, 255, 255, 87, 192, 0, 128  
CBSC, SLV22, 212, 160, 255, 148, 87, 255, 212, 160, 255  
CBSC, SLV23, 0, 128, 192, 0, 157, 192, 163, 255, 160  
CBSC, SLV24, 192, 0, 128, 255, 255, 255, 87, 87  
CBSC, SLV25, 255, 192, 160, 0, 192, 128, 255, 87, 87  
CBSC, SLV26, 192, 192, 192, 160, 255, 255, 255, 192, 160  
CBSC, SLV27, 0, 128, 57, 0, 128, 128, 0, 128, 255  
CBSC, SLV28, 0, 128, 128, 78, 0, 255, 0, 128, 128  
CBSC, SLV29, 85, 0, 192, 210, 210, 210, 192, 72, 0  
CBSC, SLV30, 255, 160, 255, 255, 87, 87, 0, 128, 128  
CBSC, SLV31, 85, 0, 192, 192, 0, 192, 128, 192, 0  
CBSC, SLV32, 0, 128, 255, 192, 0, 192, 255, 255, 255  
CBSC, SLV33, 255, 0, 128, 255, 0, 192, 255, 0, 192  
CBSC, SLV34, 0, 128, 255, 192, 0, 192, 255, 192, 87  
CBSC, SLV35, 148, 87, 255, 85, 192, 0, 255, 128, 0  
CBSC, SLV36, 192, 128, 0, 85, 192, 0, 148, 87, 255  
CBSC, SLV37, 255, 87, 128, 212, 160, 255, 255, 0, 128  
CBSC, SLV38, 255, 192, 87, 255, 160, 255, 255, 87, 128  
CBSC, SLV39, 93, 255, 87, 0, 128, 57, 255, 128, 0  
CBSC, SLV40, 255, 255, 87, 0, 128, 192, 0, 128, 128

# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

CBSC, SLV41, 255, 255, 255, 255, 160, 255, 255, 160, 255  
 CBSC, SLV42, 210, 210, 210, 163, 160, 255, 192, 192, 192  
 CBSC, SLV43, 255, 255, 87, 192, 128, 0, 255, 128, 0  
 CBSC, SLV44, 255, 255, 255, 0, 192, 192, 192, 192  
 CBSC, SLV45, 192, 192, 0, 0, 128, 192, 255, 87, 128  
 CBSC, SLV46, 255, 87, 128, 160, 255, 255, 128, 192, 0  
 CBSC, SLV47, 163, 160, 255, 78, 0, 255, 192, 0, 128  
 CBSC, SLV48, 93, 255, 87, 93, 255, 87, 255, 192, 87  
 CBSC, SLV49, 255, 255, 255, 0, 192, 192, 255, 0, 128  
 CBSC, SLV50, 78, 0, 255, 192, 0, 128, 192, 0, 192  
 CBSC, SLV51, 192, 72, 0, 0, 192, 192, 192, 192, 192  
 CBSC, SLV52, 163, 160, 255, 93, 255, 87, 255, 87, 87  
 CBSC, SLV53, 192, 192, 192, 210, 210, 210, 192, 192, 0  
 CBSC, SLV54, 255, 0, 192, 128, 192, 0, 0, 128, 128  
 CBSC, SLV55, 255, 192, 87, 85, 192, 0, 0, 128, 255  
 CBSC, SLV56, 255, 160, 255, 255, 0, 128, 160, 255, 255  
 CBSC, RARA\_1, 192, 72, 0, 192, 72, 0, 160, 255, 255  
 CBSC, RARA\_2, 0, 128, 57, 0, 192, 128, 255, 255, 255  
 CBSC, RARA\_3, 255, 192, 87, 148, 87, 255, 255, 160, 255  
 CBSC, RARA\_4, 0, 128, 128, 255, 0, 128, 85, 0, 192  
 CBSC, RARA\_5, 255, 87, 87, 255, 192, 160, 192, 192, 192  
 CBSC, RARA\_6, 192, 128, 0, 78, 0, 255, 0, 157, 192  
 CBSC, RARA\_7, 192, 192, 0, 255, 128, 0, 78, 0, 255  
 CBSC, RARA\_8, 255, 87, 87, 192, 128, 0, 146, 0, 255  
 CBSC, RARA\_9, 0, 192, 128, 160, 255, 255, 192, 192, 0  
 CBSC, RARA\_10, 255, 128, 0, 192, 192, 0, 85, 192, 0  
 CBSC, RARA\_11, 255, 192, 160, 255, 192, 87, 163, 255, 160  
 CBSC, RARA\_12, 85, 192, 0, 255, 0, 192, 148, 87, 255  
 CBSC, RARA\_13, 255, 128, 0, 85, 0, 192, 255, 160, 255  
 CBSC, RARA\_14, 192, 192, 192, 192, 192, 0, 192, 192  
 CBSC, RARA\_15, 255, 192, 160, 85, 192, 0, 160, 255, 255  
 CBSC, RARA\_16, 255, 255, 87, 85, 192, 0, 212, 160, 255  
 CBSC, FREQ\_1, 0, 192, 192, 192, 72, 0, 255, 160, 255  
 CBSC, FREQ\_2, 128, 192, 0, 0, 128, 128, 192, 72, 0  
 CBSC, FREQ\_3, 212, 160, 255, 212, 160, 255, 255, 255, 87  
 CBSC, FREQ\_4, 255, 192, 160, 255, 255, 255, 192, 192, 0  
 CBSC, FREQ\_5, 146, 0, 255, 163, 160, 255, 255, 192, 160  
 CBSC, FREQ\_6, 0, 128, 57, 163, 160, 255, 192, 128, 0  
 CBSC, FREQ\_7, 255, 255, 87, 0, 128, 255, 192, 0, 192  
 CBSC, FREQ\_8, 255, 0, 128, 85, 192, 0, 255, 87, 128  
 CBSC, FREQ\_9, 146, 0, 255, 163, 255, 160, 0, 157, 192  
 CBSC, FREQ\_10, 212, 160, 255, 255, 0, 128, 160, 255, 255  
 CBSC, FREQ\_11, 0, 192, 192, 192, 0, 212, 160, 255  
 CBSC, FREQ\_12, 0, 128, 128, 160, 192, 255, 0, 192, 128  
 CBSC, FREQ\_13, 0, 128, 192, 0, 128, 255, 163, 255, 160  
 CBSC, FREQ\_14, 255, 255, 87, 255, 160, 255, 255, 192, 87  
 CBSC, FREQ\_15, 163, 160, 255, 128, 192, 0, 255, 0, 128  
 CBSC, FREQ\_16, 0, 157, 192, 255, 87, 87, 192, 0, 128  
 CBSC, QP\_1, 0, 128, 255, 255, 160, 255, 255, 87, 128  
 CBSC, QP\_2, 192, 72, 0, 0, 157, 192, 255, 255, 255  
 CBSC, QP\_3, 192, 0, 192, 0, 128, 57, 160, 255, 255  
 CBSC, QP\_4, 212, 160, 255, 255, 192, 160, 0, 192, 128  
 CBSC, SLD1, 148, 87, 255, 210, 210, 210, 255, 160, 255  
 CBSC, SLD2, 255, 255, 87, 255, 192, 160, 192, 128, 0  
 CBSC, SLD3, 255, 255, 255, 0, 157, 192, 255, 128, 0  
 CBSC, SLD4, 255, 255, 255, 78, 0, 255, 146, 0, 255  
 CB, ENVESLU-SLV, 255, 0, 192, 192, 0, 192, 93, 255, 87  
 CB, ENVE-RARA, 93, 255, 87, 85, 0, 192, 192, 0, 128  
 CB, ENVE-FREQ, 255, 87, 87, 255, 192, 87, 255, 87, 128  
 CB, ENVE-QP, 85, 192, 0, 255, 192, 160, 163, 160, 255  
 CB, ENVE-SLD, 148, 87, 255, 255, 255, 87, 163, 255, 160  
 CBSC, ENVESLU-SLV, 128, 192, 0, 78, 0, 255, 255, 0, 192

# MIDAS

## PROJECT TITLE :

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

CBSC, ENVE-RARA, 93, 255, 87, 210, 210, 210, 192, 192, 192  
CBSC, ENVE-FREQ, 192, 192, 192, 192, 128, 0, 0, 128, 255  
CBSC, ENVE-QP, 0, 128, 57, 255, 255, 87, 0, 128, 128  
CBSC, ENVE-SLD, 128, 192, 0, 160, 255, 255, 192, 192, 0  
CBS, SLU1, 0, 192, 128, 0, 128, 57, 148, 87, 255  
CBS, SLU2, 0, 128, 255, 255, 128, 0, 255, 192, 160  
CBS, SLU3, 148, 87, 255, 148, 87, 255, 78, 0, 255  
CBS, SLU4, 192, 0, 192, 0, 128, 255, 255, 160, 255  
CBS, SLU5, 85, 0, 192, 78, 0, 255, 163, 255, 160  
CBS, SLU6, 192, 128, 0, 255, 192, 160, 255, 87, 87  
CBS, SLU7, 85, 192, 0, 160, 255, 255, 0, 192, 192  
CBS, SLU8, 255, 128, 0, 0, 128, 255, 148, 87, 255  
CBS, SLU9, 192, 128, 0, 255, 192, 160, 210, 210, 210  
CBS, SLU10, 255, 192, 160, 160, 192, 255, 160, 192, 255  
CBS, SLU11, 255, 255, 255, 210, 210, 210, 128, 192, 0  
CBS, SLU12, 128, 192, 0, 192, 128, 0, 212, 160, 255  
CBS, SLU13, 93, 255, 87, 93, 255, 87, 85, 0, 192  
CBS, SLU14, 255, 87, 87, 255, 192, 87, 210, 210, 210  
CBS, SLU15, 255, 87, 128, 192, 128, 0, 255, 255, 87  
CBS, SLU16, 0, 128, 255, 0, 157, 192, 192, 72, 0  
CBS, SLU17, 163, 160, 255, 255, 0, 192, 192, 72, 0  
CBS, SLU18, 192, 192, 192, 0, 157, 192, 0, 128, 57  
CBS, SLU19, 255, 160, 255, 128, 192, 0, 163, 255, 160  
CBS, SLU20, 255, 0, 192, 255, 255, 87, 93, 255, 87  
CBS, SLU21, 255, 0, 128, 255, 255, 255, 255, 128, 0  
CBS, SLU22, 85, 0, 192, 255, 255, 87, 255, 87, 87  
CBS, SLU23, 0, 157, 192, 85, 0, 192, 255, 192, 87  
CBS, SLU24, 210, 210, 210, 85, 0, 192, 148, 87, 255  
CBS, SLV1, 255, 87, 128, 255, 255, 87, 0, 128, 255  
CBS, SLV2, 255, 87, 87, 128, 192, 0, 148, 87, 255  
CBS, SLV3, 255, 192, 87, 255, 192, 160, 78, 0, 255  
CBS, SLV4, 255, 192, 87, 255, 255, 255, 192, 72, 0  
CBS, SLV5, 212, 160, 255, 210, 210, 210, 255, 0, 192  
CBS, SLV6, 146, 0, 255, 0, 128, 128, 163, 255, 160  
CBS, SLV7, 0, 192, 128, 160, 255, 255, 255, 192, 87  
CBS, SLV8, 192, 128, 0, 0, 157, 192, 163, 160, 255  
CBS, SLV9, 85, 0, 192, 255, 0, 192, 255, 0, 192  
CBS, SLV10, 128, 192, 0, 192, 128, 0, 0, 192, 192  
CBS, SLV11, 255, 128, 0, 255, 255, 87, 255, 192, 87  
CBS, SLV12, 0, 128, 192, 210, 210, 210, 160, 255, 255  
CBS, SLV13, 163, 160, 255, 255, 192, 87, 192, 0, 128  
CBS, SLV14, 192, 128, 0, 0, 128, 192, 0, 192, 192  
CBS, SLV15, 255, 87, 87, 255, 128, 0, 0, 128, 128  
CBS, SLV16, 0, 128, 255, 160, 192, 255, 255, 255, 87  
CBS, SLV17, 255, 87, 128, 0, 192, 128, 148, 87, 255  
CBS, SLV18, 163, 160, 255, 0, 128, 128, 255, 192, 87  
CBS, SLV19, 255, 128, 0, 85, 192, 0, 255, 160, 255  
CBS, SLV20, 255, 0, 128, 160, 255, 255, 128, 192, 0  
CBS, SLV21, 148, 87, 255, 0, 128, 255, 192, 192, 0  
CBS, SLV22, 85, 0, 192, 192, 192, 160, 192, 255  
CBS, SLV23, 163, 255, 160, 160, 255, 255, 163, 255, 160  
CBS, SLV24, 192, 0, 192, 255, 160, 255, 93, 255, 87  
CBS, SLV25, 85, 192, 0, 255, 87, 128, 0, 157, 192  
CBS, SLV26, 146, 0, 255, 255, 255, 255, 78, 0, 255  
CBS, SLV27, 85, 192, 0, 255, 87, 128, 255, 0, 192  
CBS, SLV28, 192, 0, 128, 0, 192, 128, 212, 160, 255  
CBS, SLV29, 0, 192, 128, 255, 87, 87, 255, 255, 255  
CBS, SLV30, 0, 157, 192, 85, 192, 0, 0, 192, 192  
CBS, SLV31, 255, 255, 87, 192, 0, 192, 93, 255, 87  
CBS, SLV32, 192, 0, 192, 78, 0, 255, 163, 160, 255  
CBS, SLV33, 93, 255, 87, 78, 0, 255, 192, 72, 0  
CBS, SLV34, 255, 0, 128, 192, 0, 192, 255, 128, 0

# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

CBS, SLV35, 93, 255, 87, 192, 128, 0, 148, 87, 255  
CBS, SLV36, 255, 255, 87, 146, 0, 255, 255, 255, 87  
CBS, SLV37, 85, 192, 0, 192, 0, 192, 255, 255, 255  
CBS, SLV38, 85, 0, 192, 255, 87, 128, 212, 160, 255  
CBS, SLV39, 148, 87, 255, 255, 87, 128, 0, 128, 57  
CBS, SLV40, 78, 0, 255, 255, 87, 87, 85, 0, 192  
CBS, SLV41, 148, 87, 255, 0, 128, 57, 0, 192, 192  
CBS, SLV42, 0, 192, 128, 0, 128, 255, 0, 128, 192  
CBS, SLV43, 93, 255, 87, 0, 128, 255, 0, 128, 255  
CBS, SLV44, 192, 192, 0, 160, 255, 255, 212, 160, 255  
CBS, SLV45, 255, 192, 160, 0, 192, 128, 192, 72, 0  
CBS, SLV46, 0, 157, 192, 148, 87, 255, 192, 0, 128  
CBS, SLV47, 0, 157, 192, 163, 160, 255, 163, 255, 160  
CBS, SLV48, 148, 87, 255, 210, 210, 210, 128, 192, 0  
CBS, SLV49, 146, 0, 255, 128, 192, 0, 0, 128, 192  
CBS, SLV50, 255, 255, 255, 85, 0, 192, 0, 192, 192  
CBS, SLV51, 163, 160, 255, 192, 192, 192, 192, 0, 192  
CBS, SLV52, 93, 255, 87, 192, 0, 128, 160, 192, 255  
CBS, SLV53, 192, 72, 0, 85, 0, 192, 192, 0, 192  
CBS, SLV54, 78, 0, 255, 255, 160, 255, 85, 192, 0  
CBS, SLV55, 78, 0, 255, 255, 0, 192, 146, 0, 255  
CBS, SLV56, 255, 255, 255, 255, 255, 87, 160, 192, 255  
CBS, RARA\_1, 93, 255, 87, 255, 192, 87, 0, 128, 128  
CBS, RARA\_2, 0, 192, 192, 255, 128, 0, 212, 160, 255  
CBS, RARA\_3, 128, 192, 0, 192, 128, 0, 212, 160, 255  
CBS, RARA\_4, 93, 255, 87, 78, 0, 255, 0, 192, 192  
CBS, RARA\_5, 160, 192, 255, 255, 0, 192, 0, 157, 192  
CBS, RARA\_6, 192, 0, 128, 163, 160, 255, 192, 192, 192  
CBS, RARA\_7, 192, 192, 192, 93, 255, 87, 148, 87, 255  
CBS, RARA\_8, 0, 128, 57, 255, 192, 87, 0, 157, 192  
CBS, RARA\_9, 255, 87, 87, 255, 192, 87, 163, 160, 255  
CBS, RARA\_10, 146, 0, 255, 192, 0, 128, 192, 72, 0  
CBS, RARA\_11, 255, 87, 87, 255, 255, 87, 0, 192, 128  
CBS, RARA\_12, 192, 0, 128, 255, 0, 128, 212, 160, 255  
CBS, RARA\_13, 0, 192, 192, 148, 87, 255, 192, 0, 192  
CBS, RARA\_14, 0, 128, 255, 255, 87, 128, 160, 255, 255  
CBS, RARA\_15, 255, 128, 0, 78, 0, 255, 93, 255, 87  
CBS, RARA\_16, 255, 128, 0, 160, 192, 255, 210, 210, 210  
CBS, FREQ\_1, 192, 192, 192, 192, 0, 192, 0, 128  
CBS, FREQ\_2, 210, 210, 210, 148, 87, 255, 255, 255, 87  
CBS, FREQ\_3, 85, 0, 192, 0, 128, 192, 192, 128, 0  
CBS, FREQ\_4, 192, 0, 192, 163, 160, 255, 192, 192, 192  
CBS, FREQ\_5, 85, 192, 0, 85, 192, 0, 255, 192, 160  
CBS, FREQ\_6, 255, 0, 128, 160, 255, 255, 146, 0, 255  
CBS, FREQ\_7, 160, 255, 255, 192, 128, 0, 78, 0, 255  
CBS, FREQ\_8, 255, 160, 255, 160, 192, 255, 192, 72, 0  
CBS, FREQ\_9, 163, 160, 255, 192, 128, 0, 255, 0, 128  
CBS, FREQ\_10, 0, 128, 57, 192, 192, 192, 192, 192, 192  
CBS, FREQ\_11, 192, 192, 192, 192, 0, 128, 255, 255, 87  
CBS, FREQ\_12, 192, 0, 128, 255, 255, 87, 0, 128, 255  
CBS, FREQ\_13, 255, 160, 255, 255, 255, 160, 192, 255  
CBS, FREQ\_14, 128, 192, 0, 255, 192, 160, 163, 160, 255  
CBS, FREQ\_15, 255, 128, 0, 210, 210, 210, 255, 192, 160  
CBS, FREQ\_16, 192, 0, 192, 255, 160, 255, 255, 0, 192  
CBS, QP\_1, 255, 87, 128, 255, 0, 192, 255, 128, 0  
CBS, QP\_2, 255, 255, 87, 0, 128, 57, 255, 87, 128  
CBS, QP\_3, 0, 192, 192, 255, 192, 87, 212, 160, 255  
CBS, QP\_4, 192, 192, 192, 192, 0, 128, 255, 192, 160  
CBS, SLD1, 255, 192, 87, 163, 160, 255, 78, 0, 255  
CBS, SLD2, 255, 255, 87, 255, 128, 0, 255, 0, 128  
CBS, SLD3, 255, 128, 0, 255, 160, 255, 192, 192, 0  
CBS, SLD4, 255, 128, 0, 212, 160, 255, 255, 192, 160

MIDAS

## **PROJECT TITLE :**

	Company		Client	
	Author		File Name	L2-CV02_REVISIONE_2

CBS, ENVESLU-SLV, 0, 128, 128, 255, 87, 87, 146, 0, 255  
CBS, ENVE-RARA, 255, 160, 255, 163, 255, 160, 192, 0, 128  
CBS, ENVE-FREQ, 210, 210, 210, 255, 255, 255, 255, 192, 160  
CBS, ENVE-QP, 146, 0, 255, 192, 0, 128, 192, 0, 192  
CBS, ENVE-SLD, 255, 87, 128, 210, 210, 210, 0, 192, 128  
RS, CY, 160, 192, 255, 0, 128, 255, 255, 87, 87  
CB, SLC\_X, 255, 160, 255, 255, 192, 160, 210, 210, 210  
CB, SLC\_Y, 163, 255, 160, 255, 255, 87, 255, 192, 160

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*EIGEN-CTRL      ; Eigenvalue Analysis Control
; TYPE, iFREQ, iITER, iDIM, TOL, bMINMAX, FRMIN, FRMAX, bSTRUM    ; TYPE=EIGEN
; TYPE, bINCLN, iGNUM                                         ; TYPE=RITZ(line 1)
; KIND1, CASE1/GROUND1, iNOGL, ...                            ; TYPE=RITZ(from line
2)                                             LANCZOS, 10, 20, 1, 1e-010, NO, 0, 0, NO

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*DGN-MATL      ; Modify Steel(Concrete) Material
; iMAT, TYPE, MNAME, [DATA1]                                     ; STEEL
; iMAT, TYPE, MNAME, [DATA2], [R-DATA], FCI, bSERV, SHORT, LONG ; CONC
; iMAT, TYPE, MNAME, [DATA3], [DATA2], [R-DATA]                  ; SRC
; iMAT, TYPE, MNAME, [DATA5]                                     ; STEEL(None) & KSCE-A

SD05
; [DATA1] : 1, DB, CODE, NAME or 2, ELAST, POISN, FU, FY1, FY2, FY3, FY4
;           FY5, FY6, AFT, AFT2, AFT3, FY, AFV, AFV2, AFV3
; [DATA2] : 1, DB, CODE, NAME or 2, FC, CHK, LAMBDA
; [DATA3] : 1, DB, CODE, NAME or 2, ELAST, FU, FY1, FY2, FY3, FY4
;           FY5, FY6, AFT, AFT2, AFT3, FY, AFV, AFV2, AFV3
; [DATA4] : 1, DB, CODE, NAME or 2, FC
; [DATA5] : 3, ELAST, POISN, AL1, AL2, AL3, AL4, AL5, AL6, AL7, AL8, AL9, AL10
;           MIN1, MIN2, MIN3
; [R-DATA]: RBCODE, RBMAIN, RBSUB, FY(R), FYS
;           1, STEEL, S355          , 1, EN05-PS(S) , , S355          , 2, 0, ,
;           0, 0, NO, 0.0000e+000,   0,, 0, 0,0, 0, 0,0, 0, 0,0, 0, 0,0, 0, 0,0.0000e+000,
;           0,, 0, 0,0, 0, 0,0, 0, 0,0, 0, 0,0.0000e+000,   0,, 0, 0,0, 0, 0,0, 0, 0,0,
;           0, 0,0, 0, 0,
;           2, CONC , C32/40       , 2, 0, NO, 1, , , 0, 0, 0, NO, 0, 0, ,
;           , , , , , , , , , , , , , , , , , , , , , , , , , , , ,
;           3, CONC , C32/40_NOP   , 2, 0, NO, 1, , , 0, 0, 0, NO, 0, 0, ,
;           , , , , , , , , , , , , , , , , , , , , , , , , , , , ,

```

\*LINK-KEY ; Link Key

; iKEY, TYPE, LINK KE  
523, RIGD, 2  
526, RIGD, 4  
556, RIGD, 8  
557, RIGD, 11  
558, RIGD, 14  
559, RIGD, 17  
560, RIGD, 20  
561, RIGD, 23  
562, RIGD, 26  
563, RIGD, 29  
564, RIGD, 32  
565, RIGD, 33  
566, RIGD, 7  
567, RIGD, 10  
568, RIGD, 13  
569, RIGD, 16  
570, RIGD, 19  
571, RIGD, 22  
572, RIGD, 25  
573, RIGD, 28

# MIDAS

## PROJECT TITLE :

	Company		Client	File Name
	Author			

574, RIGD, 31  
575, RIGD, 5  
576, RIGD, 6  
577, RIGD, 9  
578, RIGD, 12  
579, RIGD, 15  
580, RIGD, 18  
581, RIGD, 21  
582, RIGD, 24  
583, RIGD, 27  
584, RIGD, 30  
586, RIGD, 1  
587, RIGD, 3

\*ENDDATA